



13980 SW 252 Street, Miami, Florida 33032
 Telephone: (305) 257-1910 Fax: (305) 257-1911
 www.decotruss.com

WARNING:
**Extreme Care is required in Handling, Erecting and Bracing Trusses. Refer to
 "BCSI-B1 Summary Sheet", which is attached.**

BUILDING DESIGNER (ARCHITECT or ENGINEER of Recorded)

It is your responsibility to determine that the dimensions conform to the architectural plans and all loads utilized herein meet or exceed the actual Dead Loads Specified and /or imposed on your structure, and Live Loads imposed by The Building Code. PERMANENT BRACING shall be designed and specified by the building designer for the structural safety of the building. It shall be the responsibility of the building designer to indicate size, location and attachments for all permanent bracing as required by design analysis. The design and location of all bracing shall be such that they work together with other structural parts of the building (such as shear walls, portal frames, bearing walls, columns, beams etc.) to achieve total structural integrity Permanent Bracing includes and is not limited to The Top Chord Plane, The Bottom Chord Plane and The Web Member Planes. The Anchoring / Strapping of trusses to walls, beams, ledgers etc., shall be the responsibility of The Building Designer. Truss to Truss Connections shall be The Responsibility of The Truss Manufacturer.

TRUSS ERECTOR / INSTALLER (Builder)

The proper and safe erection of all the trusses is exclusively your responsibility. You are cautioned to seek professional advice regarding the Erection / Installation and Bracing, which is ALWAYS required to prevent toppling and dominoing. Top Chord, Bottom Chord and Webs shall be adequately braced, as per the attached HIB-91 Summary Sheet. You are also responsible to refer to the Truss Placement Plan and The Engineering when erecting Trusses, to insure proper placement. Not all trusses are symmetrical in loading and/or design. Proper Placement and/or direction are always required. Check all Flat Truss to insure they are not installed UPSIDE DOWN. Never cut or alter trusses in anyway. Never concentrate Building Materials, such as Plywood or Blocks on installed Trusses.

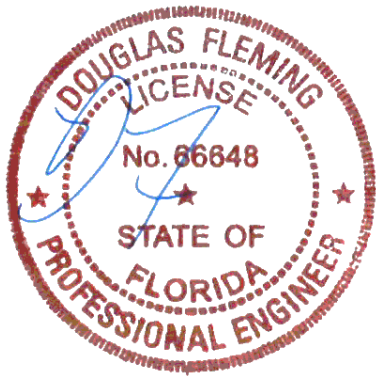
Alterations to Trusses:

"The act of cutting or attaching any item that the truss was not originally designed for"

Trusses cannot be altered from the attached engineering designs, with out proper re-designing by a Florida Registered Engineer. We highly recommend to our customers to use our engineer, however to our knowledge there is no code that states you must use the engineer, who sealed the truss engineering and layout. However, it is of the utmost importance that, any Florida Registered Engineer who takes the task of designing repairs to trusses, should be extremely knowledgeable, not only in structural designs but also have experience designing trusses. Should any alterations be made to our design or manufactured item, the engineer, contractor and homeowner waive all warranties and liabilities by Deco Truss, its officers and engineer. All future, questions regarding said structure and warranties will be held by the persons whom redesigned said trusses and altered them.



Submission No :		SUBMITTAL REVIEW	
		Z. BELFRANIN PE	
Project No :		Discipline :	
10635 S.W. 63RD AVENUE			
☒ Reviewed- No Exceptions Taken	☐ Reviewed-Review and Resubmit		
☐ Reviewed-No Tested	☐ Reviewed by Consultant Not Required		
Review is solely for general conformity with contract. The consultant does not warrant or represent that information in this submittal is accurate or complete. Review does not relieve Specialty Engineer/Contractor of responsibility for error/omissions in design, including this submittal, that are the Specialty Engineer/ Contractor's responsibility, and for conforming/correlating with all quantities/dimensions, performing the Work, selecting performance means/methods, coordinating with other parts of the Work/between trades, and performing the Work safely. Notwithstanding this review, Specialty Engineer/ Contractor remains solely responsible for contract compliance.			
By : ZTB		Date : MAY 03, 2006	



COA #0 278
Florida Certificate of Product Approval #FL1999
04/30/2026

Alpine, an ITW Company
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025
Phone: (800)755-6001
www.alpineitw.com

This item has been digitally signed by Douglas Fleming on the date adjacent to the seal.

Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Site Information:	Page 1:
Customer: Deco Truss Company, Inc.	Job Number: D15210
Job Description: 10635 SW 63 AVE RESIDENCE	
Address: 10635 SW 63 AVE, MIAMI, FL 33156	

Job Engineering Criteria:	
Design Code: FBC 8th Ed. 2023 Res.	IntelliVIEW Version: 25.02.00C JRef #: 1YJP97530002
Loading Standard: ASCE 7-22 Wind Speed (mph): 180 Building Type: Enclosed	Design Loading (psf): 55

This package contains general notes pages, 23 truss drawing(s) and 2 detail(s).

Item	Drawing Number	Truss
1	119.26.1037.36640	B1
3	119.26.1039.37150	V30
5	119.26.1038.15340	E1
7	119.26.1038.43130	K2
9	119.26.1038.51823	K5
11	119.26.1039.41397	V4
13	119.26.1039.46623	V8
15	119.26.1039.14130	V16
17	119.26.1039.21120	V24
19	119.26.1039.32737	V26
21	119.26.1037.49320	DG2
23	119.26.1038.53590	L1
25	VAL180220723	

Item	Drawing Number	Truss
2	119.26.1037.35077	A1
4	119.26.1038.17220	E2
6	119.26.1037.38957	D1
8	119.26.1038.40980	K1
10	119.26.1038.46077	K3
12	119.26.1038.37917	H1
14	119.26.1039.11040	V12
16	119.26.1039.18047	V20
18	119.26.1039.34260	V28
20	119.26.1038.35590	EG3
22	119.26.1038.49317	K4
24	BRCLBSUB0119	

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By : ZTB	Date: MAY 13, 2025	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Bearing Information:

The bearing area factor, C_b , is considered for the allowable capacity of solid sawn wood bearings supporting trusses that are located a minimum of 3" from the end of the lumber piece.

General Notes (continued)

Coated Lumber:

Coated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Coated lumber has no adjustments to lumber properties. Coated lumber may be more brittle than uncoated lumber. Special handling care must be taken to prevent breakage during all handling activities. Refer to manufacturer literature, specifications, and code evaluation reports for restrictions, details, and requirements.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

C = Coated lumber.

C-AT = AtTEK coated lumber.

C-FX = FX Lumber Guard coated lumber.

C -TE = TechWood 4400 coated lumber.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-BF = Boraflame Fire Retardant Treated lumber

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-ON = OnWood Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

FRT-PR = ProWood Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

General Notes (continued)

Key to Terms (continued):

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

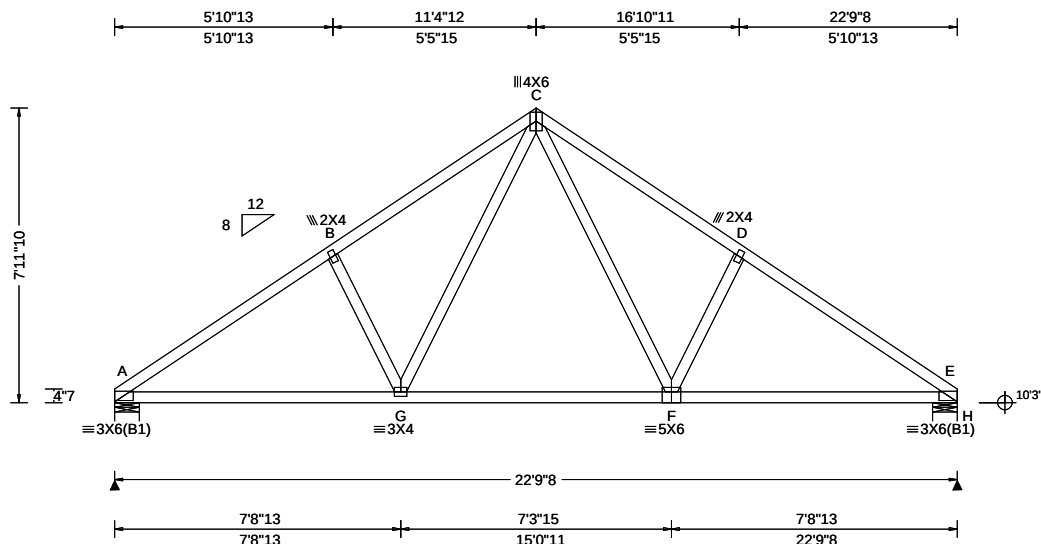
Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com

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By : ZTB		Date MAY 13, 2008	

SEQN: 23824 FROM: DLC	COMN Ply: 1 Qty: 9	Job Number: D15210 10635 SW 63 AVE RESIDENCE Truss Label: B1	Cust: R 9753 JRef: 1YJP97530002 T1 DrwNo: 119.26.1037.36640 KD / DF 04/29/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Speed: 180 mph Risk Category: II Enclosure: Enclosed EXP: D Kzt: NA TCDL: 5.0 psf BCDL: 5.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA W2: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. Load Std: ASCE 7-22 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.057 G 999 360 VERT(CL): 0.099 G 999 240 HORZ(LL): 0.025 E - - HORZ(TL): 0.043 E - - Creep Factor: 2.0 Max TC CSI: 0.452 Max BC CSI: 0.463 Max Web CSI: 0.894 VIEW Ver: 25.02.00C.0120.11	Gravity Loc R+ / R- / Rh / Rw / U / RL A 1393 - / - / - /781 /685 /491 H 1393 - / - / - /781 /685 - Non-Gravity Wind reactions based on MWFRS A Brg Wid = 8.0 Min Req = 1.6 (Truss) H Brg Wid = 8.0 Min Req = 1.6 (Truss) Bearings A & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 1388 -2037 C - D 1523 -1806 B - C 1522 -1807 D - E 1389 -2036

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

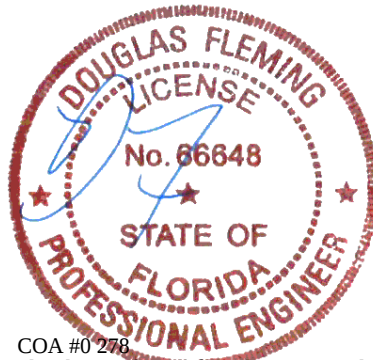
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - G	1581 -945	F - E	1580 -943
G - F	1062 -353		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - G	718 -471	C - F	727 -645
G - C	730 -644	F - D	718 -474



COA #0 278
Florida Certificate of Product Approval #FL1999
04/30/2026

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By : ZTB	Date: 04/12/2026

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

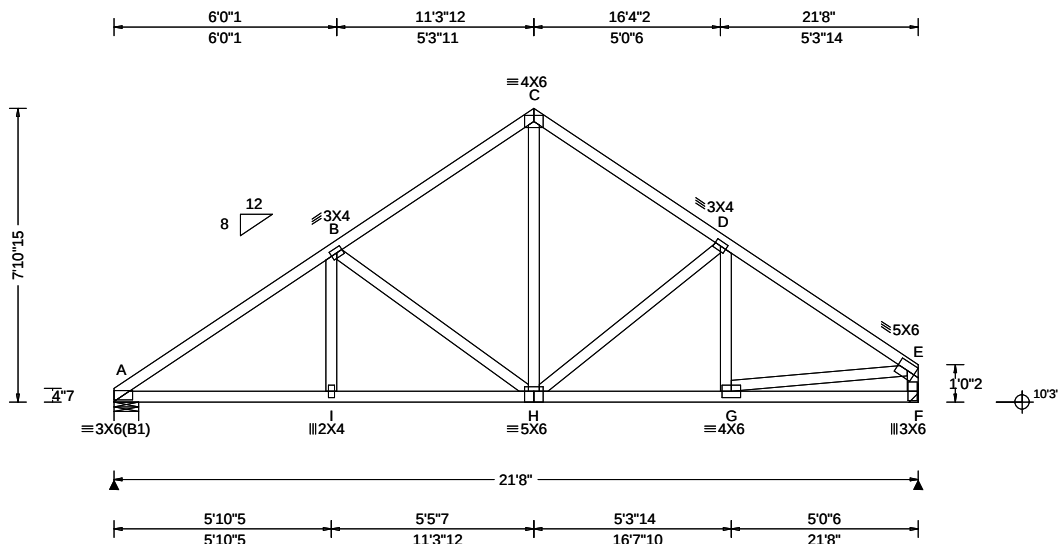
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 23821 FROM: DLC	COMN Ply: 1 Qty: 11	Job Number: D15210 10635 SW 63 AVE RESIDENCE Truss Label: A1	Cust: R 9753 JRef: 1YJP97530002 T2 DrwNo: 119.26.1037.35077 KD / DF 04/29/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
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Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

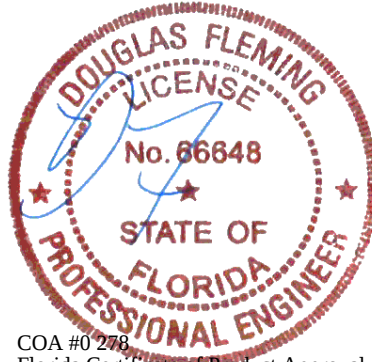
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - I	1399 -975	H - G	1250 -837
I - H	1397 -975		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - H	758 -603	G - E	1186 -756
C - H	744 -783	E - F	971 -1204
H - D	614 -496		



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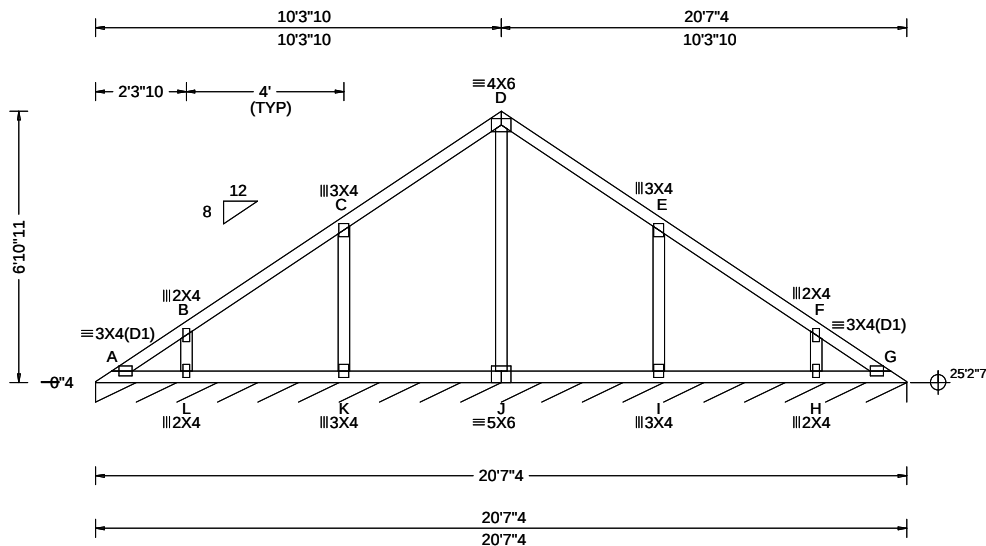
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 21756 FROM: DLC	VAL Ply: 1 Qty: 1	Job Number: D15210 10635 SW 63 AVE RESIDENCE Truss Label: V30	Cust: R 9753 JRef: 1YJP97530002 T30 DrwNo: 119.26.1039.37150 KD / DF 04/29/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Speed: 180 mph Risk Category: II Enclosure: Enclosed EXP: D Kzt: NA TCDL: 5.0 psf BCDL: 5.0 psf Mean Height: 28.79 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA W2: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. Load Std: ASCE 7-22 TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 D 999 360 VERT(CL): -0.005 E 999 240 HORZ(LL): -0.000 A - - HORZ(TL): 0.009 C - - Creep Factor: 2.0 Max TC CSI: 0.370 Max BC CSI: 0.082 Max Web CSI: 0.237 VIEW Ver: 25.02.00C.0120.11	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL G* 116 /- /- /57 /95 /21 Wind reactions based on MWFRS G Brg Wid = 247 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. C - D 488 -171 D - E 488 -171 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B - L 522 -297 I - E 796 -445 C - K 796 -445 H - F 522 -297

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

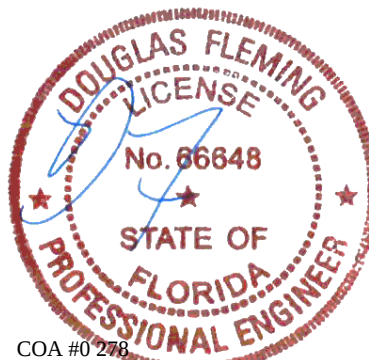
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VAL180220723 for valley details.



COA #0 278
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04/30/2026

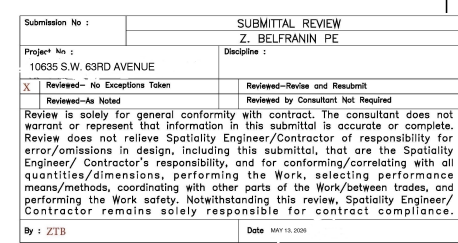
Submission No :	SUBMITTAL REVIEW
Project No :	Z. BELFRANIN PE
10635 S.W. 63RD AVENUE	Discipline :
☒ Reviewed - No Exceptions Taken	☐ Reviewed - Review and Resubmit
☐ Reviewed - No Noted	☐ Reviewed by Consultant Not Required
Review is solely for general conformity with contract. The consultant does not warrant or represent that information in this submittal is accurate or complete. Review does not relieve Spatiality Engineer/Contractor of responsibility for error/omissions in design, including this submittal, that are the Spatiality Engineer/ Contractor's responsibility, and for conforming/correlating with all quantities/dimensions, performing the Work, selecting performance means/methods, coordinating with other parts of the Work/between trades, and performing the Work safely. Notwithstanding this review, Spatiality Engineer/ Contractor remains solely responsible for contract compliance.	
By : ZTB	Date: MAY 10, 2025

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Lumber	B - C	1227	- 1021	D - E	1299	- 1368
Top chord: 2x4 SP #1;	Maximum Bot Chord Forces Per Ply (lbs)					
Bot chord: 2x4 SP #1;	<u>Chords Tens.Comp. Chords Tens. Comp.</u>					
Webs: 2x4 SP #3; W1,W9 2x6 SP #2;						

member design.	Maximum Web Forces Per Ply (lbs)			
End verticals exposed to wind pressure.	Webs	Tens.Comp.	Webs	Tens. Comp.
Deflection meets L/120.	A - J	1061 - 1017	H - D	689 - 436
Wind loading based on both gable and hip roof types.	A - I	888 - 641	G - E	888 - 645
	B - H	690 - 436	E - F	1061 - 1017
	C - H	611 - 866		



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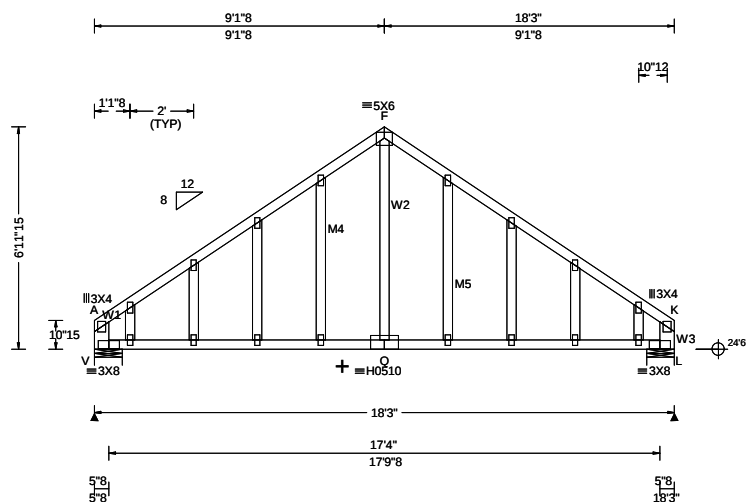
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155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

SEQN: 23854 FROM: DLC	GABL Ply: 2 Qty: 1	Job Number: D15210 10635 SW 63 AVE RESIDENCE Truss Label: E1	Cust: R 9753 JRef: 1YJP97530002 T26 DrwNo: 119.26.1038.15340 KD / DF 04/29/2026
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2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Speed: 180 mph Risk Category: II Enclosure: Part. Enc. EXP: D Kzt: NA TCDL: 5.0 psf BCDL: 5.0 psf Mean Height: 28.45 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA W2: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. Load Std: ASCE 7-22 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/def L/# VERT(LL): 0.116 O 999 360 VERT(CL): 0.481 S 455 240 HORZ(LL): 0.056 D - - HORZ(TL): 0.406 D - - Creep Factor: 2.0 Max TC CSI: 0.552 Max BC CSI: 0.648 Max Web CSI: 0.759 VIEW Ver: 25.02.00C.0120.11	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL V 1050 /- /- /909 /1238 /487 L 1050 /- /- /909 /1238 /- Wind reactions based on MWFRS V Brg Wid = 10.5 Min Req = 1.5 (Support) L Brg Wid = 10.5 Min Req = 1.5 (Support) Bearings V & M are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - F 918 -624 F - K 918 -624

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3; W1,W3 2x6 SP #2; W2,M4,
M5 2x4 SP #1;

Nailnote

Nail Schedule: 0.128"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails
in each row to avoid splitting.

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top
chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C
member design.

End verticals exposed to wind pressure.
Deflection meets L/120.

Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load
applied to face. Calculated deflection ratio is L/167.

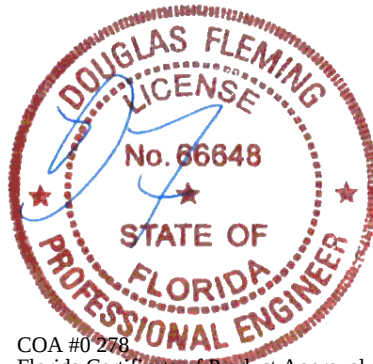
Bracing

Fasten rated sheathing to one face of this frame.

Additional Notes

Exposed portion of gable face shall be reinforced with
sheathing and the wind pressures shall be transferred
into lateral diaphragms. Connections and designs for
diaphragms is the responsibility of the Building
Designer in accordance with ANSI/TPI 1.

+ Member to be laterally braced for horizontal wind
loads. bracing system to be designed and furnished
by others.



COA #0278
Florida Certificate of Product Approval #FL1999
04/30/2026

Submission No :		SUBMITTAL REVIEW	
Project No :		Z. BELFRANIN PE	
10635 S.W. 63RD AVENUE		Discipline :	
☒ Reviewed- No Exceptions Taken	☐ Reviewed-As Noted	☐ Reviewed-Review and Resubmit	☐ Reviewed by Consultant Not Required
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By : ZTB		Date : MAY 15, 2026	

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

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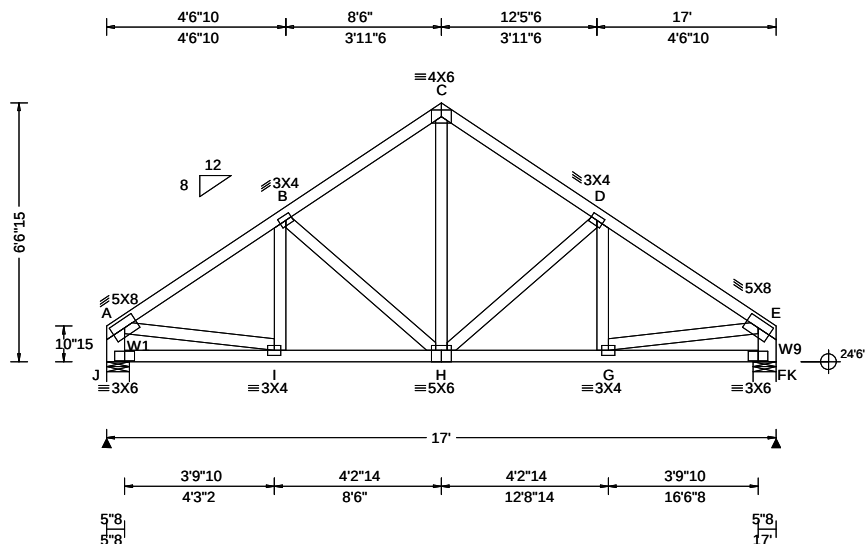
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 23848 FROM: DLC	COMN Ply: 1 Qty: 2	Job Number: D15210 10635 SW 63 AVE RESIDENCE Truss Label: D1	Cust: R 9753 JRef: 1YJP97530002 T36 DrwNo: 119.26.1037.38957 KD / DF 04/29/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Speed: 180 mph Risk Category: II Enclosure: Enclosed EXP: D Kzt: NA TCDL: 5.0 psf BCDL: 5.0 psf Mean Height: 28.24 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA W2: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. Load Std: ASCE 7-22 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.021 H 999 360 VERT(CL): 0.041 H 999 240 HORZ(LL): 0.007 B - - HORZ(TL): 0.021 B - - Creep Factor: 2.0 Max TC CSI: 0.276 Max BC CSI: 0.179 Max Web CSI: 0.627 VIEW Ver: 25.02.00C.0120.11	Gravity Loc R+ / R- / Rh / Rw / U / RL J 977 /- /- /540 /842 /460 K 977 /- /- /540 /842 /- Wind reactions based on MWFRS J Brg Wid = 7.0 Min Req = 1.5 (Support) K Brg Wid = 7.0 Min Req = 1.5 (Support) Bearings J & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 1245 -1259 C - D 1187 -946 B - C 1187 -946 D - E 1245 -1259

Lumber

Top chord: 2x4 SP SS;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3; W1,W9 2x6 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure.
Deflection meets L/120.

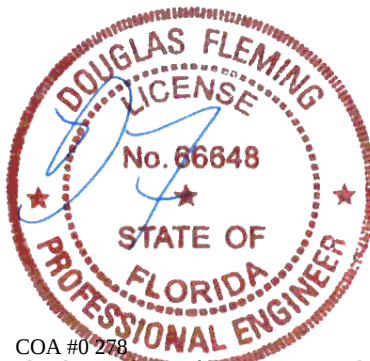
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
J - I	523 -506	H - G	959 -898
I - H	959 -891		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - J	1033 -947	H - D	665 -403
A - I	834 -646	G - E	834 -650
B - H	665 -402	E - F	1033 -947
C - H	565 -843		



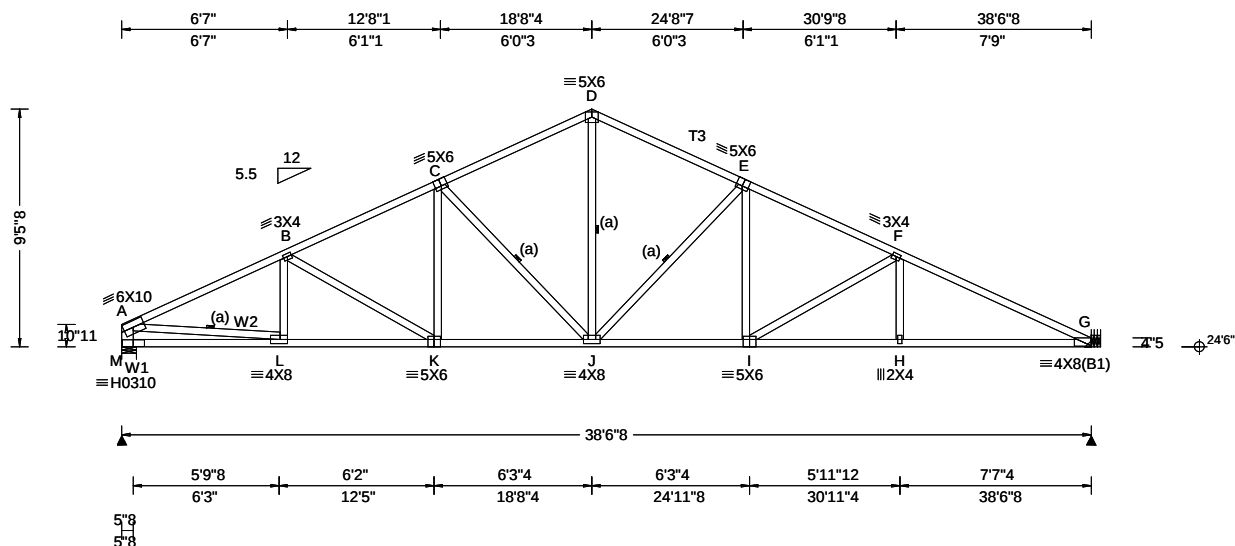
COA #0278
Florida Certificate of Product Approval #FL1999
04/30/2026

Submission No :		SUBMITTAL REVIEW	
Project# Min :		Z. BELFRANIN PE	
10635 S.W. 63RD AVENUE		Discipline :	
X	Reviewed-- No Exceptions Taken		Reviewed--Review and Resubmit
	Reviewed--As Noted		Reviewed by Consultant Not Required
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By : ZTB		Date MAY 13, 2026	

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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 23833 FROM: DLC	SPEC Ply: 1 Qty: 17	Job Number: D15210 10635 SW 63 AVE RESIDENCE Truss Label: K2	Cust: R 9753 JRef: 1YJP97530002 T23 DrwNo: 119.26.1038.43130 KD / DF 04/29/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Speed: 180 mph Risk Category: II Enclosure: Enclosed EXP: D Kzt: NA TCDL: 5.0 psf BCDL: 5.0 psf Mean Height: 29.41 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.85 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA W2: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. Load Std: ASCE 7-22 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/defl L/# VERT(LL): 0.243 I 999 360 VERT(CL): 0.439 I 999 240 HORZ(LL): 0.092 G - - HORZ(TL): 0.174 G - - Creep Factor: 2.0 Max TC CSI: 0.795 Max BC CSI: 0.739 Max Web CSI: 0.995 VIEW Ver: 25.02.00C.0120.11	Gravity Loc R+ / R- / Rh / Rw / U / RL M 2250 - / - / - /1126 /1720 /565 G 2268 - / - /1175 /1726 - Wind reactions based on MWFRS M Brg Wid = 7.0 Min Req = 1.5 (Support) G Brg Wid = - Min Req = - Bearing M is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 3915 -4002 D - E 3180 -2815 B - C 3683 -3560 E - F 3826 -3695 C - D 3176 -2815 F - G 4424 -4511

Lumber

Top chord: 2x4 SP SS; T3 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3; W1 2x6 SP #2; W2 2x4 SP #1;

Reinforcement

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

Hanger Support Required, by others

Loading

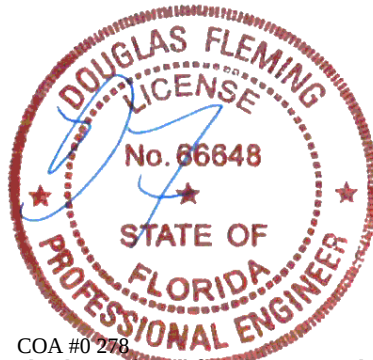
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical exposed to wind pressure.
Deflection meets L/120.

Wind loading based on both gable and hip roof types.



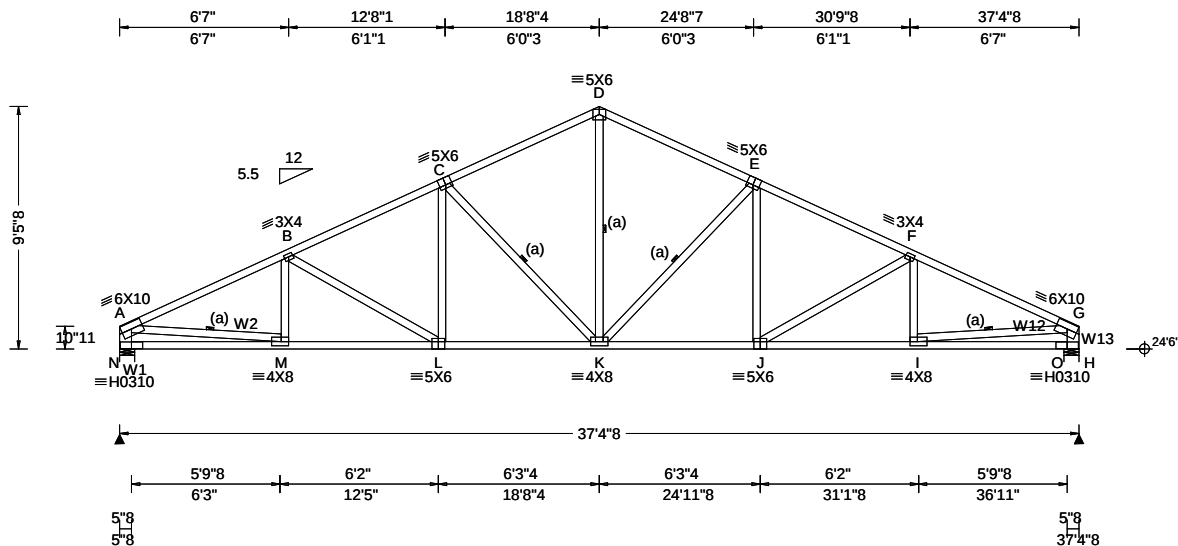
COA #0 278
Florida Certificate of Product Approval #FL1999
04/30/2026

Submission No :	SUBMITTAL REVIEW
Project No :	Z. BELFRANIN PE
10635 S.W. 63RD AVENUE	Discipline :
☒ Reviewed-- No Exceptions Taken	☐ Reviewed--Review and Resubmit
☐ Reviewed--As Noted	☐ Reviewed by Consultant Not Required
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By : ZTB	Date MAY 13, 2026

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 23830 FROM: DLC	COMN Ply: 1 Qty: 6	Job Number: D15210 10635 SW 63 AVE RESIDENCE Truss Label: K1	Cust: R 9753 JRef: 1YJP97530002 T5 DrwNo: 119.26.1038.40980 KD / DF 04/29/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Speed: 180 mph Risk Category: II Enclosure: Enclosed EXP: D Kzt: NA TCDL: 5.0 psf BCDL: 5.0 psf Mean Height: 29.67 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.74 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA W2: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. Load Std: ASCE 7-22 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/defl L/# VERT(LL): 0.212 K 999 360 VERT(CL): 0.383 K 999 240 HORZ(LL): 0.068 H - - HORZ(TL): 0.128 H - - Creep Factor: 2.0 Max TC CSI: 0.850 Max BC CSI: 0.596 Max Web CSI: 0.948 VIEW Ver: 25.02.00C.0120.11	Gravity Loc R+ / R- / Rh / Rw / U / RL N 2194 - / - / - /1093 /1715 /558 O 2194 - / - / - /1093 /1715 - Wind reactions based on MWFRS N Brg Wid = 7.0 Min Req = 1.5 (Support) O Brg Wid = 7.0 Min Req = 1.5 (Support) Bearings N & O are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3; W1,W13 2x6 SP #2; W2,
W12 2x4 SP #1;

Reinforcement

(a) Continuous lateral restraint equally spaced on member.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure.

Deflection meets L/120.

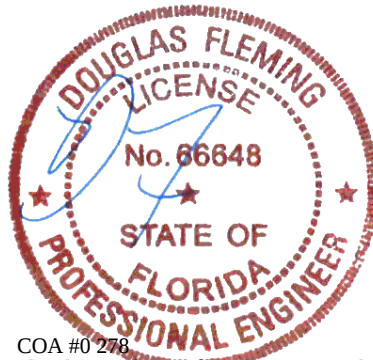
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
N - M	752 - 721	K - J	2998 - 2666
M - L	3441 - 3368	J - I	3441 - 3339
L - K	2998 - 2693	I - H	388 - 528

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - N	2201 - 2140	K - E	1236 - 964
A - M	3055 - 2793	E - J	457 - 347
B - L	775 - 499	J - F	775 - 499
L - C	457 - 347	I - G	3055 - 2800
C - K	1236 - 964	G - H	2202 - 2140
D - K	1578 - 1706		



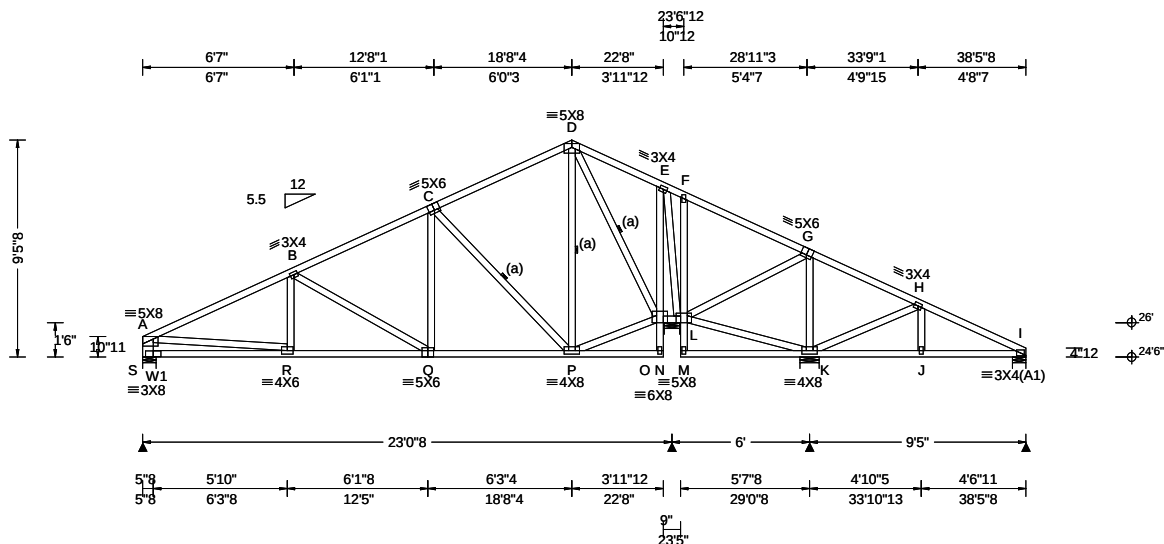
COA #0 278
Florida Certificate of Product Approval #FL1999
04/30/2026

Submission No :	SUBMITTAL REVIEW
Project No :	10635 S.W. 63RD AVENUE
Discipline :	Z. BELFRANIN PE
Reviewed - No Exceptions Taken	Reviewed - Revises and Resubmit
Reviewed - As Noted	Reviewed by Consultant Not Required
Review is solely for general conformity with contract. The consultant does not warrant or represent that information in this submittal is accurate or complete. Review does not relieve Spatiality Engineer/Contractor of responsibility for error/omissions in design, including this submittal, that are the Spatiality Engineer/ Contractor's responsibility, and for conforming/correlating with all quantities/dimensions, performing the Work, selecting performance means/methods, coordinating with other parts of the Work/between trades, and performing the Work safely. Notwithstanding this review, Spatiality Engineer/ Contractor remains solely responsible for contract compliance.	
By : ZTB	Date MAY 15, 2026

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCEA: sbceacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 23836 FROM: DLC	SPEC	Ply: 1 Qty: 3	Job Number: D15210 10635 SW 63 AVE RESIDENCE Truss Label: K5	Cust: R 9753 JRef: 1YJP97530002 T7 DrwNo: 119.26.1038.51823 KD / DF 04/29/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Speed: 180 mph Risk Category: II Enclosure: Enclosed EXP: D Kzt: NA TCDL: 5.0 psf BCDL: 5.0 psf Mean Height: 29.43 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.85 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA W2: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. Load Std: ASCE 7-22 TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.063 Q 999 360 VERT(CL): 0.115 Q 999 240 HORZ(LL): 0.016 P - - HORZ(TL): 0.034 B - - Creep Factor: 2.0 Max TC CSI: 0.688 Max BC CSI: 0.591 Max Web CSI: 0.825 VIEW Ver: 25.02.00C.0120.11	Gravity Loc R+ / R- / Rh / Rw / U / RL S 1249 - / - / - /715 /960 /564 N 1873 - / - / - /909 /1423 - / - K 1028 - / - / - /707 /789 - / - I 399 - / - / - /266 /272 - / - Wind reactions based on MWFRS S Brg Wid = 7.0 Min Req = 1.5 (Support) N Brg Wid = 8.0 Min Req = 2.2 (Truss) K Brg Wid = 10.0 Min Req = 1.5 (Truss) I Brg Wid = 7.0 Min Req = 1.5 (Truss) Bearings S, N, K, & I are a rigid surface. Members not listed have forces less than 375#

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3; W1 2x6 SP #2;

Reinforcement
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 2X4 except as noted.

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical exposed to wind pressure.
Deflection meets L/120.
Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #
04/30/2026

Maximum Top Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
A - B	1927 - 2013	F - G	478 - 190
B - C	1512 - 1388	G - H	498 - 232
C - D	925 - 556	H - I	333 - 469

Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
S - R	739 - 679	Q - P	1131 - 710
R - Q	1725 - 1534	N - L	2247 - 1073

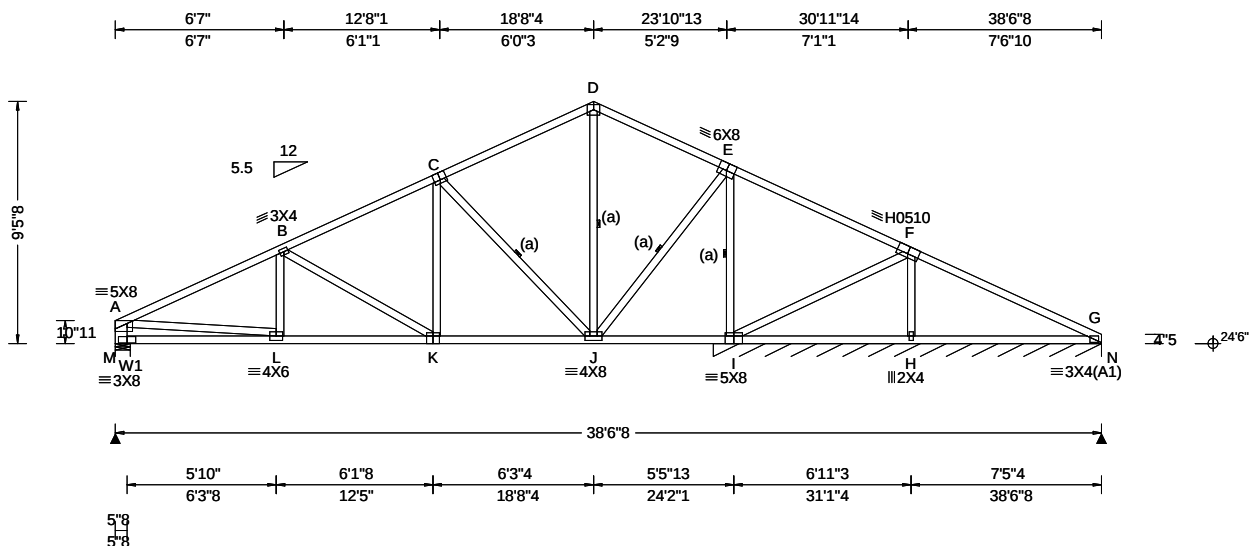
Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.	Webs	Tens. Comp.
A - S	1263 - 1199	P - N	465 - 202
A - R	1396 - 1142	N - E	448 - 228
B - Q	946 - 674	E - L	327 - 675
Q - C	556 - 434	L - F	649 - 434
C - P	1343 - 1074	L - K	646 - 375
D - P	762 - 1002	G - K	534 - 555
D - N	1389 - 1342	K - H	869 - 682

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 21721 FROM: DLC	SPEC Ply: 1 Qty: 1	Job Number: D15210 10635 SW 63 AVE RESIDENCE Truss Label: K3	Cust: R 9753 JRef: 1YJP97530002 T27 DrwNo: 119.26.1038.46077 KD / DF 04/29/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Speed: 180 mph Risk Category: II Enclosure: Enclosed EXP: D Kzt: NA TCDL: 5.0 psf BCDL: 5.0 psf Mean Height: 29.41 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.85 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA W2: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. Load Std: ASCE 7-22 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/defl L/# VERT(LL): 0.059 B 999 360 VERT(CL): 0.111 B 999 240 HORZ(LL): -0.016 G - - HORZ(TL): 0.034 B - - Creep Factor: 2.0 Max TC CSI: 0.861 Max BC CSI: 0.424 Max Web CSI: 0.874 VIEW Ver: 25.02.00C.0120.11	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL M 1248 - / - / - /720 /986 /565 N* 204 - / - /103 /162 - / - Wind reactions based on MWFRS M Brg Wid = 7.0 Min Req = 1.5 (Support) N Brg Wid = 182 Min Req = - Bearings M & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 2004 -2014 D - E 1044 -591 B - C 1597 -1380 E - F 560 -173 C - D 1014 -605 F - G 311 -409

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3; W1 2x6 SP #2;

Reinforcement

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

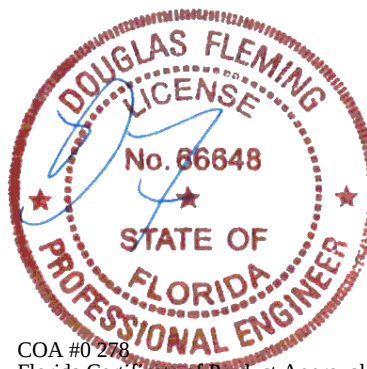
All plates are 5X6 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical exposed to wind pressure.
Deflection meets L/120.

Wind loading based on both gable and hip roof types.



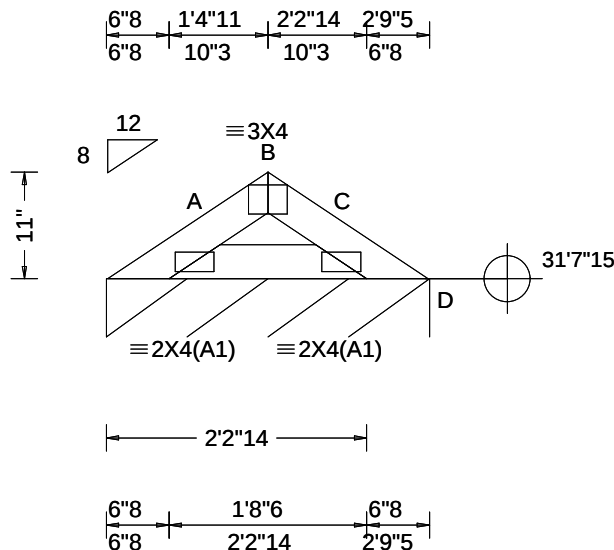
COA #0 278
Florida Certificate of Product Approval #FL1999
04/30/2026

Submission No :	SUBMITTAL REVIEW
Project No :	Z. BELFRANIN PE
Discipline :	
10635 S.W. 63RD AVENUE	
☒ Reviewed- No Exceptions Taken	☐ Reviewed-Review and Resubmit
☐ Reviewed-As Noted	☐ Reviewed by Consultant Not Required
Review is solely for general conformity with contract. The consultant does not warrant or represent that information in this submittal is accurate or complete. Review does not relieve Spatiality Engineer/Contractor of responsibility for error/omissions in design, including this submittal, that are the Spatiality Engineer/ Contractor's responsibility, and for conforming/correlating with all quantities/dimensions, performing the Work, selecting performance means/methods, coordinating with other parts of the Work/between trades, and performing the Work safety. Notwithstanding this review, Spatiality Engineer/ Contractor remains solely responsible for contract compliance.	
By : ZTB	Date MAY 13, 2026

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 21730 FROM: DLC	VAL	Ply: 1 Qty: 3	Job Number: D15210 10635 SW 63 AVE RESIDENCE Truss Label: V4	Cust: R 9753 JRef: 1YJP97530002 T13 DrwNo: 119.26.1039.41397 KD / DF 04/29/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Speed: 180 mph Risk Category: II Enclosure: Enclosed EXP: D Kzt: NA TCDL: 5.0 psf BCDL: 5.0 psf Mean Height: 32.12 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA W2: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. Load Std: ASCE 7-22 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 360 VERT(CL): -0.000 A 999 240 HORZ(LL): 0.000 A - - HORZ(TL): 0.000 A - - Creep Factor: 2.0 Max TC CSI: 0.014 Max BC CSI: 0.011 Max Web CSI: 0.000 VIEW Ver: 25.02.00C.0120.11	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL D* 71 /- /- /36 /65 /14 Wind reactions based on MWFRS D Brg Wid = 33.3 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

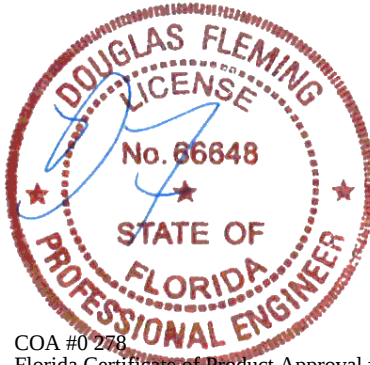
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

See DWG VAL180220723 for valley details.



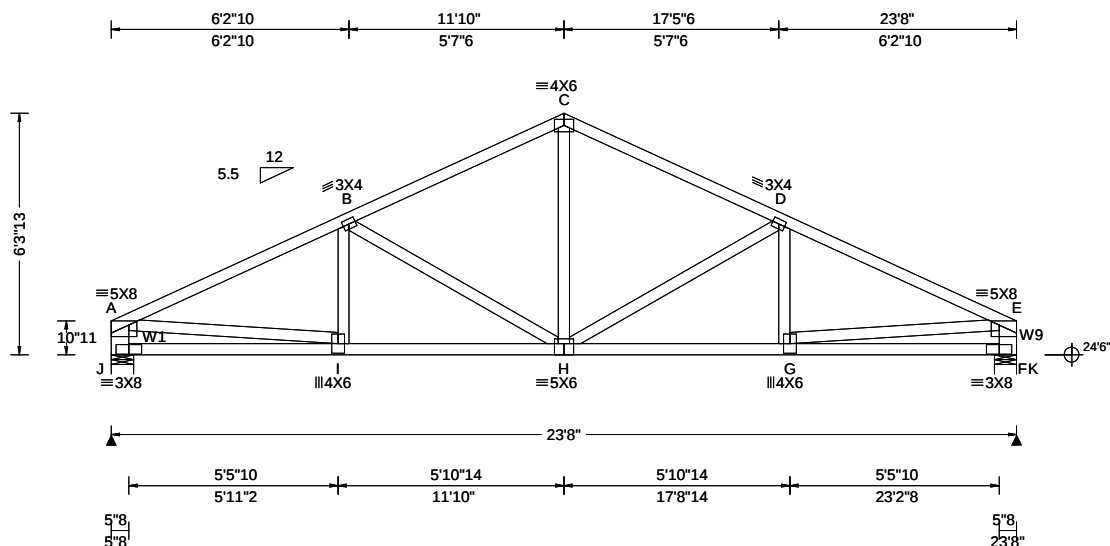
COA #0 278
Florida Certificate of Product Approval #FL1999
04/30/2026

Submission No :	SUBMITTAL REVIEW
Project No :	Z. BELFRANIN PE
10635 S.W. 63RD AVENUE	Discipline :
☒ Reviewed-- No Exceptions Taken	☐ Reviewed--Review and Resubmit
☐ Reviewed--As Noted	☐ Reviewed by Consultant Not Required
Review is solely for general conformity with contract. The consultant does not warrant or represent that information in this submittal is accurate or complete. Review does not relieve Spatiality Engineer/Contractor of responsibility for error/omissions in design, including this submittal, that are the Spatiality Engineer/ Contractor's responsibility, and for conforming/correlating with all quantities/dimensions, performing the Work, selecting performance means/methods, coordinating with other parts of the Work/between trades, and performing the Work safety. Notwithstanding this review, Spatiality Engineer/ Contractor remains solely responsible for contract compliance.	
By : ZTB	Date MAY 13, 2020

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 23827 FROM: DLC	COMN Ply: 1 Qty: 7	Job Number: D15210 10635 SW 63 AVE RESIDENCE Truss Label: H1	Cust: R 9753 JRef: 1YJP97530002 T3 DrwNo: 119.26.1038.37917 KD / DF 04/29/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Speed: 180 mph Risk Category: II Enclosure: Enclosed EXP: D Kzt: NA TCDL: 5.0 psf BCDL: 5.0 psf Mean Height: 28.10 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA W2: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. Load Std: ASCE 7-22 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.070 H 999 360 VERT(CL): 0.134 H 999 240 HORZ(LL): 0.021 E - - HORZ(TL): 0.045 E - - Creep Factor: 2.0 Max TC CSI: 0.721 Max BC CSI: 0.349 Max Web CSI: 0.931 VIEW Ver: 25.02.00C.0120.11	Gravity Loc R+ / R- / Rh / Rw / U / RL J 1328 - / - / - /649 /1184 /354 K 1328 - / - / - /649 /1184 - Wind reactions based on MWFRS J Brg Wid = 7.0 Min Req = 1.5 (Support) K Brg Wid = 7.0 Min Req = 1.5 (Support) Bearings J & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 2206 -2179 C - D 1859 -1612 B - C 1859 -1612 D - E 2206 -2179

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3; W1,W9 2x6 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure.
Deflection meets L/120.

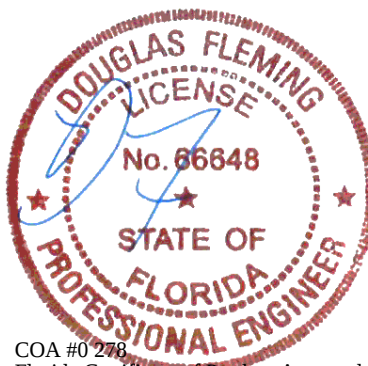
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
J - I	485 -547	H - G	1883 -1891
I - H	1883 -1925	G - F	302 -429

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - J	1390 -1281	H - D	876 -612
A - I	1586 -1448	G - E	1586 -1454
B - H	876 -612	E - F	1391 -1281
C - H	754 -838		



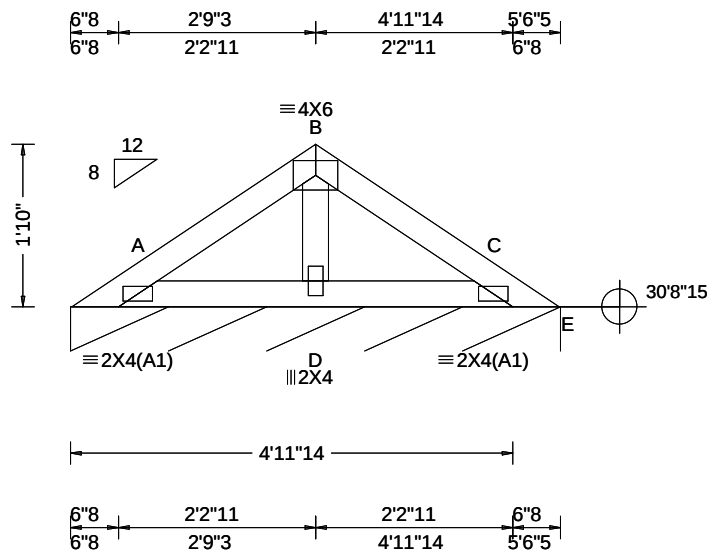
COA #0 278
Florida Certificate of Product Approval #FL1999
04/30/2026

Submission No :	SUBMITTAL REVIEW
Project No :	Z. BELFRANIN PE
10635 S.W. 63RD AVENUE	Discipline :
X Reviewed- No Exceptions Taken	Reviewed-Review and Resubmit
Reviewed-As Noted	Reviewed by Consultant Not Required
Review is solely for general conformity with contract. The consultant does not warrant or represent that information in this submittal is accurate or complete. Review does not relieve Spatiality Engineer/Contractor of responsibility for error/omissions in design, including this submittal, that are the Spatiality Engineer/ Contractor's responsibility, and for conforming/correlating with all quantities/dimensions, performing the Work, selecting performance means/methods, coordinating with other parts of the Work/between trades, and performing the Work safety. Notwithstanding this review, Spatiality Engineer/ Contractor remains solely responsible for contract compliance.	
By : ZTB	Date : MAY 13, 2026

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 21737 FROM: DLC	VAL	Ply: 1 Qty: 3	Job Number: D15210 10635 SW 63 AVE RESIDENCE Truss Label: V8	Cust: R 9753 JRef: 1YJP97530002 T9 DrwNo: 119.26.1039.46623 KD / DF 04/29/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Speed: 180 mph Risk Category: II Enclosure: Enclosed EXP: D Kzt: NA TCDL: 5.0 psf BCDL: 5.0 psf Mean Height: 31.66 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA W2: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. Load Std: ASCE 7-22 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 A 999 360 VERT(CL): -0.002 A 999 240 HORZ(LL): -0.001 C - - HORZ(TL): 0.002 A - - Creep Factor: 2.0 Max TC CSI: 0.182 Max BC CSI: 0.080 Max Web CSI: 0.018 VIEW Ver: 25.02.00C.0120.11	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 93 /- /- /47 /85 /18 Wind reactions based on MWFRS E Brg Wid = 66.3 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

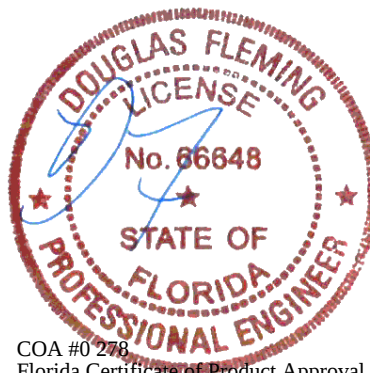
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

See DWG VAL180220723 for valley details.



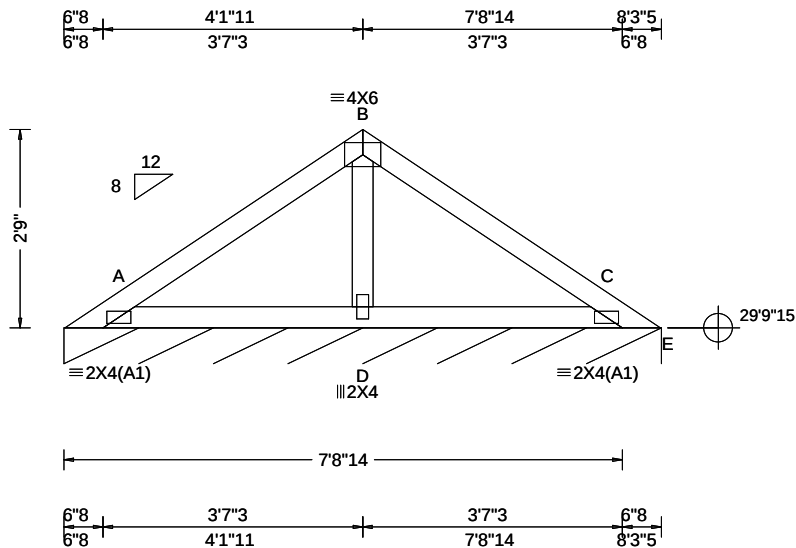
COA #0 278
Florida Certificate of Product Approval #FL1999
04/30/2026

Submission No :	SUBMITTAL REVIEW
Project No :	Z. BELFRANIN PE
10635 S.W. 63RD AVENUE	Discipline :
☒ Reviewed- No Exceptions Taken	☐ Reviewed-Review and Resubmit
☐ Reviewed-As Noted	☐ Reviewed by Consultant Not Required
Review is solely for general conformity with contract. The consultant does not warrant or represent that information in this submittal is accurate or complete. Review does not relieve Spatiality Engineer/Contractor of responsibility for error/omissions in design, including this submittal, that are the Spatiality Engineer/ Contractor's responsibility, and for conforming/correlating with all quantities/dimensions, performing the Work, selecting performance means/methods, coordinating with other parts of the Work/between trades, and performing the Work safety. Notwithstanding this review, Spatiality Engineer/ Contractor remains solely responsible for contract compliance.	
By : ZTB	Date MAY 15, 2025

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 21740 FROM: DLC	VAL	Ply: 1 Qty: 3	Job Number: D15210 10635 SW 63 AVE RESIDENCE Truss Label: V12	Cust: R 9753 JRef: 1YJP97530002 T6 DrwNo: 119.26.1039.11040 KD / DF 04/29/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Speed: 180 mph Risk Category: II Enclosure: Enclosed EXP: D Kzt: NA TCDL: 5.0 psf BCDL: 5.0 psf Mean Height: 31.20 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA W2: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. Load Std: ASCE 7-22 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 A 999 360 VERT(CL): -0.007 A 999 240 HORZ(LL): -0.002 C - - HORZ(TL): 0.009 A - - Creep Factor: 2.0 Max TC CSI: 0.441 Max BC CSI: 0.203 Max Web CSI: 0.021 VIEW Ver: 25.02.00C.0120.11	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 101 /- /- /51 /92 /20 Wind reactions based on MWFRS E Brg Wid = 99.3 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 612 -286 B - C 632 -286

Lumber

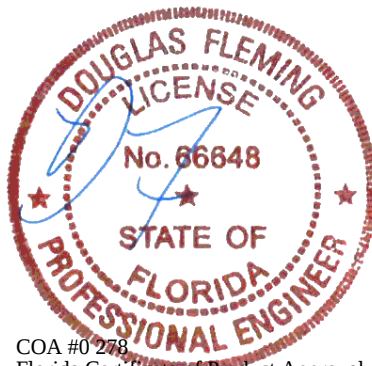
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

See DWG VAL180220723 for valley details.



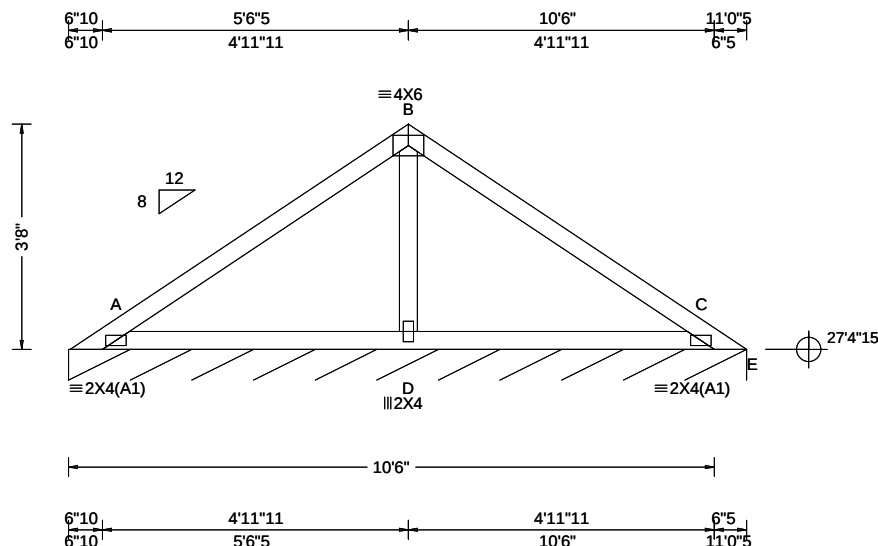
COA #0 278
Florida Certificate of Product Approval #FL1999
04/30/2026

Submission No :		SUBMITTAL REVIEW	
Project No :		Z. BELFRANIN PE	
10635 S.W. 63RD AVENUE		Discipline :	
☒ Reviewed- No Exceptions Taken	☐ Reviewed-As Noted	☐ Reviewed-Review and Resubmit	☐ Reviewed by Consultant Not Required
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By : ZTB		Date MAY 13, 2026	

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 21743 FROM: DLC	VAL	Ply: 1 Qty: 3	Job Number: D15210 10635 SW 63 AVE RESIDENCE Truss Label: V16	Cust: R 9753 JRef: 1YJP97530002 T12 DrwNo: 119.26.1039.14130 KD / DF 04/29/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Speed: 180 mph Risk Category: II Enclosure: Enclosed EXP: D Kzt: NA TCDL: 5.0 psf BCDL: 5.0 psf Mean Height: 29.24 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA W2: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. Load Std: ASCE 7-22 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.006 A 999 360 VERT(CL): -0.018 A 999 240 HORZ(LL): -0.006 C - - HORZ(TL): 0.021 A - - Creep Factor: 2.0 Max TC CSI: 0.736 Max BC CSI: 0.353 Max Web CSI: 0.079 VIEW Ver: 25.02.00C.0120.11	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 105 /- /- /53 /93 /20 Wind reactions based on MWFRS E Brg Wid = 132 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 905 -484 B - C 914 -484 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - D 291 -411 D - C 291 -411

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

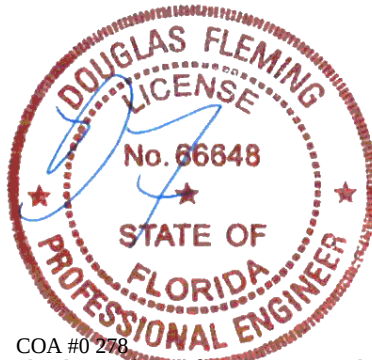
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VAL180220723 for valley details.



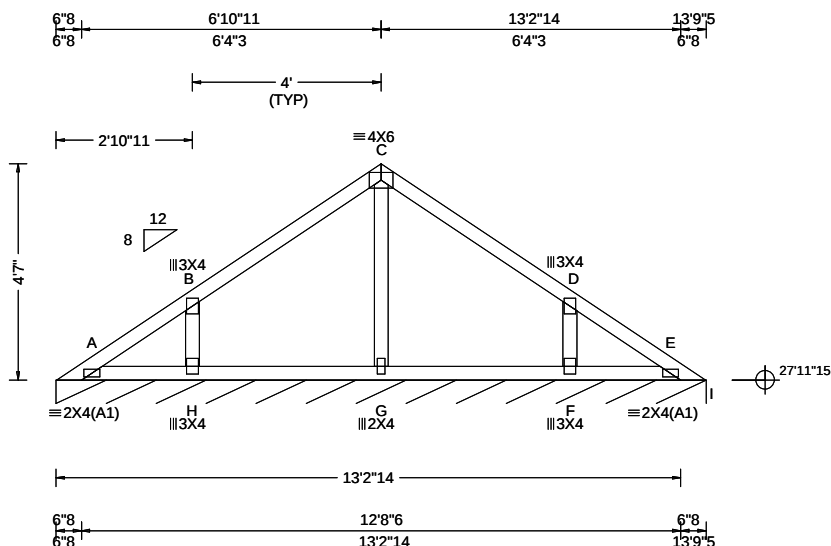
COA #0 278
Florida Certificate of Product Approval #FL1999
04/30/2026

Submission No :		SUBMITTAL REVIEW	
Project No :		Z. BELFRANIN PE	
10635 SW. 63RD AVENUE		Discipline :	
☒ Reviewed - No Exceptions Taken	☐ Reviewed - Review and Resubmit		
☐ Reviewed - As Noted	☐ Reviewed by Consultant Not Required		
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By : ZTB		Date: MAY 13, 2026	

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 21746 FROM: DLC	VAL	Ply: 1 Qty: 3	Job Number: D15210 10635 SW 63 AVE RESIDENCE Truss Label: V20	Cust: R 9753 JRef: 1YJP97530002 T8 DrwNo: 119.26.1039.18047 KD / DF 04/29/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Speed: 180 mph Risk Category: II Enclosure: Enclosed EXP: D Kzt: NA TCDL: 5.0 psf BCDL: 5.0 psf Mean Height: 30.28 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA W2: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. Load Std: ASCE 7-22 TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 C 999 360 VERT(CL): 0.002 C 999 240 HORZ(LL): -0.001 A - - HORZ(TL): 0.004 B - - Creep Factor: 2.0 Max TC CSI: 0.443 Max BC CSI: 0.086 Max Web CSI: 0.267 VIEW Ver: 25.02.00C.0120.11	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL I* 107 / - / - / 54 / 97 / 21 Wind reactions based on MWFRS I Brg Wid = 165 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B - H 896 -387 F - D 896 -387

Lumber

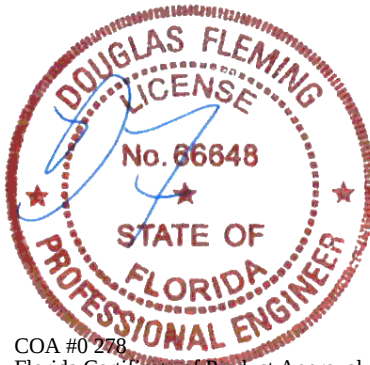
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

See DWG VAL180220723 for valley details.



COA #0 278
Florida Certificate of Product Approval #FL1999
04/30/2026

Submission No :	SUBMITTAL REVIEW
Project No :	Z. BELFRANIN PE
10635 S.W. 63RD AVENUE	Discipline :
☒ Reviewed - No Exceptions Taken	☐ Reviewed - Revise and Resubmit
☐ Reviewed - As Noted	☐ Reviewed by Consultant Not Required
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By : ZTB	Date MAY 13, 2024

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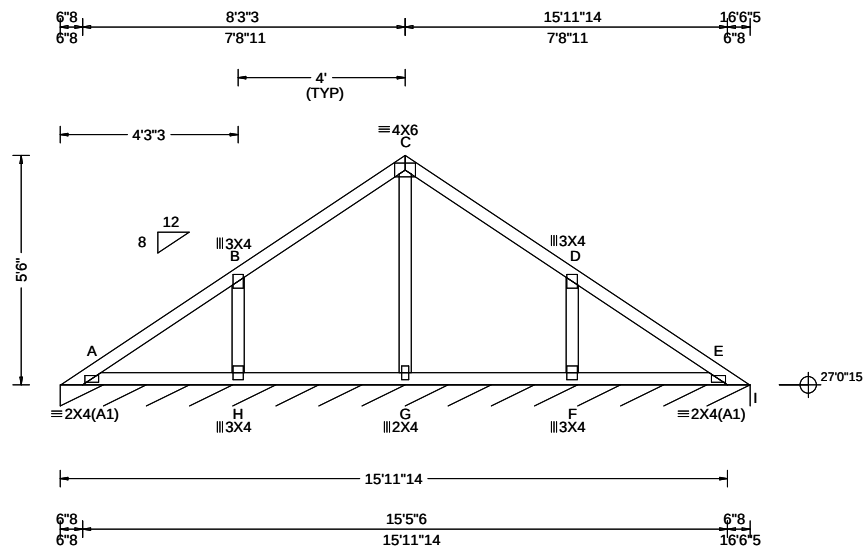
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 21749 FROM: DLC	VAL Ply: 1 Qty: 3	Job Number: D15210 10635 SW 63 AVE RESIDENCE Truss Label: V24	Cust: R 9753 JRef: 1YJP97530002 T4 DrwNo: 119.26.1039.21120 KD / DF 04/29/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Speed: 180 mph Risk Category: II Enclosure: Enclosed EXP: D Kzt: NA TCDL: 5.0 psf BCDL: 5.0 psf Mean Height: 29.83 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA W2: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. Load Std: ASCE 7-22 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 A 999 360 VERT(CL): -0.005 A 999 240 HORZ(LL): -0.001 E - - HORZ(TL): 0.010 A - - Creep Factor: 2.0 Max TC CSI: 0.369 Max BC CSI: 0.099 Max Web CSI: 0.264 VIEW Ver: 25.02.00C.0120.11	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL I* 108 /- /- /55 /98 /21 Wind reactions based on MWFRS I Brg Wid = 198 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 497 -223 C - D 496 -223 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B - H 886 -432 F - D 886 -432

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

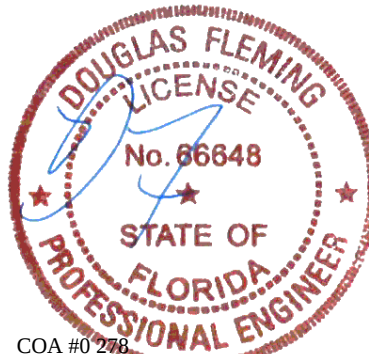
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VAL180220723 for valley details.



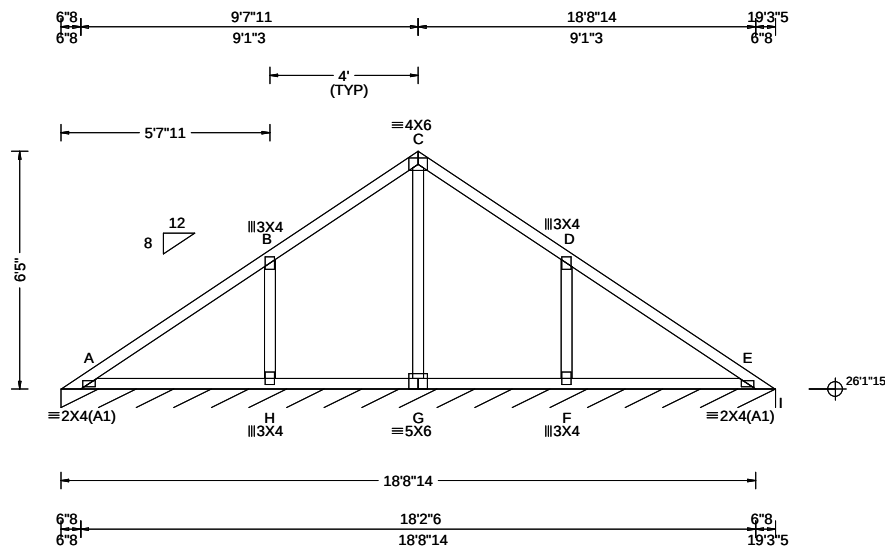
COA #0 278
Florida Certificate of Product Approval #FL1999
04/30/2026

Submission No :	SUBMITTAL REVIEW
Project No :	Z. BELFRANIN PE
10635 S.W. 63RD AVENUE	Discipline :
☒ Reviewed--No Exceptions Taken	☐ Reviewed--Review and Resubmit
☐ Reviewed--As Noted	☐ Reviewed by Consultant Not Required
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By : ZTB	Date MAY 10, 2025

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North Building, 4th Floor
Glenview, IL 60025

SEQN: 21753 FROM: DLC	VAL	Ply: 1 Qty: 1	Job Number: D15210 10635 SW 63 AVE RESIDENCE Truss Label: V28	Cust: R 9753 JRef: 1YJP97530002 T10 DrwNo: 119.26.1039.34260 KD / DF 04/29/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Speed: 180 mph Risk Category: II Enclosure: Enclosed EXP: D Kzt: NA TCDL: 5.0 psf BCDL: 5.0 psf Mean Height: 29.37 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA W2: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. Load Std: ASCE 7-22 TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.005 A 999 360 VERT(CL): -0.014 A 999 240 HORZ(LL): 0.004 A - - HORZ(TL): 0.020 A - - Creep Factor: 2.0 Max TC CSI: 0.476 Max BC CSI: 0.191 Max Web CSI: 0.279 VIEW Ver: 25.02.00C.0120.11	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL I* 110 /- /- /55 /97 /21 Wind reactions based on MWFRS I Brg Wid = 231 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 657 -303 C - D 657 -303 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B - H 937 -504 F - D 937 -504

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

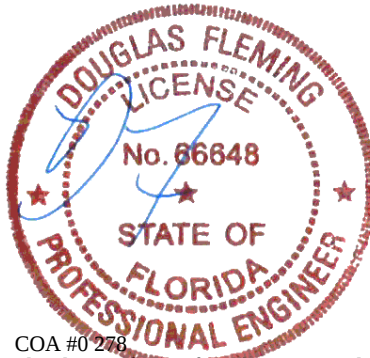
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VAL180220723 for valley details.



COA #0 278
Florida Certificate of Product Approval #FL1999
04/30/2026

Submission No :		SUBMITTAL REVIEW	
Project* No : 10635 S.W. 63RD AVENUE		Discipline : Z. BELFRANIN PE	
☒ Reviewed- No Exceptions Taken	☐ Reviewed-As Noted	☐ Reviewed-Revise and Resubmit	☐ Reviewed by Consultant Not Required
Review is solely for general conformity with contract. The consultant does not warrant or represent that information in this submittal is accurate or complete. Review does not relieve Spatiality Engineer/Contractor of responsibility for error/omissions in design, including this submittal, that are the Spatiality Engineer/ Contractor's responsibility, and for conforming/correlating with all quantities/dimensions, performing the Work, selecting performance means/methods, coordinating with other parts of the Work/between trades, and performing the Work safety. Notwithstanding this review, Spatiality Engineer/ Contractor remains solely responsible for contract compliance.			
By : ZTB		Date: MAY 15, 2026	

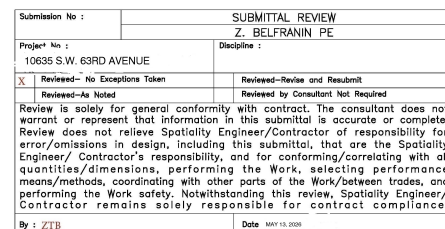
****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org

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Glenview, IL 60025

Structural drawing of a roof truss system. The truss has a peak at joint C. The roof slope is 12/8. The base is supported by a continuous foundation with joints A, H, G, F, and E. Vertical members are BH, CG, and DF. Diagonal members are AC and CE. Horizontal members are AH, HG, GF, and FE. The drawing includes dimensions: total width 18'5", peak height 6'3", and various member sizes like 4X6, 3X4, and 5X6. A 4' (TYP) dimension is shown for the spacing of the vertical members.

Lumber	Maximum Web Forces Per Ply (lbs)			
Top chord: 2x4 SP #1; Bot chord: 2x4 SP #1; Webs: 2x4 SP #3;	Webs	Tens.Comp.	Webs	Tens. Comp.
	B - H	942 - 504	F - D	942 - 504

Additional Notes
See DWG VAL180220723 for vallev details.

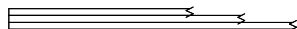


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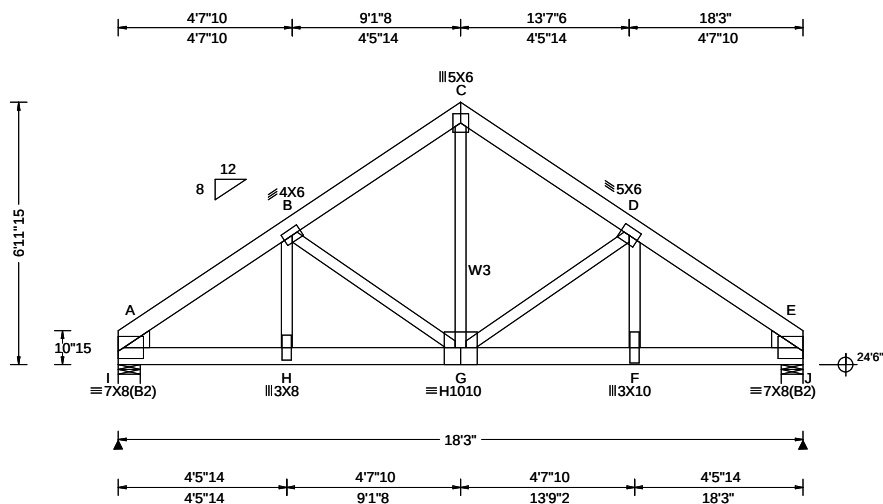
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SEQN: 23845 FROM: DLC	COMN Ply: 3 Qty: 1	Job Number: D15210 10635 SW 63 AVE RESIDENCE Truss Label: EG3	Cust: R 9753 JRef: 1YJP97530002 T17 DrwNo: 119.26.1038.35590 KD / DF 04/29/2026
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3 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Speed: 180 mph Risk Category: II Enclosure: Enclosed EXP: D Kzt: NA TCDL: 5.0 psf BCDL: 5.0 psf Mean Height: 28.45 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA W2: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. Load Std: ASCE 7-22 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/defl L/# VERT(LL): 0.073 G 999 360 VERT(CL): 0.134 G 999 240 HORZ(LL): 0.029 E - - HORZ(TL): 0.053 E - - Creep Factor: 2.0 Max TC CSI: 0.275 Max BC CSI: 0.587 Max Web CSI: 0.620 VIEW Ver: 25.02.00C.0120.11	Gravity Loc R+ / R- / Rh / Rw / U / RL I 10295 -/- /- /- /8047 -/ J 12054 -/- /- /- /9386 -/ Wind reactions based on MWFRS I Brg Wid = 7.0 Min Req = 4.0 (Truss) J Brg Wid = 7.0 Min Req = 4.7 (Truss) Bearings I & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 3603 -4607 C - D 2654 -3388 B - C 2649 -3381 D - E 3782 -4843

Lumber

Top chord: 2x6 SP SS;
Bot chord: 2x6 SP SS;
Webs: 2x4 SP #3; W3 2x4 SP #1;
Lt Wedge: 2x6 SP #2; Rt Wedge: 2x6 SP #2;

Nailnote

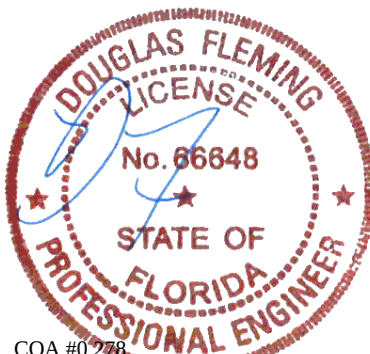
Nail Schedule: 0.128"x3", min. nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 2 Rows @ 2.50" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 96 plf at 0.00 to 96 plf at 18.25
BC: From 10 plf at 0.00 to 10 plf at 18.25
BC: 2268 lb Conc. Load at 2.06, 4.06, 6.06, 8.06
10.06, 12.06, 14.06, 16.06, 16.65

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.
THIS TRUSS MUST BE INSTALLED AS SHOWN
AND NOT END FOR END.



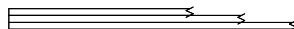
COA #0 278
Florida Certificate of Product Approval #FL1999
04/30/2026

Submission No :	SUBMITTAL REVIEW
Project No :	Z. BELFRANIN PE
Discipline :	
Reviewed--No Exceptions Taken	Reviewed--Review and Resubmit
Reviewed--As Noted	Reviewed by Consultant Not Required
Review is solely for general conformity with contract. The consultant does not warrant or represent that information in this submittal is accurate or complete. Review does not relieve Spatiality Engineer/Contractor of responsibility for error/omissions in design, including this submittal, that are the Spatiality Engineer/ Contractor's responsibility, and for conforming/correlating with all quantities/dimensions, performing the Work, selecting performance means/methods, coordinating with other parts of the Work/between trades, and performing the Work safety. Notwithstanding this review, Spatiality Engineer/ Contractor remains solely responsible for contract compliance.	
By : ZTB	Date: MAY 13, 2026

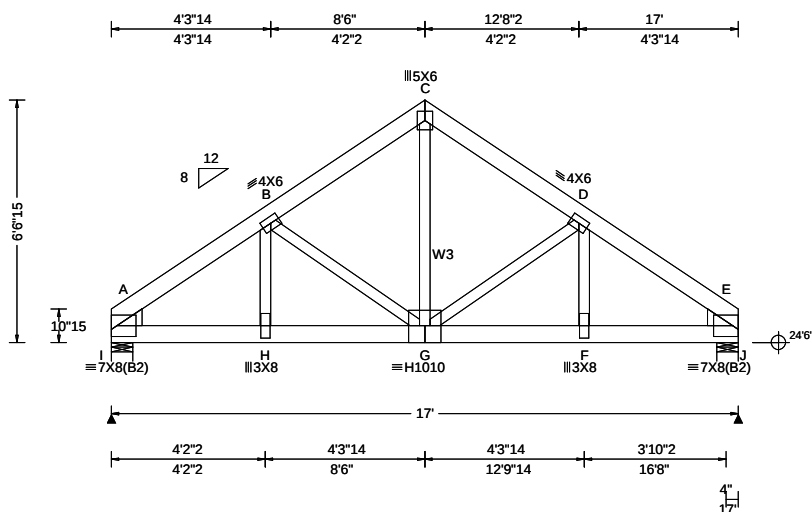
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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 23842 FROM: DLC	COMN Ply: 3 Qty: 1	Job Number: D15210 10635 SW 63 AVE RESIDENCE Truss Label: DG2	Cust: R 9753 JRef: 1YJP97530002 T37 DrwNo: 119.26.1037.49320 KD / DF 04/29/2026
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3 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Speed: 180 mph Risk Category: II Enclosure: Enclosed EXP: D Kzt: NA TCDL: 5.0 psf BCDL: 5.0 psf Mean Height: 28.24 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA W2: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. Load Std: ASCE 7-22 TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/defl L/# VERT(LL): 0.062 G 999 360 VERT(CL): 0.113 G 999 240 HORZ(LL): 0.025 E - - HORZ(TL): 0.045 E - - Creep Factor: 2.0 Max TC CSI: 0.250 Max BC CSI: 0.488 Max Web CSI: 0.485 VIEW Ver: 25.02.00C.0120.11	Gravity Loc R+ / R- / Rh / Rw / U / RL I 10716 -/- /- /- /8346 -/ J 9254 -/- /- /- /7211 -/ Wind reactions based on MWFRS I Brg Wid = 7.0 Min Req = 4.2 (Truss) J Brg Wid = 7.0 Min Req = 3.6 (Truss) Bearings I & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 3268 -4181 C - D 2389 -3050 B - C 2393 -3055 D - E 3238 -4145

Lumber

Top chord: 2x6 SP SS;
Bot chord: 2x6 SP SS;
Webs: 2x4 SP #3; W3 2x4 SP #1;
Lt Wedge: 2x6 SP #2; Rt Wedge: 2x6 SP #2;

Nailnote

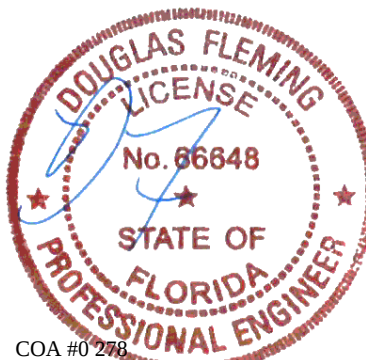
Nail Schedule: 0.128"x3", min. nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 2 Rows @ 3.00" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 96 plf at 0.00 to 96 plf at 17.00
BC: From 10 plf at 0.00 to 10 plf at 14.81
BC: From 20 plf at 14.81 to 20 plf at 17.00
BC: 2268 lb Conc. Load at 0.81, 2.81, 4.81, 6.81, 8.81, 10.81, 12.81, 14.81

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.



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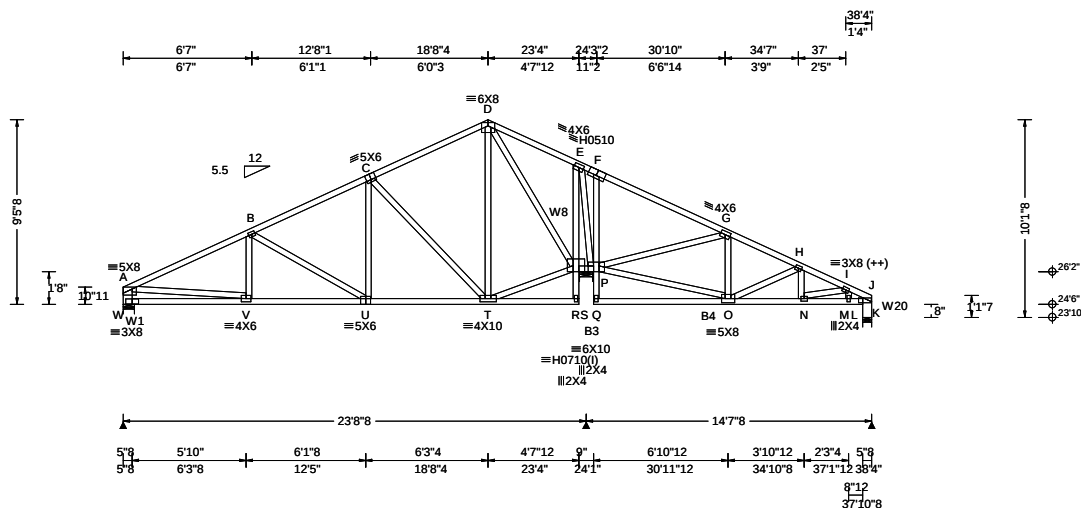
Submission No :	SUBMITTAL REVIEW
Project No :	Z. BELFRANIN PE
10635 S.W. 63RD AVENUE	
☒ Reviewed - No Exceptions Taken	☐ Reviewed - Revise and Resubmit
☐ Reviewed - As Noted	☐ Reviewed by Consultant Not Required
Review is solely for general conformity with contract. The consultant does not warrant or represent that information in this submittal is accurate or complete. Review does not relieve Spatiality Engineer/Contractor of responsibility for error/omissions in design, including this submittal, that are the Spatiality Engineer/ Contractor's responsibility, and for conforming/correlating with all quantities/dimensions, performing the Work, selecting performance means/methods, coordinating with other parts of the Work/between trades, and performing the Work safely. Notwithstanding this review, Spatiality Engineer/ Contractor remains solely responsible for contract compliance.	
By : ZTB	Date MAY 13, 2026

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 24330 FROM: DLC	SPEC Ply: 2 Qty: 8	Job Number: D15210 10635 SW 63 AVE RESIDENCE Truss Label: K4	Cust: R 9753 JRef: 1YJP97530002 T15 DrwNo: 119.26.1038.49317 KD / DF 04/29/2026
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2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Speed: 180 mph Risk Category: II Enclosure: Enclosed EXP: D Kzt: NA TCDL: 5.0 psf BCDL: 5.0 psf Mean Height: 29.46 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.83 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA W2: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. Load Std: ASCE 7-22 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/defl L/# VERT(LL): 0.030 U 999 360 VERT(CL): 0.055 U 999 240 HORZ(LL): 0.007 B - - HORZ(TL): 0.016 B - - Creep Factor: 2.0 Max TC CSI: 0.216 Max BC CSI: 0.385 Max Web CSI: 0.351 VIEW Ver: 25.02.00C.0120.11	Gravity Loc R+ / R- / Rh / Rw / U / RL W 1236 - / - / - /712 /910 /624 R 2741 - / - / - /1331 /2123 - / - K 549 - / - / - /395 /413 - / - Non-Gravity W Brg Wid = 7.0 Min Req = 1.5 (Support) R Brg Wid = 8.0 Min Req = 1.6 (Truss) K Brg Wid = 5.5 Min Req = 1.5 (Support) Bearings W, R, & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

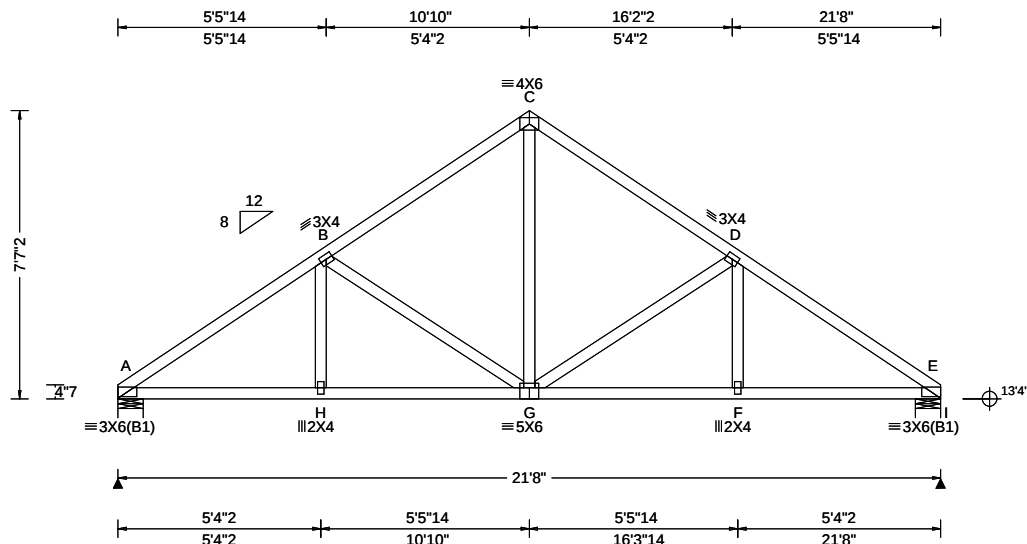
Lumber	Wind	Maximum Bot Chord Forces Per Ply (lbs)
Top chord: 2x4 SP SS; Bot chord: 2x4 SP SS; B3,B4 2x4 SP #1; Webs: 2x4 SP #3; W1,W20 2x6 SP #2; W8 2x4 SP SS;	Wind loads based on MWFRS with additional C&C member design. End verticals exposed to wind pressure. Deflection meets L/120. Extended end vertical web(s) have not been designed to resist horizontal forces or to laterally brace the support. Wind loading based on both gable and hip roof types.	Chords Tens.Comp. Chords Tens. Comp. W - V 383 -365 U - T 552 -328 V - U 851 -748 R - P 1890 -1157

Plating Notes	Maximum Web Forces Per Ply (lbs)
All plates are 3X4 except as noted. (++) - This plate works for both joints covered. (l) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.	Webs Tens.Comp. Webs Tens. Comp. A - W 605 -593 D - T 416 -549 A - V 692 -533 D - R 951 -934 B - U 483 -345 P - F 433 -292 C - T 673 -536 P - G 755 -548

Loading	Professional Engineer
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.	DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER COA #0 278 Florida Certificate of Product Approval #FL1999 04/30/2026

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SEQN: 25303 FROM: DLC	COMN Ply: 1 Qty: 10	Job Number: D15210 10635 SW 63 AVE RESIDENCE Truss Label: L1	Cust: R 9753 JRef: 1YJP97530002 T14 DrwNo: 119.26.1038.53590 KD / DF 04/29/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Speed: 180 mph Risk Category: II Enclosure: Enclosed EXP: D Kzt: NA TCDL: 5.0 psf BCDL: 5.0 psf Mean Height: 17.32 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA W2: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. Load Std: ASCE 7-22 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.043 G 999 360 VERT(CL): 0.085 G 999 240 HORZ(LL): 0.022 E - - HORZ(TL): 0.042 E - - Creep Factor: 2.0 Max TC CSI: 0.443 Max BC CSI: 0.275 Max Web CSI: 0.770 VIEW Ver: 25.02.00C.0120.11	Gravity Loc R+ / R- / Rh / Rw / U / RL A 1257 /- /- /740 /724 /472 I 1257 /- /- /740 /724 /- Wind reactions based on MWFRS A Brg Wid = 8.0 Min Req = 1.5 (Truss) I Brg Wid = 8.0 Min Req = 1.5 (Truss) Bearings A & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 1409 -1830 C - D 1185 -1285 B - C 1185 -1285 D - E 1409 -1830

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

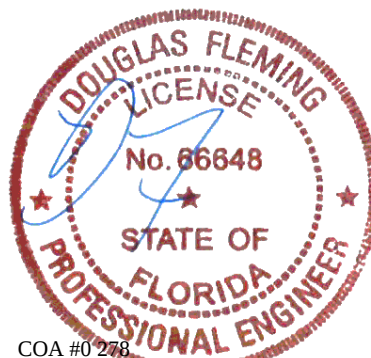
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Chords	Tens.Comp.	Chords	Tens. Comp.
A - H	1418 -970	G - F	1416 -966
H - G	1416 -971	F - E	1418 -965

Webbs	Tens.Comp.	Webbs	Tens. Comp.
B - G	740 -573	G - D	740 -581
C - G	749 -765		



COA #0 278
Florida Certificate of Product Approval #FL1999
04/30/2026

Submission No :		SUBMITTAL REVIEW	
Project# No :		Z. BELFRANIN PE	
10635 S.W. 63RD AVENUE		Discipline :	
X	Reviewed- No Exceptions Taken		Reviewed-Review and Resubmit
	Reviewed-As Noted		Reviewed by Consultant Not Required
Review is solely for general conformity with contract. The consultant does not warrant or represent that information in this submittal is accurate or complete. Review does not relieve Spatiality Engineer/Contractor of responsibility for error/omissions in design, including this submittal, that are the Spatiality Engineer/ Contractor's responsibility, and for conforming/correlating with all quantities/dimensions, performing the Work, selecting performance means/methods, coordinating with other parts of the Work/between trades, and performing the Work safety. Notwithstanding this review, Spatiality Engineer/ Contractor remains solely responsible for contract compliance.			
By : ZTB		Date MAY 13, 2026	

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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

CLR Reinforcing Member Substitution

This detail is to be used for ANSI/TPI 1-2014 standards and older when a Continuous Lateral Restraint (CLR) is specified on a truss design and an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

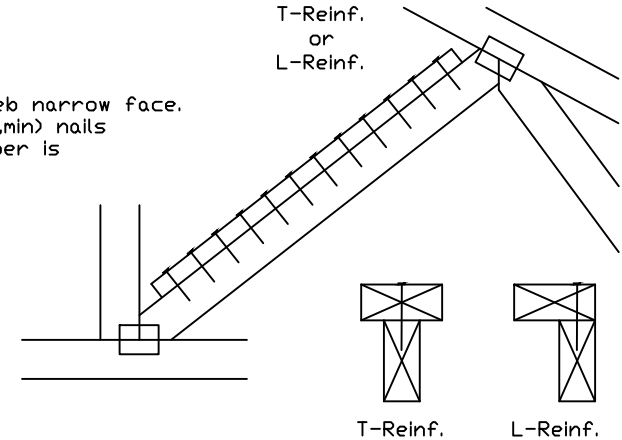
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(X)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(X)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(X) Center scab on wide face of web. Apply (1) scab to each face of web.

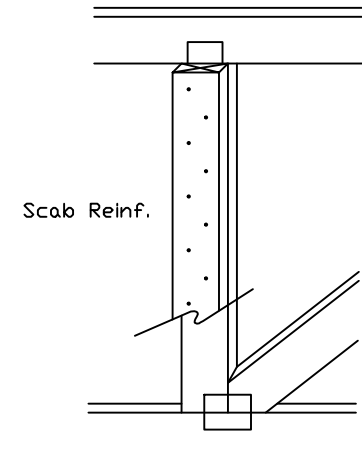
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.131"x3.0",min) nails at 6' o.c. Reinforcing member is a minimum 80% of web member length.

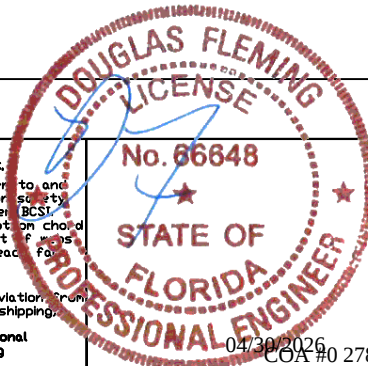


Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.131"x3.0",min) nails at 6' o.c. Reinforcing member is a minimum 80% of web member length.



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10635 S.W. 63RD AVENUE		Discipline :	
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	Reviewed-As Noted		Reviewed by Consultant Not Required
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By : ZTB		Date : 01/23/2026	



Rev: 01/23/26



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North Building, 4th Floor
Glenview, IL 60025

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For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com TPI: www.tpinet.org SBCA: www.sbcacomponents.com ICC: www.iccsafe.org

TC LL	PSF	REF	CLR Subst.
TC DL	PSF	DATE	01/02/19
BC DL	PSF	DRWG	BRCLBSUB0119
BC LL	PSF		
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

Valley Detail - ASCE 7-22: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

****Attach each valley to every supporting truss with:**
 535# connection or with (1) Simpson H2.5A or equivalent connector for
 ASCE 7-22 180 mph. 30' Mean Height, Part. Enc.
 Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00
 Or
 ASCE 7-22 160 mph. 30' Mean Height, Part. Enc.
 Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of separate 2x4 members.

All plates shown are Alpine

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7'-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.131" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with properly attached, rated sheathing applied prior to valley truss installation.

Or

Purlins at 24" o.c. or as otherwise specified on engineer's sealed design

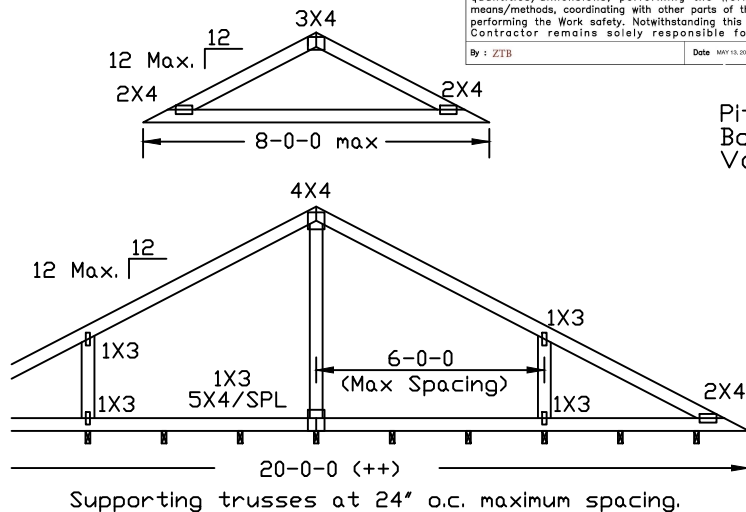
Or

By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

******* Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

Larger spans may be built as long as the vertical height does not exceed 14'-0".

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10635 S.W. 63RD AVENUE		Discipline:	
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By: ZTB		Date: MAY 12, 2023	

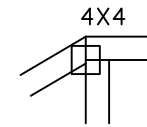
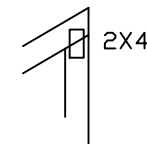


Pitched Cut Bottom Chord Valley

Valley Spacing

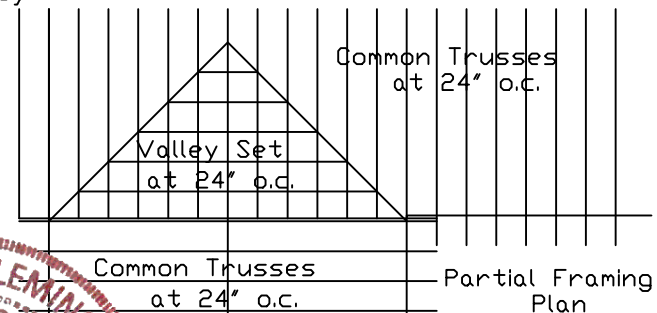
Square Cut Bottom Chord Valley

Purlin Spacing***



Stubbed Valley End Detail

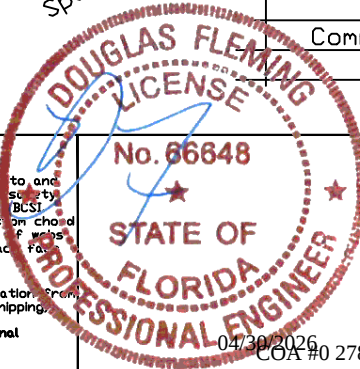
Optional Hip Joint Detail



Rev: 01/23/26



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TC LL	30	30	40PSF	REF	VALLEY DETAIL
TC DL	20	15	7PSF	DATE	07/03/2023
BC DL	10	10	10 PSF	DRWG	VAL180220723
BC LL	0	0	0 PSF		
TOT. LD.	60	55	57PSF		
DUR.FAC.	1.25/1.33	1.15	1.15		
SPACING	24.0"				



A Division of 

Hanger Approval
Submittals

Hanger Approvals for Truss-to-Truss Connections

Attached to this package are the approvals for the following Hangers/Straps.

Hanger / Strap I.D. Number	State of Florida Approval Number
LUS-MUS-HUS-HHUS-HGUS	FL10531-R6
THGB-THGBH-THGW- THGBV-THGBHV-THGWV	FL11468-R5
THA-THAC	FL10447-R7
SUR-SUL-HSUR-HSUL	FL10531-R6
LTS-MTS-HTS	FL13872-R4
THJA26	FL10447-R7
GTWS	FL6223

LUS/MUS/HUS/HHUS/HGUS

Face-Mount Joist Hangers



This product is preferable to similar connectors because of a) easier installation, b) higher loads, c) lower installed cost, or a combination of these features.

The double-shear hanger series, ranging from the light-capacity LUS hangers to the highest-capacity HGUS hangers, feature innovative double-shear nailing that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation and the use of all common nails for the same connection.

For medium-load truss applications, the MUS offers a lower-cost alternative and easier installation than the HUS or THA hangers, while providing greater load capacity and bearing than the LUS.

Material: See tables on pp. 200–201

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating. See Corrosion Information, pp. 12–15.

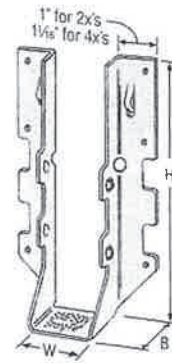
Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

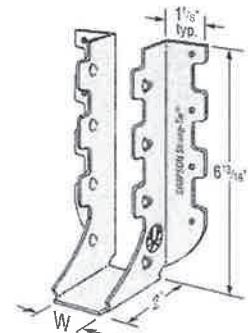
Options:

- LUS and MUS hangers cannot be modified
- Concealed flanges are not available for HGUS and HHUS
- Other sizes available; consult your Simpson Strong-Tie representative

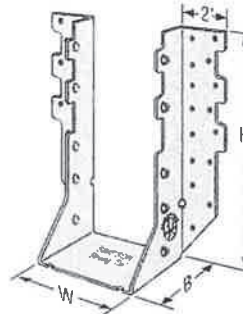
Codes: See p. 11 for Code Reference Key Chart



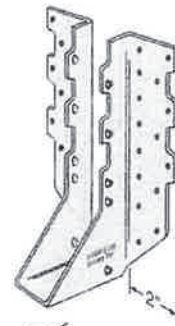
✓ LUS28



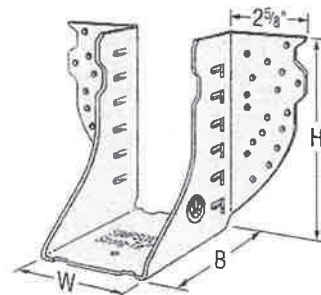
✓ MUS28



✓ HHUS210-2



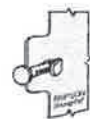
✓ HUS210
(HUS26, HUS28, and HHUS similar)



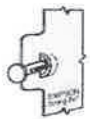
✓ HGUS28-2



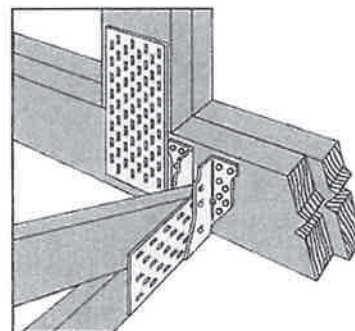
Double-Shear
Nailing
Top View



Double-Shear Nailing
Side View
Do not bend tab



Dome Double-Shear
Nailing Side View
(Available on
some models)



Typical HUS26
Installation
with Reduced
Heel Height
(multiple member
fastening
by designer)

LUS/MUS/HUS/HHUS/HGUS

Face-Mount Joist Hangers (cont.)

Model No.	Min. Heel Height	Ga.	Dimensions (in.)			Fasteners	
			W	H	B	Carrying Member	Carried Member
Single 2x Sizes							
LUS24	2 $\frac{1}{8}$	18	1 $\frac{1}{8}$	3 $\frac{1}{8}$	1 $\frac{1}{4}$	(4) 0.148 x 3	(2) 0.148 x 3
LUS26	4 $\frac{1}{4}$	18	1 $\frac{1}{8}$	4 $\frac{1}{4}$	1 $\frac{1}{4}$	(4) 0.148 x 3	(4) 0.148 x 3
MUS26	4 $\frac{1}{16}$	18	1 $\frac{1}{8}$	5 $\frac{1}{8}$	2	(6) 0.148 x 3	(6) 0.148 x 3
HUS26	4 $\frac{5}{16}$	16	1 $\frac{1}{8}$	5 $\frac{1}{8}$	3	(14) 0.162 x 3 $\frac{1}{2}$	(6) 0.162 x 3 $\frac{1}{2}$
HGUS26	4 $\frac{3}{16}$	12	1 $\frac{1}{8}$	5 $\frac{3}{8}$	5	(20) 0.162 x 3 $\frac{1}{2}$	(8) 0.162 x 3 $\frac{1}{2}$
LUS28	4 $\frac{3}{16}$	18	1 $\frac{1}{8}$	6 $\frac{1}{8}$	1 $\frac{1}{4}$	(6) 0.148 x 3	(4) 0.148 x 3
MUS28	6 $\frac{1}{16}$	18	1 $\frac{1}{8}$	6 $\frac{1}{8}$	2	(8) 0.148 x 3	(8) 0.148 x 3
HUS28	6 $\frac{1}{2}$	16	1 $\frac{1}{8}$	7	3	(22) 0.162 x 3 $\frac{1}{2}$	(8) 0.162 x 3 $\frac{1}{2}$
HGUS28	6 $\frac{1}{8}$	12	1 $\frac{1}{8}$	7 $\frac{1}{8}$	5	(36) 0.162 x 3 $\frac{1}{2}$	(12) 0.162 x 3 $\frac{1}{2}$
LUS210	4 $\frac{1}{4}$	18	1 $\frac{1}{8}$	7 $\frac{1}{8}$	1 $\frac{1}{4}$	(8) 0.148 x 3	(4) 0.148 x 3
HUS210	8 $\frac{1}{8}$	16	1 $\frac{1}{8}$	9	3	(30) 0.162 x 3 $\frac{1}{2}$	(10) 0.162 x 3 $\frac{1}{2}$
HGUS210	8 $\frac{3}{8}$	12	1 $\frac{1}{8}$	9 $\frac{1}{8}$	5	(46) 0.162 x 3 $\frac{1}{2}$	(16) 0.162 x 3 $\frac{1}{2}$

1. See table below for allowable loads.

These products are available with additional corrosion protection. For more information, see p. 14.

SS For stainless-steel fasteners, see p. 21.

SD Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 348–352 for more information.

Model No.	DF Allowable Loads					SP Allowable Loads					SPF/HF Allowable Loads					Code Ref.
	Uplift (160)	Floor (100)	Snow (115)	Roof (125)	Wind (160)	Uplift (160)	Floor (100)	Snow (115)	Roof (125)	Wind (160)	Uplift (160)	Floor (100)	Snow (115)	Roof (125)	Wind (160)	
Single 2x Sizes																
LUS24	435	670	765	820	1,045	435	725	825	890	1,120	360	495	565	605	770	IBC, FL, LA
LUS26	1,165	865	990	1,070	1,355	1,165	935	1,070	1,150	1,475	865	635	725	785	1,000	
MUS26	930	1,295	1,480	1,560	1,560	930	1,405	1,560	1,560	1,560	810	955	1,090	1,180	1,350	
HUS26	1,320	2,735	3,095	3,235	3,235	1,320	2,960	3,280	3,280	3,280	1,150	2,350	2,660	2,780	2,780	
HGUS26	1,040	4,355	4,875	5,230	5,390	1,045	4,725	5,295	5,390	5,390	1,005	3,755	4,205	4,495	5,390	
LUS28	1,165	1,100	1,260	1,350	1,725	1,165	1,195	1,360	1,465	1,730	865	810	925	1,000	1,270	
MUS28	1,320	1,730	1,975	2,125	2,255	1,320	1,875	2,135	2,255	2,255	1,150	1,270	1,455	1,575	1,955	
HUS28	1,760	4,095	4,095	4,095	4,095	1,760	4,095	4,095	4,095	4,095	1,480	3,520	3,520	3,520	3,520	
HGUS28	1,650	7,275	7,275	7,275	7,275	1,650	7,275	7,275	7,275	7,275	1,485	6,010	6,255	6,255	6,255	
LUS210	1,165	1,335	1,530	1,640	2,090	1,165	1,450	1,655	1,775	2,270	865	985	1,120	1,215	1,500	
HUS210	2,635	5,450	5,795	5,830	5,830	2,635	5,395	5,780	5,830	5,830	2,220	4,685	4,985	5,015	5,015	
HGUS210	2,090	9,100	9,100	9,100	9,100	2,090	9,100	9,100	9,100	9,100	1,545	6,340	6,730	6,730	6,730	

1. For dimensions and fastener information, see table above. See table footnotes on p. 201.

HHUS/HGUS

See Hanger Options information on pp. 97–99.

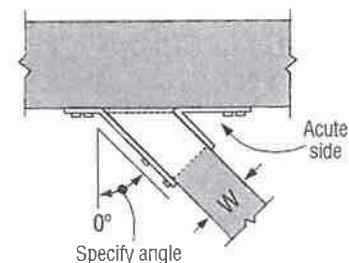
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum allowable download is 0.85 of the table load
- For sloped only or sloped and skewed hangers, the maximum allowable download is 0.65 of the table load
- Uplift loads for sloped/skewed conditions are 0.72 of the table load, not to exceed 2,475 lb.
- The joist must be bevel-cut to allow for double shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Allowable loads are:

HGUS Seat Width	Carried Member End Condition			
	Square Cut		Bevel Cut	
	Uplift	Download	Uplift	Download
W < 2"	0.46	0.62	0.46	0.72
2" < W < 6"	0.41	0.46	0.41	0.85
6" < W < 6.75"		0.40		
6.75" < W < 7.25"	Not allowed	Not allowed		



Top View HHUS Hanger Skewed Right

(joist must be bevel cut)
All joist nails installed on the outside angle (non-acute side).

LUS/MUS/HUS/HHUS/HGUS

These products are available with additional corrosion protection. For more information, see p. 14.

SS For stainless-steel fasteners, see p. 21.

SD Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 348–352 for more information.

	Model No.	Min. Heel Height	Ga.	Dimensions (in.)			Fasteners (in.)		DF/SP Allowable Loads					SPF/HF Allowable Loads					Code Ref.	
				W	H	B	Carrying Member	Carried Member	Uplift (160)	Floor (100)	Snow (115)	Roof (125)	Wind (160)	Uplift (160)	Floor (100)	Snow (115)	Roof (125)	Wind (160)		
Double 2x Sizes																				
	LUS24-2	2¼	18	3½	3½	2	(4) 0.162 x 3½	(2) 0.162 x 3½	410	800	905	980	1,245	355	690	780	845	1,070	IBC, FL, LA	
SS	LUS26-2	4⅞	18	3½	4⅞	2	(4) 0.162 x 3½	(4) 0.162 x 3½	1,060	1,030	1,170	1,265	1,595	910	885	1,005	1,090	1,370		
	HHUS26-2	4⅞	14	3⅞	5⅞	3	(14) 0.162 x 3½	(6) 0.162 x 3½	1,320	2,830	3,190	3,415	4,250	1,135	2,435	2,745	2,935	3,655		
	HGUS26-2	4⅞	12	3⅞	5⅞	4	(20) 0.162 x 3½	(8) 0.162 x 3½	2,155	4,355	4,875	5,230	5,575	2,155	3,755	4,875	5,230	5,255		
SS	LUS28-2	4⅞	18	3½	7	2	(6) 0.162 x 3½	(4) 0.162 x 3½	1,060	1,315	1,490	1,610	2,030	910	1,130	1,280	1,385	1,745		
	HHUS28-2	6⅞	14	3⅞	7¼	3	(22) 0.162 x 3½	(8) 0.162 x 3½	1,760	4,265	4,810	5,155	5,980	1,515	3,670	4,135	4,435	5,145		
	HGUS28-2	6⅞	12	3⅞	7⅞	4	(36) 0.162 x 3½	(12) 0.162 x 3½	3,235	7,460	7,460	7,460	7,460	2,780	6,415	6,415	6,415	6,415		
SS	LUS210-2	6⅞	18	3½	9	2	(8) 0.162 x 3½	(6) 0.162 x 3½	1,445	1,830	2,075	2,245	2,830	1,245	1,575	1,785	1,930	2,435		
	HHUS210-2	8⅞	14	3⅞	9⅞	3	(30) 0.162 x 3½	(10) 0.162 x 3½	3,550	5,705	6,435	6,485	6,485	3,335	4,905	5,340	5,060	5,190		
	HGUS210-2	8⅞	12	3⅞	9⅞	4	(46) 0.162 x 3½	(16) 0.162 x 3½	4,095	9,100	9,100	9,100	9,100	3,520	7,460	7,825	7,825	7,825		
Triple 2x Sizes																				
	HGUS26-3	4⅞	12	4⅞	5½	4	(20) 0.162 x 3½	(8) 0.162 x 3½	2,155	4,355	4,875	5,230	5,575	2,155	3,755	4,875	5,230	5,255	IBC, FL, LA	
	HGUS28-3	6⅞	12	4⅞	7¼	4	(36) 0.162 x 3½	(12) 0.162 x 3½	3,235	7,460	7,460	7,460	7,460	2,780	6,415	6,415	6,415	6,415	FL	
	HHUS210-3	8⅞	14	4⅞	8⅞	3	(30) 0.162 x 3½	(10) 0.162 x 3½	3,405	5,640	6,380	6,485	6,485	2,930	4,850	5,485	5,575	5,575	IBC, FL, LA	
	HGUS210-3	8⅞	12	4⅞	9¼	4	(46) 0.162 x 3½	(16) 0.162 x 3½	4,095	9,100	9,100	9,100	9,100	3,520	7,825	7,825	7,825	7,825		
	HGUS212-3	10⅞	12	4⅞	10¾	4	(56) 0.162 x 3½	(20) 0.162 x 3½	5,205	11,915	13,330	14,290	14,350	4,730	10,280	12,420	12,420	12,420		
	HGUS214-3	12⅞	12	4⅞	12¾	4	(66) 0.162 x 3½	(22) 0.162 x 3½	5,360	13,860	14,350	14,350	14,350	4,915	11,960	12,420	12,420	12,420		
Quadruple 2x Sizes																				
	HGUS26-4	5½	12	6⅞	5⅞	4	(20) 0.162 x 3½	(8) 0.162 x 3½	2,155	4,355	4,875	5,230	5,575	2,155	3,755	4,875	5,230	5,255	IBC, FL, LA	
	HGUS28-4	7¼	12	6⅞	7⅞	4	(36) 0.162 x 3½	(12) 0.162 x 3½	3,235	7,460	7,460	7,460	7,460	2,780	6,415	6,415	6,415	6,415	FL	
	HHUS210-4	8⅞	14	6⅞	8⅞	3	(30) 0.162 x 3½	(10) 0.162 x 3½	3,405	5,640	6,380	6,485	6,485	2,930	4,850	5,485	5,575	5,575	IBC, FL, LA	
	HGUS210-4	9¼	12	6⅞	9⅞	4	(46) 0.162 x 3½	(16) 0.162 x 3½	4,095	9,100	9,100	9,100	9,100	3,520	7,825	7,825	7,825	7,825		
	HGUS212-4	10⅞	12	6⅞	10⅞	4	(56) 0.162 x 3½	(20) 0.162 x 3½	5,205	11,915	13,330	14,290	14,350	4,730	10,280	12,420	12,420	12,420		
	HGUS214-4	12⅞	12	6⅞	12⅞	4	(66) 0.162 x 3½	(22) 0.162 x 3½	5,360	13,860	14,350	14,350	14,350	4,915	11,960	12,420	12,420	12,420		
4x Sizes																				
	LUS46	4⅞	18	3⅞	4¾	2	(4) 0.162 x 3½	(4) 0.162 x 3½	1,060	1,030	1,170	1,265	1,595	910	885	1,005	1,090	1,370	IBC, FL, LA	
	HGUS46	4⅞	12	3⅞	4⅞	4	(20) 0.162 x 3½	(8) 0.162 x 3½	2,155	4,355	4,875	5,230	5,575	2,155	3,755	4,875	5,230	5,255		
	HHUS46	4⅞	14	3⅞	5⅞	3	(14) 0.162 x 3½	(6) 0.162 x 3½	1,320	2,830	3,190	3,415	4,250	1,135	2,435	2,745	2,935	3,655		
	LUS48	4⅞	18	3⅞	6¾	2	(6) 0.162 x 3½	(4) 0.162 x 3½	1,060	1,315	1,490	1,610	2,030	910	1,130	1,280	1,385	1,745		
	HUS48	6⅞	14	3⅞	7	2	(6) 0.162 x 3½	(6) 0.162 x 3½	1,320	1,580	1,790	1,930	2,415	1,135	1,360	1,540	1,660	2,075		
	HHUS48	6½	14	3⅞	7½	3	(22) 0.162 x 3½	(8) 0.162 x 3½	1,760	4,265	4,810	5,155	5,980	1,515	3,670	4,135	4,435	5,145		
	HGUS48	6⅞	12	3⅞	7⅞	4	(36) 0.162 x 3½	(12) 0.162 x 3½	3,235	7,460	7,460	7,460	7,460	2,780	6,415	6,415	6,415	6,415		
	LUS410	6¼	18	3⅞	8¾	2	(8) 0.162 x 3½	(6) 0.162 x 3½	1,445	1,830	2,075	2,245	2,830	1,245	1,575	1,785	1,930	2,435		
	HHUS410	8⅞	14	3⅞	9	3	(30) 0.162 x 3½	(10) 0.162 x 3½	3,550	5,705	6,435	6,485	6,485	3,265	4,905	5,535	5,575	5,575		
	HGUS410	8⅞	12	3⅞	9⅞	4	(46) 0.162 x 3½	(16) 0.162 x 3½	4,095	9,100	9,100	9,100	9,100	3,520	7,825	7,825	7,825	7,825		
	HGUS412	10⅞	12	3⅞	10⅞	4	(56) 0.162 x 3½	(20) 0.162 x 3½	5,205	11,915	13,330	14,290	14,350	4,730	10,280	12,420	12,420	12,420		
	HGUS414	11⅞	12	3⅞	12⅞	4	(66) 0.162 x 3½	(22) 0.162 x 3½	5,360	13,860	14,350	14,350	14,350	4,915	11,960	12,420	12,420	12,420		
Double 4x Sizes																				
	HGUS7.37/10	8⅞	12	7⅞	8⅞	4	(46) 0.162 x 3½	(16) 0.162 x 3½	4,095	9,100	9,100	9,100	9,100	3,520	7,825	7,825	7,825	7,825	IBC, FL, LA	
	HGUS7.37/12	10⅞	12	7⅞	10⅞	4	(56) 0.162 x 3½	(20) 0.162 x 3½	5,205	11,915	13,330	14,290	14,350	4,730	10,280	12,420	12,420	12,420		
	HGUS7.37/14	11⅞	12	7⅞	12⅞	4	(66) 0.162 x 3½	(22) 0.162 x 3½	5,360	13,860	14,350	14,350	14,350	4,915	11,960	12,420	12,420	12,420		

1. Uplift loads have been increased for earthquake or wind loading with no further increase allowed. Reduce where other loads govern.
2. Wind (160) is a download rating.
3. Minimum heel height shown is required to achieve full table loads. For less than minimum heel height, see technical bulletin T-C-REDHEEL at strongtie.com.
4. Truss chord cross-grain tension may limit allowable loads in accordance with ANSI/TPI 1-2014. Simpson Strong-Tie® Connector Selector™ software includes the evaluation of cross-grain tension in its hanger allowable loads. For additional information, contact Simpson Strong-Tie.
5. Loads shown are based on a two-ply 2x carrying member minimum for nailed hangers. With 3x carrying members: when 0.162" x 3 1/2" nails are specified, use 0.162" x 2 1/2" nails into the header and 0.162" x 3 1/2" nails into the joist with no load reduction. When 0.148" x 3" nails are specified, use 0.148" x 2 1/2" nails into the header and 0.148" x 3" nails into the joist with no load reduction. With 2x carrying members: when 0.162" x 3 1/2" nails are specified, use 0.148" x 1 1/2" nails into the header and 0.148" x 3" nails into the joist and reduce the load to 0.64 of the table values. When 0.148" x 3" nails are specified, use 0.148" x 1 1/2" nails into the header and 0.148" x 3" nails into the joist and reduce the load to 0.77 of the table values.
6. Fasteners: Nail dimensions are listed diameter by length. See pp. 21–22 for fastener information.

THGB/THGBH/THGW/THGBV/THGBHV/THGWV

Truss and SCL-to-Truss Girder Hangers

These high-capacity, welded hangers are designed for attaching two-ply, three-ply or four-ply-sized structural composite lumber (SCL) and multi-ply girder trusses to a girder truss.

The THGBV and THGB offer optional installation with Strong-Drive® SDS Heavy-Duty Connector screws, while the bolted THGBHV, THGWV, THGBH and THGW offer higher load capacities.

For the THGBV/THGBHV/THGWV series, two bucket heights are available for each width to accommodate a range of SCL sizes. Options for skewing or dropping the buckets for conditions where the SCL joist is lower than the girder bottom chord provide additional design flexibility for a variety of SCL-to-truss connections.

See the THGQ/THGQH series for a lower-cost alternative that uses Strong-Drive SDS Heavy-Duty Connector screws instead of bolts and offers high-load capacities.

Material: 3 gauge

Finish: Simpson Strong-Tie gray paint

Installation:

- Use all specified fasteners; see General Notes.
- All multiple members must be fastened together to act as a single unit.
- Maximum 11 7/8" bottom chord in the carrying member to allow for the minimum bolt end distance.
- Bolts must be installed symmetrically when using fewer than eight bolts on the eight-bolt backplate.
- Girders must be adequately laterally braced to prevent excessive displacement due to secondary torsional stresses (Ref. ANSI/TPI 1-2014 Section 7.5.3.5).
- Shall be attached to a two-ply girder truss to allow for required minimum screw penetration. See footnote 4.

Options:

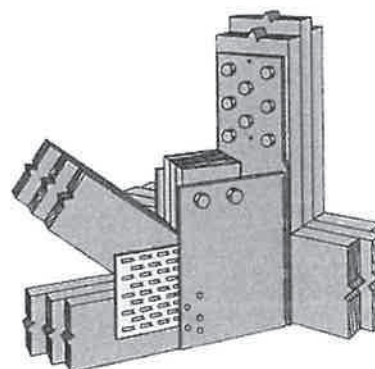
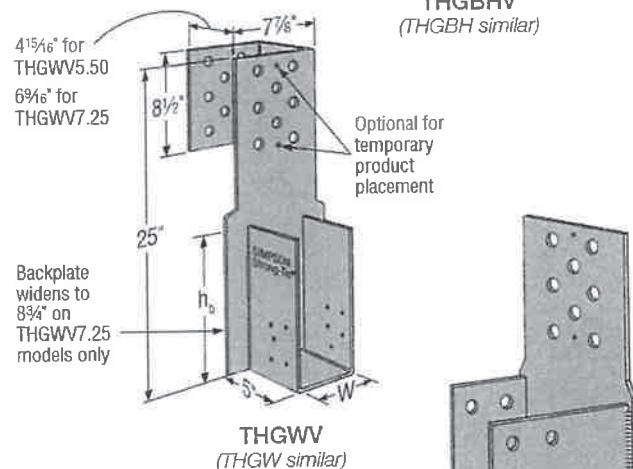
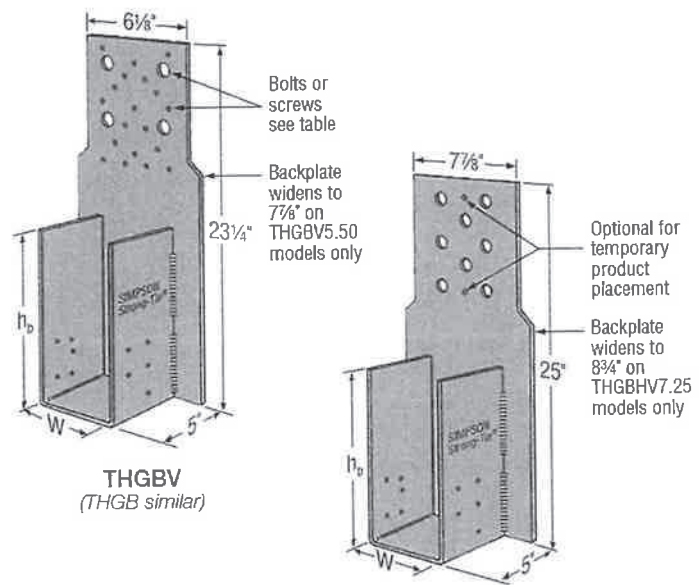
Skewed Seat

- THGB/THGBH/THGBV/THGBHV hangers can be skewed to a maximum of 45°. The maximum allowable down load and uplift load for skew is 0.87 of the table load.

Dropped Bucket

- THGBV/THGBHV backplates can be extended to allow for up to a 6" dropped bucket.
- Allowable loads are 100% of the table loads.
- Order as "X" version, specify the total backplate height, BK_PLT, equal to the hanger height (H) plus the dropped bucket amount (db). Ex: a THGBV3.62/9 with a 4" dropped bucket would have a total backplate height of 27 1/4".

Codes: See p. 11 for Code Reference Key Chart

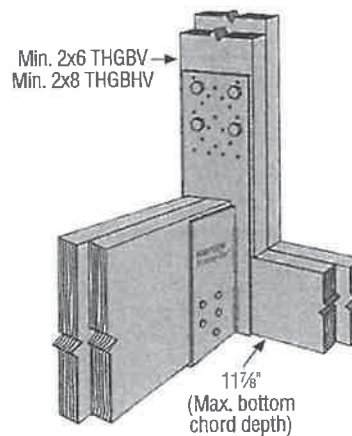
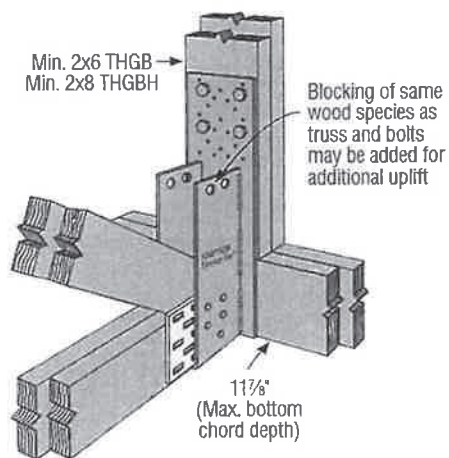
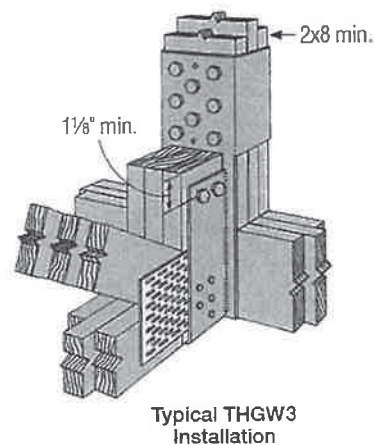


Typical THGBH Installation
Skewed Left

THGB/THGBH/THGW/THGBV/THGBHV/THGWV

Truss and SCL-to-Truss Girder Hangers (cont.)

Joist Dimensions (in.)		Model No.	Hanger Dimensions (in.)	
Width	Depth		W	h _b
3½	9¼ – 14	THGBV3.62/9	3⅝	9
		THGBHV3.62/9		9
	11¼ – 20	THGBV3.62/11		11
		THGBHV3.62/11		11
5¼	9¼ – 14	THGBV5.50/9	5½	9
		THGBHV5.50/9		9
		THGWV5.50/9		9
	11¼ – 20	THGBV5.50/11		11
		THGBHV5.50/11		11
		THGWV5.50/11		11
7	9¼ – 14	THGBHV7.25/9	7¼	9
		THGWV7.25/9		9
	11¼ – 20	THGBHV7.25/11		11
		THGWV7.25/11		11



THGB/THGBH/THGW/THGBV/THGBHV/THGWV

Truss and SCL-to-Truss Girder Hangers (cont.)

Model No.	Beam Width (in.)	Fasteners (in.)		Length of Bolt in Carrying Member	DF/SP Allowable Loads					SPF/HF Allowable Loads					Code Ref.
		Carried Member	Carrying Member		Uplift (160)	Floor (100)	Snow (115)	Roof (125)	Wind (160)	Uplift (160)	Floor (100)	Snow (115)	Roof (125)	Wind (160)	
THGBV3.62/9 THGBV3.62/11	3½	(10) 0.148 x 3	(4) ¾ MB	3	2,570	6,030	6,835	7,375	8,715	2,570	5,160	5,840	6,290	7,320	
				4½		6,910	7,780	8,350	8,715		6,385	7,185	7,320	7,320	
				6		6,910	7,780	8,350	8,715		6,400	7,200	7,320	7,320	
				(19) ¼ x 3 SDS		7,980	8,675	8,675	8,675		5,700	6,245	6,245	6,245	
THGBHV3.62/9 THGBHV3.62/11	3½	(10) 0.148 x 3	(8) ¾ MB	3	2,570	10,105	10,345	10,505	10,915	2,570	7,465	7,675	7,815	8,285	
				4½		10,915	10,915	10,915	10,915		9,165	9,165	9,165	9,165	
				6		10,915	10,915	10,915	10,915		9,165	9,165	9,165	9,165	
THGBV5.50/9 THGBV5.50/11	5¼	(10) 0.148 x 3	(4) ¾ MB	3	2,570	6,030	6,835	7,375	8,715	2,570	5,160	5,840	6,290	7,320	
				4½		6,910	7,780	8,350	8,715		6,385	7,185	7,320	7,320	
				6		6,910	7,780	8,350	8,715		6,400	7,200	7,320	7,320	
				(19) ¼ x 3 SDS		7,980	8,675	8,675	8,675		5,700	6,245	6,245	6,245	
THGBHV5.50/9 THGBHV5.50/11	5¼	(10) 0.148 x 3	(8) ¾ MB	3	2,570	10,915	10,915	10,915	10,915	2,570	9,165	9,165	9,165	9,165	
				4½		12,665	12,665	12,665	12,665		10,500	10,640	10,640	10,640	
				6		12,815	12,815	12,815	12,815		10,500	10,710	10,765	10,765	
THGBHV7.25/9 THGBHV7.25/11	7	(10) 0.148 x 3	(4) ¾ MB	3	2,570	6,040	6,850	7,390	8,715	2,570	5,165	5,845	6,295	7,320	
				4½		6,910	7,780	8,350	8,715		6,385	7,185	7,320	7,320	
				6		6,910	7,780	8,350	8,715		6,400	7,200	7,320	7,320	
THGBHV7.25/9 THGBHV7.25/11	7	(10) 0.148 x 3	(6) ¾ MB	3	2,570	9,065	10,010	10,010	10,010	2,570	7,750	8,410	8,410	8,410	
				4½		10,010	10,010	10,010	10,010		8,410	8,410	8,410	8,410	
				6		10,010	10,010	10,010	10,010		8,410	8,410	8,410	8,410	
THGBHV7.25/9 THGBHV7.25/11	7	(10) 0.148 x 3	(8) ¾ MB	3	2,570	10,915	10,915	10,915	10,915	2,570	9,165	9,165	9,165	9,165	
				4½		13,830	15,060	15,060	15,060		12,650	12,650	12,650	12,650	
				6		13,830	15,060	15,060	15,060		12,650	12,650	12,650	12,650	
THGWV5.50/9 THGWV5.50/11	5¼	(10) 0.148 x 3	(8) ¾ MB	4½	2,570	21,320	21,835	21,835	21,835	2,570	18,340	18,340	18,340	18,340	
THGWV7.25/9 THGWV7.25/11	7	(10) 0.148 x 3	(8) ¾ MB	6	2,570	24,165	24,165	24,165	24,165	2,570	20,300	20,300	20,300	20,300	

1. Allowable loads are based on a SCL (LVL, PSL, or LSL) carried member with an allowable F'_c of 750 psi and equivalent Specific Gravity of 0.50 or higher.

2. Uplift loads have been increased for earthquake or wind loading with no further increase allowed. Reduce where other loads govern.

3. A three-ply carrying member is required for the THGWV5.50/9 and THGWV5.50/11; a four-ply carrying member is required for the THGWV7.25/9 and THGWV7.25/11.

4. Strong-Drive® SDS Heavy-Duty Connector screws require a minimum two-ply (3") carrying member.

5. Bolts and Strong-Drive SDS Heavy-Duty Connector screws (not included) may be installed through metal truss connector plates as approved by the Truss Designer, provided the requirements of ANSI/TPI 1-2014, Sections 7.5.3.4 and 8.9.2 are met.

6. Wind (160) is a download rating.

7. To achieve published loads, install bolts into the girder truss with the nut on the opposite side of the hanger with a standard-cut washer (except THGWVs). Standard-cut washers are required for THGBV and THGBHV.

8. Fasteners: Nail dimensions are listed diameter by length. SDS screws are Simpson Strong-Tie® Strong-Drive SDS Heavy-Duty Connector screws. See pp. 21–22 for fastener information.

THGB/THGBH/THGW/THGBV/THGBHV/THGWV

Truss and SCL-to-Truss Girder Hangers (cont.)

Model No.	Width (W) (in.)	Fasteners (in.)		Length of Bolt in Carrying Member	DF/SP Allowable Loads					SPF/HF Allowable Loads					Code Ref.
		Carried Member	Carrying Member		Uplift (160)	Floor (100)	Snow (115)	Roof (125)	Wind (160)	Uplift (160)	Floor (100)	Snow (115)	Roof (125)	Wind (160)	
THGB2	3 $\frac{5}{16}$	(10) 0.148 x 3 and (2) $\frac{3}{4}$ MB	(4) $\frac{3}{4}$ MB	3	9,250	6,030	6,840	7,375	9,250	7,765	5,160	5,845	6,290	7,770	FL
				4 $\frac{1}{2}$	9,700	6,915	7,780	8,350	9,350	8,145	6,390	7,185	7,320	7,855	
				6	9,700	6,915	7,780	8,350	9,350	8,145	6,405	7,200	7,320	7,855	
THGB2	3 $\frac{5}{16}$	(10) 0.148 x 3 and (2) $\frac{3}{4}$ MB	(19) $\frac{1}{4}$ x 3 SDS	—	9,510	7,980	9,175	9,510	9,510	6,845	5,700	6,605	6,845	6,845	
THGBH2	3 $\frac{5}{16}$	(10) 0.148 x 3 and (2) $\frac{3}{4}$ MB	(8) $\frac{3}{4}$ MB	3	9,700	10,105	10,345	10,505	10,915	8,145	7,465	7,675	7,815	8,285	
				4 $\frac{1}{2}$	9,700	10,105	10,345	10,505	10,915	8,145	8,490	8,690	8,825	9,165	
				6	9,700	10,105	10,345	10,505	10,915	8,145	8,490	8,690	8,825	9,165	
THGB3	4 $\frac{1}{16}$	(10) 0.148 x 3 and (2) $\frac{3}{4}$ MB	(4) $\frac{3}{4}$ MB	3	9,250	6,030	6,840	7,375	9,250	7,765	5,160	5,845	6,290	7,770	
				4 $\frac{1}{2}$	9,700	6,915	7,780	8,350	9,350	8,145	6,390	7,185	7,320	7,855	
				6	9,700	6,915	7,780	8,350	9,350	8,145	6,405	7,200	7,320	7,855	
THGB3	4 $\frac{1}{16}$	(10) 0.148 x 3 and (2) $\frac{3}{4}$ MB	(19) $\frac{1}{4}$ x 3 SDS	—	9,510	7,980	9,175	9,510	9,510	6,845	5,700	6,605	6,845	6,845	
THGBH3	4 $\frac{1}{16}$	(10) 0.148 x 3 and (2) $\frac{3}{4}$ MB	(8) $\frac{3}{4}$ MB	3	9,700	10,915	10,915	10,915	10,915	8,145	9,165	9,165	9,165	9,165	
				4 $\frac{1}{2}$	9,700	12,665	12,665	12,665	12,665	8,145	10,500	10,640	10,640	10,640	
				6	9,700	12,815	12,815	12,815	12,815	8,145	10,500	10,710	10,765	10,765	
THGBH4	6 $\frac{3}{16}$	(10) 0.148 x 3 and (2) $\frac{3}{4}$ MB	(4) $\frac{3}{4}$ MB	3	9,270	6,040	6,850	7,390	9,270	7,785	5,165	5,845	6,295	7,785	
				4 $\frac{1}{2}$	9,700	6,915	7,780	8,350	9,350	8,145	5,810	7,185	7,320	7,855	
				6	9,700	6,915	7,780	8,350	9,350	8,145	5,810	7,200	7,320	7,855	
THGBH4	6 $\frac{3}{16}$	(10) 0.148 x 3 and (2) $\frac{3}{4}$ MB	(6) $\frac{3}{4}$ MB	3	9,700	9,065	10,010	10,010	10,010	8,145	7,750	8,410	8,410	8,410	
				4 $\frac{1}{2}$	9,700	10,010	10,010	10,010	10,010	8,145	8,410	8,410	8,410	8,410	
				6	9,700	10,010	10,010	10,010	10,010	8,145	8,410	8,410	8,410	8,410	
THGBH4	6 $\frac{3}{16}$	(10) 0.148 x 3 and (2) $\frac{3}{4}$ MB	(8) $\frac{3}{4}$ MB	3	9,700	10,915	10,915	10,915	10,915	8,145	9,165	9,165	9,165	9,165	
				4 $\frac{1}{2}$	9,700	13,830	15,060	15,060	15,060	8,145	12,650	12,650	12,650	12,650	
				6	9,700	13,830	15,060	15,060	15,060	8,145	12,650	12,650	12,650	12,650	
THGW3-3 ^e	4 $\frac{1}{16}$	(10) 0.148 x 3 and (2) $\frac{3}{4}$ MB	(8) $\frac{3}{4}$ MB	4 $\frac{1}{2}$	9,700	20,630	20,630	20,630	20,630	8,145	17,330	17,330	17,330	17,330	
THGW3-4 ^e				6	9,700	20,630	20,630	20,630	20,630	8,145	17,330	17,330	17,330	17,330	
THGW4-3 ^e	6 $\frac{3}{16}$	(10) 0.148 x 3 and (2) $\frac{3}{4}$ MB	(8) $\frac{3}{4}$ MB	4 $\frac{1}{2}$	9,700	22,840	22,840	22,840	22,840	8,145	19,185	19,185	19,185	19,185	
THGW4-4 ^e				6	9,700	22,840	22,840	22,840	22,840	8,145	19,185	19,185	19,185	19,185	

- Uplift loads have been increased for earthquake or wind loading with no further increase allowed. Reduce where other loads govern.
- A three-ply carrying member is required for the THGW3-3 and THGW4-3; a four-ply carrying member is required for the THGW3-4 and THGW4-4. For all other models, a minimum two-ply carrying member is required.
- Strong Drive® SDS Heavy-Duty Connector screws require a minimum two-ply (3") carrying member.
- Bolts and Strong Drive SDS Heavy-Duty Connector screws (not included) may be installed through metal truss connector plates as approved by the Truss Designer, provided the requirements of ANSI/TPI 1-2014, Sections 7.5.3.4 and 8.9.2 are met.
- (10) 0.148" x 3" nails for the carried member will achieve the maximum downloads. Uplift loads are 2,570 lb. for DF/SP and 2,210 lb. for SPF/HF. To achieve maximum uplift, install nails and bolts listed in the table.
- Loads for THGW models require that the supported member have minimum 2x6 end verticals to ensure end-grain bearing. Contact Simpson Strong-Tie for loads when horizontal members are bearing in the hanger seat.
- Wind (160) is a download rating.
- To achieve published loads, install bolts into the girder truss with the nut on the opposite side of the hanger with a standard-cut washer (except THGWs). Standard-cut washers are required for THGB and THGBH.
- Fasteners:** Nail dimensions are listed diameter by length, SDS screws are Simpson Strong-Tie® Strong-Drive SDS Heavy-Duty Connector screws. See pp. 21-22 for fastener information.

THA/THAC

Adjustable Truss Hangers



This product is preferable to similar connectors because of (a) easier installation, (b) higher loads, (c) lower installed cost, or a combination of these features.

The THA series have extra long straps that can be field-formed to give height adjustability and top-flange hanger convenience. THA hangers can be installed as top-flange or face-mount hangers.

THA4x and THA2x-2 models feature a dense nail pattern in the straps, which provides more installation options and allows for easy top-flange installation.

Material: See table

Finish: Galvanized. Some products available in ZMAX® or HDG coating. See Corrosion Information, pp. 12–15.

Installation:

- Use all specified fasteners: see General Notes.

The following installation methods may be used:

- **Top-Flange Installation** — The straps must be field formed over the header 2" minimum (27/16" for the THA29). Install top and face nails according to the table. Top nails shall not be within 1/4" from the edge of the top-flange members. For all top-flange (max.) conditions, nails used for joist attachment must be driven at an angle so that they penetrate through the joist and into the header. For top-flange (min.) installations (not applicable to the THA29), straighten the double-shear nailing tabs and install the nails straight into the joist. Top-flange (max.) installations require full backing to allow for joist slanted fasteners to be properly installed.
- **Face-Mount (Min.) Installation** — Install face nails according to the table, with at least half of the required fasteners in the top half of the header. Not all nail holes in the straps will be filled. Nails must have a minimum 1/2" edge distance. Straighten the double-shear nailing tabs and install the joist nails straight into the joist. The face-mount (min.) installation option accommodates conditions where the supported member hangs either partially or entirely below the header.
- **Face-Mount (Max.) Installation** — Install face nails according to the table. Not all nail holes in the straps will be filled except for the following models: THA29, THA213, THA218 and THA413. For all other models with more nail holes than required, the straps may be installed straight or wrapped over with tabulated quantity of face nails installed into the face and top of the header. The lowest four face holes must be filled. Nails used for the joist attachment must be driven at an angle so that they penetrate through the corner of the joist into the header.
- **Uplift** — Lowest face nails must be filled to achieve uplift loads.

Options:

- THA hangers available with the header flanges turned in for 3/8" (except THA413) and larger, with no load reduction — order THAC hanger.

Codes: See p. 11 for Code Reference Key Chart



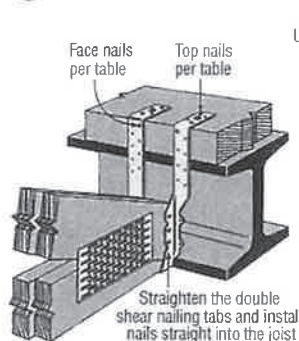
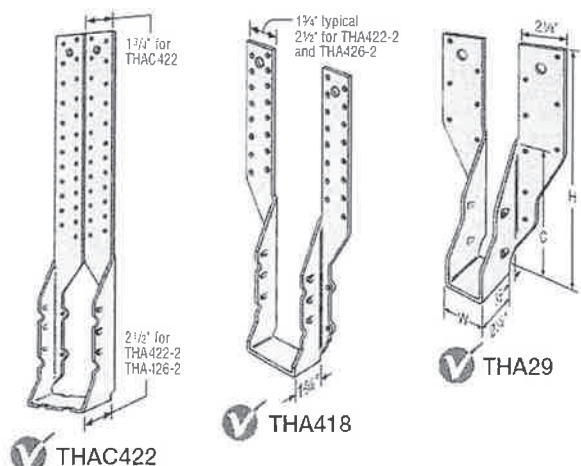
Double-Shear Nailing Top View



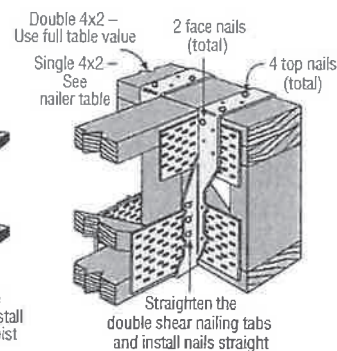
Double-Shear Nailing Side View: Do not bend tab



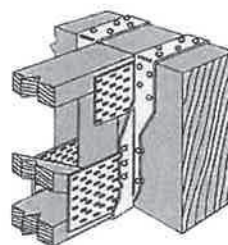
Dome Double-Shear Nailing Side View (Available on some models)



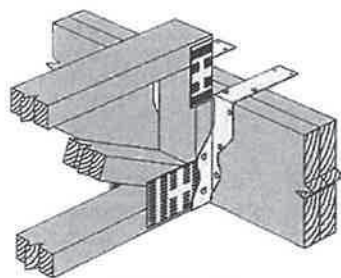
Typical THA Top Flange (Min.) Installation on a Nailer (except THA29)



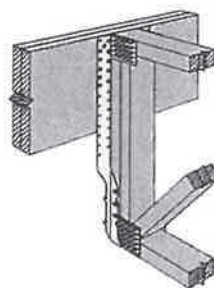
Typical THA422 Top Flange (Min.) Installation on a 4x2 Floor Truss



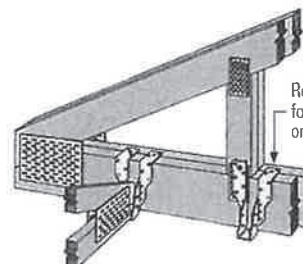
THA422 Face-Mount (Max.) Installation
Straps may be installed straight or wrapped over with tabulated face nails installed into top and face of header.



THA413 Top Flange (Max.) Installation



THA422 Face-Mount (Min.) Installation



Typical THA29 Top Flange Installation

Refer to footnote 5 on p. 193

THA/THAC

Adjustable Truss Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 14.

Model No.	Ga.	Dimensions (in.)			Min. Header Depth (in.)	Fasteners (in.)			DF/SP Allowable Loads					SPF/HF Allowable Loads					Code Ref.
		W	H	C		Carrying Member		Carried Member	Uplift (160)	Floor (100)	Snow (115)	Roof (125)	Wind (160)	Uplift (160)	Floor (100)	Snow (115)	Roof (125)	Wind (160)	
						Top	Face												
Top-Flange Installation ¹																			
THA29 Max.	18	1 5/8	9 1/8	5 1/2	5 1/2	(4) 0.148 x 3	(6) 0.148 x 3	(4) 0.148 x 3	465	2,560	2,560	2,560	2,560	405	2,040	2,040	2,040	2,040	IBC, FL, LA
THA213 Min.	18	1 5/8	13 5/8	5 1/2	—	(4) 0.148 x 3	(2) 0.148 x 3	(4) 0.148 x 1 1/2	—	1,430	1,430	1,430	1,430	—	1,170	1,170	1,170	1,170	
THA213 Max.					7 7/8	(4) 0.148 x 3	(6) 0.148 x 3	(4) 0.148 x 3	855	2,090	2,090	2,090	2,090	750	1,620	1,620	1,620	1,620	
THA218 Min.	18	1 5/8	17 3/8	5 1/2	—	(4) 0.148 x 3	(2) 0.148 x 3	(4) 0.148 x 1 1/2	—	1,430	1,430	1,430	1,430	—	1,170	1,170	1,170	1,170	
THA218 Max.					7 7/8	(4) 0.148 x 3	(6) 0.148 x 3	(4) 0.148 x 3	855	2,090	2,090	2,090	2,090	750	1,620	1,620	1,620	1,620	
THA218-2 Min.	16	3 1/2	17 1/8	8	—	(4) 0.162 x 3 1/2	(2) 0.162 x 3 1/2	(6) 0.148 x 3	—	2,245	2,245	2,245	2,245	—	1,835	1,835	1,835	1,835	
THA218-2 Max.					11 1/4	(4) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	1,855	3,670	3,670	3,670	3,670	1,670	2,790	2,790	2,790	2,790	
THA222-2 Min.	16	3 1/2	22 3/8	8	—	(4) 0.162 x 3 1/2	(2) 0.162 x 3 1/2	(6) 0.148 x 3	—	2,245	2,245	2,245	2,245	—	1,835	1,835	1,835	1,835	
THA222-2 Max.					11 1/4	(4) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	1,855	3,670	3,670	3,670	3,670	1,615	2,790	2,790	2,790	2,790	
THA413 Min.	18	3 3/8	13 5/8	4 1/2	—	(4) 0.148 x 3	(2) 0.148 x 3	(4) 0.148 x 3	—	1,430	1,430	1,430	1,430	—	1,055	1,055	1,055	1,055	
THA413 Max.					7 1/4	(4) 0.148 x 3	(6) 0.148 x 3	(4) 0.148 x 3	855	2,090	2,090	2,090	2,090	750	1,620	1,620	1,620	1,620	
THA418 Min.	16	3 3/8	17 1/2	7 7/8	—	(4) 0.162 x 3 1/2	(2) 0.162 x 3 1/2	(6) 0.148 x 3	—	2,245	2,245	2,245	2,245	—	1,835	1,835	1,835	1,835	
THA418 Max.					11 1/4	(4) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	1,855	3,670	3,670	3,670	3,670	1,670	2,790	2,790	2,790	2,790	
THA422 Min.	16	3 3/8	22	7 7/8	—	(4) 0.162 x 3 1/2	(2) 0.162 x 3 1/2	(6) 0.148 x 3	—	2,245	2,245	2,245	2,245	—	1,835	1,835	1,835	1,835	
THA422 Max.					11 1/4	(4) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	1,855	3,670	3,670	3,670	3,670	1,670	2,790	2,790	2,790	2,790	
THA426 Min.	14	3 3/8	26	7 7/8	—	(4) 0.162 x 3 1/2	(4) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	—	2,870	2,870	2,870	2,870	—	2,270	2,270	2,270	2,270	
THA426 Max.					11 1/4	(4) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	1,855	3,755	3,755	3,755	3,755	1,670	2,945	2,945	2,945	2,945	
THA422-2 Min.	14	7 1/4	22 1/8	9 3/4	—	(4) 0.162 x 3 1/2	(4) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	—	3,330	3,330	3,330	3,330	—	2,465	2,465	2,465	2,465	
THA422-2 Max.					11 1/4	(4) 0.162 x 3 1/2	(8) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	1,855	4,210	4,210	4,210	4,210	1,670	3,285	3,285	3,285	3,285	
THA426-2 Min.	14	7 1/4	26 1/8	9 3/4	—	(4) 0.162 x 3 1/2	(4) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	—	3,330	3,330	3,330	3,330	—	2,465	2,465	2,465	2,465	
THA426-2 Max.					11 1/4	(4) 0.162 x 3 1/2	(8) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	1,855	4,210	4,210	4,210	4,210	1,670	3,285	3,285	3,285	3,285	
Face-Mount (Max.) Installation ⁴																			
THA29	18	1 5/8	9 1/8	5 1/2	9 1/8	—	(16) 0.148 x 3	(4) 0.148 x 3	465	2,295	2,305	2,305	2,305	405	1,965	2,250	2,250	2,250	IBC, FL, LA
THA213	18	1 5/8	13 5/8	5 1/2	13 5/8	—	(14) 0.148 x 3	(4) 0.148 x 3	855	2,060	2,210	2,210	2,210	750	1,760	2,020	2,145	2,145	
THA218	18	1 5/8	17 3/8	5 1/2	17 3/8	—	(18) 0.148 x 3	(4) 0.148 x 3	855	2,210	2,210	2,210	2,210	750	2,145	2,145	2,145	2,145	
THA218-2	16	3 1/2	17 1/8	8	14 1/8	—	(22) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	1,855	3,695	3,695	3,695	3,695	1,670	3,330	3,535	3,535	3,535	
THA222-2	16	3 1/2	22 3/8	8	14 1/8	—	(22) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	1,855	3,695	3,695	3,695	3,695	1,670	3,330	3,535	3,535	3,535	
THA413	18	3 3/8	13 5/8	4 1/2	13 3/8	—	(14) 0.148 x 3	(4) 0.148 x 3	855	2,060	2,210	2,210	2,210	750	1,760	2,020	2,145	2,145	
THA418	16	3 3/8	17 1/2	7 7/8	14 1/8	—	(22) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	1,855	3,695	3,695	3,695	3,695	1,670	3,330	3,535	3,535	3,535	
THA422	16	3 3/8	22	7 7/8	14 1/8	—	(22) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	1,855	3,695	3,695	3,695	3,695	1,670	3,330	3,535	3,535	3,535	
THA426	14	3 3/8	26	7 7/8	16 1/8	—	(30) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	1,955	4,315	4,315	4,315	4,315	1,670	3,225	3,225	3,225	3,225	
THA422-2	14	7 1/4	22 1/8	9 3/4	16 3/8	—	(30) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	1,855	5,170	5,520	5,520	5,520	1,670	4,440	5,010	5,010	5,010	
THA426-2	14	7 1/4	26 1/8	9 3/4	18	—	(38) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	1,855	5,520	5,520	5,520	5,520	1,670	5,010	5,010	5,010	5,010	

1. Uplift loads have been increased for earthquake or wind loading with no further increase allowed. Reduce where other loads govern.

2. Wind (160) is a download rating.

3. Top flange installation loads are based on a minimum two-ply 2x carrying member. For top flange (min.) installation on 4x2 truss carrying members with double top chords, use the specified fasteners for full tabulated values; for single 4x2 top chord or nailer applications, refer to the Nailer Table.

4. Face-mount installation loads are based on a two-ply 2x carrying member minimum. For single 2x carrying members, use 0.148" x 1 1/2" nails in the carrying member and tabulated fasteners in the carried member, and use 0.80 of the table value for 18 gauge, and 0.68 of the table value for 16 gauge and 14 gauge.

5. For the THA 2x models, one strap may be installed vertically according to the face-mount nailing requirements and the other strap wrapped over the truss chord according to the top-flange (min.) nailing requirements (see drawing on p. 192) and achieve full tabulated top flange (min.) installation loads.

6. Refer to installation instructions regarding fastener installation into carried (joist) member. Based on the installation condition, nails will be installed either straight with straightened double-shear nailing tabs or slanted.

7. THA29 may be installed on a single 2x6 or 2x8 carrying member using (2) 0.148" x 3" top nails, (6) 0.148" x 3" face nails and (4) 0.148" x 3" slant nails with an allowable download of 2,020 lb. for DF/SP and 1,500 lb. for SPF/HF.

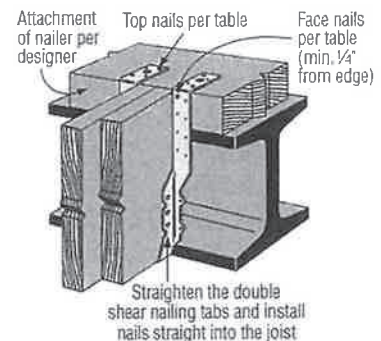
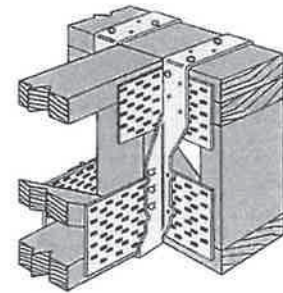
8. Fasteners: Nail dimensions are listed diameter by length. See pp. 21–22 for fastener information.

THA/THAC

Adjustable Truss Hangers (cont.)

Nailer Table

Model No.	Nailer	Top Nailing (in.)	Face Nailing (in.)	Joist Nailing (in.)	Allowable Loads (100/115/125)	
					DF/SP	SPF/HF
THA218-2/ THA222-2	2x	(4) 0.148 x 1 1/2	(2) 0.148 x 1 1/2	(6) 0.148 x 1 1/2	1,335	1,245
		(4) 0.148 x 1 1/2	(2) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	1,415	1,245
	(2) 2x	(4) 0.148 x 3	(2) 0.148 x 3	(6) 0.148 x 3	1,835	1,680
	4x	(4) 0.162 x 3 1/2	(2) 0.162 x 3 1/2	(6) 0.148 x 3	2,245	1,930
THA418/ THA422	2x	(4) 0.148 x 1 1/2	(2) 0.148 x 1 1/2	(6) 0.148 x 1 1/2	1,335	1,245
		(4) 0.148 x 1 1/2	(2) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	1,415	1,245
	(2) 2x	(4) 0.148 x 3	(2) 0.148 x 3	(6) 0.148 x 3	1,735	1,680
	4x	(4) 0.162 x 3 1/2	(2) 0.162 x 3 1/2	(6) 0.148 x 3	2,245	1,930
THA426	2x	(4) 0.148 x 1 1/2	(2) 0.148 x 1 1/2	(6) 0.148 x 1 1/2	1,785	1,360
		(4) 0.148 x 1 1/2	(2) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	2,255	1,940
	(2) 2x	(4) 0.148 x 3	(2) 0.148 x 3	(6) 0.148 x 3	1,735	1,680
	4x	(4) 0.162 x 3 1/2	(4) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	2,435	2,095
THA422-2/ THA426-2	2x	(4) 0.148 x 1 1/2	(2) 0.148 x 1 1/2	(6) 0.148 x 1 1/2	1,375	1,325
		(8) 0.148 x 1 1/2	(2) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	2,345	2,015
	(2) 2x	(4) 0.148 x 3	(4) 0.148 x 3	(6) 0.148 x 3	1,970	1,970
	4x	(4) 0.162 x 3 1/2	(4) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	3,330	2,865

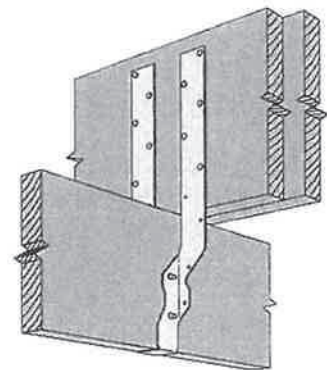


1. Loads for 2x Nailers are applicable to single 4x2 top chord carrying members provided:
 - 1) the hanger is located at a top chord panel point; 2) there is no splice at that panel point location;
 - 3) the floor truss girder must have adequate lateral bracing to prevent excessive displacement due to secondary torsional stresses (refer to ANSI/TPI 1-2014, Section 7.5.3.5).
2. Attachment of nailer to supporting member is the responsibility of the designer.
3. Refer to table on p. 193 for hanger dimensions, minimum top flange requirements and additional footnotes.

Allowable Loads for Face-Mount (Min.) Nailing Installation

Model No.	Ga.	Dimensions (in.)		Fasteners (in.)		Allowable Loads (lb.)					
		W	H	Header (Face) ³	Joist	DF/SP			SPF/HF		
						Floor (100)	Snow (115)	Roof (125)	Floor (100)	Snow (115)	Roof (125)
THA213	18	1 5/8	13 5/8	(10) 0.148 x 3	(4) 0.148 x 1 1/2	1,180	1,200	1,200	1,020	1,160	1,200
THA218	18	1 5/8	17 3/8	(10) 0.148 x 3	(4) 0.148 x 1 1/2	1,180	1,200	1,200	1,020	1,160	1,200
THA218-2	16	3 1/8	17 1/8	(20) 0.148 x 3	(6) 0.148 x 1 1/2	2,440	2,485	2,485	2,100	2,140	2,140
THA222-2	16	3 1/8	22 3/8	(20) 0.148 x 3	(6) 0.148 x 1 1/2	2,440	2,485	2,485	2,100	2,140	2,140
THA413	18	3 3/8	13 3/8	(10) 0.148 x 3	(4) 0.148 x 1 1/2	1,180	1,200	1,200	1,020	1,160	1,200
THA418	16	3 3/8	17 1/2	(20) 0.148 x 3	(6) 0.148 x 1 1/2	2,440	2,485	2,485	2,100	2,140	2,140
THA422	16	3 5/8	22	(20) 0.148 x 3	(6) 0.148 x 1 1/2	2,440	2,485	2,485	2,100	2,140	2,140
THA426	14	3 3/8	26	(30) 0.148 x 3	(6) 0.148 x 1 1/2	3,225	3,225	3,225	2,770	2,770	2,770

1. Loads are based on a min. 2-ply 2x carrying member. 0.148" x 2 1/2" nails may be used instead of the specified 10d commons at 1.00 of the table load. For single-ply 2x or 1 3/4" wide carrying members, use 0.148" x 1 1/2" nails and use 0.77 of the table value. Alternately, #9 x 1 1/2" Strong-Drive® SD Connector screws may be used in place of the specified header and joist nails for full table loads.
2. The joist nails should be installed straight into the carried member by straightening the THA double shear nailing tabs. When used to support 2x4 joists, the THA213 or THA218 may be installed with (2) 0.148" x 1 1/2" nails into the joist (one each side).
3. At least half of the face fasteners must be installed into the upper half of the header, unless some other means of mechanical reinforcement is used to resist the tension perpendicular to grain stresses. Nails must have a minimum 1/2" edge distance.
4. For installations with fewer face fasteners than specified, reduce the allowable load as follows:
Allowable load = No. of Face Nails Used/No. Face Nails in Table x Table Load
5. Fasteners: Nail dimensions are listed diameter by length. SD screws are Simpson Strong-Tie® Strong-Drive SD Connector screws. See pp. 21–22 for fastener information.



Typical THA Face-Mount
Min. Nailing Installation for
Supporting a Suspended Joist

SUR/SUL/HSUR/HSUL

Skewed 45° Face-Mount Hangers



This product is preferable to similar connectors because of (a) easier installation, (b) higher loads, (c) lower installed cost, or a combination of these features.

The SU and HSU series of hangers are skewed 45° left or right. Angled nail slots direct nails for proper installation.

Material: SUR and SUL — 16 gauge; HSUR and HSUL — 14 gauge

Finish: Galvanized. Some products available in ZMAX® coating. See Corrosion Information, pp. 12–15.

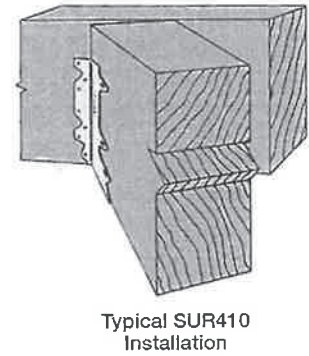
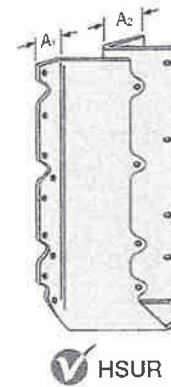
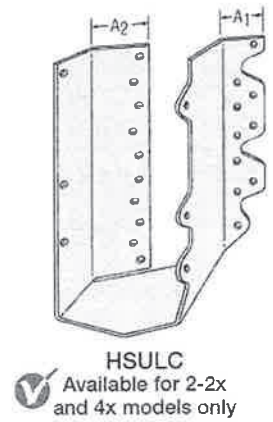
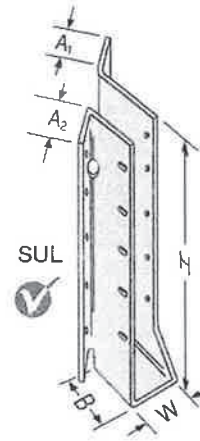
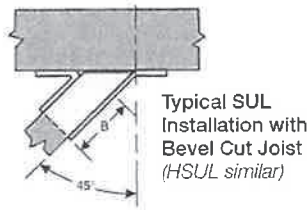
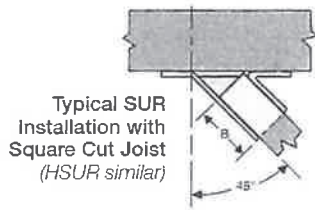
Installation:

- Use all specified fasteners; see General Notes
- These hangers will normally accommodate a 40° to 50° skew
- Illustration shows left and right skews SUR/L (SUR = skewed right; SUL = skewed left)
- The joist end may be square cut or bevel cut

Options:

- Available with the A₂ flange turned in on the 2-2x and 4x models only (see illustration)
- To order, add "C" (for concealed) to the product name
- For example, specify HSURC46, HSULC46, SURC46, or SULC46

Codes: See p. 11 for Code Reference Key Chart



These products are available with additional corrosion protection. For more information, see p. 14.

For stainless-steel fasteners, see p. 21.

Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 348–352 for more information.

Joist Size	Model No.	Dimensions (in.)					Fasteners (in.)		DF/SP Species Header Allowable Loads				SPF/HF Species Header Allowable Loads				Code Ref.
		W	H	B	A ₁	A ₂	Face	Joist	Uplift (160)	Floor (100)	Snow (115)	Roof (125)	Uplift (160)	Floor (100)	Snow (115)	Roof (125)	
2x4	SUR/L24	1 1/8	3 1/2	2	1 1/8	1 1/4	(4) 0.162 x 3 1/2	(4) 0.148 x 1 1/2	395	575	650	705	340	495	560	605	IBC, FL, LA
2x6, x8	SUR/L26	1 1/8	5	2	1 1/8	1 1/8	(6) 0.162 x 3 1/2	(6) 0.148 x 1 1/2	675	865	980	1,055	580	745	845	905	
2x10, x12	SUR/L210	1 1/8	8 3/8	2	1 1/8	1 1/8	(10) 0.162 x 3 1/2	(10) 0.148 x 1 1/2	1,250	1,440	1,630	1,760	1,075	1,240	1,400	1,515	
2x14	SUR/L214	1 1/8	10	2	1 1/8	1 1/8	(12) 0.162 x 3 1/2	(12) 0.148 x 1 1/2	1,890	1,730	1,955	2,110	1,625	1,490	1,680	1,815	
3x10, x12	SUR/L2.56/9	2 1/8	8 1/8	3 1/8	1 1/8	2 1/8	(14) 0.162 x 3 1/2	(2) 0.148 x 1 1/2	210	2,015	2,280	2,465	180	1,735	1,960	2,120	
3x14	SUR/L2.56/11	2 1/8	11 3/8	3 1/8	1 1/8	2 1/8	(16) 0.162 x 3 1/2	(2) 0.148 x 1 1/2	210	2,305	2,610	2,665	180	1,980	2,245	2,290	
(2) 2x6, x8	SUR/L26-2	3 1/8	4 1/8	2 1/8	1 1/8	2 3/8	(8) 0.162 x 3 1/2	(4) 0.162 x 2 1/2	725	1,150	1,305	1,325	625	990	1,120	1,140	
(2) 2x6, x8	HSUR/L26-2	3 1/8	4 1/8	2 1/8	1 1/8	2 3/8	(12) 0.162 x 3 1/2	(4) 0.162 x 2 1/2	725	1,790	1,795	1,795	625	1,540	1,545	1,545	
(2) 2x10, x12	SUR/L210-2	3 1/8	8 1/8	2 1/8	1 1/8	2 3/8	(14) 0.162 x 3 1/2	(6) 0.162 x 2 1/2	1,150	2,015	2,280	2,345	990	1,735	1,960	2,015	
(2) 2x10, x12	HSUR/L210-2	3 1/8	8 1/8	2 1/8	1 1/8	2 3/8	(20) 0.162 x 3 1/2	(6) 0.162 x 2 1/2	1,150	2,980	3,360	3,410	990	2,565	2,890	2,935	
(2) 2x14	HSUR/L214-2	3 1/8	12 1/8	2 1/8	1 1/8	2 3/8	(26) 0.162 x 3 1/2	(8) 0.162 x 2 1/2	1,490	3,875	4,370	4,680	1,280	3,335	3,760	4,025	
4x6, x8	SUR/L46	3 1/8	4 3/4	2 1/8	1	2 3/8	(8) 0.162 x 3 1/2	(4) 0.162 x 3 1/2	730	1,150	1,265	1,265	630	990	1,090	1,090	
4x6, x8	HSUR/L46	3 1/8	4 3/4	2 1/8	1	2 3/8	(12) 0.162 x 3 1/2	(4) 0.162 x 3 1/2	725	1,790	1,795	1,795	625	1,540	1,545	1,545	
4x10, x12	SUR/L410	3 1/8	8 1/2	2 1/8	1	2 3/8	(14) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	1,140	2,015	2,245	2,245	980	1,735	1,930	1,930	
4x10, x12	HSUR/L410	3 1/8	8 1/2	2 1/8	1	2 3/8	(20) 0.162 x 3 1/2	(6) 0.162 x 3 1/2	1,150	2,980	3,360	3,410	990	2,565	2,890	2,935	
4x14	SUR/L414	3 1/8	12 1/2	2 1/8	1	2 3/8	(18) 0.162 x 3 1/2	(8) 0.162 x 3 1/2	1,490	2,400	2,400	2,400	1,280	2,065	2,065	2,065	
4x14	HSUR/L414	3 1/8	12 1/2	2 1/8	1	2 3/8	(26) 0.162 x 3 1/2	(8) 0.162 x 3 1/2	1,490	3,875	4,370	4,680	1,280	3,335	3,760	4,025	

1. Uplift loads have been increased for earthquake or wind loading with no further increase allowed. Reduce where other loads govern.
2. Roof loads are 125% of floor loads unless limited by other criteria. Floor loads may be adjusted for load durations according to the code provided they do not exceed those in the roof column.
3. Truss chord cross-grain tension may limit allowable loads in accordance with ANSI/TPI 1-2014. Simpson Strong-Tie® Connector Selector® software includes the evaluation of cross-grain tension in its hanger allowable loads. For additional information, contact Simpson Strong-Tie.
4. **Fasteners:** Nail dimensions are listed diameter by length. See pp. 21–22 for fastener information.

LTS/MTS/HTS

Twist Straps

Twist straps provide a tension connection between two wood members. They resist uplift at the heel of a truss economically. LTS/MTS have a 2°-bend section and HTS has a 3¼°-bend section that eliminates interference at the transition points between the two members.

Material: LTS — 18 gauge; MTS — 16 gauge; HTS — 14 gauge

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating. See Corrosion Information, pp. 12–15.

Installation:

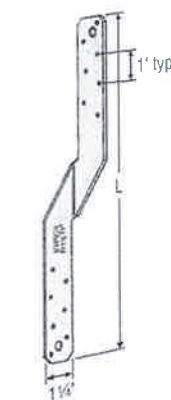
- Use all specified fasteners, with half into each member being connected, to achieve the listed loads; see General Notes.
- LTS, MTS and HTS are available with the bend reversed. Specify "-REV" after the model number, such as MTS16-REV.

Codes: See p. 11 for Code Reference Key Chart

These products are available with additional corrosion protection. For more information, see p. 14.

For stainless-steel fasteners, see p. 21.

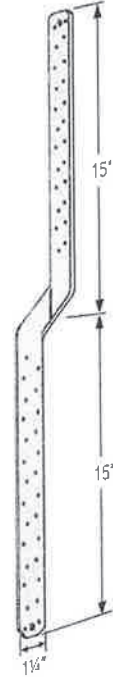
SD Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 348–352 for more information.



LTS12
(MTS and HTS similar)

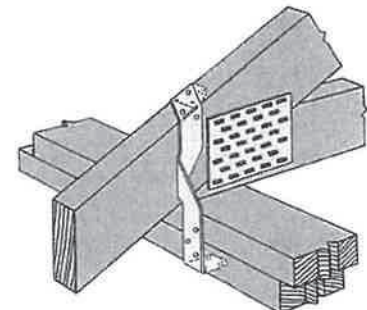


MTS30
(HTS30 similar)



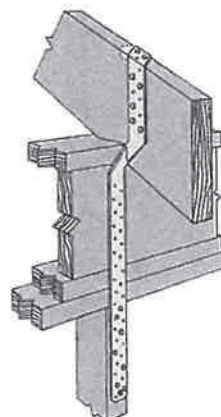
HTS30C
(MTS30C similar)

Model No.	Strap Length (in.)	Total Quantity of Fasteners		DF/SP Allowable Uplift Loads (160)		SPF/HF Allowable Uplift Loads (160)		Code Ref.	
		0.148" x 3" Nails	0.148" x 1 ½" Nails	0.148" x 3" Nails	0.148" x 1 ½" Nails	0.148" x 3" Nails	0.148" x 1 ½" Nails		
SS	LTS12	12	12	12	660	645	570	515	IBC, FL, LA
	LTS16	16							
	LTS20	20							
SS	MTS12	12	14	14	990	990	850	850	
	MTS16	16							
SS	MTS20	20							
	MTS24C	24							
	MTS30	30							
	MTS30C	30							
	HTS16	16	16	16	1,415	1,415	1,215	1,215	
	HTS20	20							
	HTS24	24							
	HTS30	30							
	HTS30C	30							

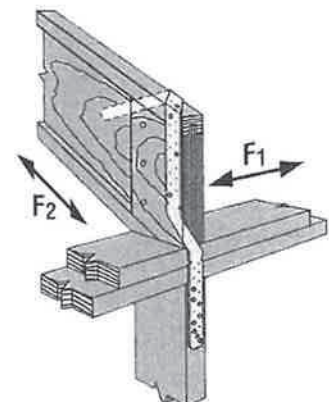


MTS Installation as a Truss-to-Top Plate Tie

1. See pp. 266–267 for Straps and Ties General Notes.
2. All LTS, MTS and HTS models (except for MTS12 and HTS16) have additional nail holes.
3. All straps except the MTS30 and HTS30 have the twist in the center of the strap.
4. Twist straps do not need to be wrapped over the truss to achieve the allowable load.
5. May be installed on the inside face of the stud.
6. Allowable lateral loads are $F_1 = 75$ lb. and $F_2 = 125$ lb., when the following installation requirements are met. The first seven nail holes on each side of the bend must be filled with 0.148" x 1½" minimum nails. All additional fasteners may be installed in any remaining strap holes.
7. For simultaneous loads in more than one direction, the connector must be evaluated using either the Unity Equation or the 75% Rule, as described in Straps and Ties General Notes on p. 267.
8. **Fasteners:** Nail dimensions are listed diameter by length. See pp. 21–22 for fastener information.



Typical MTS30 Installation



MTS30 Installation with I-Joist Rafter

THJA26/LTHJA26

Multiple-Truss Hip/Jack Hanger



This product is preferable to similar connectors because of (a) easier installation, (b) higher loads, (c) lower installed cost, or a combination of these features.

The versatile THJA26 can accommodate right- or left-hand hips (at 45-degree skews), and can be installed before or after the hip and jack. Can also be used for double (terminal) hips.

The LTHJA26 is a lighter-capacity version of the THJA26 and offers the lowest-cost alternative for light hip/jack load applications.

Material: THJA26 — 14 gauge; LTHJA26 — 18 gauge

Finish: Galvanized

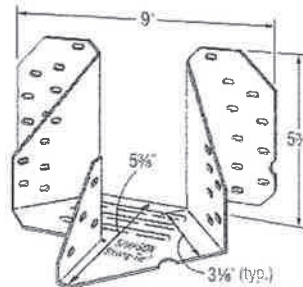
Installation:

- Use all specified fasteners; see General Notes.
- All multi-ply members must be fastened together to act as a single unit.
- Shall be attached to a double girder truss to allow for required minimum nail penetration. See footnote 3 for girder truss requirements.
- LTHJA26 only: 0.148" x 1 1/2" nails must be installed into bottom of hip members through bottom of hanger seat for table loads.

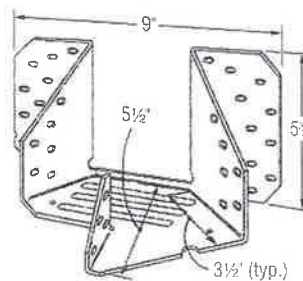
Options:

- These hangers cannot be modified

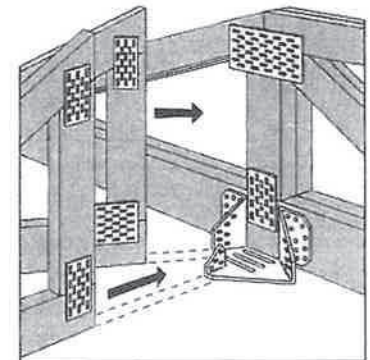
Codes: See p. 11 for Code Reference Key Chart



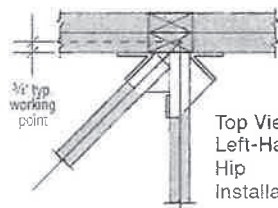
THJA26



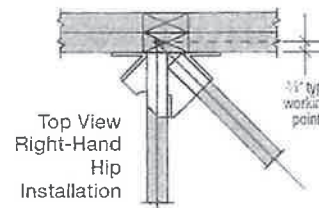
LTHJA26



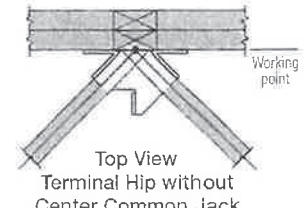
Typical THJA26 Installation
(LTHJA26 similar)



Top View
Left-Hand
Hip
Installation



Top View
Right-Hand
Hip
Installation



Top View
Terminal Hip without
Center Common Jack

Model No.	Carried Member Combination	Fasteners (in.)			Carried Member	DF/SP Allowable Loads					SPF/HF Allowable Loads					Code Ref.
		Carrying Member	Hip (each)	Jack		Uplift (160)	Floor (100)	Snow (115)	Roof (125)	Wind (160)	Uplift (160)	Floor (100)	Snow (115)	Roof (125)	Wind (160)	
LTHJA26	Side hip and center jack	(20) 0.148 x 3	(7) 0.148 x 1 1/2	(4) 0.148 x 1 1/2	Hip	220	845	845	845	845	195	710	710	710	710	IBC, FL
					Jack	75	280	280	280	280	65	235	235	235	235	
					Hip and jack	295	1,125	1,125	1,125	1,125	250	945	945	945	945	
	Double (terminal) hip	(20) 0.148 x 3	(7) 0.148 x 1 1/2	—	Hip (each)	285	630	630	630	630	240	530	530	530	530	
					Two hips	565	1,260	1,260	1,260	1,260	475	1,060	1,060	1,060	1,060	
THJA26	Side hip and center jack	(20) 0.162 x 3 1/2	(6) 0.148 x 1 1/2	(4) 0.148 x 1 1/2	Hip	685	1,890	2,130	2,205	2,205	575	1,590	1,790	1,850	1,850	
					Jack	230	630	710	735	735	195	530	595	615	615	
					Hip and jack	915	2,520	2,840	2,940	2,940	770	2,115	2,385	2,470	2,470	
	Double (terminal) hip	(20) 0.162 x 3 1/2	(6) 0.148 x 1 1/2	—	Hip (each)	460	1,260	1,420	1,470	1,470	385	1,060	1,195	1,235	1,235	
					Two hips	915	2,520	2,840	2,940	2,940	770	2,115	2,385	2,470	2,470	

1. Uplift loads have been increased for earthquake or wind loading with no further increase allowed. Reduce where other loads govern.
2. Wind (160) is a download rating.
3. Loads shown are based on a minimum (2) 2x6 carrying member. For single 2x carrying members (min. 2x6), use 0.148" x 1 1/2" nails and use 0.67 of the table value. For (2) 2x4 carrying members, multiply the download by 0.50.
4. Tabulated hip and jack allowable loads assume that 75% of the total load is distributed to the hip and 25% to the jack. It is permitted to distribute 65% to 85% of the tabulated total load to the hip, and the remaining percentage of total load to the jack. The combined hip and jack load may not exceed the published Total Load.
5. Truss chord cross-grain tension may limit allowable loads in accordance with ANSI/TPI 1-2014, Simpson Strong-Tie® Connector Selector™ software includes the evaluation of cross-grain tension in its hanger allowable loads. For additional information, contact Simpson Strong-Tie.
6. **Fasteners:** Nail dimensions are listed diameter by length. See pp. 21-22 for fastener information.

The GTWS series girder-to-girder hangers feature high uplift capacities along with high gravity load ratings.

Materials: 10 gauge

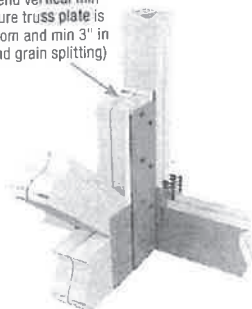
Finish: G90 galvanizing

Codes: FL

Installation:

- Use all specified fasteners. See Product Notes, page 18.
- MiTek's WS structural wood screws are included with hangers where specified.
- **GTWS2T** shall be installed to a minimum 2x4 vertical member of a girder truss with no restriction on the size of the bottom chord.
- **GTWS3T** shall be installed to a minimum 2x6 vertical member of a girder truss with no restriction on the size of the bottom chord.
- **GTWS4T** shall be installed to a minimum 2x8 vertical member of a girder truss with no restriction on the size of the bottom chord.

Supported truss end vertical min 2x6 required. Ensure truss plate is installed flush bottom and min 3" in height (to resist end grain splitting)



Typical GTWS installation



GTWS2T



GTWS3T

Plated Truss

MiTek Stock No.	Ref. No.	Steel Gauge	Dimensions (in)			Fastener Schedule ^{2,3,4}				No. of Plies	DF/SP Allowable Loads (Lbs.)				Code Ref.
						Supporting Truss		Supported Truss			100%	115%	125%	Uplift ¹ 160%	
			W	H	D	Qty	Type	Qty	Type						
			GTWS2T	--	10	3-1/4	16	4	22						
GTWS3T	--	10	4-7/8	16	5	28	WS3	24	WS3	3	11100	12470	12470	12490	
GTWS4T	--	10	6-1/2	16	5	28	WS3	24	WS3	4	11100	12470	12470	12490	

- 1) Uplift loads have been increased 60% for wind or seismic loads; no further increase shall be permitted.
- 2) MiTek's WS3 structural wood screws require a minimum 3" wood penetration.
- 3) MiTek's WS3 (1/4" dia. x 3" long) structural wood screws are included with the GTWS hangers.
- 4) MiTek's WS3 structural wood screws may be installed in both vertical and horizontal members.

General Notes

Trusses are not marked in any way to identify the frequency or location of temporary lateral restraint and diagonal bracing. Follow the recommendations for handling, installing and temporary restraining and bracing of trusses. Refer to **BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses** for more detailed information.

Truss Design Drawings may specify locations of permanent lateral restraint or reinforcement for individual truss members. Refer to the **BCSI-B3*** for more information. All other permanent bracing design is the responsibility of the building designer.

⚠ DANGER The consequences of improper handling, erecting, installing, restraining and bracing can result in a collapse of the structure, or worse, serious personal injury or death.

¡PELIGRO! El resultado de un manejo, levantamiento, instalación, restricción y arrioste incorrecto puede ser la caída de la estructura o aún peor, heridos o muertos.

⚠ CAUTION Exercise care when removing banding and handling trusses to avoid damaging trusses and prevent injury. Wear personal protective equipment for the eyes, feet, hands and head when working with trusses.

¡CAUTELA! Utilice cautela al quitar las ataduras o los pedazos de metal de sujetar para evitar daño a los trusses y prevenir la herida personal. Lleve el equipo protector personal para ojos, pies, manos y cabeza cuando trabaja con trusses.

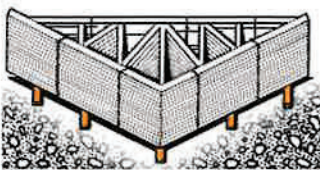
Handling — Manejo

NOTICE Avoid lateral bending.

Evite la flexión lateral.

NOTICE The contractor is responsible for properly receiving, unloading and storing the trusses at the jobsite. Unload trusses to smooth surface to prevent damage.

El contratista tiene la responsabilidad de recibir, descargar y almacenar adecuadamente los trusses en la obra. Descargue los trusses sobre una superficie lisa para prevenir el daño.



Trusses may be unloaded directly on the ground at the time of delivery or stored temporarily in contact with the ground after delivery. If trusses are to be stored for more than one week, place blocking of sufficient height beneath the stack of trusses at 8' (2.4 m) to 10' (3 m) on-center (oc).

Los trusses pueden ser descargados directamente en el suelo en aquel momento de entrega o almacenados temporalmente en contacto con el suelo después de entrega. Si los trusses estarán guardados para más de una semana, ponga bloques de altura suficiente debajo de la pila de los trusses a 8 hasta 10 pies en centro (oc).

If trusses are to be stored for more than one week, cover bundles to protect from the environment.

Para trusses guardados por más de una semana, cubra los paquetes para protegerlos del ambiente.

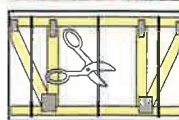
Refer to **BCSI*** for more detailed information pertaining to handling and jobsite storage of trusses.

Vea el folleto **BCSI*** para información más detallada sobre el manejo y almacenado de los trusses en el sitio de trabajo.

Notas generales

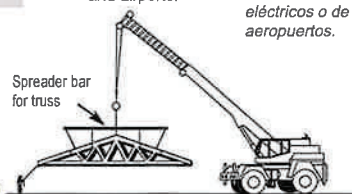
Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de restricción lateral y arrioste diagonal temporales. Use las recomendaciones de manejo, instalación, restricción y arrioste temporal de los trusses. Vea el folleto **BCSI - Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arrioste de los Trusses de Madera Conectados con Placas de Metal*** para información más detallada.

Los dibujos de diseño de los trusses pueden especificar las localizaciones de restricción lateral permanente o refuerzo en los miembros individuales del truss. Vea la hoja resumen **BCSI-B3*** para más información. El resto de los diseños de arriostres permanentes son la responsabilidad del diseñador del edificio.



⚠ CAUTION Use special care in windy weather or near power lines and airports.

¡CAUTELA! Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.



Use proper rigging and hoisting equipment.

Use equipo apropiado para levantar e improvisar.



DO NOT store unbraced bundles upright.

NO almacene verticalmente los trusses sueltos.



DO NOT store on uneven ground.

NO almacene en tierra desigual.



Holisting and Placement of Truss Bundles

Recomendaciones para levantar paquetes de trusses

⚠ DON'T overload the crane.

NO sobrecargue la grúa.

⚠ NEVER use banding to lift a bundle.

NUNCA use las ataduras para levantar un paquete.



⚠ WARNING Do not overload supporting structure with truss bundle.

¡ADVERTENCIA! No sobrecargue la estructura apoyada con el paquete de trusses.

Place truss bundles in stable position.

Puse paquetes de trusses en una posición estable.

A single lift point may be used for bundles of top chord pitch trusses up to 45' (13.7 m) and parallel chord trusses up to 30' (9.1 m).

Use at least two lift points for bundles of top chord pitch trusses up to 60' (18.3 m) and parallel chord trusses up to 45' (13.7 m). Use at least three lift points for bundles of top chord pitch trusses >60' (18.3 m) and parallel chord trusses >45' (13.7 m).

Puede usar un solo lugar de levantar para paquetes de trusses de la cuerda superior hasta 45' y trusses de cuerdas paralelas de 30' o menos. Use por lo menos dos puntos de levantar con grupos de trusses de cuerda superior inclinada hasta 60' y trusses de cuerdas paralelas hasta 45'.

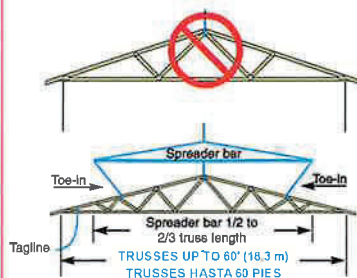
Use por lo menos dos puntos de levantar con grupos de trusses de cuerda superior inclinada mas de 60' y trusses de cuerdas paralelas mas de 45'.

Mechanical Hoisting Recommendations for Single Trusses

Recomendaciones para levantar trusses individuales

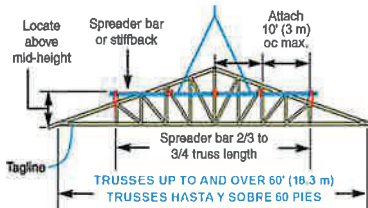
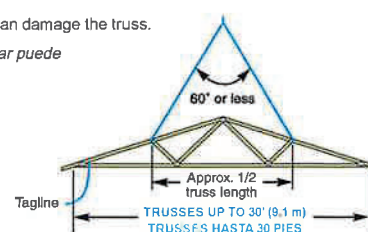
NOTICE Using a single pick-point at the peak can damage the truss.

El uso de un solo lugar en el pico para levantar puede hacer daño al truss.



Hold each truss in position with the erection equipment until top chord temporary lateral restraint is installed and the truss is fastened to the bearing points.

Sostenga cada truss en posición con equipo de grúa hasta que la restricción lateral temporal de la cuerda superior esté instalada y el truss está asegurado en los soportes.

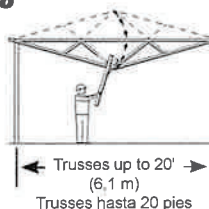


Installation of Single Trusses by Hand

Recomendaciones de levantamiento de trusses individuales por la mano

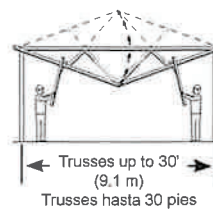
Trusses 20' (6.1 m) or less, support near peak.

Soporte cerca del pico los trusses de 20 pies o menos.



Trusses 30' (9.1 m) or less, support at quarter points.

Soporte en los puntos de cuarto los trusses de 30 pies o menos.



Temporary Restraint & Bracing

Restricción y arrioste temporal

NOTICE Refer to **BCSI-B2*** for more information.

Vea el resumen **BCSI-B2*** para más información.

Locate ground braces directly in line with all rows of top chord temporary lateral restraint (see table in the next column).

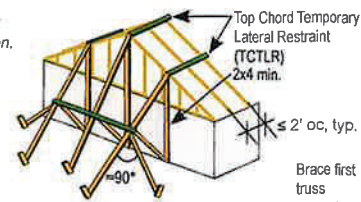
Coloque los arriostres de tierra para el primer truss directamente en línea con cada una de las filas de restricción lateral temporal de la cuerda superior (vea la tabla en la próxima columna).

DO NOT walk on unbraced trusses.

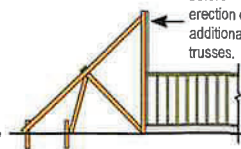
NO camine sobre trusses sueltos.

DO NOT stand on truss overhangs until Structural Sheathing has been applied to the truss and overhangs.

NO se pare en voladizos cerchas hasta Revestimiento estructural ha sido aplicado a la armadura y voladizos.



Brace first truss securely before erection of additional trusses.



Steps to Setting Trusses

Las medidas de la instalación de los trusses

1) Install ground bracing. 2) Set first truss and attach securely to ground bracing. 3) Set next 4 trusses with short member temporary lateral restraint (see below). 4) Install top chord diagonal bracing (see below). 5) Install web member plane diagonal bracing to stabilize the first five trusses (see below). 6) Install bottom chord temporary lateral restraint and diagonal bracing (see below). 7) Repeat process with groups of four trusses until all trusses are set.

1) Instale los arriostres de tierra. 2) Instale el primero truss y afe seguramente al arriostre de tierra. 3) Instale los próximos 4 trusses con restricción lateral temporal de miembro corto (vea abajo). 4) Instale el arriostre diagonal de la cuerda superior (vea abajo). 5) Instale arriostre diagonal para los planos de los miembros secundarios para estabilice los primeros cinco trusses (vea abajo). 6) Instale la restricción lateral temporal y arriostre diagonal para la cuerda inferior (vea abajo). 7) Repita este procedimiento en grupos de cuatro trusses hasta que todos los trusses estén instalados.

NOTICE

Refer to **BCSI-B10** for trusses spaced greater than 2' oc. Consult **BCSI-B10** para trusses espaciados a más de 2' en el centro.

Restraint/Bracing for All Planes of Trusses

Restricción/Arriostre para todos planos de trusses

Minimum lumber used for lateral restraint and diagonal bracing is 2x4 stress-graded lumber. Attach to each truss with at least 2-10d (0.128x3"), 2-12d (0.128x3.25") or 2-16d (0.131x3.5") nails. La madera 2x4 clasificada para astrés es la madera mínima utilizada para restricción lateral y arriostamiento diagonal. Atarlas a cada braguero con al mínimo 2 clavos 10d (0.128x3"), 12d (0.128x3.25") o 16d (0.131x3.5").

This restraint and bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses (PCTs). See top of next column for temporary restraint and bracing of PCTs. Este método de restricción y arriostre es para todo trusses excepto trusses de cuerdas paralelas (PCTs) 3x2 y 4x2. Vea la parte superior de la columna para la restricción y arriostre temporal de PCTs.

1) TOP CHORD PLANE — CUERDA SUPERIOR

Truss Span Longitud de Tramo	Top Chord Temporary Lateral Restraint (TCTLR) Spacing Espaciamiento del Arriostre Temporal de la Cuerda Superior
Up to 30' (9.1 m)	10' (3 m) oc max.
30' - 45' (9.1 m - 13.7 m)	8' (2.4 m) oc max.
45' - 60' (13.7 m - 18.3 m)	6' (1.8 m) oc max.
60' - 80' * (18.3 m - 24.4 m) *	4' (1.2 m) oc max.

*Consult a Registered Design Professional for trusses longer than 60' (18.3 m).
*Consulte a un Profesional Registrado de Diseño para trusses más de 60 pies.

See **BCSI-B2*** for TCTLR options.

Vea el **BCSI-B2*** para las opciones de TCTLR.

NOTICE

Refer to **BCSI-B3*** for Gable End Frame restraint/bracing/reinforcement information.

Para información sobre restricción/arriostre/refuerzo para Armazones Haciales vea el resumen **BCSI-B3***

Note: Ground bracing not shown for clarity.

Repeat diagonal braces for each set of 4 trusses.

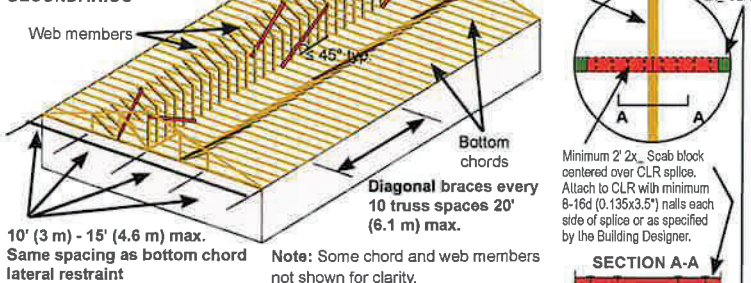
Repita los arriostres diagonales para cada grupo de 4 trusses.

NOTICE

LATERAL RESTRAINT & DIAGONAL BRACING ARE VERY IMPORTANT

¡LA RESTRICCIÓN LATERAL Y EL ARRIOSTRE DIAGONAL SON MUY IMPORTANTES!

2) WEB MEMBER PLANE — PLANO DE LOS MIEMBROS SECUNDARIOS



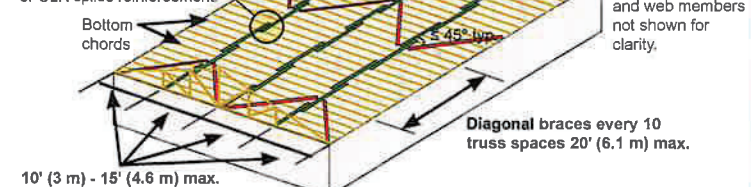
10' (3 m) - 15' (4.6 m) max.

Same spacing as bottom chord lateral restraint

Note: Some chord and web members not shown for clarity.

3) BOTTOM CHORD PLANE — CUERDA INFERIOR

Lateral Restraints - 2x4x12' or greater lapped over two trusses or CLR splice reinforcement.



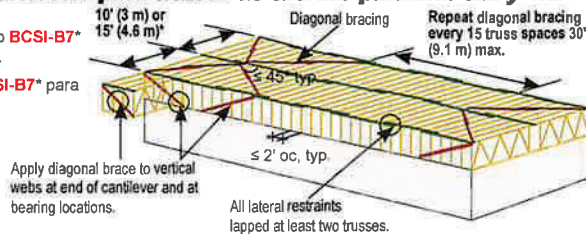
10' (3 m) - 15' (4.6 m) max.

Restraint & Bracing for 3x2 and 4x2 Parallel Chord Trusses

Restricción y arriostre para trusses de cuerdas paralelas 3x2 y 4x2

NOTICE

Refer to **BCSI-B7*** for more information. Vea el resumen **BCSI-B7*** para más información.

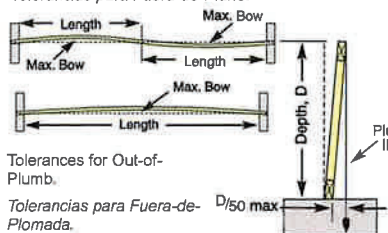


*Top chord temporary lateral restraint spacing shall be 10' (3 m) oc max. for 3x2 chords and 15' (4.6 m) oc for 4x2 chords.

Installing — Instalación

Tolerances for Out-of-Plane.

Tolerancias para Fuera-de-Plano.



Tolerances for Out-of-Plumb.

Tolerancias para Fuera-de-Plomada.

Out-of-Plumb		Out-of-Plane	
D/50	D (ft.)	Max. Bow	Truss Length
1/4"	1'	3/4"	12.5'
(6 mm)	(0.3 m)	(19 mm)	(3.8 m)
1/2"	2'	7/8"	14.6'
(13 mm)	(0.6 m)	(22 mm)	(4.5 m)
3/4"	3'	1"	16.7'
(19 mm)	(0.9 m)	(25 mm)	(5.1 m)
1"	4'	1-1/8"	18.8'
(25 mm)	(1.2 m)	(29 mm)	(5.7 m)
1-1/4"	5'	1-1/4"	20.8'
(32 mm)	(1.5 m)	(32 mm)	(6.3 m)
1-1/2"	6'	1-3/8"	22.9'
(38 mm)	(1.8 m)	(35 mm)	(7.0 m)
1-3/4"	7'	1-1/2"	25.0'
(44 mm)	(2.1 m)	(38 mm)	(7.6 m)
2"	8'	1-3/4"	29.2'
(51 mm)	(2.4 m)	(44 mm)	(8.9 m)
		2"	≥33.3'
		(51 mm)	(10.1 m)

Construction Loading

Carga de construcción

DO NOT proceed with construction until all lateral restraint and bracing is securely and properly in place.

NO proceda con la construcción hasta que todas las restricciones laterales y los arriostres estén colocados en forma apropiada y segura.

DO NOT exceed maximum stack heights in table at right.

Refer to **BCSI-B4*** for more information.

NO exceda las alturas máximas de montón. Vea el resumen **BCSI-B4*** para más información.



NEVER stack materials near a peak, at mid-span, on cantilevers or overhangs.

NUNCA apile los materiales cerca de un pico, a centro de la luz, en cantilevers o aleros.

DO NOT overload small groups or single trusses.

NO sobrecargue pequeños grupos o trusses individuales.

Place loads over as many trusses as possible.

Coloque las cargas sobre tantos trusses como sea posible.

Position loads over load bearing walls.

Coloque las cargas sobre las paredes soportantes.

Alterations — Alteraciones

NOTICE Refer to **BCSI-B5.***

Vea el resumen **BCSI-B5.***

DO NOT cut, alter, or drill any structural member of a truss unless specifically permitted by the truss design drawing.

NO corte, altere o perfore ningún miembro estructural de un truss, a menos que esté específicamente permitido en el dibujo del diseño del truss.

NOTICE Trusses that have been overloaded during construction or altered without the Truss Manufacturer's prior approval may render the Truss Manufacturer's limited warranty null and void.

Trusses que se han sobrecargado durante la construcción o han sido alterados sin la autorización previa del Fabricante de Trusses, pueden hacer nulo y sin efecto la garantía limitada del Fabricante de Trusses.

*Contact the Component Manufacturer for more information or consult a Registered Design Professional for assistance.

NOTE: The truss manufacturer and truss designer rely on the presumption that the contractor and crane operator (if applicable) are professionals with the capability to undertake the work they have agreed to do on any given project. If the contractor believes it needs assistance in some aspect of the construction project, it should seek assistance from a competent party. The methods and procedures outlined in this document are intended to ensure that the overall construction techniques employed will put the trusses into place SAFELY. These recommendations for handling, installing, restraining and bracing trusses are based upon the collective experience of leading personnel involved with truss design, manufacture and installation, but must, due to the nature of responsibilities involved, be presented only as a GUIDE for use by a qualified building designer or contractor. It is not intended that these recommendations be interpreted as superior to the building designer's design specialization for handling, installing, restraining and bracing trusses and it does not preclude the use of other equivalent methods for restraining/bracing and providing stability for the walls, columns, floors, roofs and all the interrelated structural building components as determined by the contractor. Thus, SBCA expressly disclaims any responsibility for damages arising from the use, application, or reliance on the recommendations and information contained herein.

SBCA

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