

MIAMI-DADE COUNTY**DEPARTMENT OF REGULATORY AND ECONOMIC RESOURCES**<http://www.miamidade.gov/building/home.asp>

4/15/2026 3:25:16 PM

Tracking #	Process #	Permit #
2026011895	M2026011895	2026039338

THIS COPY OF PLANS MUST BE AVAILABLE ON BUILDING SITE OR AN INSPECTION WILL NOT BE MADE.				
Process #	Review	Disposition	Reviewer	Date
M2026011895	FIRE	A	FRIAS, JOSE	4/15/2026
M2026011895	IMPACT FEES	N	RICHARDSON, KEITH	4/10/2026
M2026011895	PAVING & DRAINAGE	A	DE TORRES, MAYRA	4/8/2026
M2026011895	RER ENVIRONMENTAL CORE	A	SARDINA, YALENYS	4/15/2026
M2026011895	UPFRONT FEES	A	WEB APPLICATION ID	3/23/2026
M2026011895	WASA	A	GARCIA, MAYTEE	4/8/2026

Disclaimer.

Subject to compliance with all Federal, State, and County Laws, rules and regulations. Miami-Dade County assumes no responsibility for accuracy of or results of these plans.

NOTICE: In addition to the requirements of this permit, there may be additional restrictions applicable to the property that may be found in the public records of this county, and there may be additional permits required from other governmental entities such as water management districts, state agencies or federal agencies.

Stamp Name	Trade	Disposition	Stamp Description
Approved	DERM	A	Approved
Approved	FIRE	A	Approved
Approved As Noted	FIRE	P	Approved as noted. See comments, markups and stamps.
Reference Only.	FIRE	R	Reference only.
Void	FIRE	V	Void

e-Permitting**miamidade.gov**

Resident

Visitor

Business

Employee

**MUNICIPAL INSPECTION REQUIREMENTS AND RECORD 04/15/2026**

MUNICIPAL NO.2026-039338 FOLIO: 2040360000750

JOB SITE ADDRESS 6575 SW 88 ST

PROPOSED USE SCHOOL BUILDINGS /MODULAR

LEGAL 36 54 40 9.43AC W1/2 OF SW1/4 OF SW1/4 LYG S OF

APPLICATION TYPE NEW 8000 SQFT 1 UNITS 1 FLOORS

OWNER NAME GULLIVER PREPARATORY

CONTRACTOR

QUALIFIER

PERMIT TYPE MUNICIPAL BLDG

CATEGORIES 0001 MUNICIPAL GENERAL BUILDING

DATE: 4/15/2026 PROCESS NUMBER: M2026011895 NEW *AMOUNT PAID 1032.12

DERM 1 UP FRONT FEE- 90.00 DERM 1 MIN COMM REV(101.25

DERM 1 PAVING AND DR 210.00 DERM 1 SEWER CAP. CE 120.00

FIRE 8000 NEW BUILDING 313.60 FIRE 8000 FIRE UPFRONT 72.80

FIRE 1 SRI PLAN REVI 267.98 FRWK 313 1ST FIRE MAJO 156.50

RSUR 3 RER 7.5% SUR 25.59 UBS1 1 BLDG 7.5% UPF 5.25

UPMU 70 UPFRONT FEE F 70.00 URS1 1 RER 7.5% UPFR 6.75

4/15/2026 15:25 BNZWEB1 182604152217 WEBIPAS 1032.12

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REQUIRED INSPECTIONS INIT DATE

FIRE

0001 FIRE INSPECTIONS RECOMMENDED

200 FIRE HYDRANTS

208 FIRE TCO INSPECTION

211 PRELIMINARY

209 FIRE FINAL

_____	_____
_____	_____
_____	_____

MUNICIPAL INSPECTION REQUIREMENTS AND RECORD 04/15/2026

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PROPOSED USE SCHOOL BUILDINGS /MODULAR

TO SCHEDULE A FIRE INSPECTION, PLEASE VISIT THE WEB AT
 WWW.MIAMIDADE.GOV/BUILDING OR WWW.MIAMIDADE.GOV/FIRE. YOU WILL
 NEED TO PROVIDE YOUR TEN DIGIT MUNICIPAL INSPECTION NUMBER AND
 INSPECTION TYPE. THE INSPECTION TYPE CAN BE FOUND ON YOUR
 INSPECTION REQUIREMENTS AND RECORDS CARD.

IF YOU HAVE ANY QUESTIONS OR CONCERNS REGARDING AN INSPECTION,

SCHEDULING A PRELIMINARY INSPECTION, OR LOAD BANK TEST
INSPECTION, PLEASE CALL FIRE PREVENTION AT 786-331-4800.

IF YOU HAVE ANY QUESTIONS OR CONCERNS REGARDING A PLAN REVIEW,
PLEASE CALL FIRE ENGINEERING AT (786) 315-2771.

**BE ADVISED THIS IS NOT A PERMIT. PERMIT IS TO BE ISSUED BY
YOUR CORRESPONDING MUNICIPAL BUILDING DEPARTMENT.

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mobile modular

Your Project – Our Commitment

MECHANICAL NOTES:

1. ALL SUPPLY AIR REGISTERS SHALL BE 10 INCHES X 10 INCHES ADJUSTABLE w/ 10 INCHES X 20 INCHES (INSIDE) OVERHEAD FIBERGLASS DUCT, UNLESS OTHERWISE SPECIFIED. DUCTS LOCATED IN VENTILATED ATTIC SPACES SHALL HAVE AN R-6 INSULATION VALUE. DUCTS LOCATED IN UNCONDITIONED INTERIOR SPACE, INTERIOR SPACES SHALL HAVE AN R-4.2 INSULATION VALUE.
2. RESTROOM VENT FANS SHALL PROVIDE 50 CFM MINIMUM PER WATER CLOSET AND / OR URINAL.
3. VENT FANS SHALL BE DUCTED TO THE EXTERIOR AND TERMINATE AT AN APPROVED VENT CAP.
4. HVAC EQUIPMENT SHALL BE EQUIPPED WITH OUTSIDE FRESH INTAKES PROVIDING 20 CFM FOR EACH OCCUPANT OR 50 CFM FOR EACH WATER HEATER CLOSET AND EACH URINAL, WHICHEVER IS GREATER.

PLUMBING NOTES:

1. CUSTOMER ASSUMES ALL RESPONSIBILITY FOR DRINKING WATER FACILITIES AND SERVICE SINK WHEN NOT SHOWN ON THE FLOOR PLAN.
2. TOILETS SHALL BE ELONGATED WITH NON-ABSORBENT OPEN FRONT SEAT.
3. RESTROOMS WALLS SHALL BE COVERED WITH NON-ABSORBENT MATERIAL TO A MINIMUM HEIGHT OF 72 INCHES A.F.F.
4. ALL PLUMBING FIXTURES SHALL HAVE SEPARATE SHUT-OFF VALVES.
5. WATER HEATER SHALL HAVE SAFETY PAN WITH 1 INCH DRAIN TO EXTERIOR, T & P RELIEF VALVE WITH DRAIN TO EXTERIOR, AND A SHUT OFF VALVE WITHIN 3 FEET ON A COLD WATER SUPPLY LINE.
6. DWV SYSTEM SHALL BE EITHER ABS OR PVC - DWV.
7. WATER SUPPLY LINES SHALL BE POLYBUTYLENE, CPVC, OR COPPER; WHEN POLYBUTYLENE SUPPLY LINES ARE INSTALLED THE MAXIMUM WATER HEATER TEMPERATURE SETTING IS 180° F. THE POLYBUTYLENE PIPE SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURERS LIMITATIONS AND INSTRUCTIONS.
8. WATER CLOSETS ARE TANK TYPE AND URINALS ARE FLUSH TANK TYPE UNLESS OTHERWISE SPECIFIED.
9. BUILDING DRAIN AND CLEANOUTS ARE DESIGNED AND SITE INSTALLED BY OTHERS.
10. SHOWERS SHALL BE CONTROLLED BY AN APPROVED MIXING VALVE WITH A MAXIMUM WATER OUTLET TEMPERATURE OF 120° F (48.8° C).
11. THERMAL EXPANSION DEVICE, IF REQUIRED BY WATER HEATER INSTALLED, AND IF NOT SHOWN ON PLUMBING PLAN, IS DESIGNED AND SITE INSTALLED BY OTHERS, SUBJECT TO LOCAL JURISDICTION AND APPROVAL.

ELEVATION NOTES (TYP.)

1. SEE CROSS SECTION FOR METHOD OF ROOF VENTILATION
2. HANDICAP RAM(S), STAIRS (S), AND HANDRAILS ARE TO BE DESIGNED AND SITE INSTALLED BY OTHERS, SUBJECT TO LOCAL JURISDICTION AND APPROVAL.
3. FOUNDATION ENCLOSURE (WHEN PROVIDED) MUST HAVE 1 SQUARE FOOT NET VENT AREA PER 1/1 1/150th OF THE FLOOR AREA, AND AN 18" X 24" MINIMUM CRAWL SPACE ACCESS. SITE INSTALLED BY OTHERS, SUBJECT TO LOCAL JURISDICTION AND APPROVAL.

NOTE:
THIS BUILDING DOES NOT MEET THE FLORIDA ENHANCED HURRICANE PROTECTION AREA REQUIREMENTS.

NOTE:
THESE PLANS HAVE BEEN DESIGNED IN ACCORDANCE WITH FAC RULE 61-41.

NOTE:
BUILDING PLACED IN A HIGH RISK ENVIRONMENT FOR LIGHTNING STRIKE REQUIRES A HIGH RISK LIGHTNING STRIKE EVALUATION.

NOTE:
THIS BUILDING HAS BEEN DESIGNED TO MEET THE REQUIREMENTS FOR PLACEMENT IN THE HIGH VELOCITY HURRICANE ZONE.

NOTE:
MINIMUM PLUMBING FACILITIES TO BE PROVIDED IN ACCORDANCE WITH FBC 403.4.1 AND SUBJECT TO THE APPROVAL BY THE LOCAL AUTHORITY HAVING JURISDICTION.

NOTE:
THESE PLANS HAVE BEEN DESIGNED IN ACCORDANCE WITH FLORIDA FIRE PREVENTION CODE, 6TH EDITION (2017).

NOTE:
THE FLOOR AND ROOF DESIGN OF THIS PLAN IS "LIGHT-FRAME TRUSS TYPE CONSTRUCTION" AS REFERENCED IN F.A.C. RULE 69A.3.012. POSTING OF NOTICE SIGN(S) AS REQUIRED BY F.A.C. RULE 69A.3.012(6) SHALL BE SITE INSTALLED AND IS THE RESPONSIBILITY OF THE BUILDING OWNER.

NOTE:
THE EXTERIOR WALLS OF THIS UNIT HAVE BEEN CONSTRUCTED TO PROVIDE A 1 HOUR FIRE RATING. THE SIDING PANELS HAVE BEEN TESTED IN ACCORDANCE WITH ASTM E119 AND HAVE BEEN APPROVED UNDER ASSEMBLY #: N015328-103870.

GENERAL NOTES:

1. ACCESS TO BUILDING FOR PERSONS IN WHEELCHAIRS IS DESIGNED BY AND FIELD BUILT BY OTHERS AND SUBJECT TO ALL LOCAL JURISDICTIONS. AT LEAST 50% OF PUBLIC ENTRANCES (INCLUDING PRIMARY ENTRANCE) AND ALL REQUIRED EXITS MUST BE ACCESSIBLE.
2. ALL DOORS SHALL BE OPENABLE FROM THE EGRESS SIDE WITHOUT THE USE OF A KEY, TOOL, SPECIAL KNOWLEDGE OR EFFORT. MANUALLY OPERATED FLUSH BOLTS OR SURFACE BOLTS SHALL NOT BE USED.
3. ALL GLAZING WITHIN A 48 INCH ARC OF DOORS, WHOSE BOTTOM EDGE IS LESS THAN 80 INCHES ABOVE THE FLOOR, AND ALL GLAZING IN DOORS SHALL BE SAFETY, TEMPERED OR ACRYLIC PLASTIC SHEET.
4. ALL STEEL STRAPS REFERENCED ON FLOOR PLAN SHALL BE 1.5 INCH X 26 GA. W/ (8) 15 GA. X 7/16 INCH CROWN X 1/2 INCH STAPLES WITH A MINIMUM OF 1" PENETRATION EACH END OF STRAP OR 5 MIN. CORRIDOR WIDTH IS 44 INCHES.
5. MIN. CORRIDOR FINISH IS CLASS B (GYPSUM).
7. WINDOW AND DOOR HIGH WIND STORM COVERINGS PER CODE TO BE SUPPLIED AND SITE INSTALLED BY OTHERS SUBJECT TO LOCAL JURISDICTION AND APPROVAL.
8. PLAN REVIEW AND INSPECTION REQUIRED BY CHAPTER 833 F.S. TO BE DONE BY THE LOCAL FIRE SAFETY INSPECTOR.
9. PORTABLE FIRE EXTINGUISHER PER N.F.P.A. - 101 INSTALLED BY OTHERS ON SITE, AND SUBJECT TO LOCAL JURISDICTION AND APPROVAL.
10. THIS BUILDING REQUIRES A FIRE SEPARATION DISTANCE IN ACCORDANCE WITH TABLE 602 OF THE FLORIDA BUILDING CODE, 6TH EDITION (2017).
11. WHEN LOW SIDE OF ROOF PROVIDES LESS THAN 6" OF OVERHANG GUTTERS AND DOWNSPOUTS WILL BE REQUIRED, SITE INSTALLED AND SUBJECT TO LOCAL JURISDICTION.
12. IN WIND BORNE DEBRIS REGIONS, EXTERIOR GLAZING SHALL BE PROTECTED WITH AND IMPACT RESISTANT COVERING OR WITH MINIMUM 7/16" WOOD STRUCTURAL PANELS PER SECTION 1609.1.2 OF THE FBC. PANELS SHALL BE PRECUT TO COVER THE GLAZED OPENINGS WITH ATTACHMENT HARDWARE PROVIDED PER TABLE 1609.1.2 THE IMPACT RESISTANT COVERING OR WOOD STRUCTURAL PANELS ARE TO BE PROVIDED ON SITE BY OTHERS, SUBJECT TO LOCAL JURISDICTION AND APPROVAL.
13. WIND BORNE DEBRIS REGIONS ARE AREAS WITHIN ONE MILE OF THE COASTAL, NEAR HIGH WATER LINE WHERE THE ULTIMATE DESIGN WIND SPEED IS 130 MPH OR GREATER AND AREAS WHERE THE ULTIMATE DESIGN WIND SPEED IS 140 MPH OR GREATER.
14. ALL MATERIALS USED IN THE CONSTRUCTION OF THE BUILDING WHICH ARE COVERED BY THE FLORIDA BUILDING COMMISSION CHAPTER 61020-3.006 RULES SHALL HAVE CURRENT FLORIDA PRODUCT APPROVAL.
15. THESE PLANS COMPLY WITH THE FLORIDA BUILDING CODE, 6TH EDITION (2017).
16. THE RAISED SEAL SET OF PLANS ARE ON FILE IN THE THIRD PARTY AGENCY'S OFFICE AS DIRECTED BY DBPR.
16. EMERGENCY LIGHTING SHALL BE CAPABLE OF PROVIDING INITIAL ILLUMINATION THAT IS AT LEAST AN AVERAGE OF 1 FOOT-CANDLE AND A MIN. OF 1 FC MEASURED ALONG THE EGRESS AT THE FLOOR LEVEL. ILLUMINATION LEVELS SHALL BE PERMITTED TO DECLINE TO .6 C AVERAGE AND A MINIMUM AT ANY POINT OF .06 FC AT THE END OF THE EMERGENCY LIGHT TIME DURATION. A MAXIMUM TO-MINIMUM ILLUMINATION UNIFORMITY RATIO OF 40 TO 1 SHALL NOT BE EXCEEDED. THE EMERGENCY POWER SYSTEM SHALL PROVIDE POWER FOR A DURATION OF NOT LESS THAN 90 MINUTES.

ELECTRICAL NOTES:

1. ALL CIRCUITS AND EQUIPMENT SHALL BE GROUNDED IN ACCORDANCE WITH THE APPROPRIATE ARTICLES OF THE NATIONAL ELECTRICAL CODES (NEC).
2. WHEN LIGHT FIXTURES ARE INSTALLED IN CLOSETS THEY SHALL BE SURFACE MOUNTED OR RECESSED. INCANDESCENT FIXTURES SHALL HAVE COMPLETELY ENCLOSED LAMPS. SURFACE MOUNTED INCANDESCENT FIXTURES SHALL HAVE A MINIMUM CLEARANCE OF 12 INCHES AND ALL OTHER FIXTURES SHALL HAVE A MINIMUM CLEARANCE OF 6 INCHES FROM "STORAGE AREA" AS DEFINED BY NEC 410-8.8 (a).
3. WHEN WATER HEATERS ARE INSTALLED THEY SHALL BE PROVIDED WITH READILY ACCESSIBLE DISCONNECTS ADJACENT TO THE WATER HEATERS SERVED. THE BRANCH CIRCUIT SWITCH OR CIRCUIT BREAKER SHALL BE PERMITTED TO SERVE AS THE DISCONNECTING MEANS ONLY WHERE THE SWITCH OR CIRCUIT BREAKER IS WITHIN SIGHT FROM THE WATER HEATER OR IS CAPABLE OF BEING LOCKED IN THE OPEN POSITION.
4. HVAC EQUIPMENT SHALL BE PROVIDED WITH READILY ACCESSIBLE DISCONNECTS ADJACENT TO THE EQUIPMENT SERVED. A UNIT SWITCH WITH A MARKED "OFF" POSITION THAT IS A PART OF THE HVAC EQUIPMENT AND DISCONNECTS ALL UNGROUNDED CONDUCTORS SHALL BE PERMITTED AS THE DISCONNECTING MEANS WHERE OTHER DISCONNECTING MEANS ARE ALSO PROVIDED BY A READILY ACCESSIBLE CIRCUIT BREAKER.
5. PRIOR TO ENERGIZING THE ELECTRICAL SYSTEM THE INTERRUPTING RATING OF THE MAIN BREAKER MUST BE DESIGNED AND VERIFIED AS BEING IN COMPLIANCE WITH SECTION 110-9 OF THE NEC BY LOCAL ELECTRICAL CONSULTANT.
6. THE MAIN ELECTRICAL PANEL AND FEEDERS ARE DESIGNED BY OTHERS. SITE INSTALLED AND SUBJECT TO LOCAL JURISDICTION APPROVAL.
7. ALL CIRCUITS CROSSING OVER MODULE MATING LINE(S) SHALL BE SITE CONNECTED WITH APPROVED ACCESSIBLE JUNCTION BOXES OR CABLE CONNECTORS.
8. REFERENCE STATE APPROVED PACKAGE FOR ELECTRICAL RISER DIAGRAM.
9. FIRE ALARM PULL STATION OPERABLE DEVICE SHALL BE LOCATED 42 TO 45 INCHES ABOVE THE FLOOR. FIRE ALARM HORN / STROBE DEVICE SHALL BE WALL MOUNTED WITH THE BOTTOM EDGE 80 INCHES ABOVE THE FLOOR APPLICABLE.
10. EXTERIOR LIGHTS NOT INTENDED FOR 24 HOUR USE AND SHALL BE CONNECTED TO A PHOTOCELL OR TIMER.
11. ALL RECEPTACLS INSTALLED IN WET LOCATIONS (EXTERIOR) SHALL BE IN WEATHER PROOF (W.P.) ENCLOSURES. THE INTEGRITY OF WHICH IS NOT AFFECTED WHEN AN ATTACHMENT PLUG CAP IS INSERTED OR REMOVED.
12. WHEN NOT SHOWN ON THE PLANS PROVISIONS FOR EXIT DISCHARGE LIGHTING (INCLUDING EXIT DISCHARGE EMERGENCY LIGHTING) ARE DESIGNED BY OTHERS AND THE RESPONSIBILITY OF THE BUILDINGS OWNER AND SUBJECT TO LOCAL JURISDICTION APPROVAL.

ACCESSIBILITY NOTES:

1. THE INTERNATIONAL SYMBOL OF ACCESSIBILITY SIGN SHALL BE DISPLAYED AT ALL ACCESSIBLE RESTROOM FACILITIES AND AT ACCESSIBLE BUILDING ENTRANCES UNLESS ALL ENTRANCES ARE ACCESSIBLE. INACCESSIBLE ENTRANCES SHALL HAVE DIRECTIONAL SIGNS INDICATING THE ROUTE TO THE NEAREST ACCESSIBLE ENTRANCE.
2. ACCESSIBLE DRINKING FOUNTAINS SHALL HAVE A SPOUT HEIGHT NO HIGHER THAN 36 INCHES ABOVE THE FLOOR AND EDGE OF BASIN NO HIGHER THAN 34 INCHES ABOVE THE FLOOR FOR INDIVIDUALS IN WHEELCHAIRS. ADDITIONALLY, DRINKING WATER PROVISIONS SHALL BE MADE FOR INDIVIDUALS WHO HAVE DIFFICULTY IN BENDING.
3. WHERE STORAGE FACILITIES SUCH AS CABINETS, SHELVES, CLOSETS, AND DRAWERS ARE PROVIDED AT LEAST ONE OF EACH TYPE PROVIDED SHALL CONTAIN STORAGE SPACE COMPLYING WITH THE FOLLOWING: DOORS, ETC. TO SUCH SPACES SHALL BE ACCESSIBLE (i.e. TOUCH LATCHES, U-SHAPED PULLS); SPACES SHALL BE WITHIN 15 INCHES MINIMUM AND 48 INCHES MAXIMUM OF THE FLOOR FOR FORWARD REACH OR 9 INCHES MINIMUM AND 54 INCHES MAXIMUM, OF THE FLOOR FOR SIDE REACH; CLOTHES RODS SHALL BE A MINIMUM OF 54 INCHES ABOVE THE FLOOR (48 INCHES MAXIMUM WHEN DISTANCE FROM WHEELCHAIR TO ROD EXCEEDS 10 INCHES).
4. CONTROLS, DISPENSERS, RECEPTACLES AND OTHER OPERABLE EQUIPMENT SHALL BE NO HIGHER THAN 45 INCHES ABOVE THE FLOOR FOR FRONT APPROACH OR 54 INCHES ABOVE THE FLOOR FOR SIDE APPROACH. RECEPTACLES ON WALLS SHALL BE MOUNTED NO LESS THAN 15 INCHES ABOVE THE FLOOR. EXCEPTION: HEIGHT LIMITATIONS DO NOT APPLY WHERE THE USE OF SPECIAL EQUIPMENT DICTATES OTHERWISE OR WHERE ELECTRICAL RECEPTACLES ARE NOT NORMALLY INTENDED FOR USE BY BUILDING OCCUPANTS.
5. WHERE EMERGENCY WARNING SYSTEMS ARE PROVIDED, THEY SHALL INCLUDE BOTH AUDIBLE AND VISUAL ALARMS. THE VISUAL ALARMS SHALL BE LOCATED THROUGHOUT, INCLUDING RESTROOMS, AND PLACED 80 INCHES ABOVE THE FLOOR OR 6 INCHES BELOW CEILING, WHICHEVER IS LOWER.
6. DOORS TO ALL ACCESSIBLE SPACES SHALL HAVE ACCESSIBLE HARDWARE (i.e. LEVER - OPERATED, PUSH - TYPE, U - SHAPED) MOUNTED NO HIGHER THAN 48 INCHES ABOVE THE FLOOR.
7. ALL DOORS SHALL BE OPENABLE BY A SINGLE EFFORT. THE MAXIMUM FORCE REQUIRED TO OPEN A DOOR SHALL NOT EXCEED 8.5 LBS. FOR EXTERIOR SWINGING DOORS AND 5 LBS. FOR ALL SLIDING, FOLDING, AND INTERIOR SWINGING DOORS.
8. FLOOR SURFACES SHALL BE STABLE, FIRM, AND SLIP-RESISTANT. CHANGES IN LEVEL BETWEEN 0.25 INCHES AND 0.5 INCH SHALL BE BEVELED WITH A SLOPE NO GREATER THAN 1:2. CHANGES IN LEVEL GREATER THAN 0.5 INCH REQUIRE RAMPS. CARPET PILE THICKNESS SHALL BE 0.5 INCH MAX. GRATINGS IN FLOOR SHALL HAVE SPACES NO GREATER THAN 0.5 INCH WIDE IN ONE DIRECTION. DOORWAY THRESHOLDS SHALL NOT EXCEED 0.5 INCH IN HEIGHT.
9. ACCESSIBLE WATER CLOSETS SHALL BE 17 INCHES FROM THE FLOOR TO THE TOP OF THE SEAT. GRAB BARS SHALL BE 36 INCHES LONG MINIMUM WHEN LOCATED BEHIND THE WATER CLOSET AND 42 INCHES MINIMUM WHEN LOCATED ALONG THE SIDE OF THE WATER CLOSET, AND SHALL BE MOUNTED 33" MIN. & 36" MAX. FROM FLOOR TO THE TOP OF THE RAIL.
10. ACCESSIBLE URINALS SHALL BE STALL-TYPE OR WALL HUNG WITH ELONGATED RIMS AT A MAXIMUM OF 17 INCHES ABOVE THE FLOOR.
11. ACCESSIBLE LAVATORIES SHALL BE MOUNTED WITH THE RIM NO HIGHER THAN 34 INCHES ABOVE THE FLOOR AND A CLEARANCE OF AT LEAST 28 INCHES ABOVE THE FLOOR TO THE BOTTOM OF THE APRON.
12. ACCESSIBLE SINKS SHALL BE MOUNTED WITH RIM NO HIGHER THAN 34 INCHES ABOVE THE FLOOR AND A CLEARANCE OF AT LEAST 27 INCHES HIGH, 30 INCHES WIDE, AND 19 INCHES DEEP UNDERNEATH SINK. THE SINK DEPTH SHALL BE 6.5 INCHES MAXIMUM.
13. HOT WATER AND DRAIN PIPES UNDER ACCESSIBLE LAVATORIES AND SINKS SHALL BE INSULATED OR OTHERWISE CONFIGURED TO PROTECT CONTACT. INSULATION OR PROTECTION MATERIAL MAY BE SITE INSTALLED. THERE SHALL BE NO SHARP OR ABRASIVE SURFACES UNDER ACCESSIBLE LAVATORIES AND SINKS.
14. ACCESSIBLE LAVATORIES AND SINKS SHALL HAVE ACCESSIBLE FAUCETS (i.e. LEVER-OPERATED, PUSH-TYPE, ELECTRONICALLY CONTROLLED).
15. WHERE MIRRORS ARE TO BE PROVIDED ABOVE A LAVATORY OR COUNTERTOP, IT SHALL BE INSTALLED WITH THE BOTTOM EDGE OF THE REFLECTING SURFACE NO HIGHER THAN 40 INCHES ABOVE FINISHED FLOOR.
16. WHERE MEDICINE CABINETS ARE PROVIDED, AT LEAST ONE SHALL BE LOCATED WITH A USABLE SHELF NO HIGHER THAN 44 INCHES ABOVE THE FLOOR.
17. GRAB BARS REQUIRED FOR ACCESSIBILITY SHALL BE 1.25 INCHES TO 2 INCHES IN DIAMETER WITH 1.5 INCHES CLEAR SPACE BETWEEN THE BAR AND THE WALL.
18. TOILET STALL DOORS SHALL BE THE SELF-CLOSING TYPE.
19. A TOWEL DISPENSER SHALL BE LOCATED ADJACENT TO ALL ACCESSIBLE LAVATORIES.
20. WATER CLOSET FLUSH CONTROL SHALL BE MOUNTED ON THE WIDE SIDE OF THE CLOSET.

ADDITIONAL PROJECT INFORMATION

-BUILDING IS TO BE CERTIFIED.
SERIAL #: 0000000

" NOTICE "

PLEASE REVIEW PLANS COMPLETELY. ANY COMPONENTS CROSSING MATE. LINES WILL BE SITE INSTALLED BY SET UP CREW.

ADDITIONAL 3rd PARTY INFORMATION:

A SIGNED/SEALED SET OF PLANS IS MAINTAINED ON FILE IN THE 3rd PARTY AGENCY OFFICE.

THESE PLANS COMPLY WITH FLORIDA PRODUCT APPROVAL RULE 61020-3.006.

STRUCTURAL LOAD LIMITATIONS:

DESIGN CODES
FLORIDA BUILDING CODE, 6TH EDITION (2017)
ASCE 7-10 CODE

BUILDING DEAD LOADS

- A. ROOF = 10 PSF
- B. FLOOR = 10 PSF
- C. WALLS = 5 PSF

BUILDING LIVE LOADS

- A. ROOF = 30 PSF
- B. FLOOR = 50 PSF
- C. CORRIDOR = 100 PSF

CONCENTRATED LOAD,
OVER 30 INCH C 20 INCH AREA
LOCATED ANYWHERE ON FLOOR.

ROOF SNOW LOAD: N/A

WIND LOAD CRITERIA:

1. 144 Vasd/186 Vult MPH (HVHZ) WIND SPEED
2. II RISK CATEGORY
3. II BUILDING CATEGORY
4. ENCLOSURE CLASSIFICATION: ENCLOSED
Gcpl = 0.18 INTERNAL PRESSURE COEFFICIENT.
5. C EXPOSURE FACTOR
6. 0.85 WIND DIRECTIONALITY FACTOR (Kd)
7. 0.85 GUST RESPONSE FACTOR (Gf)
8. COMPONENT & CLADDING LOAD:
(ROOF)
P1 = ZONE1: -31.5 PSF, ZONE 2: -62.2 PSF, ZONE 3: -91.2 PSF (WALL)
PW = ZONE 4: -31.8 PSF, ZONE 5: -42.1 PSF
9. ENCLOSED BUILDING ENCLOSURE CLASSIFICATION
10. THIS BUILDING IS NOT DESIGNED FOR PLACEMENT ON THE UPPER HALF OF A HILL OR ESCARPMENT EXCEEDING 15 FEET IN HEIGHT.

FLOOD LOAD:

THIS BUILDING IS NOT DESIGNED TO BE SUBMERGED OR SUBJECTED TO WAVE ACTION WHEN LOCATED IN A FLOOD PRONE OR ZONE AREA. FINISH FLOOR ELEVATION MUST BE LOCATED ABOVE THE BUILDING SITE FLOOD PLANE LEVEL.

SITE INSTALLED NOTES:

- NOTE THAT THIS LIST DOES NOT NECESSARILY LIMITS THE ITEMS OF WORK AND MATERIALS THAT MAY BE REQUIRED FOR A COMPLETE INSULATION. ALL SITE RELATED ITEMS ARE SUBJECT TO LOCAL JURISDICTION AND APPROVAL.
1. THE COMPLETE FOUNDATION SUPPORT AND TIE DOWN SYSTEM.
 2. RAMPS, STAIRS AND GENERAL ACCESS TO THE BUILDING.
 3. PORTABLE FIRE EXTINGUISHERS (S).
 4. DRINKING FOUNTAIN, BUILDING DRAINS, CLEAN-OUTS, AND HOOK-UP TO PLUMBING SYSTEM.
 5. ELECTRICAL SERVICE HOOK-UP (INCLUDING FEEDERS) TO THE BUILDING.
 6. THE MAIN ELECTRICAL PANEL AND SUB-FEEDERS (MULTI-UNITS ONLY).
 7. CONNECTION OF ELECTRICAL CIRCUITS CROSSING OVER MODULE MATING LINES (S) - (MULTI-UNITS ONLY).
 8. STRUCTURAL AND AESTHETIC INTERCONNECTIONS BETWEEN WINDOW AND DOOR HIGH WIND STORM COVERINGS PER CODE.
 9. GUTTERS AND DOWNSPOUTS (IF APPLICABLE).
 11. SINK AND CABINETS.
 12. FIRE ALARM SYSTEM, WIRING, SMOKE DETECTOR, ETC (IF APPLICABLE).
 13. THERMAL EXPANSION DEVICE IF REQUIRED.
 14. FIRE EXTINGUISHERS (IF APPLICABLE).
 15. COVERED PLATFORM FOR ENTRY.

DRAWING INDEX:

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A3.3	DETAILS
A3.4	DETAILS

CLEMONS, RUTHERFORD & ASSOCIATES, INC. 2027 Thomasville Road, Tallahassee, Florida 32308 PH: 850-385-4151 F: 850-386-8420 www.craarchitects.com	
DBPR PLAN REVIEW	
AGENCY PLAN #: 18002.06	
CODE VERSION:	FBC 2017 6th Edition, SREF
CONSTRUCTION TYPE:	II OCCUPANCY: Group E AREA: 880
MANUFACTURE:	Mobile Modular Management, MFT-7792
FIRE PROTECTION:	Owner provided where required EHPA: 80
# OF STORIES:	1 MODULES: 2 EXT. WALL RATING: 1
WIND VELOCITY:	186 mph, Exposure C HVHZ: YES
FLOOR LOAD:	MAX (1000) LIVE (50) DEAD (10)
ROOF LOAD:	MAX () LIVE (30) DEAD (10)
"R" FLOOR, WALL, ROOF: 15.32, 15.6, 21.9	
FLOOD ZONE: Inundation must comply with FBC 603.2.5.1	
APPROVED DATE:	06/2018 REVIEWER: Billy Tyson, SMP-46
This approval IN NO way alleviates the builder from complying with all Applicable Current Codes, which may not be identified in this Review.	

FIRE PLAN REVIEW
Pursuant to FAC 69A-04.006(4)(e)
a plan review of 18002.06
found items that are indicated to be installed for compliance with the Florida Fire Prevention Code
In the Factory [] yes [x] no
on site by others [x] yes [] no
An inspection pursuant to SS 633.081(1) is required.
Reviewer: Walt Spann, Lic. # 62373
Date: 06/06/2018

CLEMONS, RUTHERFORD & ASSOC.

DBPR PLAN REVIEW
18002.06 6/5/2018

CONSULTING ENGINEER

ROBERT T. HAUG
FLORIDA
P.E. LICENSE #24575
1820 E EDGEWOOD DR
LAKELAND, FLORIDA 33803

REVISION DATE:

03-30-2018

DATE: 03-20-2018

DRAWN: R.L.G.

JOB #: MPD-1518

SHEET NO.

C1

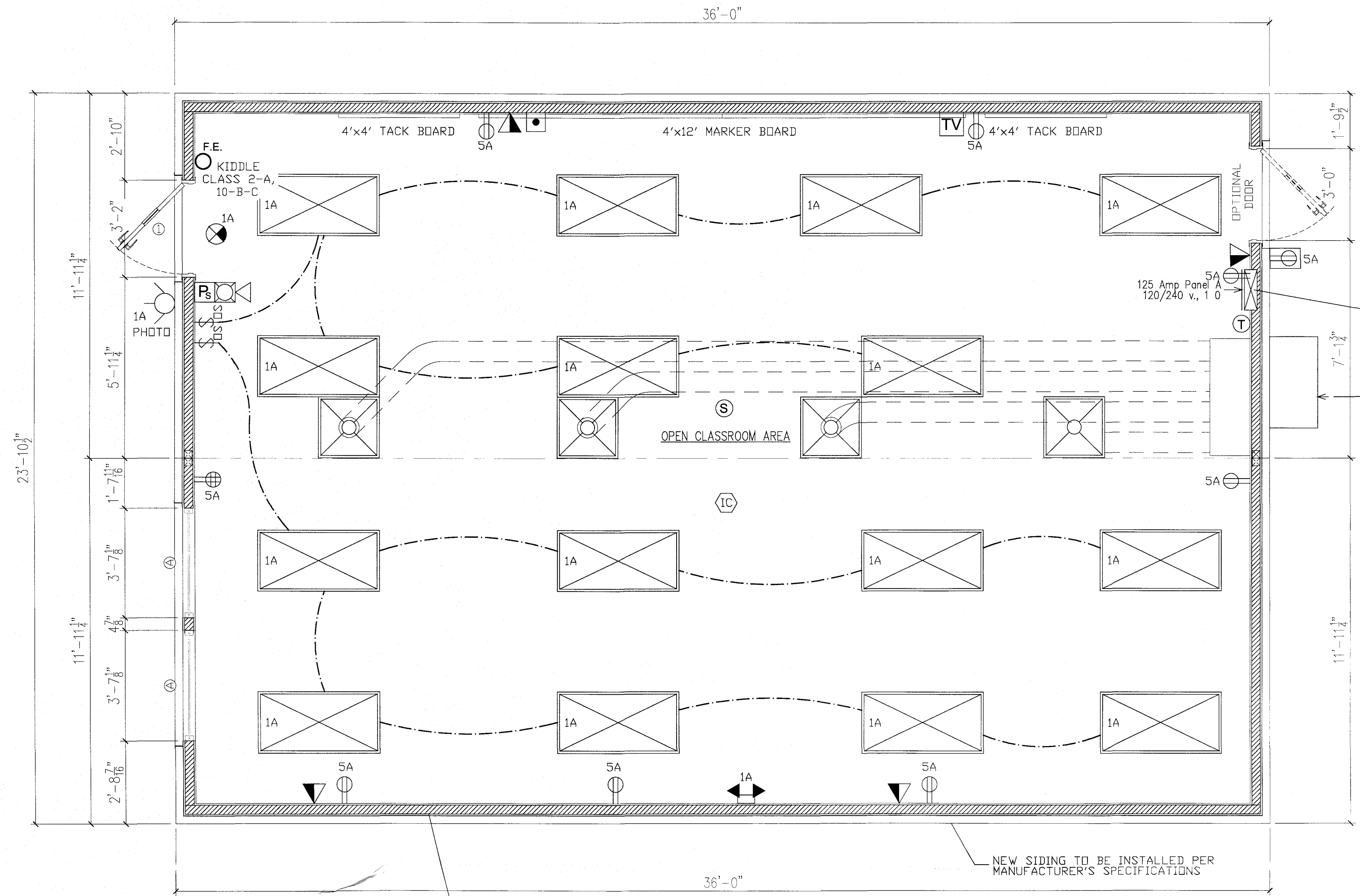
MODULAR STRUCTURE FOR:
Mobile Modular
(24' X 36')

MODULAR PLANS DESIGN, CO.

MODULAR
PLANS DESIGN, CO.

mobile
modular
Your Project – Our Commitment

1074 S. FLORIDA AVE., SUITE 201
LAKELAND, FLORIDA
33803
PH: 863-688-1054
FAX: 863-688-7118
MODULARPLANS@TAMPABAYFLOR.COM



ELECTRICAL PANEL MUST BE LOCKABLE AND CONTAIN 20% SPARE CIRCUIT CAPACITY.
NOTE: ELECTRICAL PANEL IS IN COMPLIANCE WITH SREF DESIGN GUIDELINES.
3.5 TON HVAC W/10 KW HEAT STRIP 1000 CFM @ 5 LWC ESP 240V, 10 HVAC UNIT PROVIDED WITH FRESH AIR INTAKE CRT#2,4A

ELECTRICAL PANEL "A" SIZING		
DESCRIPTION:		
.0035 KW SF X .864 SF X 1.25		3.78
1 HVAC @ 10.9 KW		10.9
9 RECEPTS @ 180 VA / 1000		1.62
1 WATER HEATER @ 1.9 KW X 1.25		--
1 FANS @ .3 KW X 1.25		--
TOTAL 16.30 KW		
TOTAL / 240 X 1000 67.92 AMPS		
INSTALL 125 AMP PANEL 120 / 240V 10		

ELECTRICAL SCHEDULE			
CIRCUIT	NOMENCLATURE	BREAKER (AMPS)	WIRE SIZE CU. MC W/G.
2 & 4	HVAC	60A 2P	6-2
1	LIGHTING	20A 1P	12-2
5	RECEPTS	20A 1P	12-2
ALL PORTABLE APPLIANCES, INCLUDING W/H TO HAVE DISCONNECT IF NOT WITHIN VIEW OF ELECTRIC PANEL			

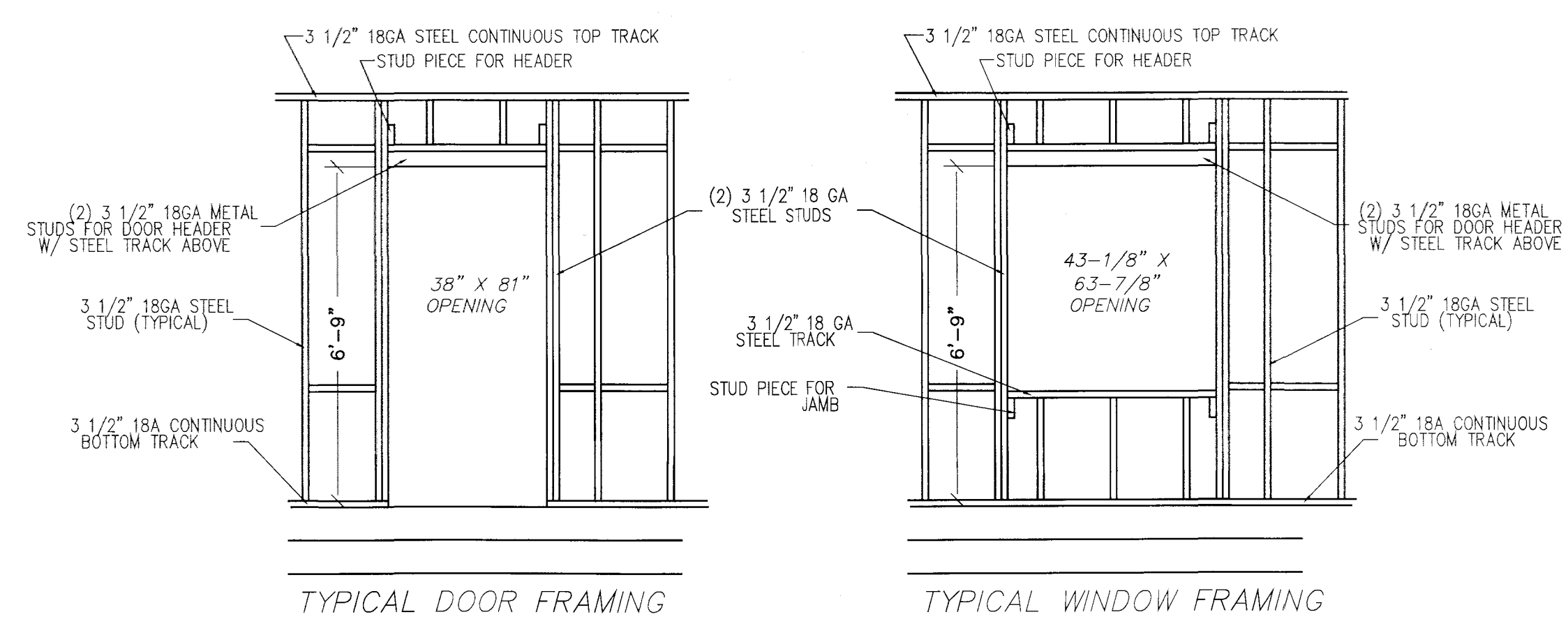
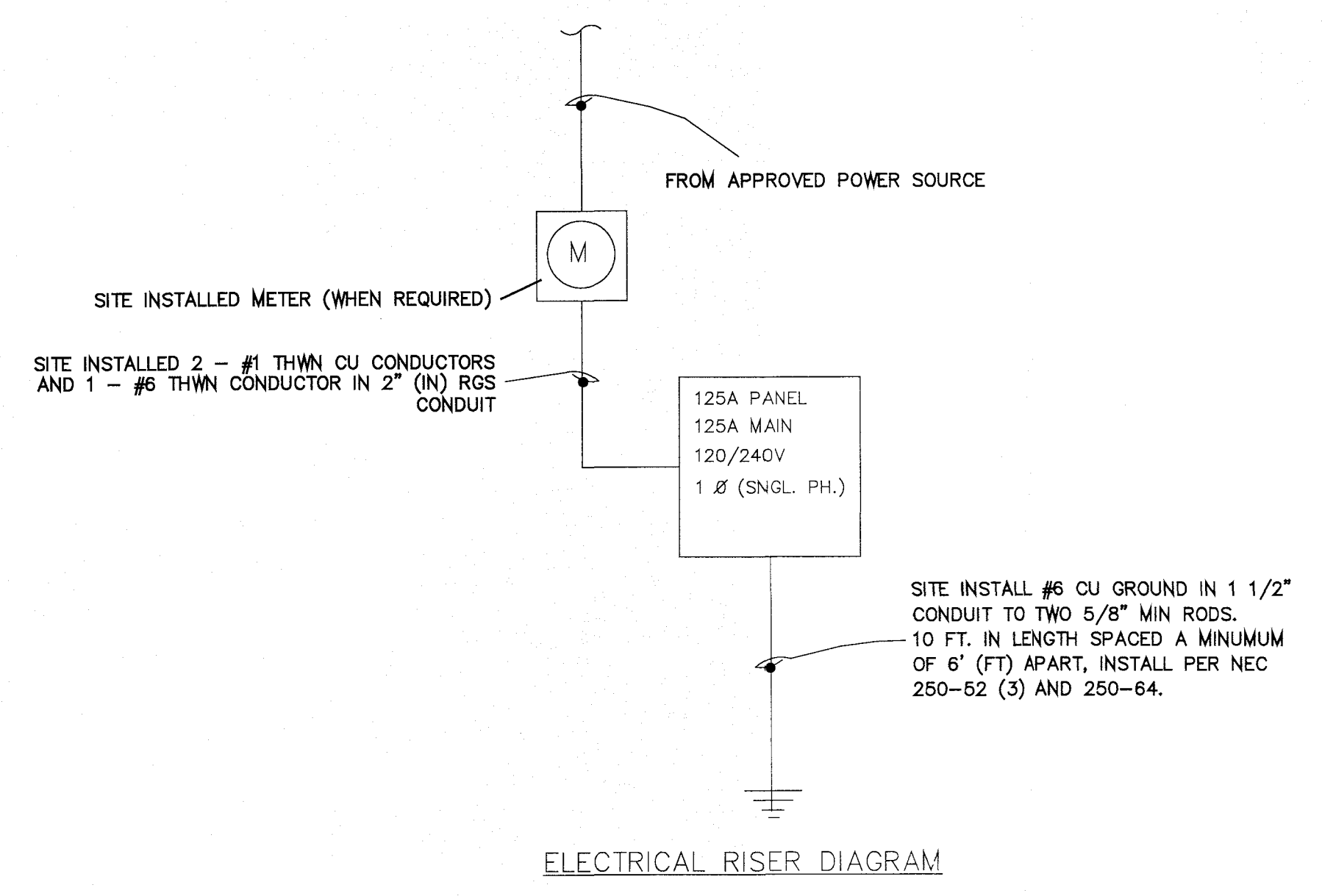
ELECTRICAL LEGEND	
⊕ DUPLEX RECEPT @ 16" A.F.F.	⊙ EXHAUST FAN
⊕ DUPLEX RECEPT @ 42" A.F.F.	⊙ EXHAUST FAN / LIGHT
⊕ RECEPT 220 VOLT	⬇ EMERGENCY LIGHT
⊕ QUAD RECEPT @ 16" A.F.F.	⬆ J BOX FOR MANUAL PULL STATION
⊕ SWITCH	⊕ 12" X 12" SUPPLY AIR REGISTER
⬆ PHONE JACK @ 42" A.F.F.	⊕ THERMOSTAT
⬆ PHONE JACK @ 16" A.F.F.	⊕ 12" X 12" RETURN AIR
⬆ DATA OUTLET @ 16" A.F.F.	⊕ EXIT SIGNAGE LIGHTING, CEILING MNT
⬆ INCANDESCENT LIGHT 15/25W MAX	⊕ EMERGENCY / EXIT LIGHTING COMBO
⬆ EYEBALL LIGHT 40W MAX	⊕ FLUORESCENT FIXTURE OR LED EQUIVALENT
⬆ WATER PROOF PORCH LIGHT 60W MAX/W/ PHOTO CELL	⊕ EEMAX W/H
⬆ FIRE ALARM PULL STATION	⊕ FIRE ALARM STROBE
⬆ FIRE ALARM HORN STROBE	⊕ INTERCOM SPEAKER
⬆ SMOKE DETECTOR	⊕ INTERCOM CALL SWITCH

FLOOR PLAN
SCALE: 3/8" = 1'-0"

NOTE:
THE EXTERIOR WALLS OF THIS UNIT HAVE BEEN CONSTRUCTED TO PROVIDE A 1 HOUR FIRE RATING. THE SIDING PANELS HAVE BEEN TESTED IN ACCORDANCE WITH ASTM E119 AND HAVE BEEN APPROVED UNDER ASSEMBLY #: NDI5328-103870.

DOOR SCHEDULE						
#	SIZE	DESCRIPTION	QTY	HEADER	JACKS	COMMENTS
①	36x80	STEEL/STEEL W/9"x40" V.B.	1	DBL 2x4	1	FL #: 6378 R1 / 1 OPTIONAL
HARDWARE: CLASSROOM FUNCTION LOCK SET, CLOSER, & ADA THRESHOLD						

WINDOW SCHEDULE						
#	SIZE	DESCRIPTION	QTY	HEADER	JACKS	COMMENTS
②	41X60	VS/BRONZE/WHITE/CLEAR	2	DBL 2x4	1	FL #: 10970 1



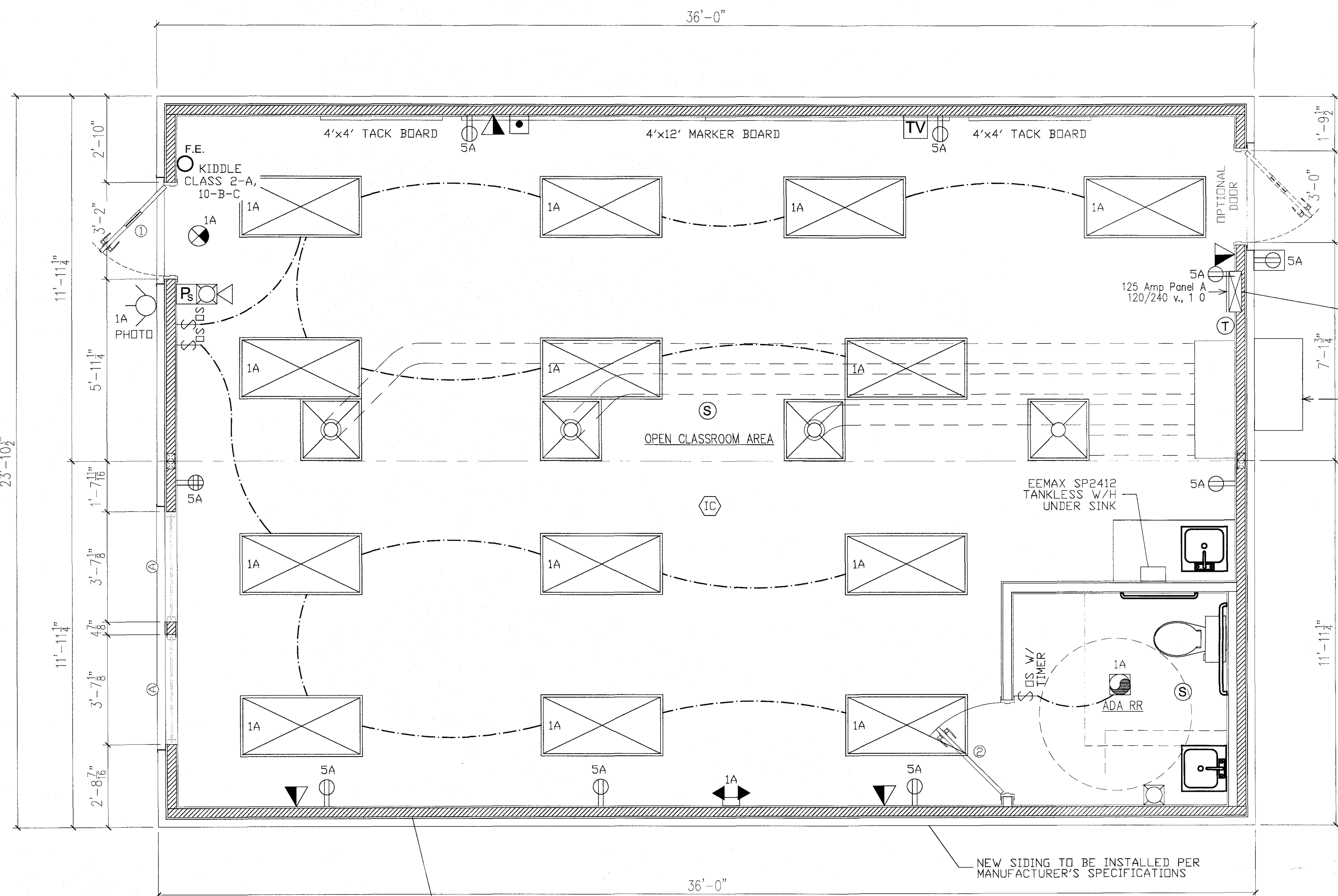
TYPICAL FRAMING
SCALE: 3/8" = 1'-0"

CLERMONT, RUTHERFORD & ASSOC.
DBPR PLAN REVIEW
18002 06 6/5/2018

CONSULTING ENGINEER
ROBERT T. HAUG
FLORIDA
P.E. LICENSE #24575
1820 E EDGEWOOD DR
LAKELAND, FLORIDA 33803

REVISION DATE:
03-30-2018

DATE: 06-02-2018
DRAWN: R.L.G.
JOB #: MPD-1518
SHEET NO. A1



FLOOR PLAN
SCALE: 3/8" = 1'-0"

NOTE:
THE EXTERIOR WALLS OF THIS UNIT HAVE BEEN CONSTRUCTED TO PROVIDE A 1 HOUR FIRE RATING. THE SIDING PANELS HAVE BEEN TESTED IN ACCORDANCE WITH ASTM E119 AND HAVE BEEN APPROVED UNDER ASSEMBLY #: N015328-103870.

ELECTRICAL PANEL "A" SIZING	
DESCRIPTION	
.0035 KW SF X 864 SF X 1.25	3.78
1 HVAC @ 10.9 KW	10.9
9 RECEPTS @ 180 VA / 1000	1.62
1 EMAX W/H @ 2.4 KW X 1.25	3.0
1 FANS @ .3 KW X 1.25	0.38
TOTAL	19.68 KW
TOTAL / 240 X 1000	82.00 AMPS
INSTALL 125 AMP PANEL 120 / 240V 10	

ELECTRICAL SCHEDULE			
CIRCUIT	NOMENCLATURE	BREAKER (AMPS)	WIRE SIZE CU, MC W/G.
2 & 4	HVAC	60A 2P	6-2
1	LIGHTING	20A 1P	12-2
5	RECEPTS	20A 1P	12-2
6	EMAX W/H	PER MFRS SPEC	
ALL PORTABLE APPLIANCES, INCLUDING W/H, TO HAVE DISCONNECT IF NOT WITHIN VIEW OF ELECTRIC PANEL			

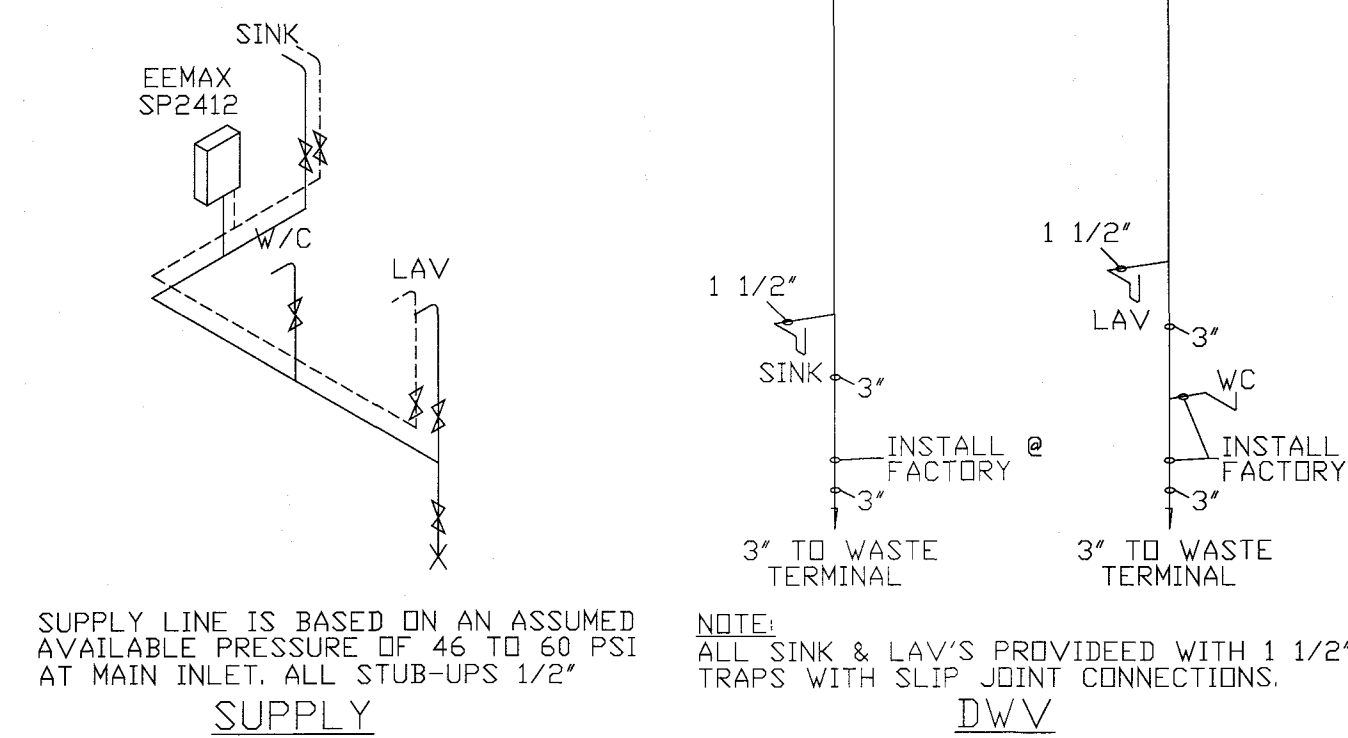
ELECTRICAL LEGEND	
⊕ DUPLEX RECEPT @ 16" A.F.F.	⊙ EXHAUST FAN
⊕ DUPLEX RECEPT @ 42" A.F.F.	⊕ EXHAUST FAN / LIGHT
⊕ RECEPT 220 VOLT	⚡ EMERGENCY LIGHT
⊕ QUAD RECEPT @ 16" A.F.F.	Ⓜ J BOX FOR MANUAL PULL STATION
Ⓢ SWITCH	Ⓢ 12" X 12" SUPPLY AIR REGISTER
▼ PHONE JACK @ 42" A.F.F.	Ⓢ 12" X 12" RETURN AIR
▼ PHONE JACK @ 16" A.F.F.	Ⓢ THERMOSTAT
▼ DATA OUTLET @ 16" A.F.F.	Ⓢ EXIT SIGNAGE LIGHTING, CEILING MNT
Ⓢ INCANDESCENT LIGHT 15/25W MAX	Ⓢ EMERGENCY / EXIT LIGHTING COMBO
Ⓢ EYEBALL LIGHT 40W MAX	Ⓢ FLUORESCENT FIXTURE OR LED EQUIVALENT
Ⓢ WATER PROOF PORCH LIGHT 60W MAX W/ PHOTO CELL	Ⓢ EEMAX W/H
Ⓢ FIRE ALARM PULL STATION	Ⓢ FIRE ALARM STROBE
Ⓢ FIRE ALARM HORN STROBE	Ⓢ INTERCOM SPEAKER
Ⓢ SMOKE DETECTOR	Ⓢ INTERCOM CALL SWITCH

WATER CLOSETS SERVING CHILDREN AGES 3 THROUGH 12			
	AGES 3 & 4	AGES 5 THRU 8	AGES 9 THRU 12
W/C CENTERLINE	12 INCHES	12 TO 15 INCHES	15 TO 18 INCHES
TOILET SEAT HEIGHT	11 TO 12 INCHES	12 TO 15 INCHES	15 TO 17 INCHES
GRAB BAR HEIGHT	18 TO 20 INCHES	20 TO 25 INCHES	25 TO 27 INCHES
DISPENSER HEIGHT	14 INCHES	14 TO 17 INCHES	17 TO 19 INCHES

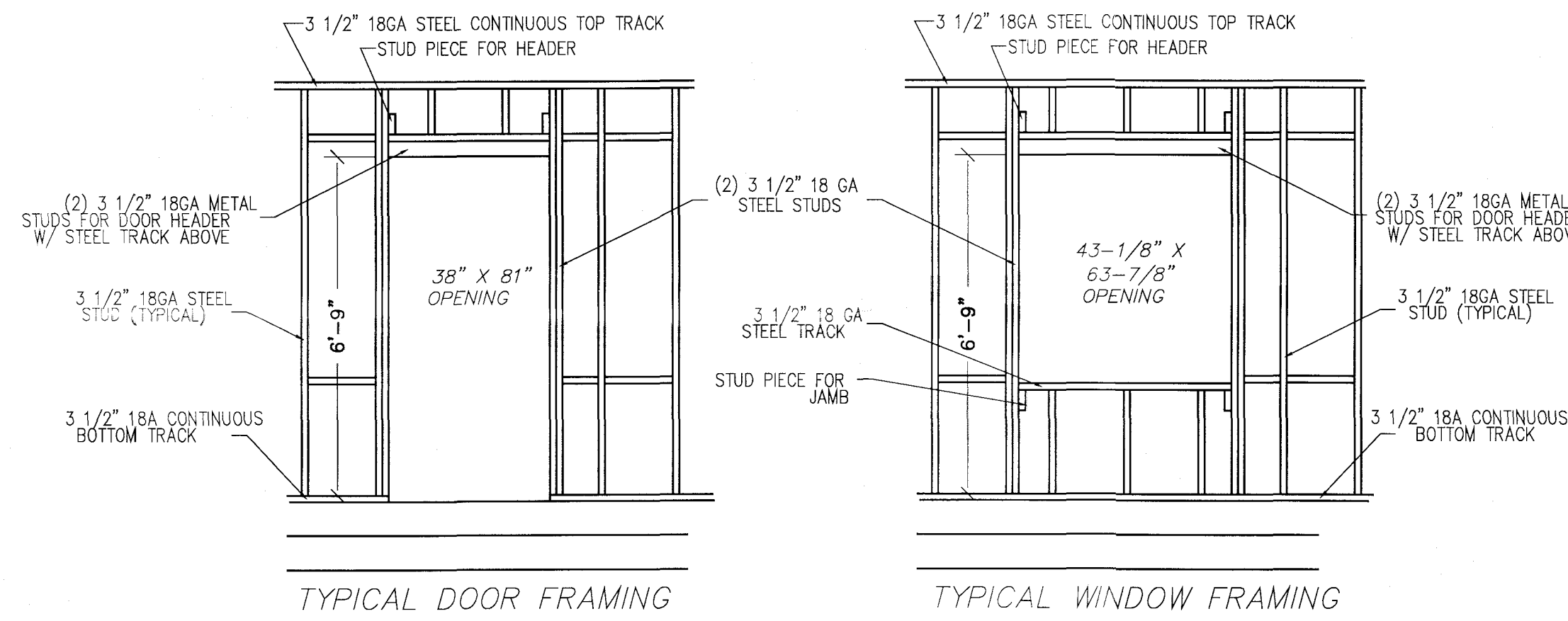
TYPICAL ADA RESTROOM DIMENSIONS	
LAVATORY CLEARANCE WIDTH - 30"	WATER CLOSET CLEARANCE WIDTH - 60"
LAVATORY CLEARANCE LENGTH - 48"	WATER CLOSET CLEARANCE DEPTH - 56"
LAVATORY CENTERLINE FROM SIDE WALL - 18"	WATER CLOSET CENTERLINE FROM SIDE WALL - 18"
TOP OF LAVATORY HEIGHT - 34" MAX	WATER CLOSET SEAT HEIGHT - 17"-19"
BOTTOM OF LAVATORY HEIGHT - 29" MAX	GRAB BAR HEIGHTS - 33"-36" (TO TOP)
MIRROR HEIGHT - 40" MAX (TO BOTTOM OF REFLECTIVE SURFACE)	REAR GRAB BAR LENGTH - 36" MIN
DOOR CLEARANCE - 32" MIN. OPENING	REAR GRAB BAR TO CORNER - 12" MAX
DOOR HANDLE HEIGHT - 34"-48"	SIDE GRAB BAR LENGTH - 42" MIN
DISPENSER HEIGHT - 15"-48"	SIDE GRAB BAR TO CORNER - 12" MAX
	DISPENSER TO WATER CLOSET - 7"-9"

DOOR SCHEDULE						
#	SIZE	DESCRIPTION	QTY	HEADER	JACKS	KINGS
1	36x80	STEEL/STEEL W/9"x40" V.B.	1	DBL 2x4	1	1
HARDWARE: CLASSROOM FUNCTION LOCK SET, CLOSER, & ADA THRESHOLD						
2	36x80	S.C. - PRE-HUNG STAINED	1	SGL 2x4	0	2
HARDWARE: PRIVACY LOCKSET						

WINDOW SCHEDULE					
#	SIZE	DESCRIPTION	QTY	HEADER	JACKS
A	41X60	VS/BRONZE/WHITE/CLEAR	2	DBL 2x4	1
FL# 10970.1					

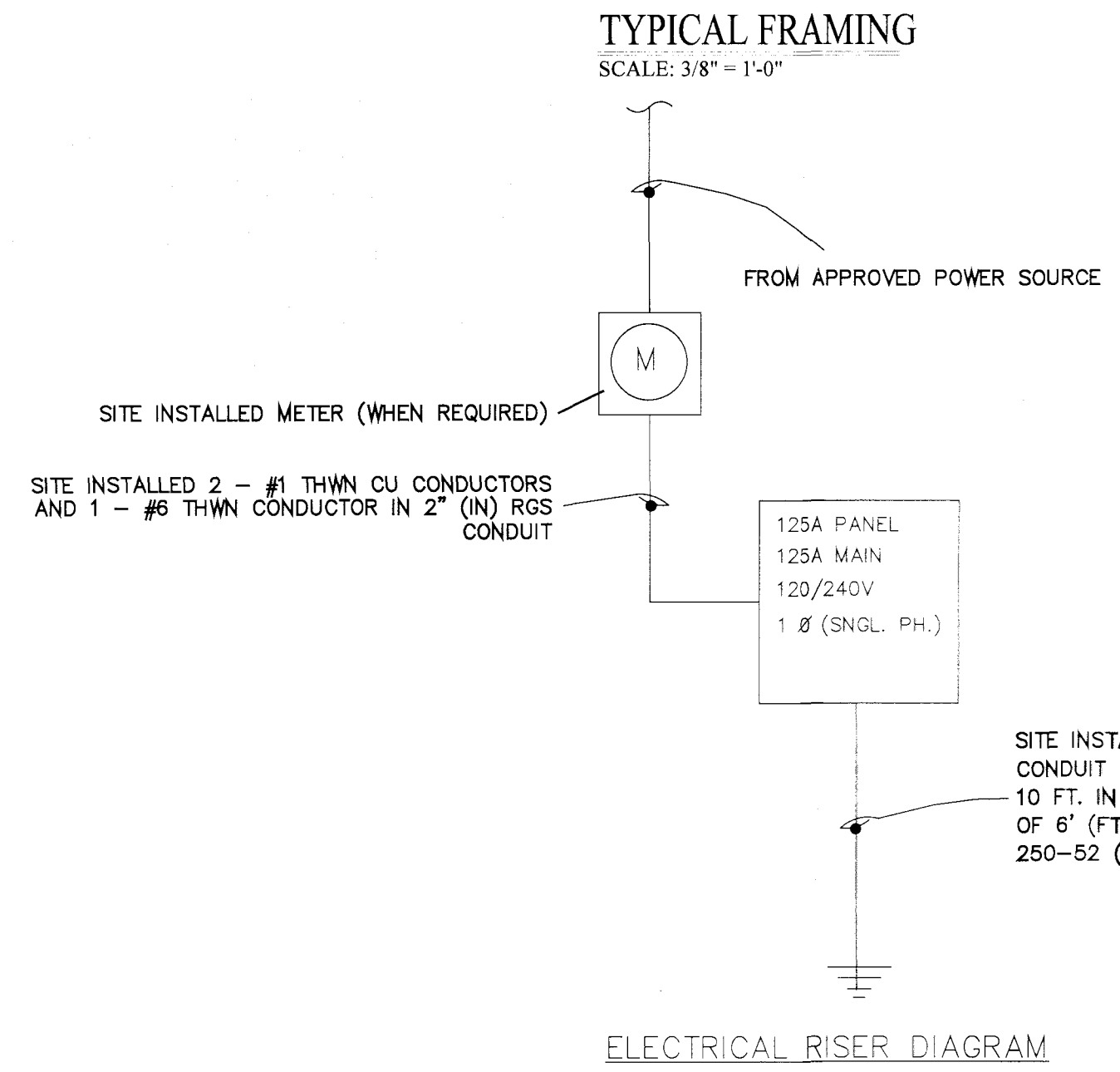


PLUMBING RISER
SCALE: 3/8" = 1'-0"



TYPICAL DOOR FRAMING

TYPICAL WINDOW FRAMING



ELECTRICAL RISER DIAGRAM

CLERMONT, RUTHERFORD & ASSOC.
DBPR PLAN REVIEW
18002.06 6/5/2018

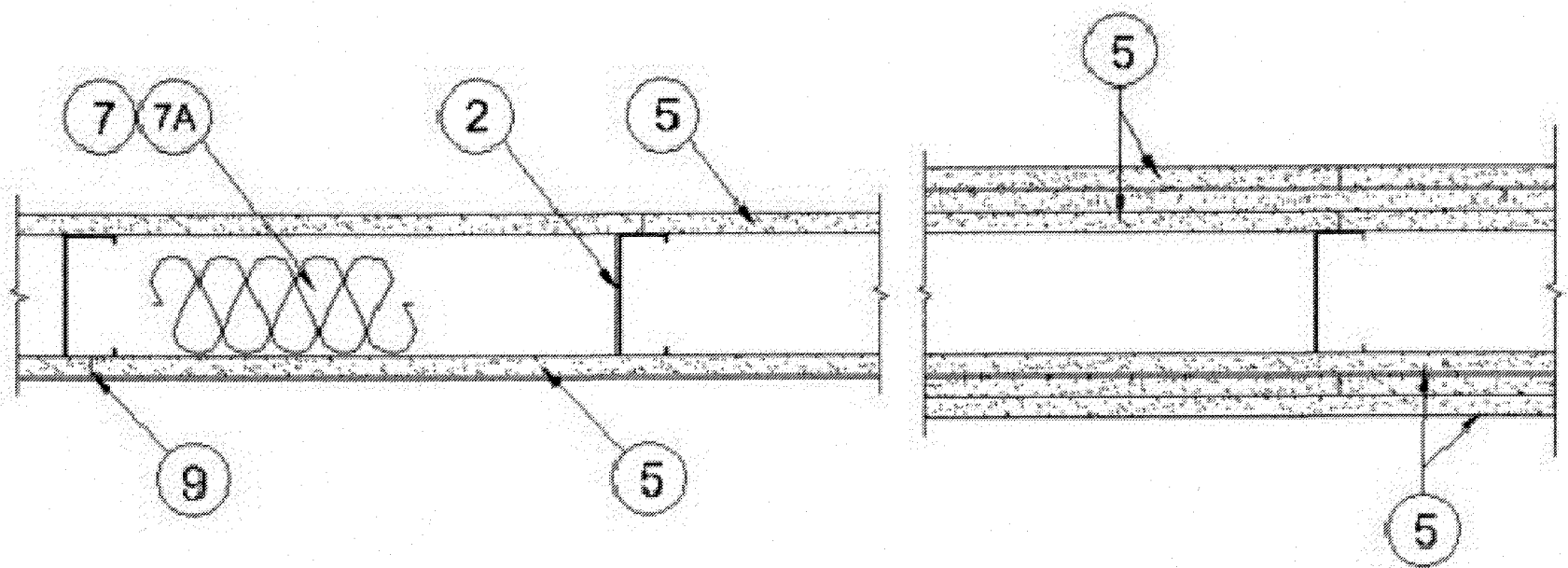
CONSULTING ENGINEER
ROBERT T. HAUG
FLORIDA
P.E. LICENSE #24575
1820 E EDGEWOOD DR
LAKELAND, FLORIDA 33803

REVISION DATE: A 03-30-2018	DATE: 06-04-2018
	DRAWN: R.L.G.
	JOB #: MPD-1518
	SHEET NO. A1.1

MODULAR STRUCTURE FOR:
Mobile Modular
(24' X 36')

MODULAR PLANS DESIGN, CO.
1074 S. FLORIDA AVE., SUITE 201
LAKELAND, FLORIDA 33803
PH: 883.688.1054
FAX: 883.688.1118
MODULARPLANS@TAMPABAY.RR.COM

mobile modular
Your Project - Our Commitment



Design No. U423

March 07, 2012

Bearing Wall Ratings : 45 min, 1, 1-1/2 or 2 Hr (See Items 5 & 7)
Load Restricted for Canadian Applications —See Guide BXUV7

1. Floor and Ceiling Runners : (Not shown) : Channel shaped, fabricated from min 0.0329 in. bare metal thickness (No. 20 MSG) corrosion-protected steel, that provide a sound structural connection between steel studs and adjacent assemblies such as floors, ceilings and/or other walls. Attached to floor and ceiling assemblies with steel fasteners spaced not greater than 24 in. OC.
- 1A. Floor and Ceiling Runners : (Not shown, As an alternate to Item 1, For Use With Item 5A and 5C) —Channel shaped runners min 3-1/2 in. deep with 1-1/4 in. flanges fabricated from min No. 20 MSG corrosion-protected steel. Attached to floor and ceiling assemblies with steel fasteners spaced not greater than 24 in. OC.
2. Steel Studs : Min 0.0329 in. bare metal thickness (No. 20 MSG) corrosion-protected steel studs, min 3-1/2 in. wide, cold formed, designed in accordance with the current edition of the Specification for the Design of Cold-Formed Steel Structural Members by the American Iron and Steel Institute (AISI). All design details enhancing the structural integrity of the wall assembly, including the axial design load of the studs, shall be as specified by the steel stud designer and/or producer, and shall meet the requirements of all applicable local code agencies. The max stud spacing shall not exceed 24 in. OC. Studs attached to floor and ceiling runners with 1/2 in. long Type S-12 steel screws on both sides of the studs or by welded or bolted connections designed in accordance with the AISI specifications.
- 2A. Steel Studs : (As an alternate to Item 2, For use with Item 5A and 5C) Channel shaped, fabricated from min 20 MSG corrosion-protected or galv steel, 3-1/2 in. min width, min 1-1/2 in. flanges and 1/4 in. return, spaced a max of 16 in. OC. Studs friction-fit into floor and ceiling runners.
- 2B. Steel Studs : (As an alternate to Item 2 and 2A, For Use With Item 5B) - Min 0.0329 in. (No. 20 MSG) corrosion-protected cold formed steel studs, min 3-1/2 in. deep by 1-5/8 in. wide with 1/2 in. returns. Braced at mid-height and designed in accordance with the current edition of the Specification for the Design of Cold-Formed Steel Structural Members by the American Iron and Steel Institute (AISI). All design details enhancing the structural integrity of the wall assembly, including the axial design load of the studs, shall be as specified by the steel stud designer and/or producer, and shall meet the requirements of all applicable local code agencies. The max stud spacing shall not exceed 24 in. OC. Studs attached to floor and ceiling runners with 1/2 in. long Type S-12 steel screws on both sides of the studs or by welded or bolted connections designed in accordance with the AISI specifications.
- 2C. Framing Members - Steel Studs : (As an alternate to Item 2, For use with Item 5C) Channel shaped, fabricated from min 20 MSG (0.0327 in. thick) corrosion-protected or galv steel, 3-1/2 in. min width, min 1-1/2 in. flanges and 1/4 in. return, spaced a max of 16 in. OC. Studs friction-fit into floor and ceiling runners. Studs to be cut 5/8 to 3/4 in. less than assembly height.
3. Lateral Support Members : (Not shown) —Where required for lateral support of studs, support shall be provided by means of steel straps, channels or other similar means as specified in the design of a particular steel stud wall system.
4. Wood Structural Panel Sheathing : (Optional, For use with Item 5 only)~ (Not Shown) - 4 ft wide, 7/16 in. thick oriented strand board (OSB) or 15/32 in. thick structural 1 sheathing (plywood) complying with DDC PS1 or PS2, or APA Standard PRP-108, manufactured with exterior glue, applied horizontally or vertically to the steel studs. Vertical joints centered on studs, and staggered one stud space from wallboard joints. Attached to studs with flat-head self-drilling tapping screws with a min. head diam. of 0.292 in. at maximum 6 in. OC. in the perimeter and 12 in. OC. in the field. When used, fastener lengths for gypsum panels increased by min. 1/2 in.
- The maximum loading on the steel studs was evaluated with the steel studs braced at mid-height and not braced by the plywood sheathing.
5. Gypsum Board* : Gypsum panels with beveled, square or tapered edges, applied vertically or horizontally. Vertical joints centered over studs and staggered one stud cavity on opposite sides of studs. Vertical joints in adjacent layers (multilayer systems) staggered one stud cavity. Horizontal joints need not be backed by steel framing. Horizontal edge joints and horizontal butt joints on opposite sides of studs need not be staggered when load is reduced to 90 percent of max stud capacity. When load is at 100 percent, horizontal edge joints and horizontal butt joints on opposite sides of studs staggered a min of 12 in. Horizontal edge joints and horizontal butt joints in adjacent layers (multilayer systems) staggered a min of 12 in. When used in widths other than 48 in., gypsum panels to be installed horizontally. The thickness and number of layers and percent of design load for the 45 min, 1 hr, 1-1/2 hr, and 2 hr ratings are as follows:
Wallboard Protection on Each Side of Wall

Rating	No. of Layers & Thickness	% of Design Load
45 Min	1 layer, 1/2 in. thick	100
1 hr	1 layer, 5/8 in. thick	100
1-1/2 hr	2 layers, 1/2 in. thick	100
2 hr	2 layers, 5/8 in. thick	80
2 hr	2 layers, 5/8 in. thick	100
2 hr	3 layers, 1/2 in. thick	100
2 hr	2 layers, 3/4 in. thick	100

®Rating applicable when Batts and Blankets (Item 7) are used.

5A. Gypsum Board* : (As an alternate to Item 5 when used as the base layer on one or both sides of wall, For direct attachment only, not to be used with Item 4) - Nom 5/8 in. or ¾ in. may be used as alternate to all 5/8 in. or ¾ in. shown in Item 5, Wallboard Protection on Each Side of Wall table. Nom 5/8 in. or ¾ in. thick lead backed gypsum panels with beveled, square or tapered edges, applied vertically. Vertical joints centered over 20 MSG steel studs and staggered min 1 stud cavity on opposite sides of studs. See Items 1A, 2A 8, 8A(a). Wallboard secured to studs with 1-1/4 in. long Type S-12 steel screws spaced 8 in. OC at perimeter and 12 in. OC in the field. To be used with Lead Batten Strips (see Item 12) or Lead Discs or Tabs (see Item 13).

RAY-BAR ENGINEERING CORP —Type RB-LBG

5B. Gypsum Board* : (As an alternate to Items 5 and 5A,) - Nom 5/8 in. thick gypsum panels with square edges, applied horizontally or vertically. For the hour single layer system when the gypsum board panels are installed horizontally the joints are to be staggered by a minimum of 12 in. on opposite sides of assembly, they are to be secured on each side of the studs with 1-1/4 in. long Type S-12 bugle head steel screws spaced 8 in. OC to the top and bottom tracks and in the field with screws 1 in. and 4 in. from the horizontal joints. When the gypsum board panels are installed vertically all vertical joints must be centered over studs and staggered min 1 stud cavity on opposite sides of studs. Gypsum board secured to studs with 1-1/4 in. long Type S-12 steel screws spaced 8 in. OC to the top and bottom tracks and in the field with screws 1 in. and 4 in. from the perimeter. For the 2 hour double layer system when the gypsum board panels are installed horizontally the joints need not be staggered on opposite sides of assembly. Base layer secured on each side of the studs with 1-1/4 in. long Type S-12 bugle head steel screws spaced 16 in. OC to the top and bottom track and in the field with screws beginning 1 in. and 8 in. from the horizontal joints. Face layer horizontal joints staggered 8 in. from base layer joints and secured with 1-5/8 in. long Type S-12 bugle head steel screws spaced 16 in. OC to the top and bottom tracks and in the field with screws beginning 1 in. and 8 in. from the horizontal joints. Face layer screws offset 8 in. from base layer screws. When the gypsum board panels are installed vertically all vertical joints must be centered over studs and staggered min 1 stud cavity on opposite sides of studs. Face layer gypsum boards secured to studs with 1-1/4 in. long Type S-12 steel screws spaced 16 in. OC with screws 2 in. and 16 in. from the perimeter. Base layer gypsum boards secured to studs with 1-1/4 in. long Type S-12 steel screws spaced 16 in. OC with screws 1-1/2 in. and 8 in. from the perimeter. Face layer screws offset 8 in. from base layer screws.

UNITED STATES GYPSUM CO —5/8 in. thick Type USGX (Joint tape and compound, Item 9, optional with Type USGX)

PRODUCT APPROVAL			
PRODUCT CATEGORY	SUB. CATEGORY	MANUFACTURER	STATE OF FLORIDA APPROVAL #:
DOORS	EXTERIOR	FLEMING-BARGON	FL#: 14237
WINDOWS	SLIDER	HR WINDOWS	FL#: 14039.1
WINDOWS	SLIDER	HR WINDOWS	NDA#: 16-0505.02
ROOFING	SINGLE PLY ROOFING	GACO WESTERN	NDA# 17-0329.08
EXT. SIDING	SIDING	STRUCTALL	FL#: 19974.1

5C. Gypsum Board* : (As an alternate to Item 5 when used as the base layer on one or both sides of wall, For direct attachment only, not to be used with Item 4). Nominal 5/8 in. thick lead backed gypsum panels with beveled, square or tapered edges, applied vertically. Vertical joints centered over studs and staggered min 1 stud cavity on opposite sides of studs. Wallboard secured to studs with 1-1/4 in. long Type S-12 (or #6 by 1-1/4 in. long bugle head fine drillers) steel screws spaced 8 in. OC at perimeter and 12 in. OC in the field.

NEW ENGLAND LEAD BURNING CO INC, DBA

5D. Gypsum Board* : (As an alternate to Item 5 when used as the base layer on one or both sides of wall, For direct attachment only, not to be used with Item 4) - Nom 5/8 or 3/4 in. may be used as alternate to all 5/8 or 3/4 in. shown in Item 5, Wallboard Protection on Each Side of Wall table. Nom 5/8 or 3/4 in. thick lead backed gypsum panels with beveled, square or tapered edges, applied vertically. Vertical joints centered over 20 MSG steel studs and staggered min 1 stud cavity on opposite sides of studs. See Items 1A, 2A 8, 8A(a). Wallboard secured to studs with 1-1/4 in. long Type S-12 steel screws spaced 8 in. OC at perimeter and 12 in. OC in the field. To be used with Lead Batten Strips (see Item 12A) or Lead Discs (see Item 13A).

MAYCO INDUSTRIES INC —Type X-Ray Shielded Gypsum

5E. Gypsum Board* —(As an alternate to Item 5 when used as the base layer on one or both sides of wall, For direct attachment only, not to be used with Item 4). Nom 5/8 in. may be used as alternate to all 5/8. shown in Item 5, Wallboard Protection on Each Side of Wall table. Nom 5/8 or 3/4 in. thick lead backed gypsum panels with beveled, square or tapered edges, applied vertically. Vertical joints centered over 20 MSG steel studs and staggered min 1 stud cavity on opposite sides of studs. See Items 1A, 2A 8, 8A(a). Wallboard secured to studs with 1-1/4 in. long Type S-12 steel screws spaced 8 in. OC at perimeter and 12 in. OC in the field. Lead batten strips required behind vertical joints of lead backed gypsum wallboard and optional at remaining stud locations. Lead batten strips, min 2 in. wide, max 8 ft long with a max thickness of 0.14 in. placed on the face of studs and attached to the stud with construction adhesive and two 1 in. long Type S-12 pan head steel screws, one at the top of the strip and one at the bottom of the strip. Lead discs, nominal 3/8 in. diam by max 0.085 in. thick. Compression fitted or adhered over the screw heads. Lead batten strips and discs to have a purity of 99.9% meeting the Federal specification QQ-L-201F, Grade 'C'.

RADIATION PROTECTION PRODUCTS INC —Type RPP-LBG

6. Fasteners : (Not shown) : For use with Item 5 - Type S-12 steel screws used to attach panels to runners (Item 1 or 1A) and studs (Item 2 or 2A) or Furring channels (Item 8). Single layer systems: 1 in. long for 1/2 and 5/8 in. thick panels or 1-1/4 in. long for 3/4 in. thick panels, spaced 8 in. OC when panels are applied horizontally, or 12 in. OC when panels are applied vertically. Two layer systems: First layer~ 1 in. long for 1/2 and 5/8 in. thick panels or 1-1/4 in. long for 3/4 in. thick panels, spaced 16 in. OC. Second layer~ 1-5/8 in. long for 1/2 in. and 5/8 in. thick panels or 2-1/4 in. long for 3/4 in. thick panels, spaced 16 in. OC with screws offset 8 in. from first layer. Three-layer systems: First layer~ 1 in. long for 1/2 in. thick panels, spaced 24 in. OC. Second layer~ 1-5/8 in. long for 1/2 in. thick panels, spaced 24 in. OC. Third layer~ 2-1/4 in. long for 1/2 in. thick panels, spaced 12 in. OC. Screws offset min 6 in. from layer below.

7. Batts and Blankets* —(Required as indicated under Item 5) —Nom 2 in. thick mineral wool batts, friction fitted between studs and runners. See Batts and Blankets (BKNV or BJJZ) Categories for names of Classified companies.

7A. Batts and Blankets* —(Optional, not shown) —Placed in stud cavities, any glass fiber or mineral wool insulation bearing the UL Classification Marking as to Surface Burning Characteristics and/or Fire Resistance. See Batts and Blankets (BKNV or BJJZ) Categories for names of Classified companies.

7B. Batts and Blankets* —(Optional, not shown) —Placed in stud cavities, glass fiber insulation bearing the UL Classification Marking as to Surface Burning Characteristics and/or Fire Resistance.

DWENS CORNING —Type QuietZone Acoustic Batts

8. Furring Channels : (Optional on one or both sides, not shown, for single or double layer systems) —Resilient furring channels fabricated from min 25 MSG corrosion-protected steel, spaced vertically a max of 24 in. OC. Flange portion attached to each intersecting stud with 1/2 in. long Type S-12 panhead steel screws. Not for use with type FRX-G gypsum panels and Item 5A or 5C.

8A. Steel Framing Members (Not Shown)* —(Optional on one or both sides, not shown, for single or double layer systems) —As an alternate to Item 8, furring channels and Steel Framing Members as described below:

a. Furring Channels —Formed of No. 25 MSG galv steel, 2-3/8 in. wide by 7/8 in. deep, spaced max. 24 in. OC perpendicular to studs. Channels secured to studs as described in Item 6. Gypsum board attached to furring channels as described in Item 6. Not for use with type FRX-G gypsum panels and Item 5A or 5C.

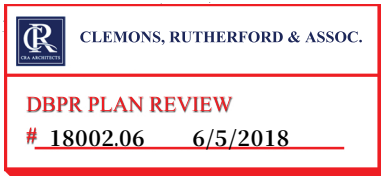
b. Steel Framing Members* —Used to attach furring channels (Item 8a) to studs (Item 2). Clips spaced max. 48 in. OC, and secured to studs with No. 8 x 1-1/2 in. minimum self-drilling, S-12 steel screw through the center grommet. Furring channels are friction fitted into clips.

PAC INTERNATIONAL INC —Type RSIC-1.

9. Joint Tape and Compound : Vinyl or casein, dry or premixed joint compound applied in two coats to joints and screw heads of outer layers. Paper tape, nom 2 in. wide, embedded in first layer of compound over all joints of outer layers. Paper tape and joint compound may be omitted when gypsum boards are supplied with square edges.

10. Siding, Brick or Stucco —(Optional, not shown) —Aluminum, vinyl or steel siding, brick veneer or stucco, meeting the requirements of local code agencies. Brick veneer attached to studs with corrugated metal wall ties attached to each stud with steel screws, not more than each sixth course of brick.

11. Caulking and Sealants* —(Optional, not shown) —A bead of acoustical sealant applied around the partition perimeter for sound control.

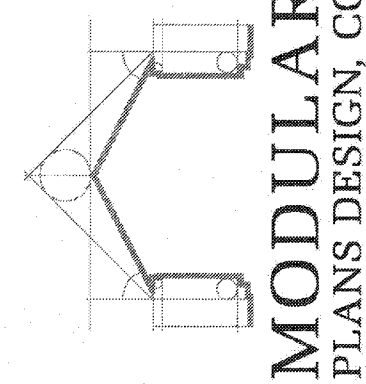


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ROBERT T. HAUG
FLORIDA
P. E. LICENSE #24575
1820 E EDGEWOOD DR
LAKELAND, FLORIDA 33803

REVISION DATE:	DATE: 03-20-2018
	DRAWN: R.L.G.
	JOB #: MPD-1518
	SHEET NO. A1.2

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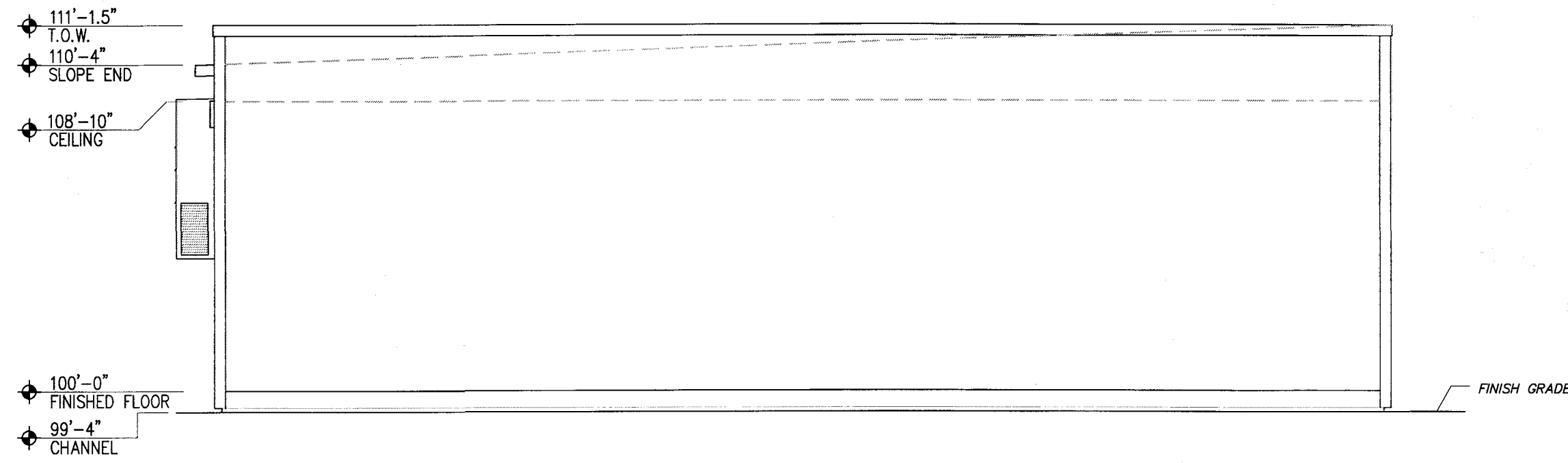
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33803

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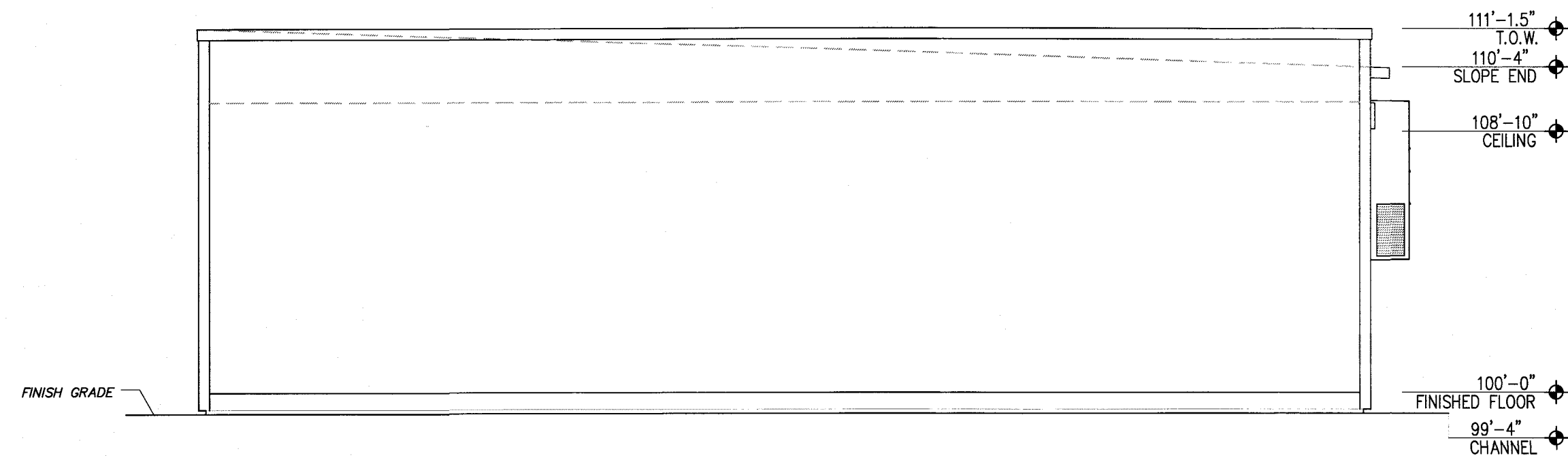


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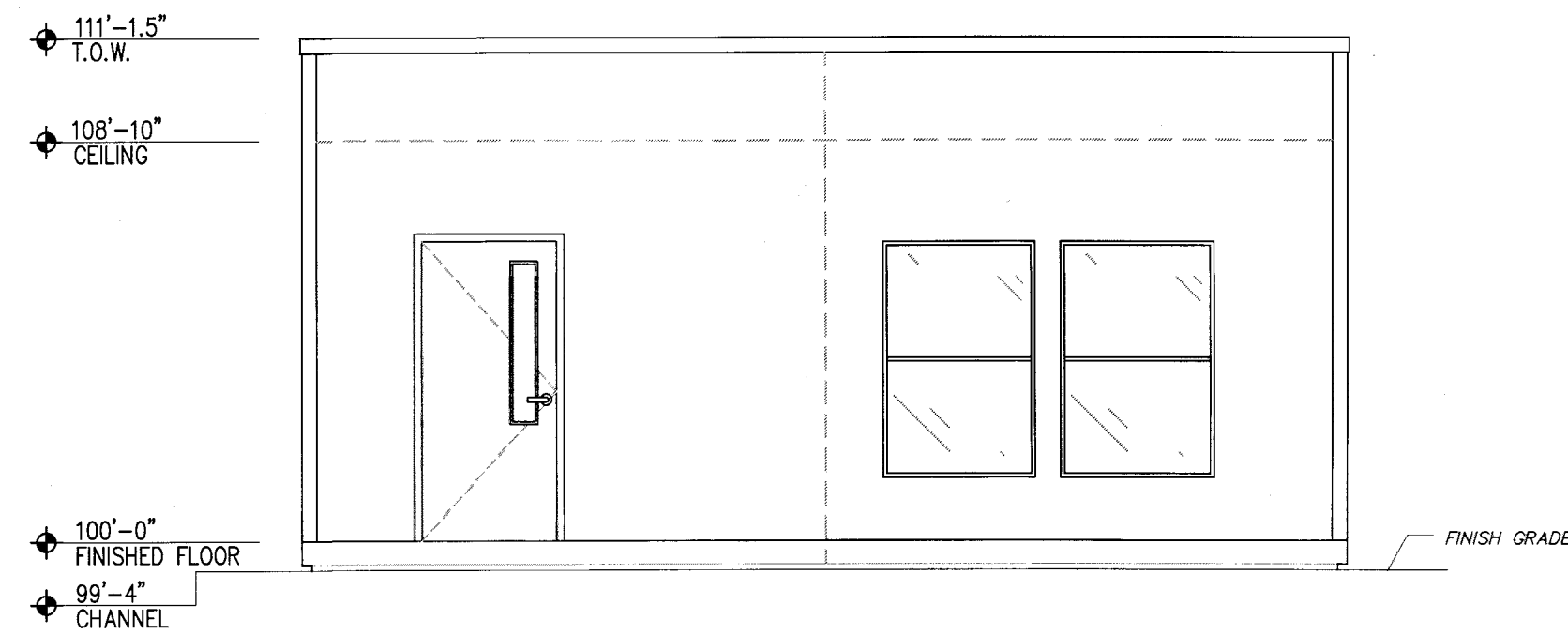
MODULAR STRUCTURE FOR:
Mobile Modular
(24' X 36')



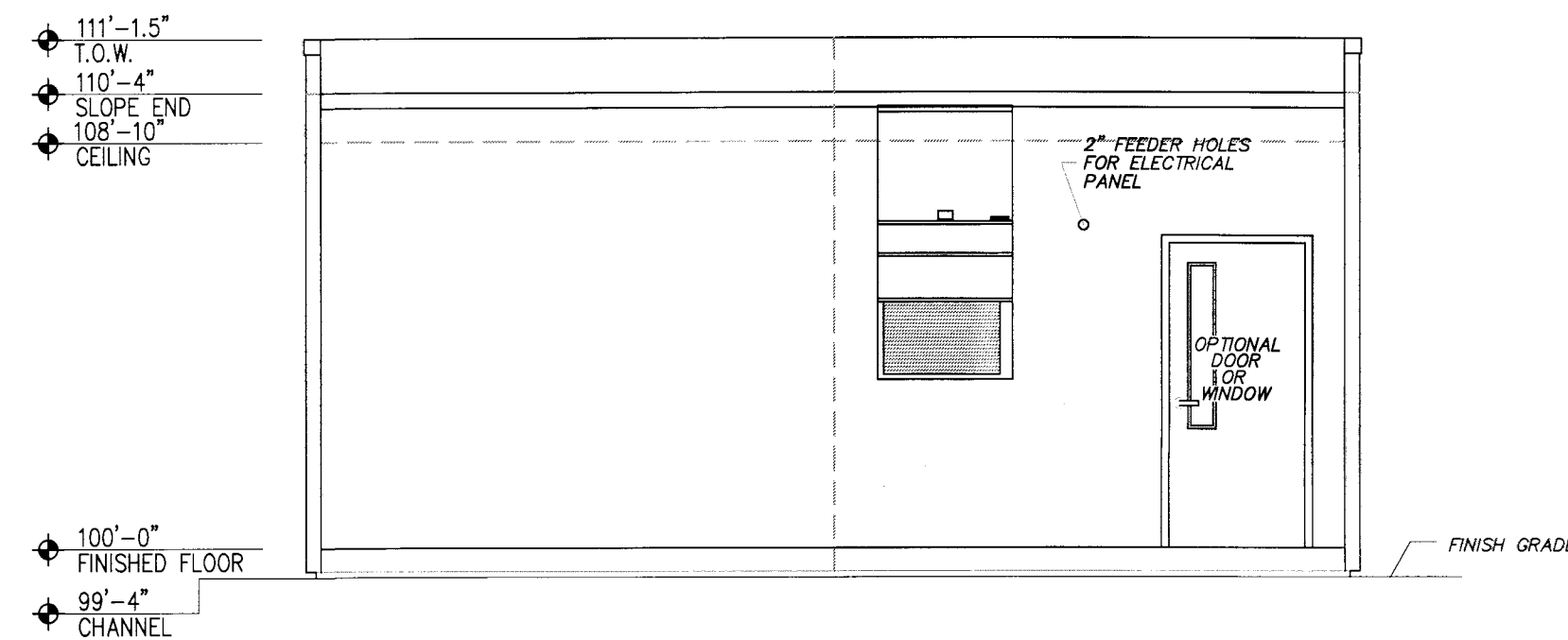
LEFT SIDE ELEVATION
SCALE: 1/4" = 1'-0"



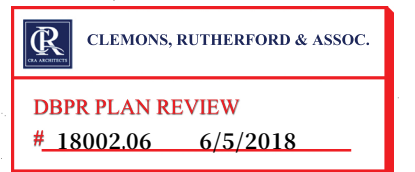
RIGHT SIDE ELEVATION
SCALE: 1/4" = 1'-0"



FRONT ELEVATION
SCALE: 1/4" = 1'-0"



BACK ELEVATION
SCALE: 1/4" = 1'-0"

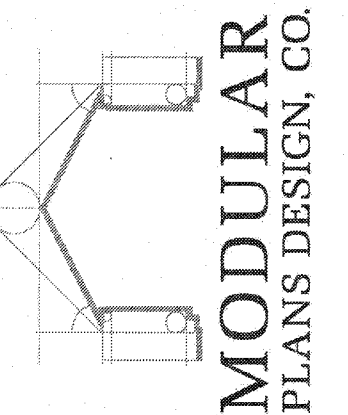


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REVISION DATE:	DATE:	03-20-2018
	DRAWN:	R.L.G.
	JOB #:	MPD-1518
	SHEET NO.	A2

3/21/18

MODULAR STRUCTURE FOR:
Mobile Modular
(24' X 36')

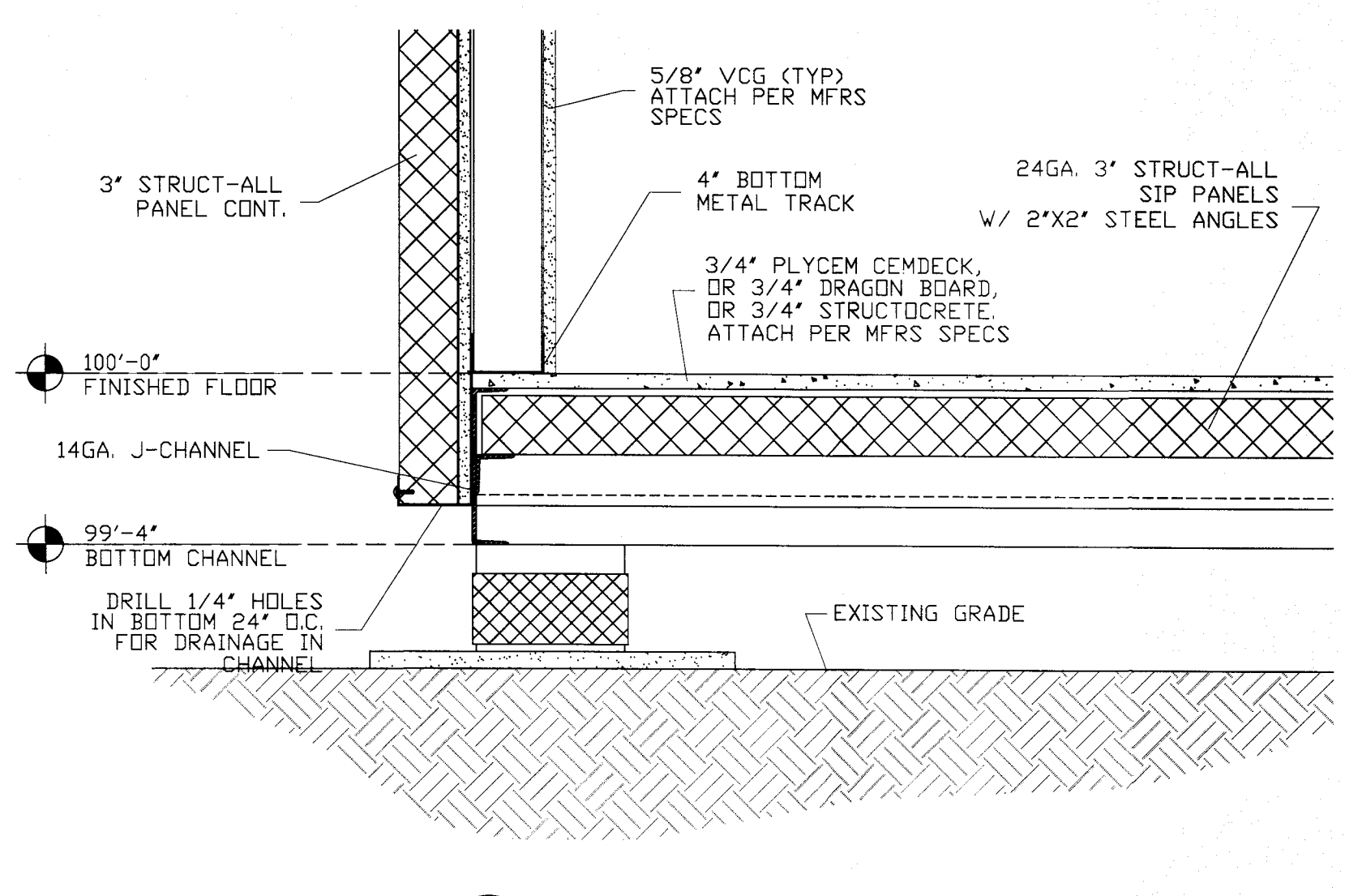
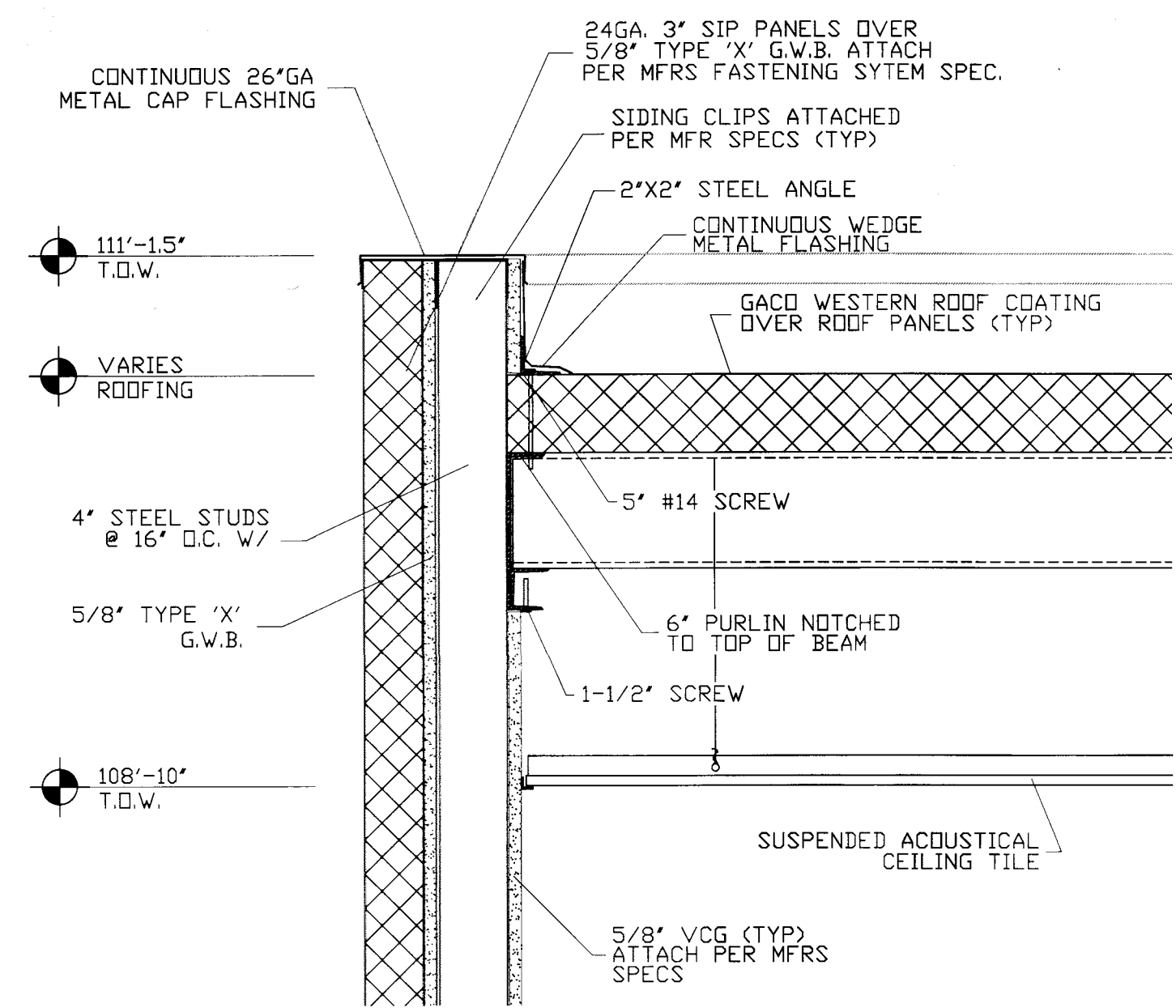
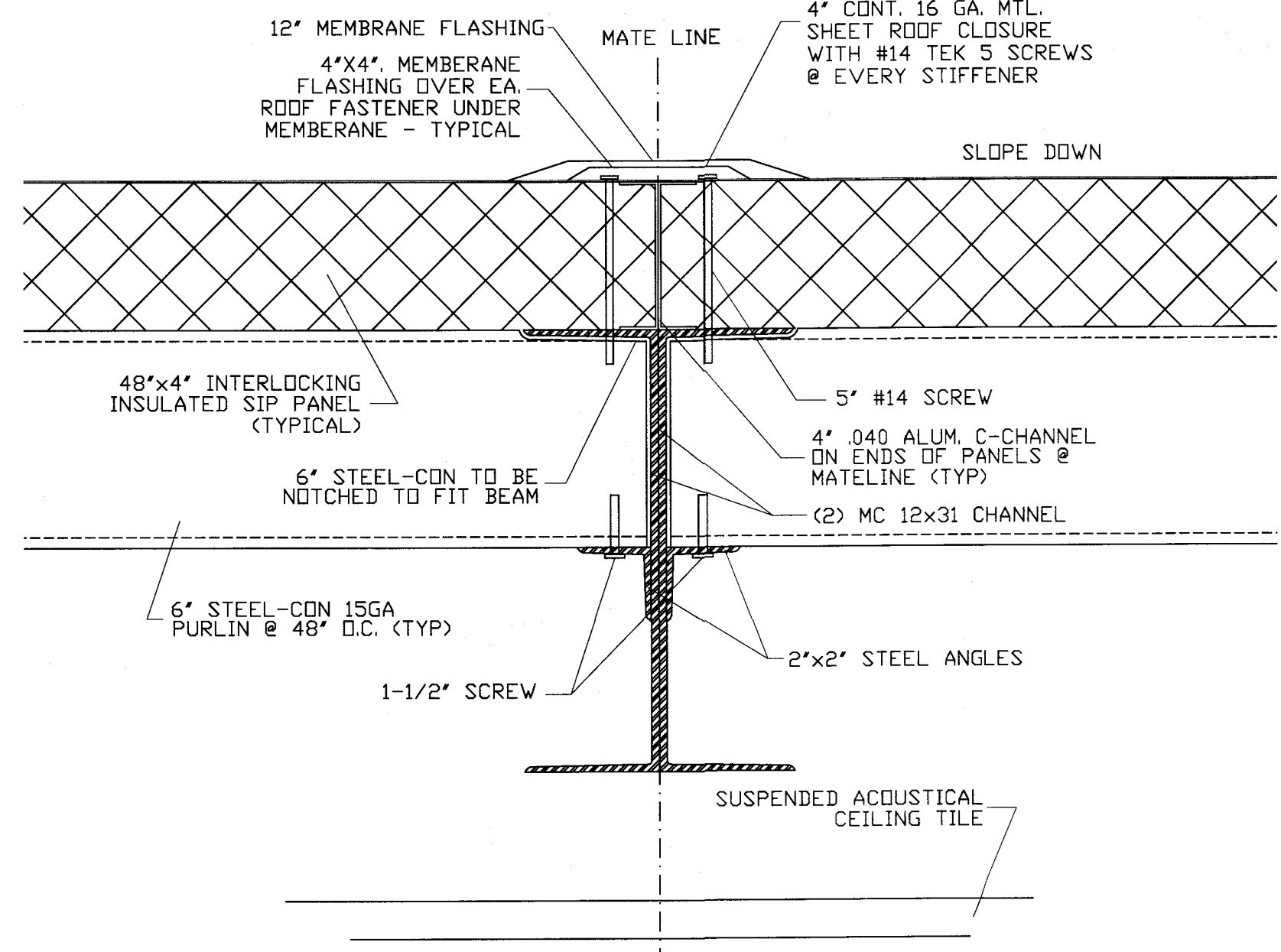
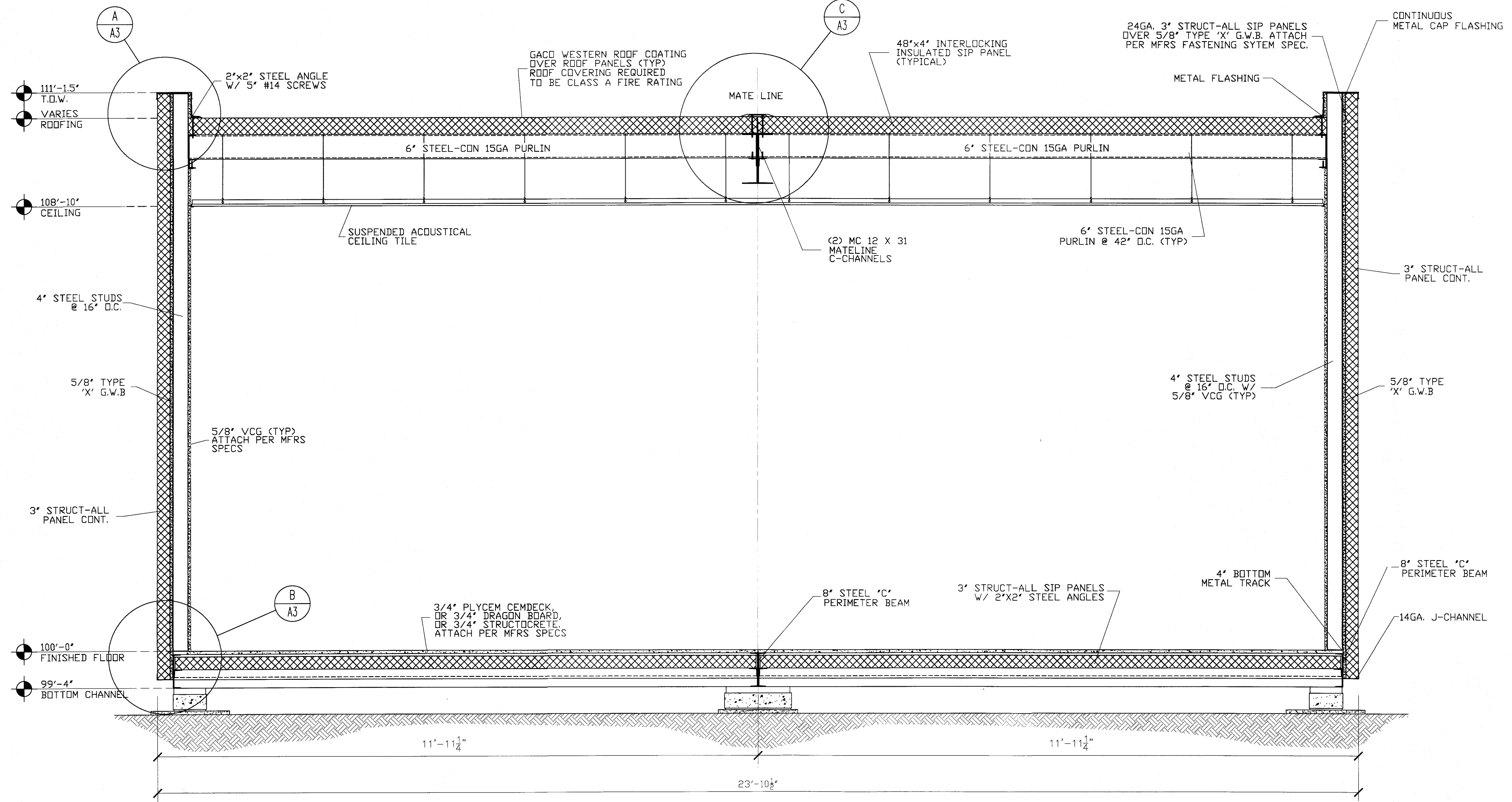


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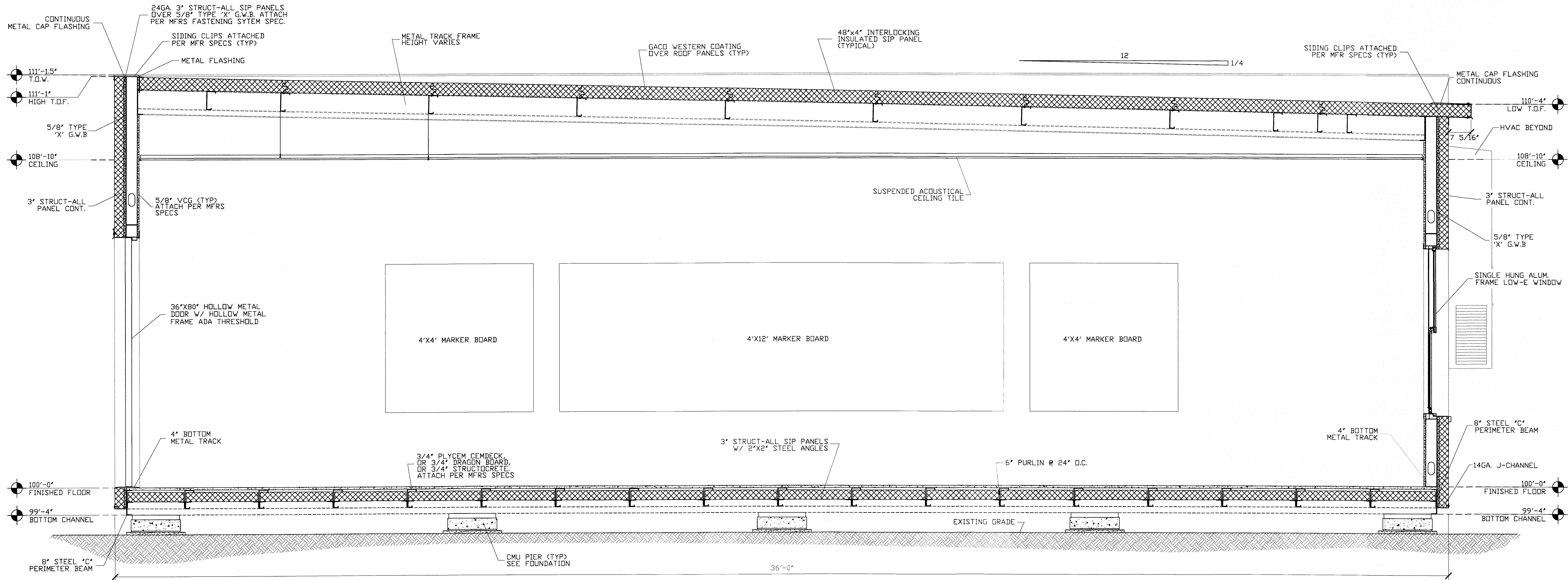
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REVISION DATE:

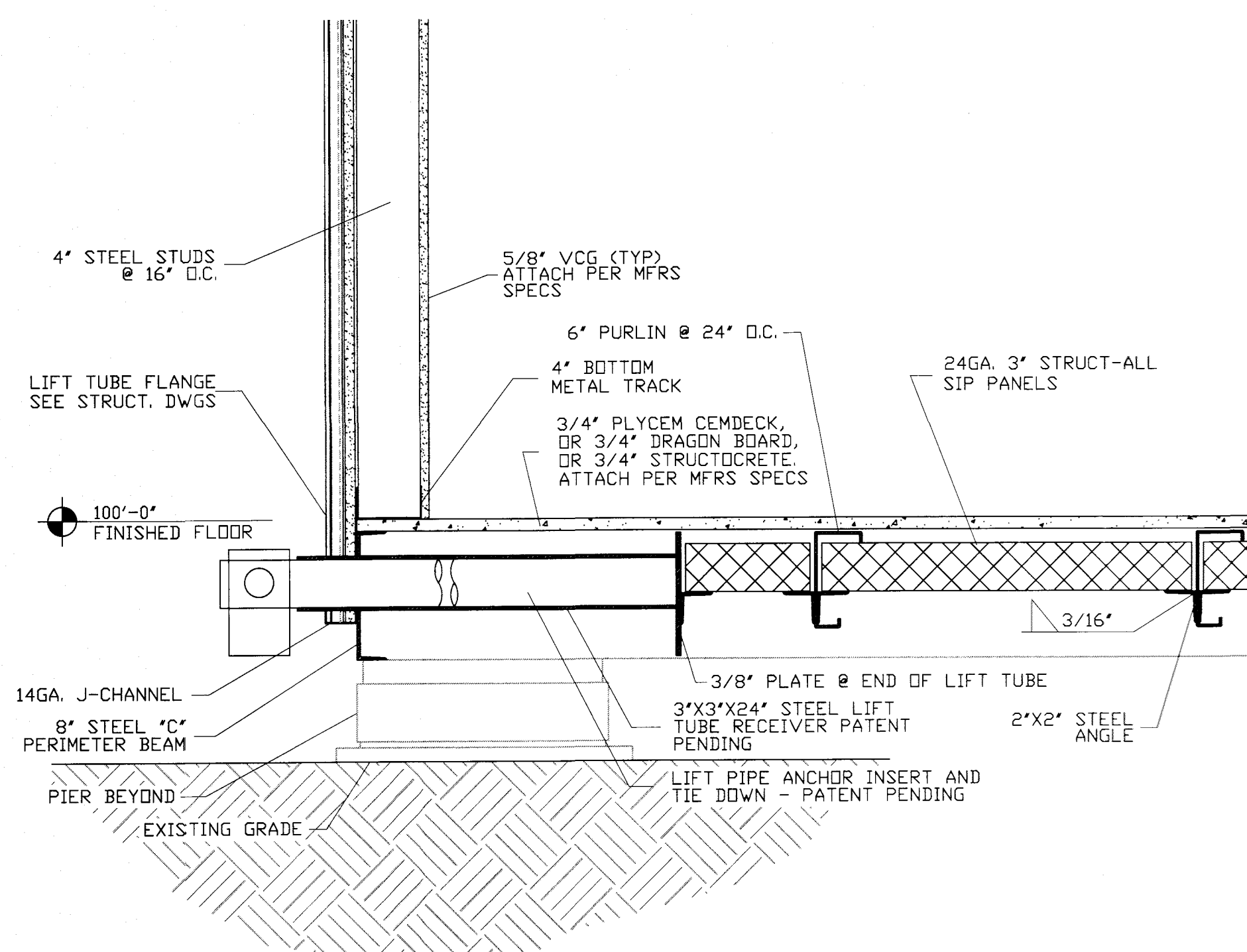
3/21/18

DATE: 03-20-2018
DRAWN: R.L.G.
JOB #: MPD-1518
SHEET NO. A3

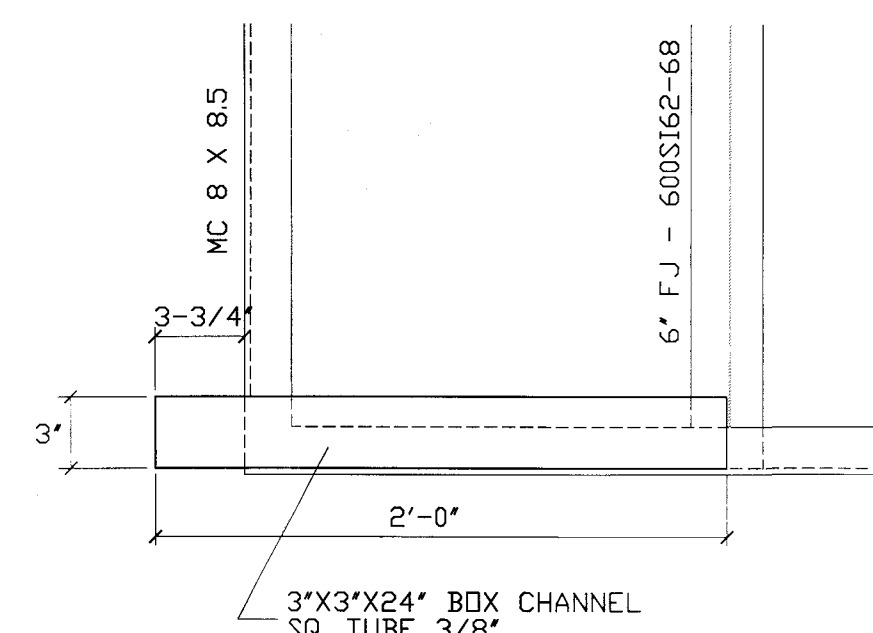
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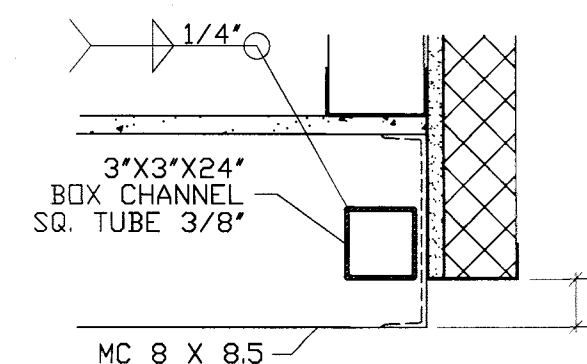
SIDE CROSS SECTION
SCALE: 3/4" = 1'-0"



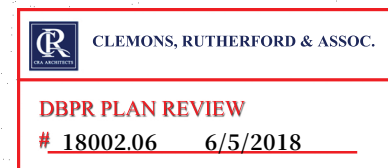
LIFT TUBE RECEIVER @ ENDWALL
SCALE: 1 1/2" = 1'-0"



LIFT TUBE RECEIVER TOP VIEW
SCALE: 1 1/2" = 1'-0"



LIFT TUBE RECEIVER FRONT VIEW
SCALE: 1 1/2" = 1'-0"



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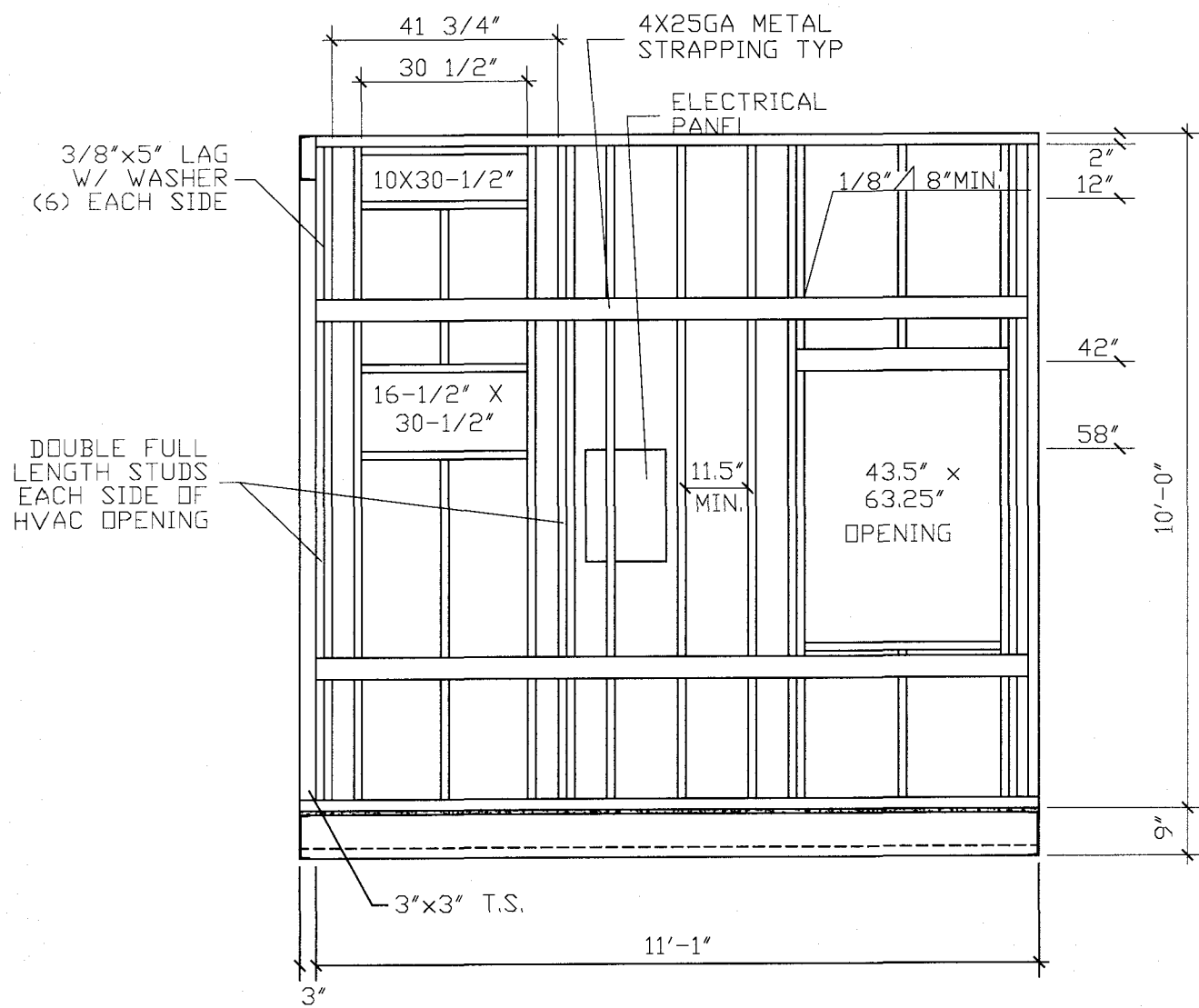
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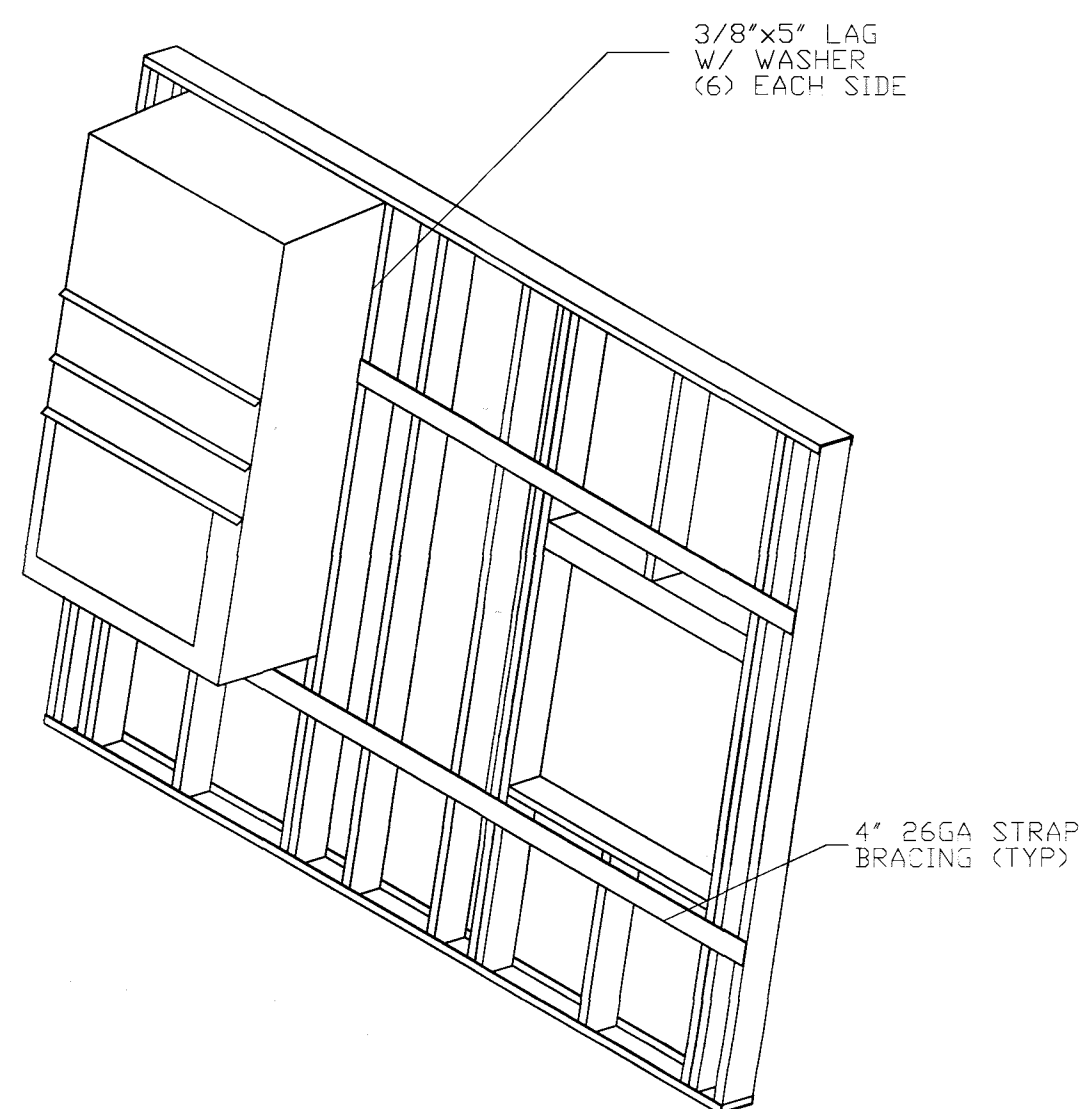
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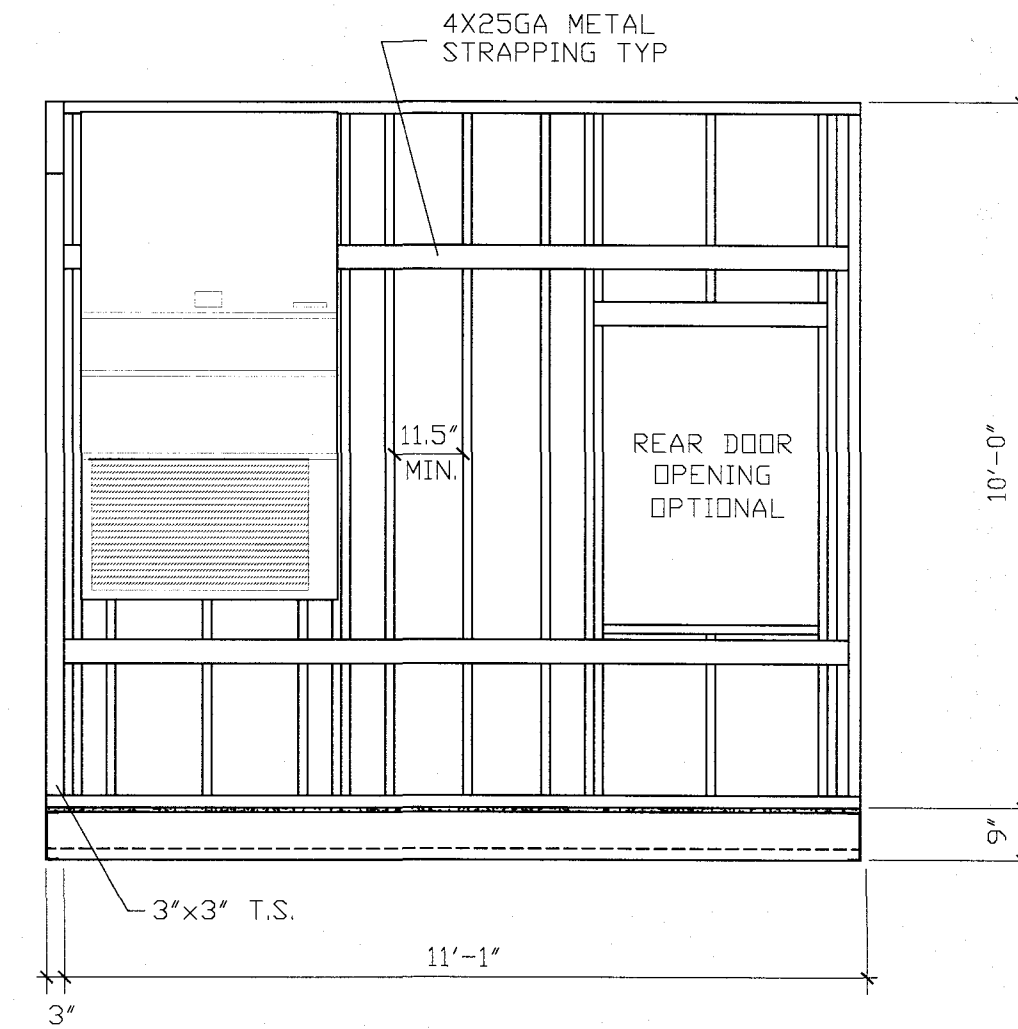
MODULAR STRUCTURE FOR:
Mobile Modular
(24' X 36')



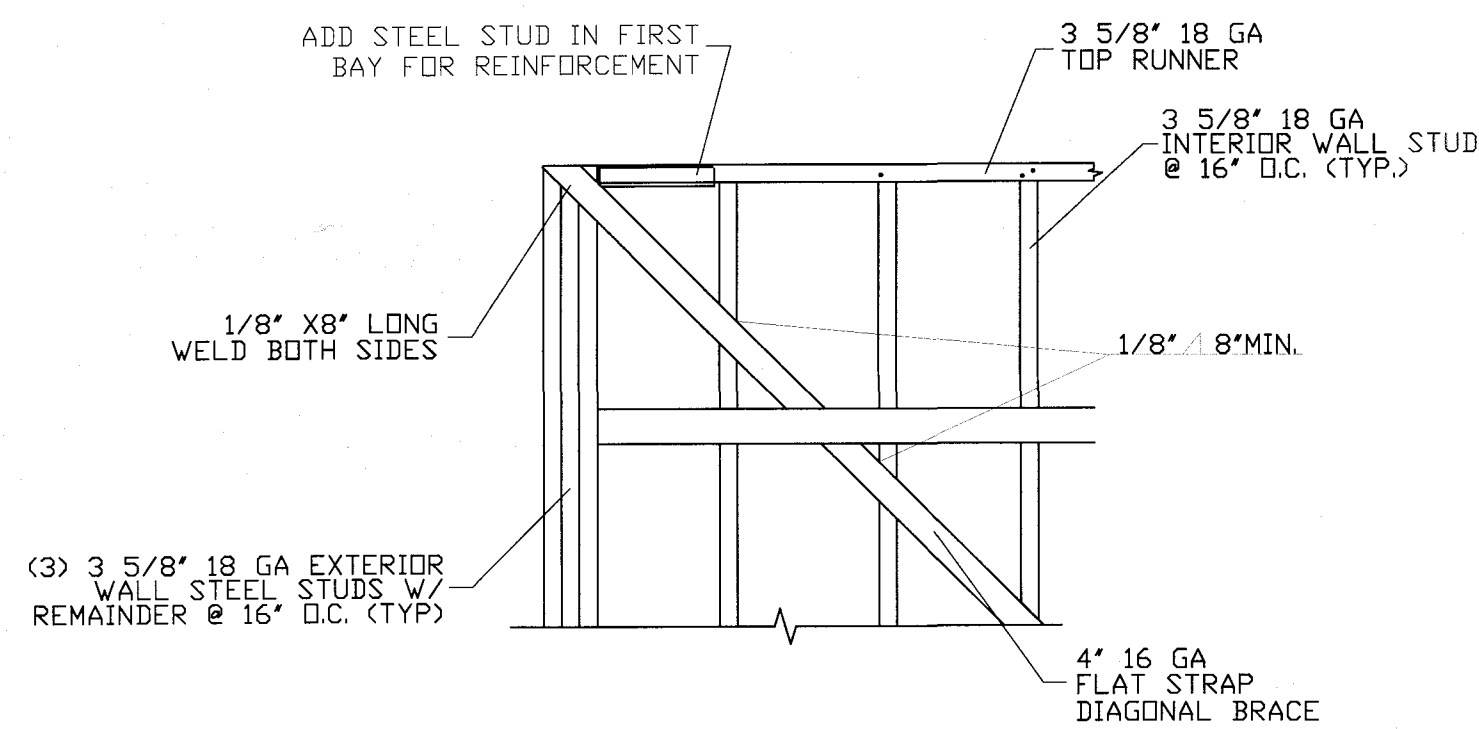
ENDWALL-HVAC
SCALE: 3/8" = 1'-0"



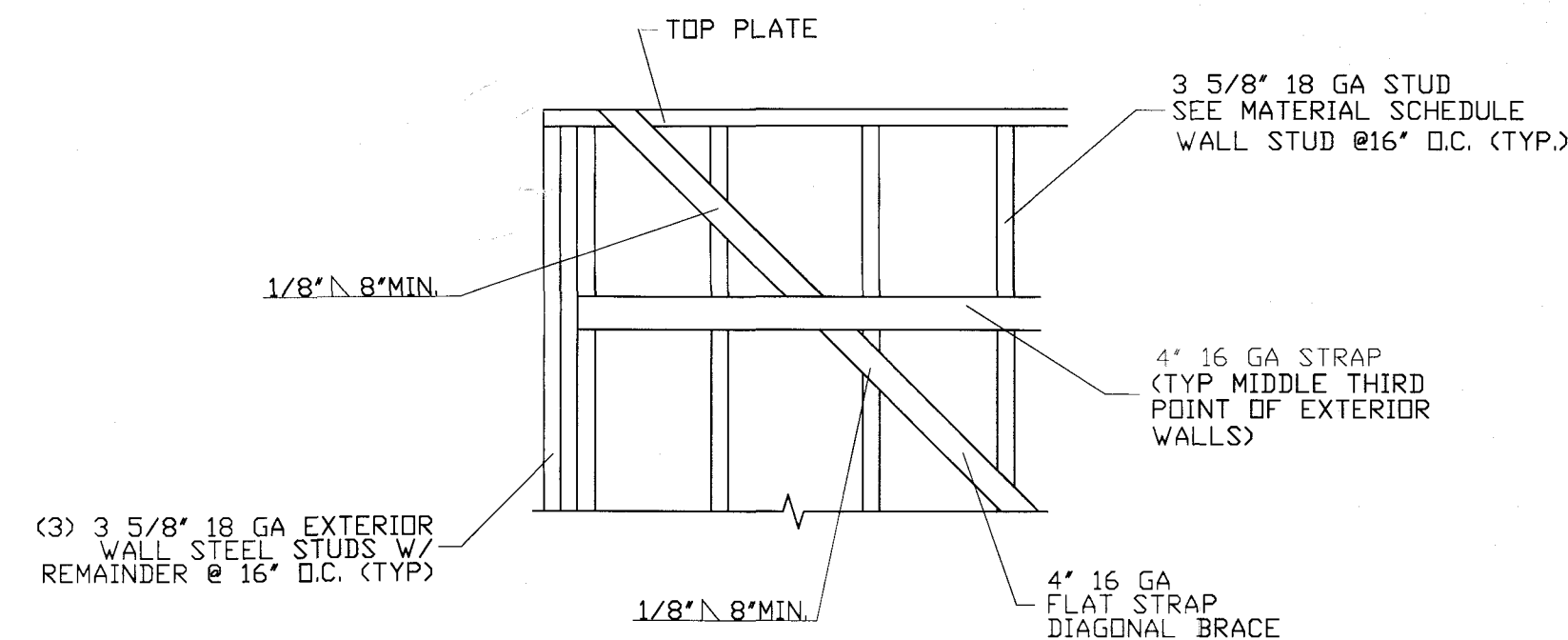
ENDWALL ISO
SCALE: 3/8" = 1'-0"



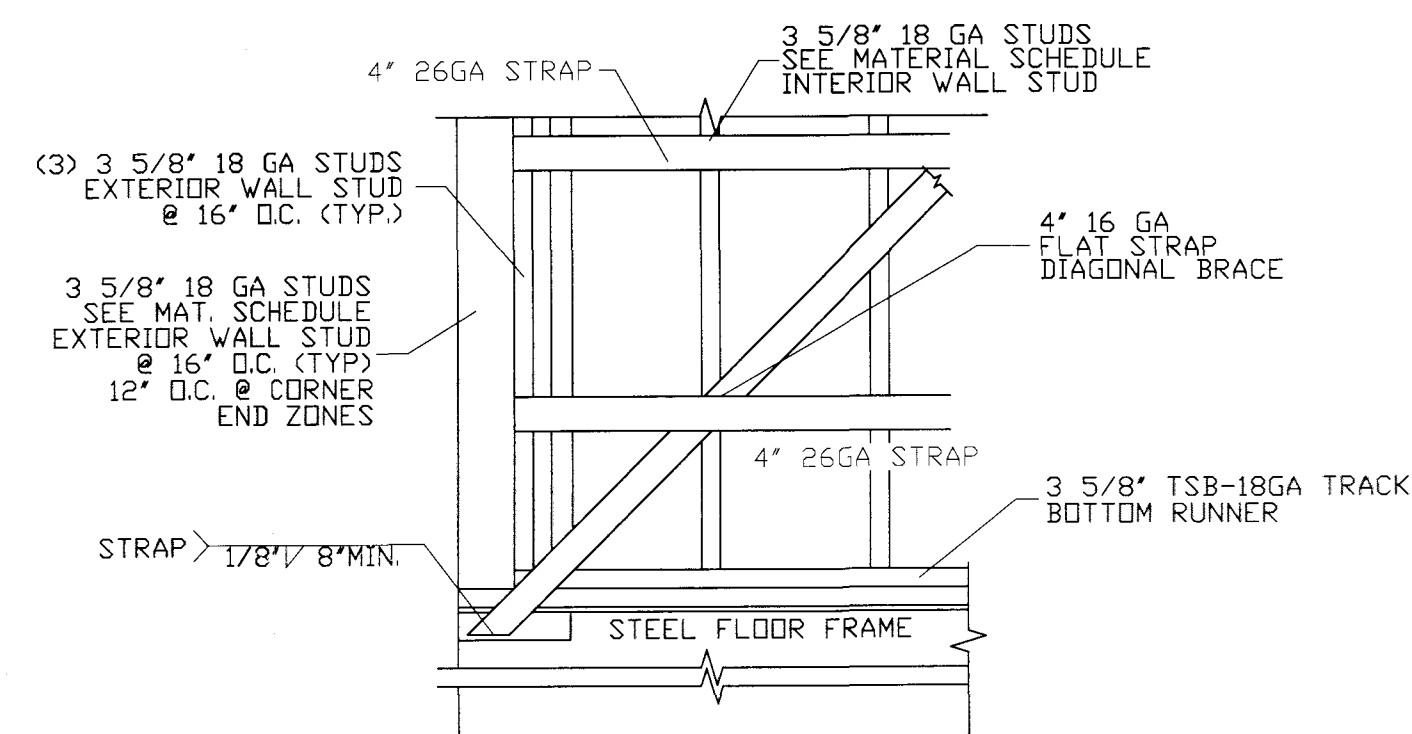
ENDWALL-HVAC
SCALE: 3/8" = 1'-0"



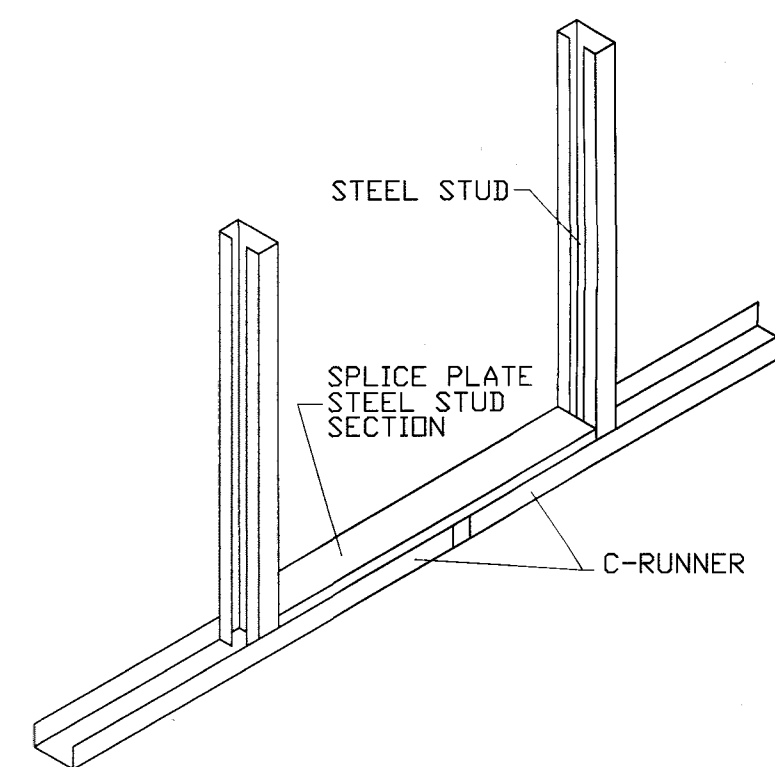
X-BRACING TO TOP OF SIDEWALL
SCALE: NTS



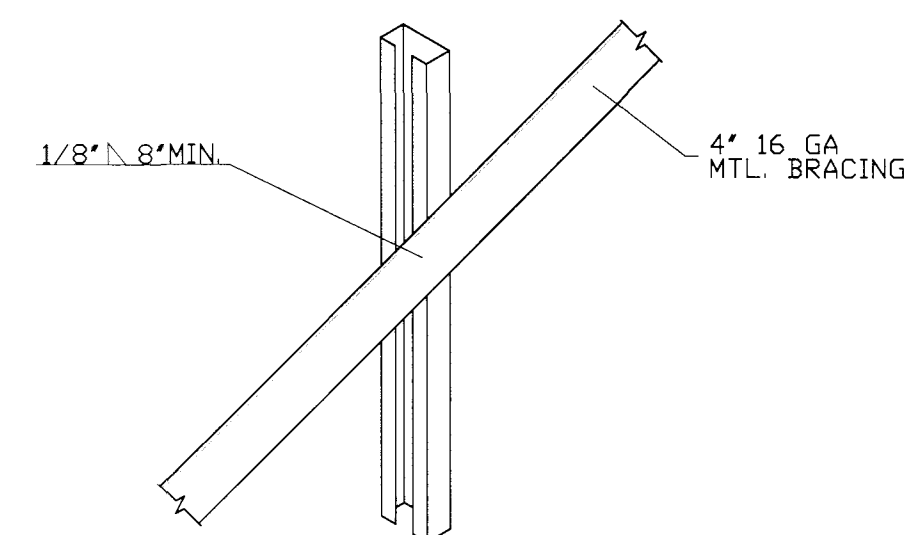
X-BRACING TO TOP OF ENDWALL
SCALE: NTS



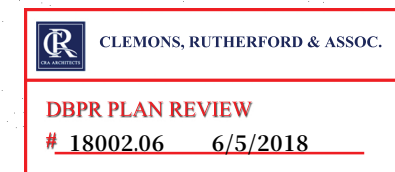
X-BRACING TO FLOOR FRAMING
SCALE: NTS



WALL FRAMING DETAIL
SCALE: NTS



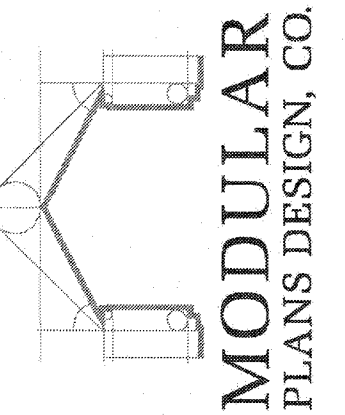
DIAGONAL BRACING DETAIL
SCALE: NTS



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	JOB #: MPD-1518
	SHEET NO. A3.2

MODULAR STRUCTURE FOR:
Mobile Modular
(24' X 36')

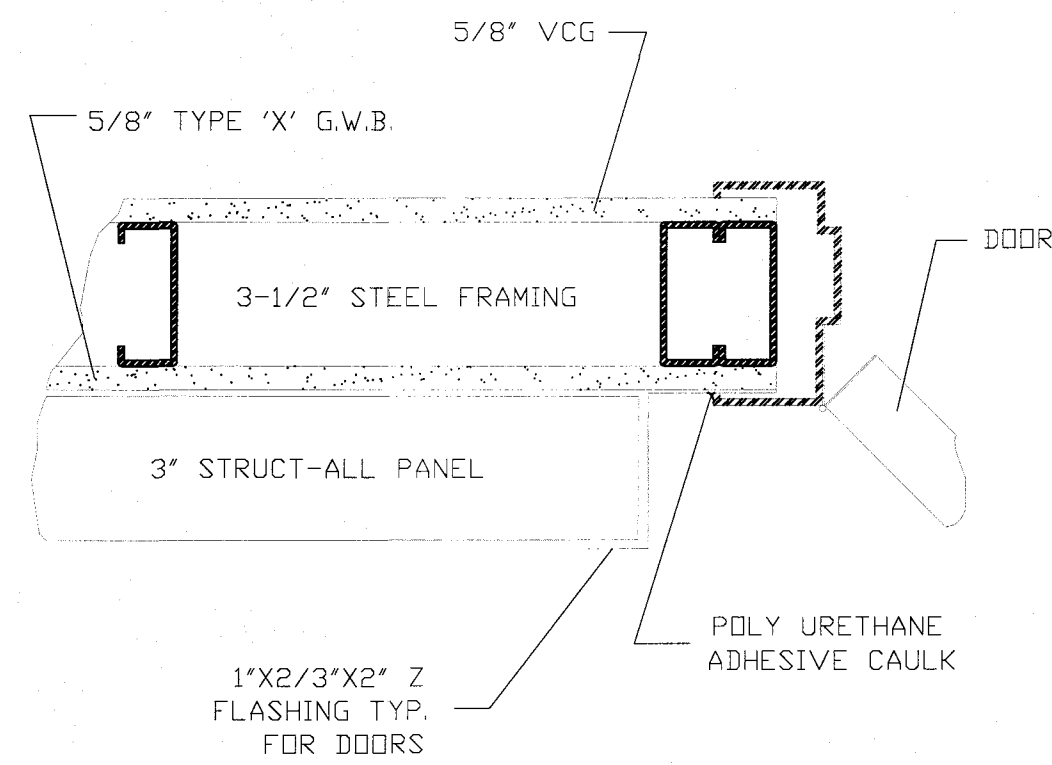


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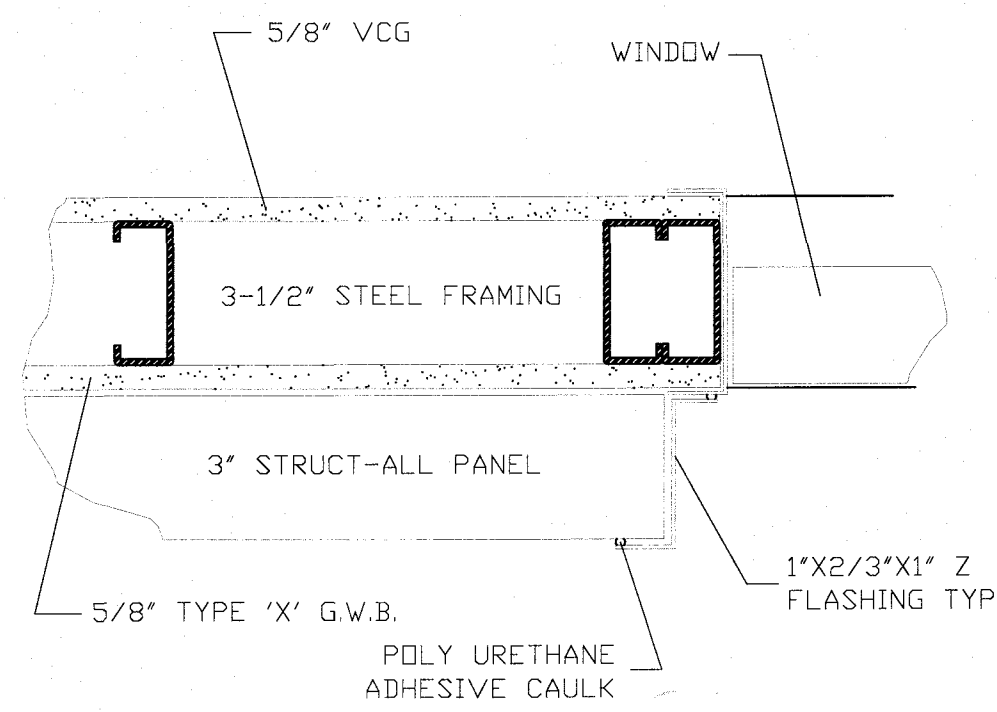
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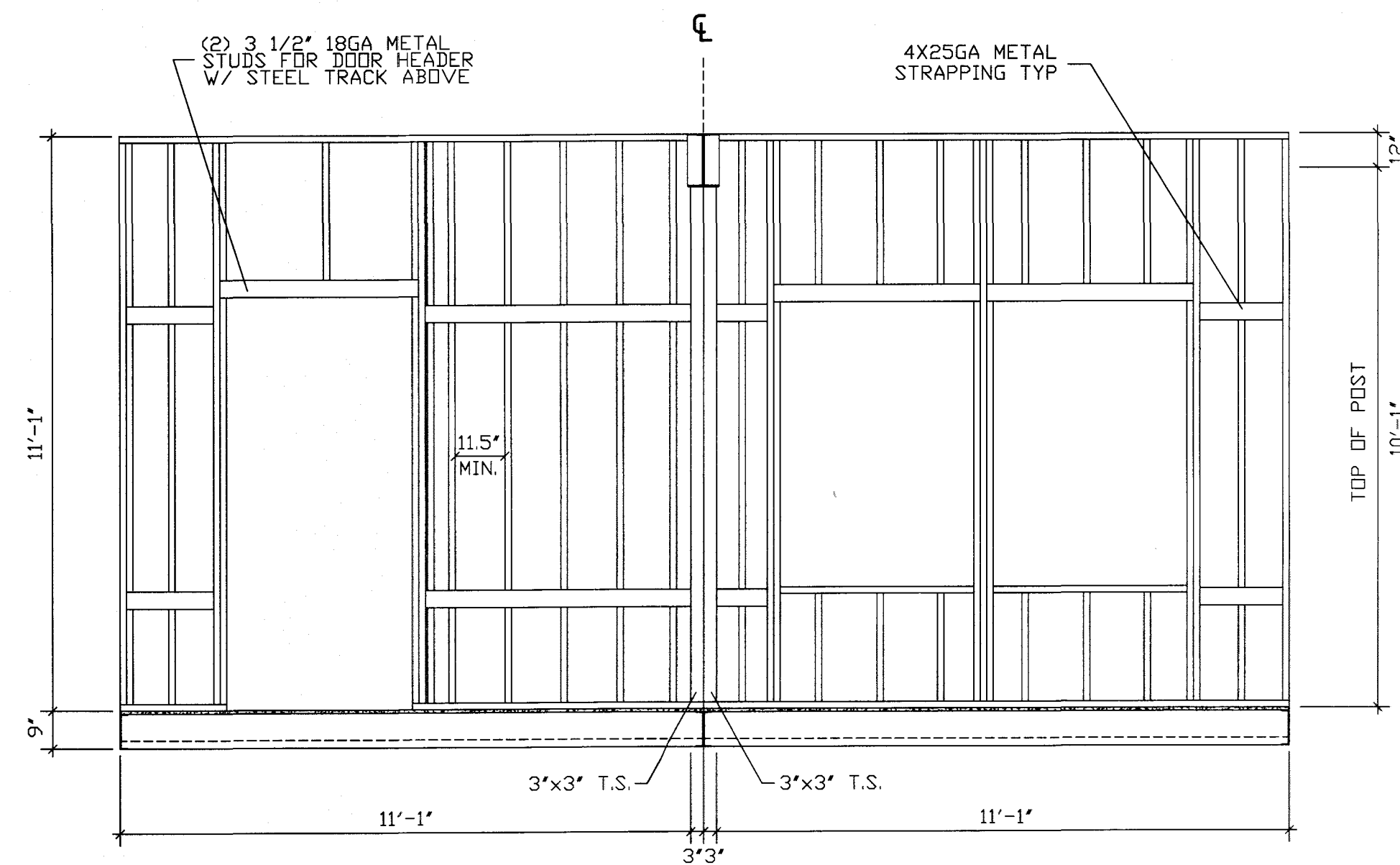




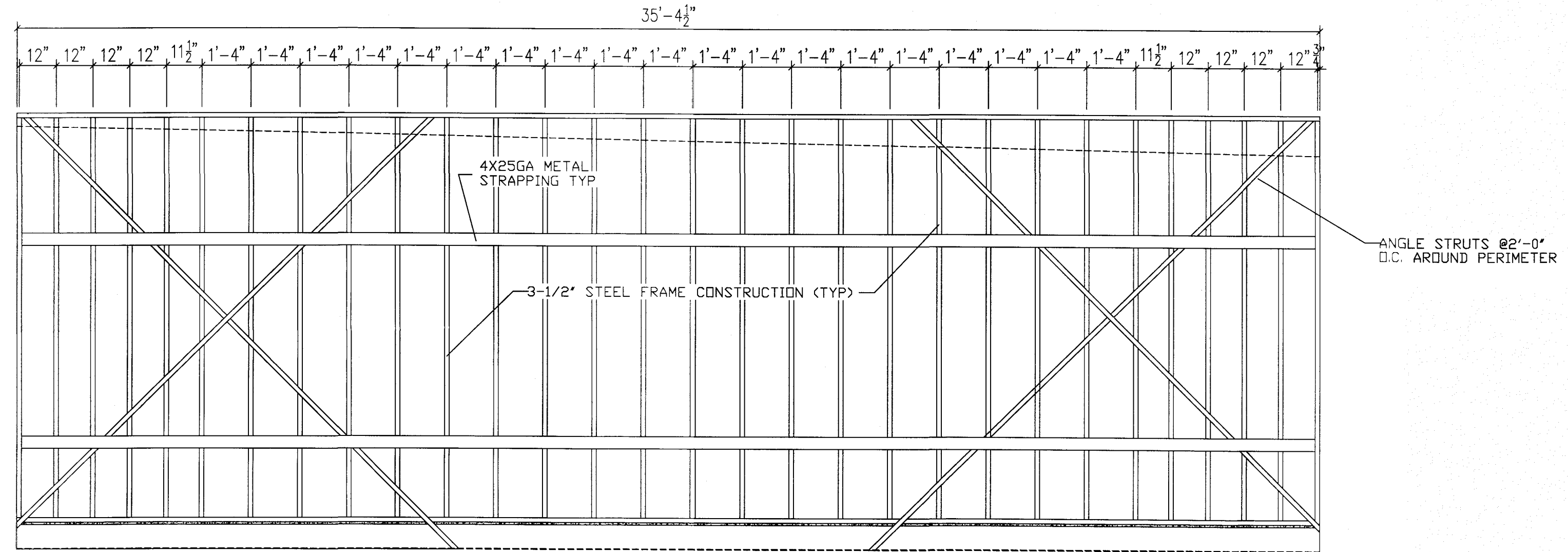
DOOR FRAMING DETAIL
SCALE: 3" = 1'-0"



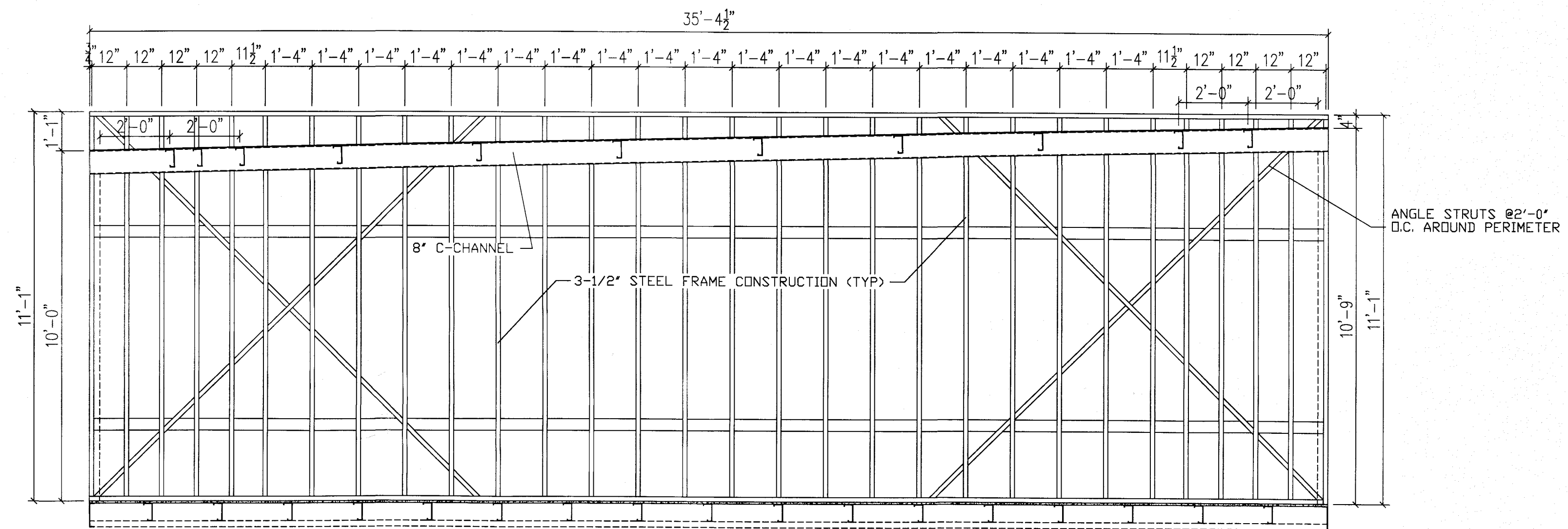
WINDOW FRAMING DETAIL
SCALE: 3" = 1'-0"



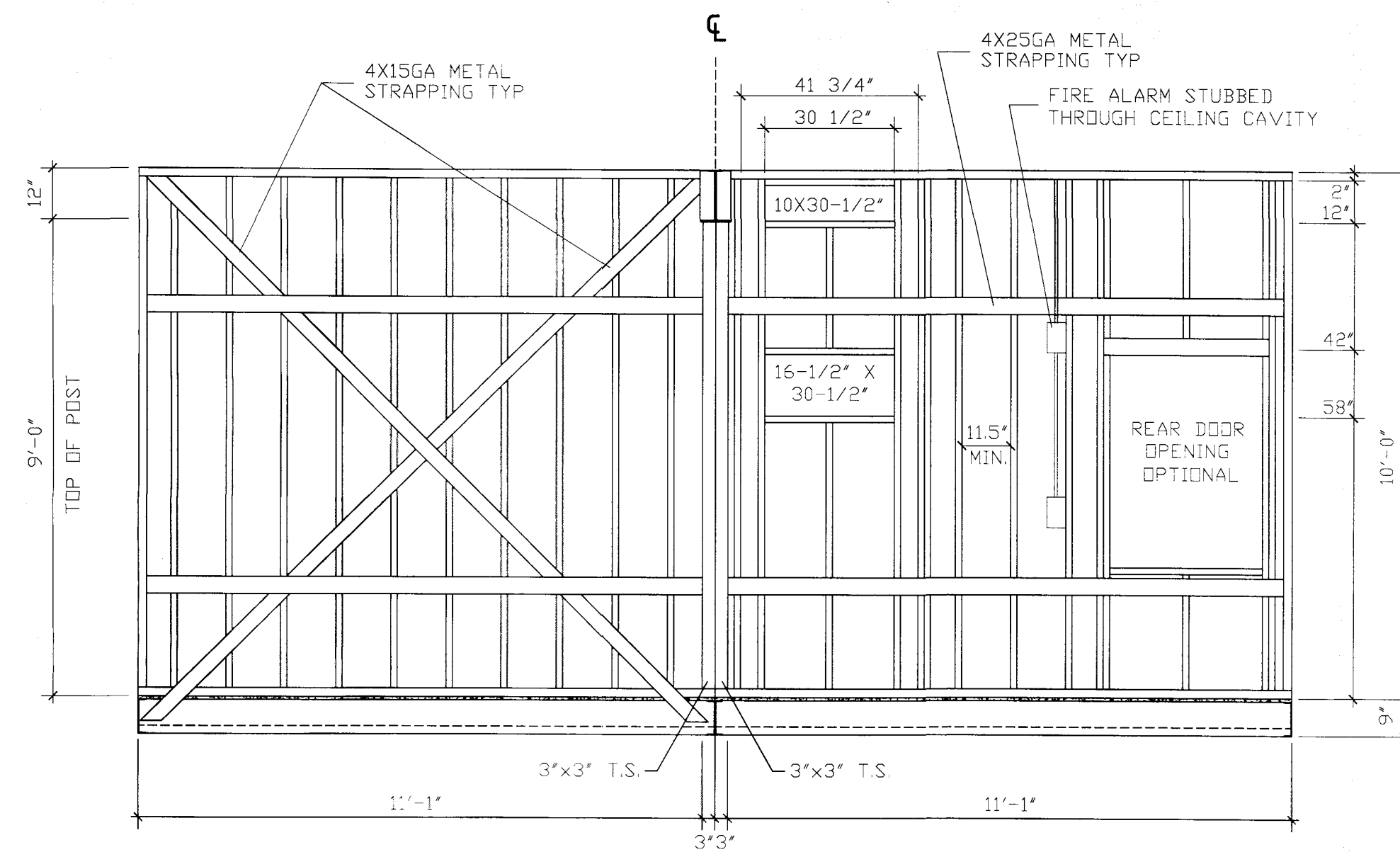
ENTRY ENDWALL FRAMING ELEVATION
SCALE: 3/8" = 1'-0"



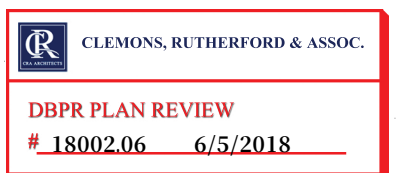
SIDEWALL FRAMING ELEVATION
SCALE: 3/8" = 1'-0"



SIDEWALL INTERIOR FRAMING
SCALE: 3/8" = 1'-0"



HVAC ENDWALL FRAMING ELEVATION
SCALE: 3/8" = 1'-0"



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JOB #: MPD-1518
SHEET NO.
A3.3

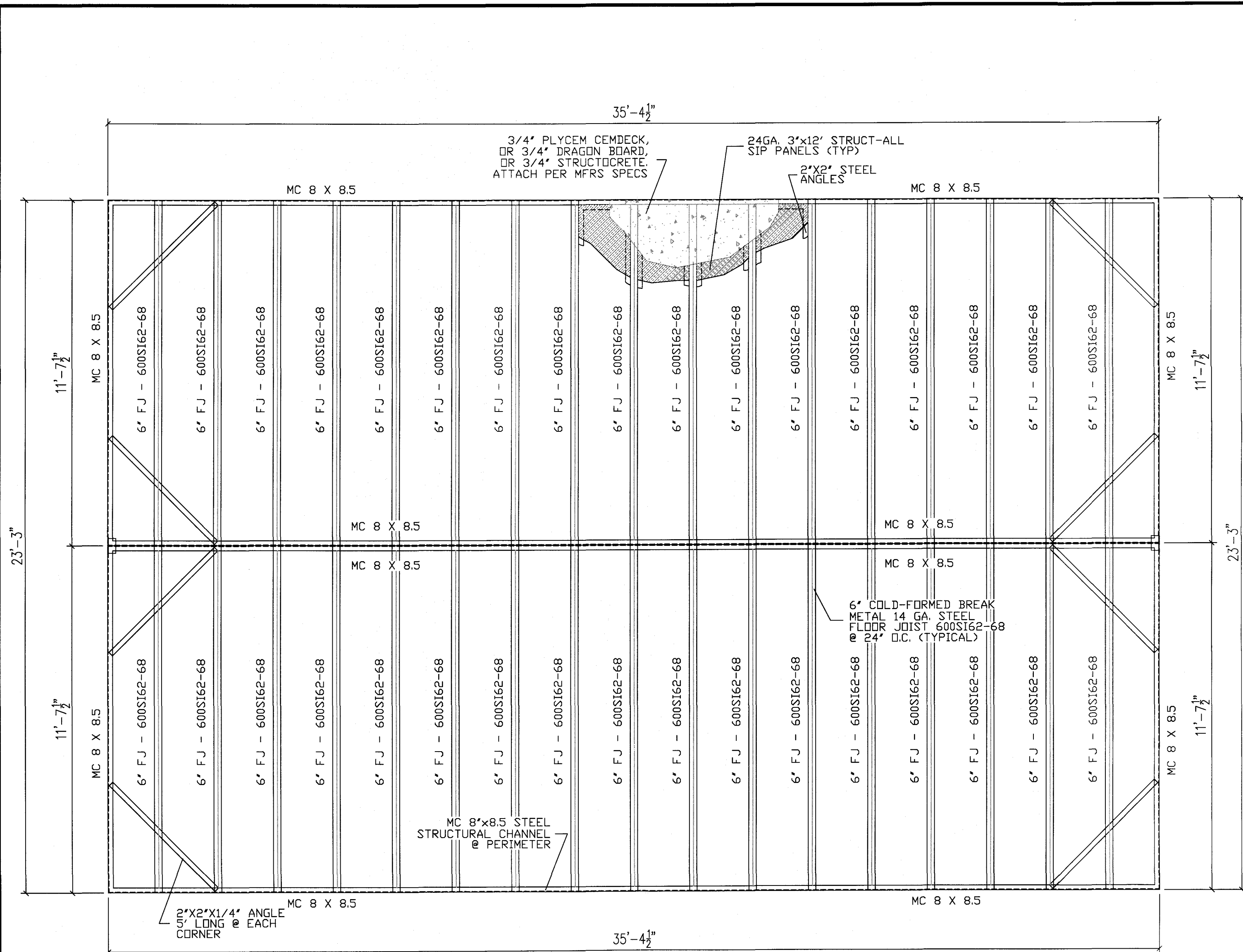
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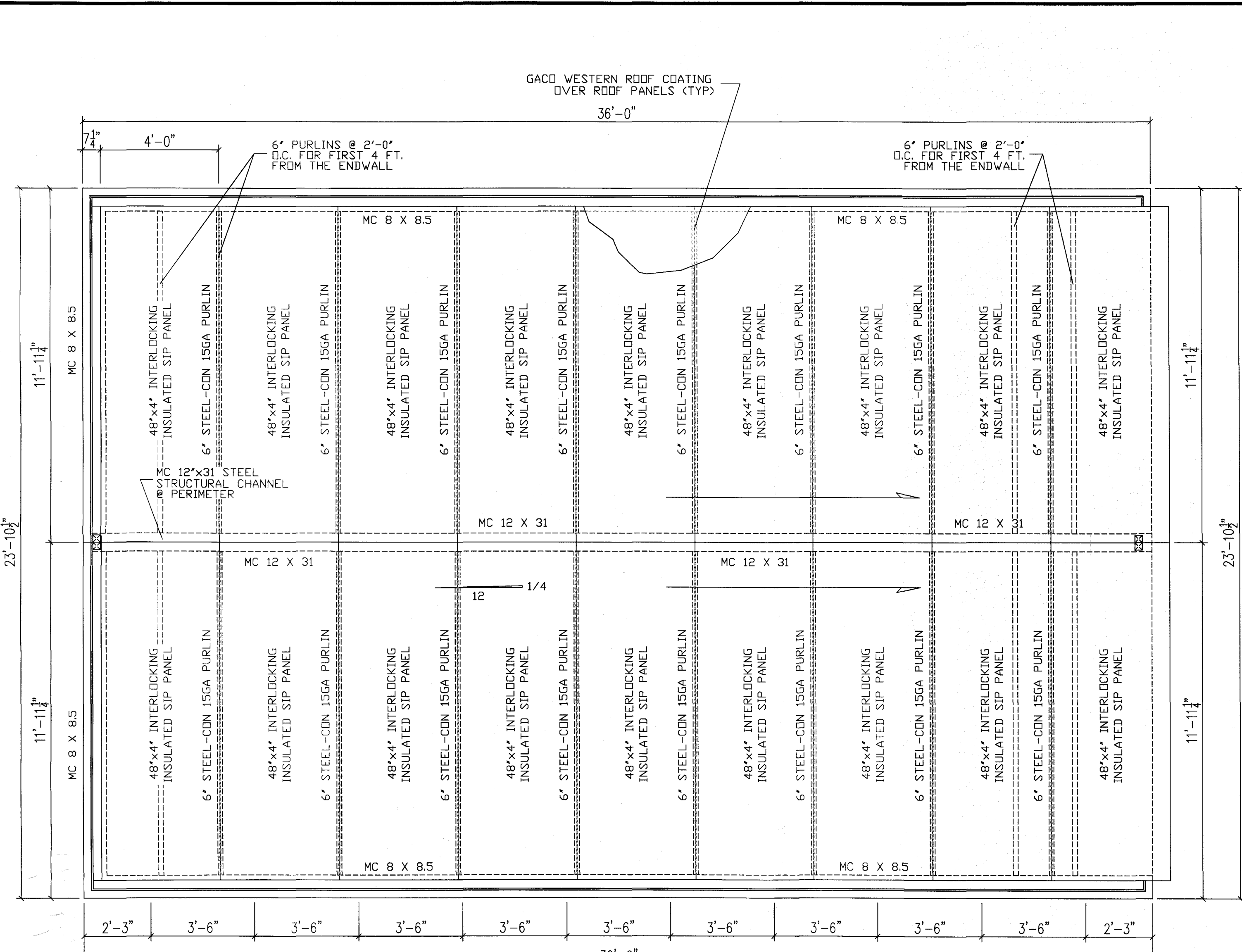
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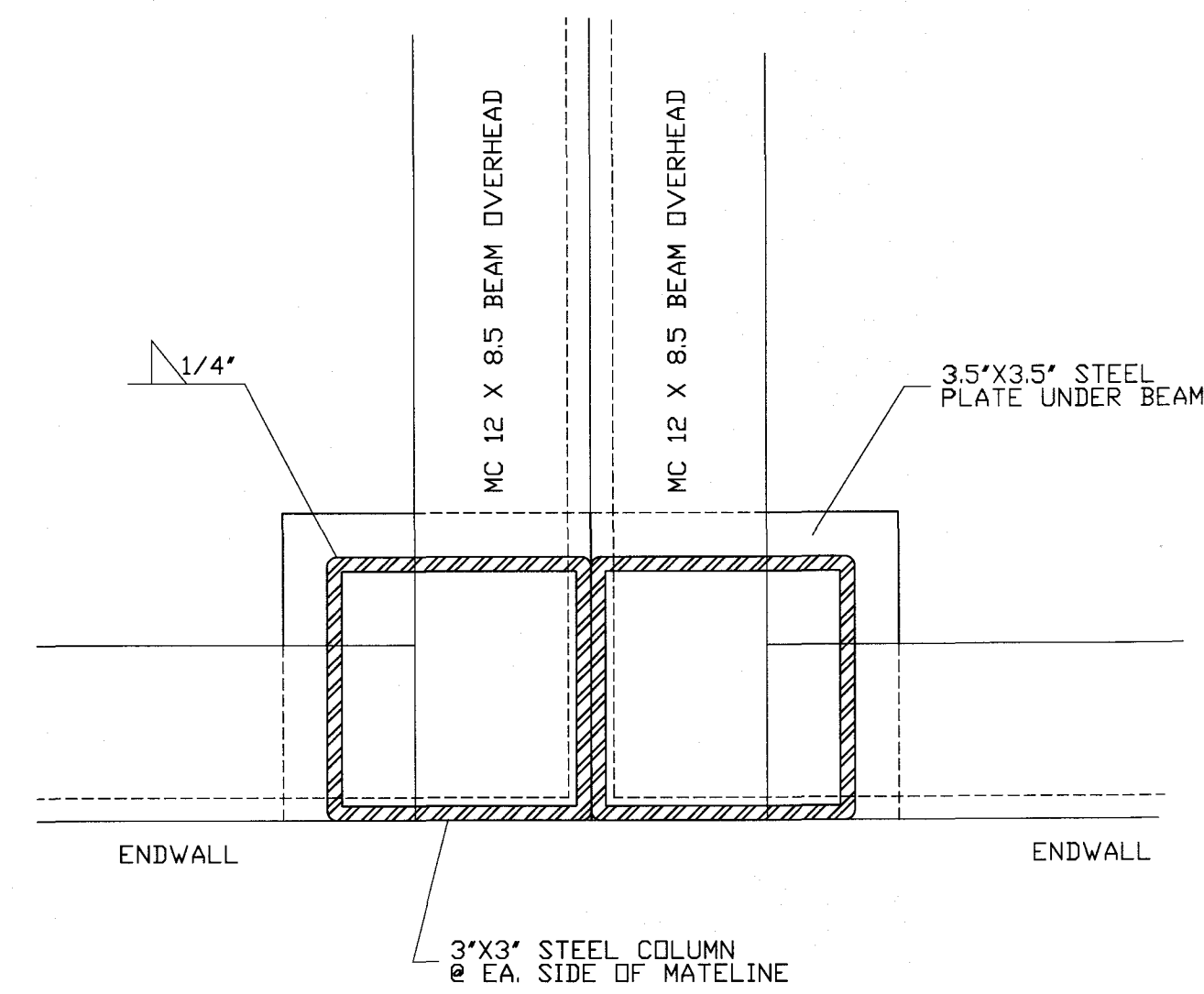
MODULAR STRUCTURE FOR:
Mobile Modular
(24' X 36')



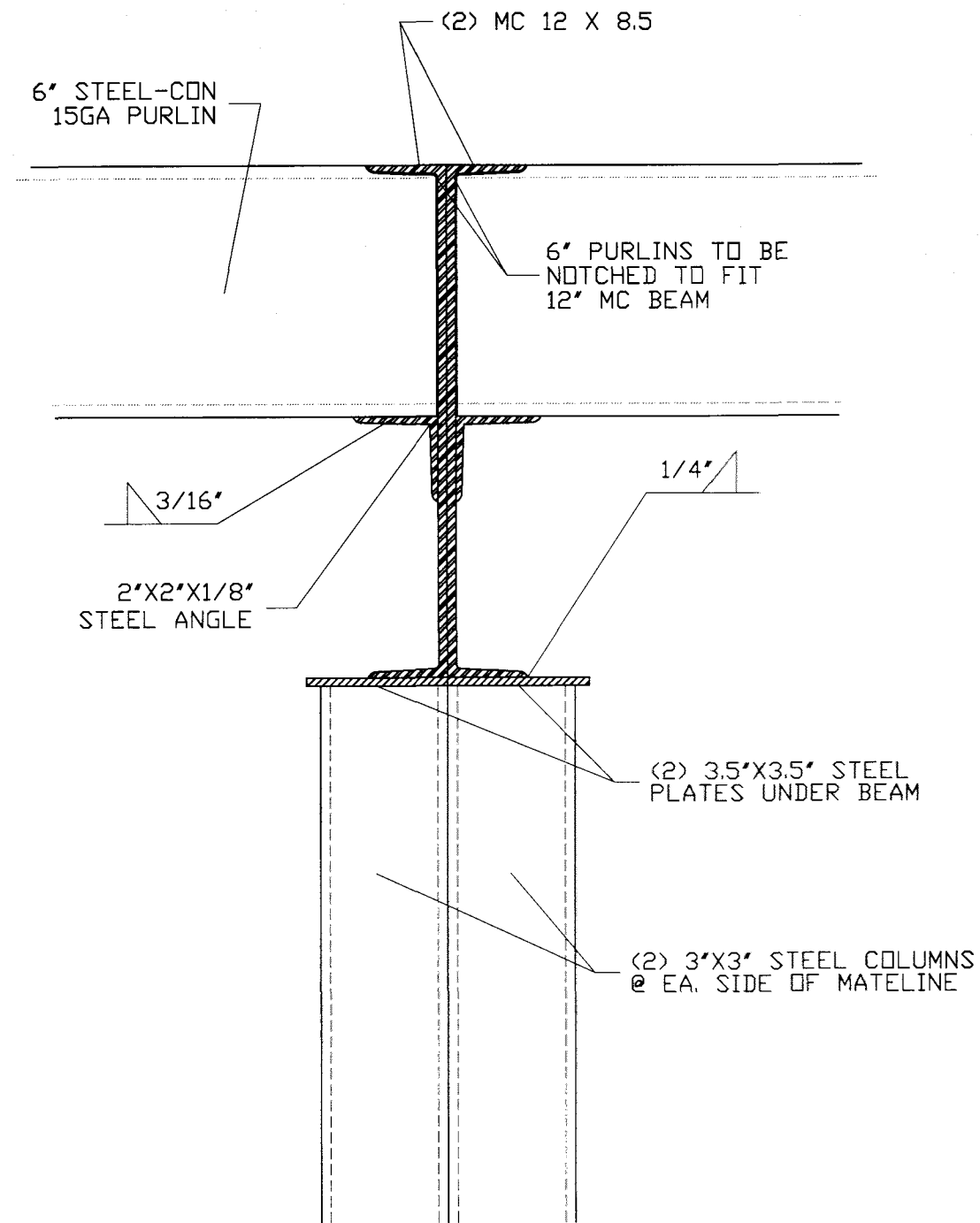
FLOOR FRAMING PLAN
SCALE: 3/8" = 1'-0"



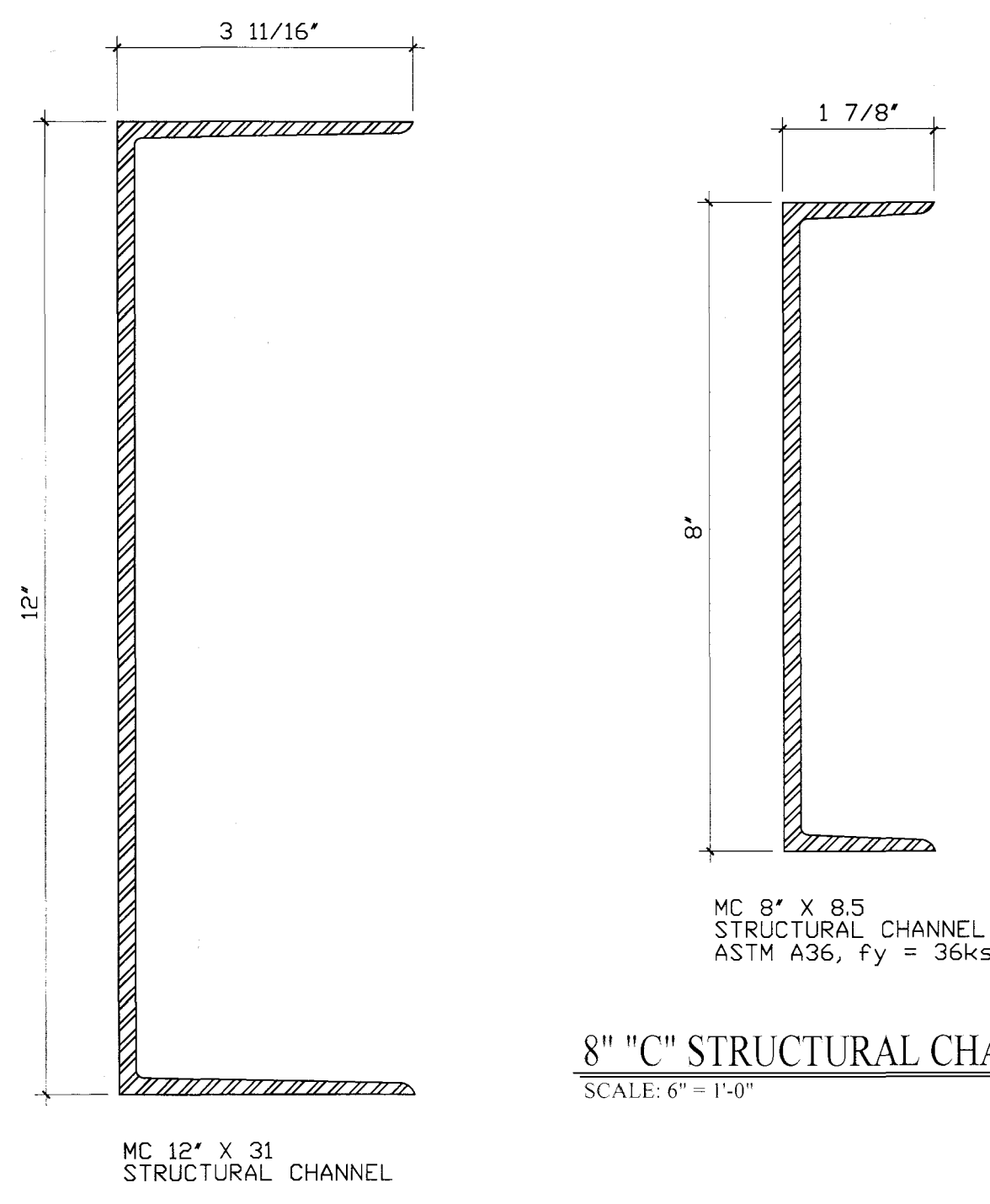
ROOF FRAMING PLAN
SCALE: 3/8" = 1'-0"



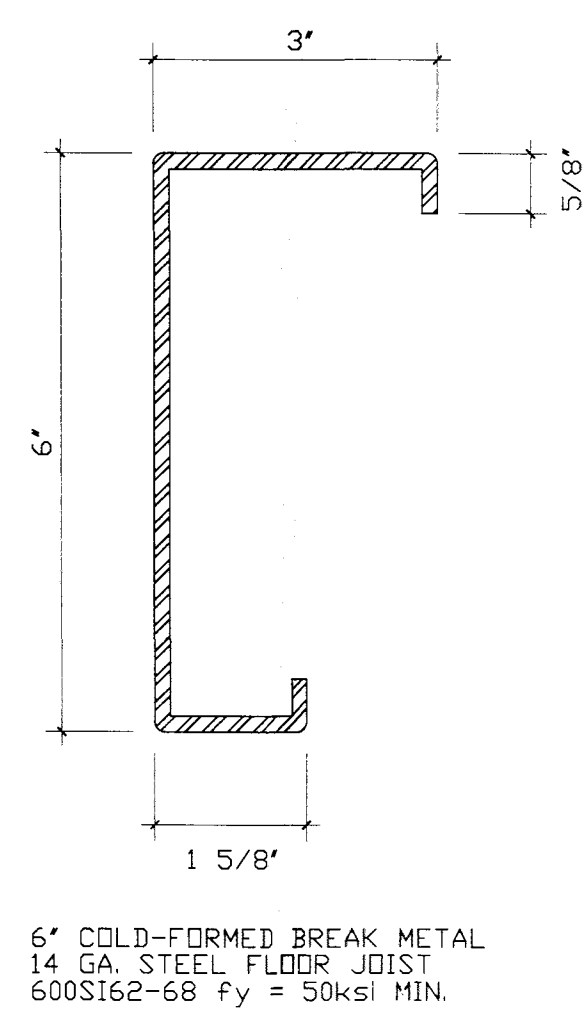
COLUMN TO BEAM CONNECTION DETAIL
SCALE: 6" = 1'-0"



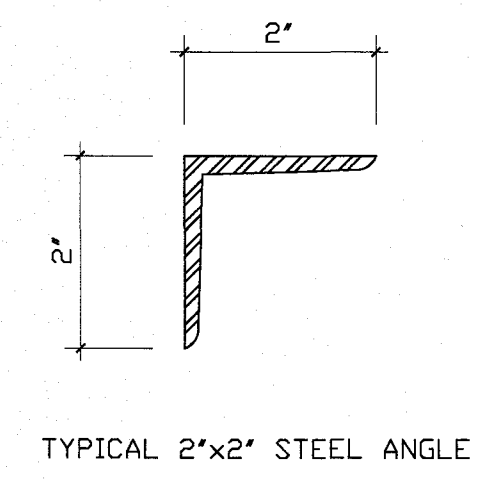
COLUMN TO BEAM CONNECTION DETAIL
SCALE: 3" = 1'-0"



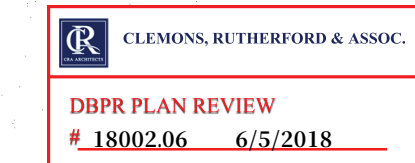
8" "C" STRUCTURAL CHANNEL
SCALE: 6" = 1'-0"



6" STEEL FLOOR JOIST
SCALE: 6" = 1'-0"



2" STEEL ANGLING
SCALE: 6" = 1'-0"

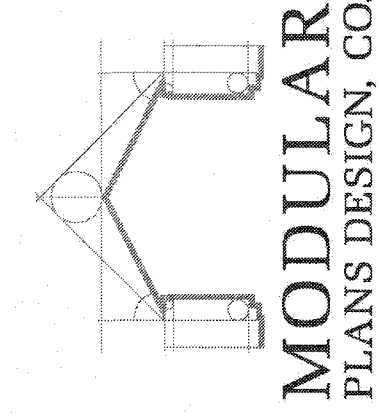


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LAKELAND, FLORIDA 33803

REVISION DATE:	DATE: 03-20-2018
	DRAWN: R.L.G.
	JOB #: MPD-1518
	SHEET NO. A3.4

MODULAR STRUCTURE FOR:
Mobile Modular
(24' X 36')

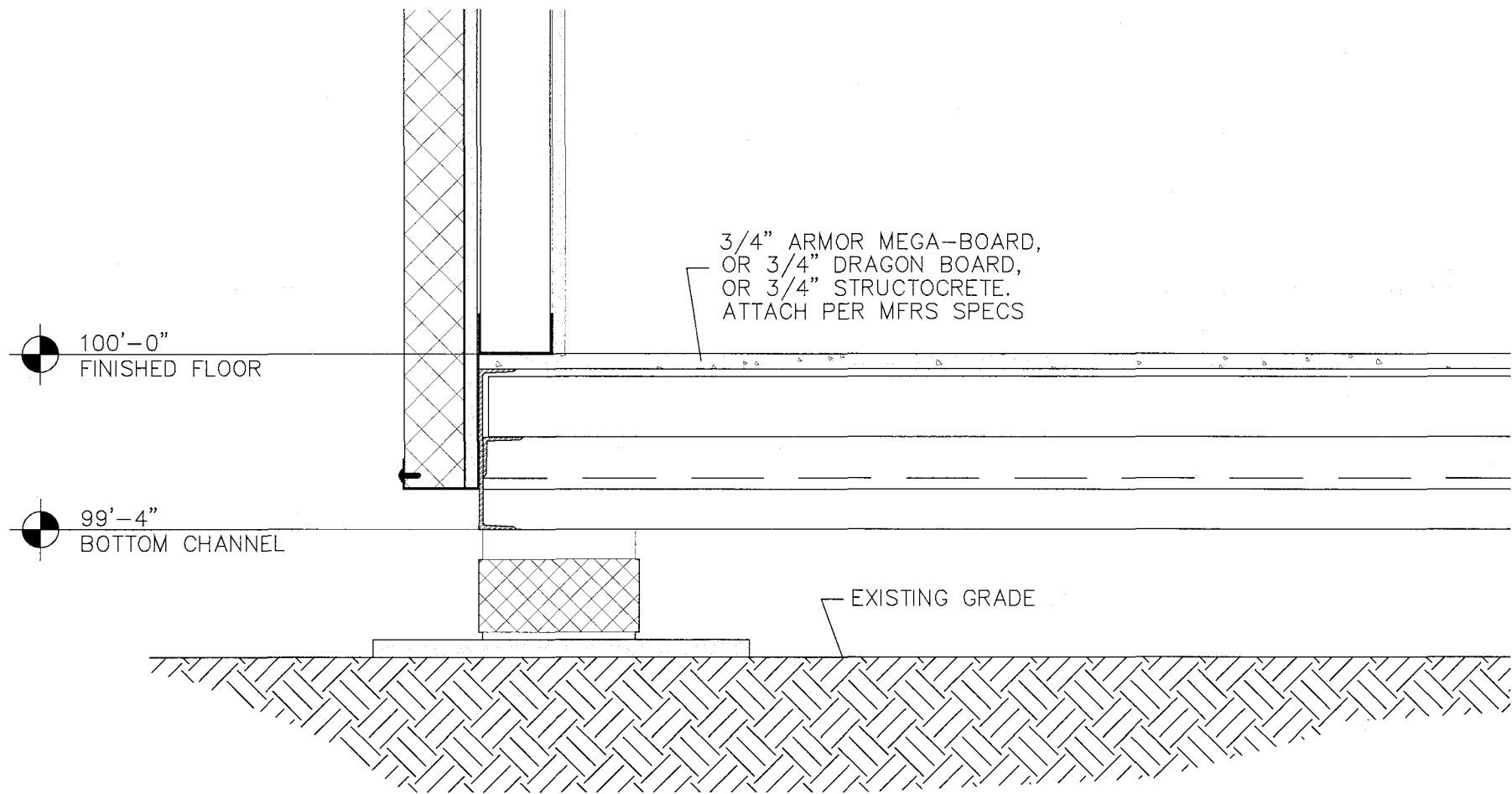
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PRODUCT APPROVAL			
PRODUCT CATEGORY	SUB. CATEGORY	MANUFACTURER	STATE OF FLORIDA APPROVAL #:
DOORS	EXTERIOR	VISION HOLLOW METAL	FL#: 19036.2
DOORS	EXTERIOR	VISION HOLLOW METAL	FL#: 19036.4
FLOORING	SUB-FLOOR	ECTEK INTERNATIONAL	ARMOR-MEGA BOARD



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DBPR PLAN REVIEW

18002.06 6/5/2018

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P.E. LICENSE #24575
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LAKELAND, FLORIDA 33803

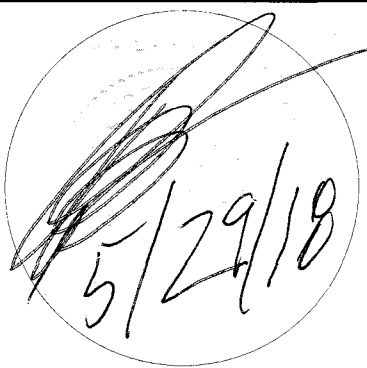
FLOORING @ SIDEWALL
SCALE: 1 1/2" = 1'-0"

MOBILE MODULAR
ADDITIONAL APPROVED PRODUCTS
CRA #: 18002.06



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DATE:	05-24-2018
DRAWN:	R.C.S.
JOB #:	MPD-1518
SHEET NO.	

SD-1

VERIFICATION FORM

DATE:	April 8, 2026	BLDG PROCESS #:	M2026011895
VF#	26-2026-W-VF - 4518	INVOICE(S)#:	N00190939 N00190939

THIS FORM EXPIRES ONE YEAR FROM THE DATE ON THE FORM

PROJECT NAME:	TEMP TRAILER FOR CLASS ROOM GULLIVER PREP 6575 SW 88 ST M2026011895
PROJECT/AGREEMENT NUMBER:	2026
PROJECT DESCRIPTION:	8050 SQFT OF 10 TEMP PORTABAL TRAILERS PER PLANS REPLACING VACANTLAND
IS THIS AN AFFORDABLE HOUSING PROJECT?	No

Folio	Address	Zip Code	Lot	Block	Proposed sq. ft.	Previous sq. ft.
2040360000750	6575 SW 88 ST	33156	M	B	8,050	0
Connection Type	Reason for Connection Information	Critical Habitat	Wetlands	Affordable Housing	SIR Inspection #	
Water, Sewer	New Construction	No	No	No		

THIS VERIFICATION LETTER CERTIFIES THAT AVAILABILITY OF A WATER AND/OR SEWER MAIN ONLY, AND IT DOES NOT GUARANTEE THE EXISTENCE OF A WATER SERVICE LINE, FIRE LINE OR OF A SEWER LATERAL WITH SUFFICIENT DEPTH TO SERVE THE PROPERTY. FOR ADDITIONAL INFORMATION EMAIL NEWBUSINESSSUPVLIST@MIAMIDADE.GOV. SHOULD IT BECOME NECESSARY TO INSTALL A SERVICE LINE AND/OR A SEWER LATERAL MDWASD REQUIRES THAT THE DEVELOPER RETAINS SERVICES FROM DESIGNERS AND CONTRACTORS WITH SKILL SETS FOR DESIGNING, BUILDING, AND CONNECTING TO PUBLIC WATER AND SEWER SYSTEMS. A WATER AND/OR SEWER AGREEMENT MAY BE REQUIRED. **AN INSPECTION FOR ANY EXISTING SERVICES WILL BE PROCESSED WITH THIS FORM, AND A SERVICE UPGRADE MAY BE REQUIRED WHICH MAY TAKE UP TO 12 WEEKS.**

THIS IS TO CERTIFY THAT THE MIAMI-DADE WATER AND SEWER DEPARTMENT **DOES HAVE** A(N) 12 INCH WATER MAIN AND/OR **DOES HAVE** A(N) 8 INCH GRAVITY SEWER MAIN ABUTTING THE SUBJECT PROPERTY.

Proposed Use	Square Footage/ # Units	Water Gallons Per Day	Sewer Gallons Per Day
School Regular (12 gpd/100 sq ft)	8050	966	966
Previous Use	Square Footage/ # Units	Water Gallons Per Day	Sewer Gallons Per Day
Vacant Land (0 gpd)	1	0	0

Water Service Area	WASD Water	Sewer Service Area	WASD Sewer
Net Water GPD	966	Net Sewer GPD	966
Net Water Cost (\$)	\$1,342.74	Net Sewer Cost (\$)	\$5,409.60
Water Basin Charge (\$)	No	Sewer Basin Charge (\$)	No
Total Connection Charges (\$)			\$6,752.34
Total Construction Connection Charges (\$) (accrues interest daily)			\$ 0.00
Construction Connection Charges Status			N/A

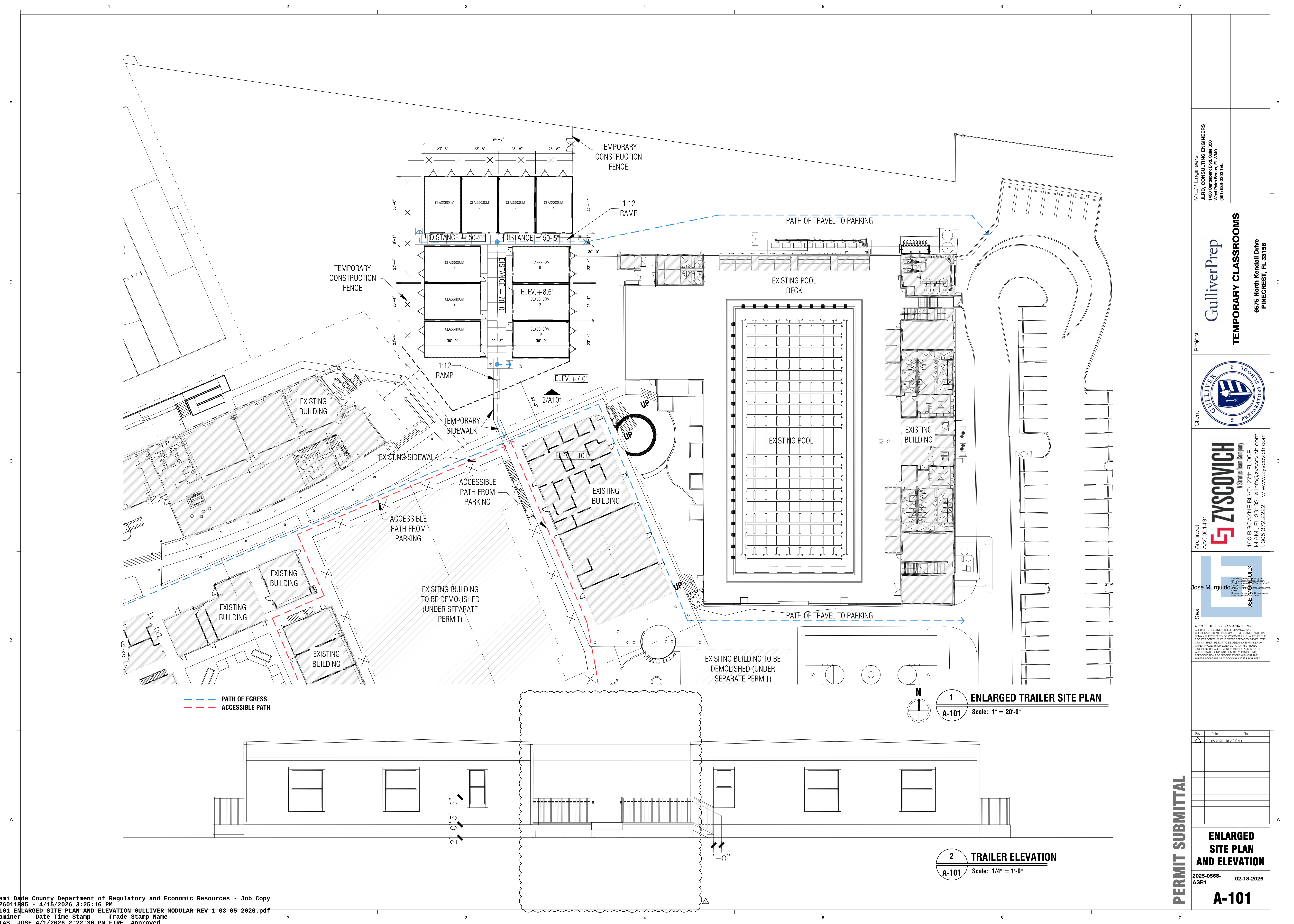
WE ARE WILLING TO SERVE THE SUBJECT PROPERTY, SUBJECT TO PROHIBITIONS, OR RESTRICTIONS OF GOVERNMENTAL AGENCIES HAVING JURISDICTION OVER MATTERS OF THE ANTICIPATED DAILY WATER AND/OR SEWAGE FLOW FOR THIS PROJECT WHICH WILL BE THE NUMBER OF GALLONS PER DAY INCREASE STATED ABOVE. IF "WILL HAVE", UPON PROPER CONVEYANCE AND PLACEMENT INTO SERVICE OF WATER AND/OR SEWER FACILITIES BY THE DEVELOPER UNDER AGREEMENT IF APPLICABLE WITH THE DEPARTMENT. **FURTHERMORE, APPROVAL OF ALL SEWAGE FLOWS INTO THE DEPARTMENT'S SYSTEM MUST BE OBTAINED FROM D.E.R.M. SUBJECT TO RER'S TERMS AND CONDITIONS SET FORTH IN THE CONSENT DECREE (CASE NO. 1:12-CV-24400-FAM) OR DOH ONSITE SEWER TREATMENT & DISPOSAL SYSTEM RULES & STATUES.**

Fixed Fee Item	Price (\$)	Total Quantity	Total Fees (\$)
Water Allocation Certificate - Initial	90	1.00	90.00
Verification Form - Non-Residential Existing Water	75	1.00	75.00
Verification Form - Non-Residential Existing Sewer	75	1.00	75.00
Total Fixed Fee			\$ 240.00
Verification Form Total (\$)			\$6,992.34
Connection Charges Paid?			Yes

COMMENTS: Refunds are based on the payment date and subject to State Statute 95-11. Some fees are not refundable.

CUSTOMER NAME: JOHN HENCH	CUSTOMER PHONE:
Prepare by: Harold Chica	Approved by: Karina Campos
Printed Name of Reviewer	Printed Name of Supervisor

Attached is the Comprehensive Planning and Water Supply Certification Letter



M/E/P Engineers
JLRD, CONSULTING ENGINEERS
1450 Centpark Blvd Suite 300
West Palm Beach, FL 33411
(561) 688-8333 FAX

Project
GulliverPrep
TEMPORARY CLASSROOMS
6575 North Kendall Drive
PINECREST, FL 33156



Client
ZYSCOVICH
A Stratus Team Company
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MIAMI, FL 33132 e info@zyscovich.com
t 305.372.2222 w www.zyscovich.com

Architect
AAC001431
JOSE MURGUIDO
Seal

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Rev	Date	Note
1	03.05.2025	REVISION 1

**ENLARGED
SITE PLAN
AND ELEVATION**
2025-0568-
ASR1
02-18-2026
A-101

FIRE EXTINGUISHERS ARE TO BE PROVIDED PER THE REQUIREMENTS OF NFPA 10 (TABLE 5.2.1) WHICH REQUIRES (1) ONE 2-A 10-BC FIRE EXTINGUISHER FOR EVERY 2500 SQUARE FEET MIN. MAX. 75' TRAVEL DISTANCE TO A F.E.

ALL INTERIOR FINISHES TO COMPLY WITH
NFPA-101:CHAPTER 28 NEW HOTEL AND CHAPTER
10/TABLE A10.2.2 (FFPC2023)

A-201



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STRUCTURAL CALCULATIONS

Project No: 26037

Ramp Calculations

Live Load = 100 psf

Dead Load = 5 psf

For

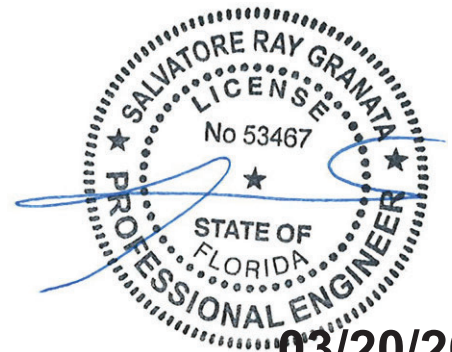
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137 Pine Forest Drive

Locust, NC 28097

(704)-888-0327

This document has been
electronically signed and
sealed by Salvatore Ray Granata
Professional Engineer of the
State of Florida, United States.
Printed Copies of this document
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The signature must be verified
on any electronic copies.



Designed by: S. Granata
Florida P.E. #: 53467

03/20/26

Salvatore
Date: 03/20/26 Ray
Granata

Digitally signed by
Salvatore Ray
Granata
Date: 2026.03.20
12:14:45 -05'00'

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JOB TITLE Ramp Calcs

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www.struware.com

Code Search**Code:** International Building Code 2021 & 2024**Occupancy:**

Occupancy Group = E Educational

Risk Category & Importance Factors:

Risk Category = III

Wind Factor = 1.00 use 0.60 NOTE: Output will be nominal wind pressures

Snow Factor = 1.00

Seismic Importance factor = 1.25

Type of Construction:

Fire Rating:

Roof = 0.0 hr

Floor = 0.0 hr

Building Geometry:Roof angle (θ) 0.00 / 12 0.0 deg

Building length 6.5 ft

Least width 6.5 ft

Mean Roof Ht (h) 2.7 ft

Parapet ht above grd 0.0 ft

Minimum parapet ht 0.0 ft

hb for Elevated bldg 0.0 ft

Live Loads:

Roof 0 to 200 sf: 20 psf
200 to 600 sf: 24 - 0.02Area, but not less than 12 psf
over 600 sf: 12 psf

Stairs and exit ways 100 psf

Floor:

Typical Floor 100 psf

Partitions N/A

Stairs and exit ways 100 psf

0 psf

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Wind Loads :

ASCE 7- 22

Ultimate Wind Speed 200 mph
 Nominal Wind Speed 154.9 mph
 Risk Category III
 Exposure Category C
 Enclosure Classif. Open Building
 Internal pressure +/-0.00
 Bldg Directionality (Kd) 0.85
 Kh MWFRS<=60 0.851
 Kh all other 0.851
 Type of roof Monoslope

Topographic Factor (Kzt)

Topography Flat
 Hill Height (H) 80.0 ft
 Half Hill Length (Lh) 100.0 ft
 Actual H/Lh = 0.80
 Use H/Lh = 0.50
 Modified Lh = 160.0 ft
 From top of crest: x = 50.0 ft
 Bldg up/down wind? downwind

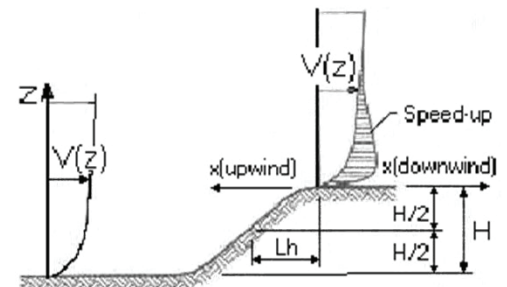
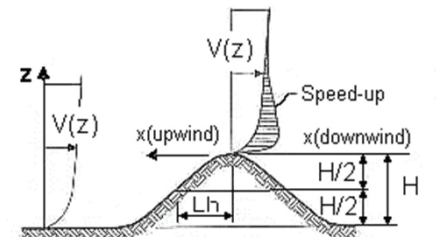
$$H/Lh = 0.50 \quad K_1 = 0.000$$

$$x/Lh = 0.31 \quad K_2 = 0.792$$

$$z/Lh = 0.09 \quad K_3 = 1.000$$

At Mean Roof Ht:

$$K_{zt} = (1 + K_1 K_2 K_3)^2 = 1.00$$

**ESCARPMENT****2D RIDGE or 3D AXISYMMETRICAL HILL****Gust Effect Factor**

h = 2.7 ft
 B = 6.5 ft
 /z (0.6h) = 15.0 ft

Flexible structure if natural frequency < 1 Hz (T > 1 second).

If building h/B > 4 then may be flexible and should be investigated.

h/B = 0.42 Rigid structure (low rise bldg)

G = 0.85 Using rigid structure default**Rigid Structure**

$\bar{e} = 0.20$
 $l = 500$ ft
 $z_{min} = 15$ ft
 $c = 0.20$
 $g_Q, g_v = 3.4$
 $L_z = 427.1$ ft
 $Q = 0.97$
 $I_z = 0.23$
 $G = 0.91$ use G = 0.85

Flexible or Dynamically Sensitive Structure

Natural Frequency (η_1) = 0.0 Hz
 Damping ratio (β) = 0.01
 $/b = 0.660$
 $/\alpha = 0.156$
 $V_z = 171.2$
 $N_1 = 0.00$
 $R_n = 0.000$
 $R_h = 28.282$ $\eta = 0.000$ h = 2.7 ft
 $R_B = 28.282$ $\eta = 0.000$
 $R_L = 28.282$ $\eta = 0.000$
 $g_R = 0.000$
 $R = 0.000$
 $G_f = 0.000$

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Wind Loads - Rooftop Structures & Canopies

Nominal Wind Pressures

Building (L) = 6.5 ft
 Building (B) = 6.5 ft
 Directionality (Kd) = 0.85

Rooftop Structures & Equipment #1

Equipment length parallel to L = 6.5 ft
 Equipment length parallel to B = 4.5 ft
 Height of equipment = 6.9 ft

Base pressure (Kd qh) = **44.5 psf**

<u>Vertical wind pressure</u>		<u>Wind normal to building B</u>		<u>Wind normal to building L</u>	
Ar =	29.3 sf	Af =	30.9 sf	Af =	44.7 sf
GCr =	1.171	GCr =	1.00	GCr =	1.00
F = KdqhGCr Ar =	52.0 Ar (psf)	F = KdqhGCr Af =	44.5 Af (psf)	F = KdqhGCr Af =	44.5 Af (psf)
Fv =	1.5 kips	Fh =	1.4 kips	Fh =	2.0 kips

Horizontal Wind Load : $V_w = 52.0 \text{ psf} \times 3 \text{ ft}^2 = 156 \text{ lb (Nom.)} / 260 \text{ lbs (Ult.)}$

Rooftop Structures & Equipment #2

Equipment length parallel to L = 3.0 ft
 Equipment length parallel to B = 3.0 ft
 Height of equipment = 10.0 ft

Base pressure (Kd qh) = **44.5 psf**

<u>Vertical wind pressure</u>		<u>Wind normal to building B</u>		<u>Wind normal to building L</u>	
Ar =	9.0 sf	Af =	30.0 sf	Af =	30.0 sf
GCr =	1.437	GCr =	1.00	GCr =	1.00
F = KdqhGCr Ar =	63.9 Ar (psf)	F = KdqhGCr Af =	44.5 Af (psf)	F = KdqhGCr Af =	44.5 Af (psf)
Fv =	0.6 kips	Fh =	1.3 kips	Fh =	1.3 kips

Attached Canopies on Buildings : $h \leq 60'$

Mean Roof Ht (h) = 2.7 ft
 Mean eave height (he) = 60.0 ft
 Mean Canopy height (hc) = 45.0 ft
 hc/he = 0.75

Base pressure (Kd qh) = **44.5 psf**

Area	C&C Surface Pressure (psf)				User input
	10 sf	20 sf	50 sf	100 sf	75 sf
Separate Individual Surfaces:					
Upper surface negative pressure	-51.1	-45.8	-38.7	-33.3	-35.6
Lower surface negative pressure	-35.6	-33.6	-30.9	-28.9	-29.7
Upper or Lower surface pos pressure	35.6	32.9	29.3	26.7	27.8
Combined Upper & Lower Surfaces (net):					
Negative pressure	-40.0	-36.7	-32.2	-28.9	-30.3
Positive pressure	40.0	36.7	32.2	28.9	30.3

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Snow Loads : ASCE 7- 22

Ultimate Snow Forces

Roof slope = 0.0 deg
 Horiz. eave to ridge dist (W) = 6.5 ft
 Roof length parallel to ridge (L) = 6.5 ft

Type of Roof Monoslope
 Ground Snow Load Pg = 20.0 psf
 Risk Category = III
 Snow Factor = 1.0
 Roof R value Rroof = 30
 Thermal Factor Ct = 1.190
 Exposure Factor Ce = 1.10
 $P_f = 0.7 \cdot C_e \cdot C_t \cdot I \cdot P_g = 18.3 \text{ psf}$
 Unobstructed Slippery Surface yes

Sloped-roof Factor Cs = 1.00
 Balanced Snow Load = **18.3 psf**

Rain on Snow Surcharge Angle 0.13 deg
 Code Maximum Rain Surcharge 8.0 psf
 Rain on Snow Surcharge = 8.0 psf
 Ps plus rain surcharge = 26.3 psf
 Minimum Snow Load Pm = 20.0 psf

Uniform Roof Design Snow Load = **26.3 psf**

Near ground level surface balanced snow load = **20.0 psf**

NOTE: Alternate spans of continuous beams shall be loaded with half the design roof snow load so as to produce the greatest possible effect - see code for loading diagrams and exceptions for gable roofs

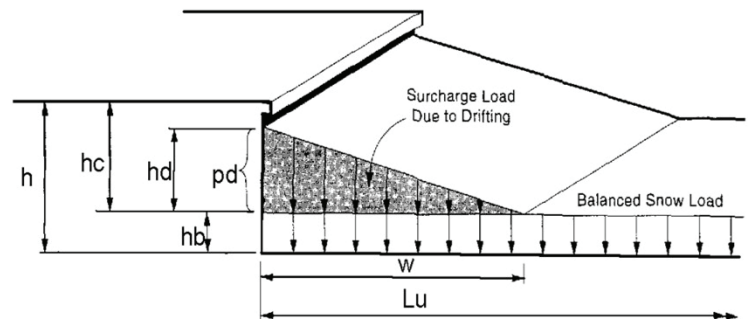
Winter Wind Parameter W2 = 0.55

Snow Drift 1 - Against roof projections, parapets, etc

Up or downwind fetch lu = 12.0 ft
 Projection height h = 8.5 ft
 Projection width/length lp = 4.5 ft
 Snow density $\gamma = 16.6 \text{ pcf}$
 Balanced snow height hb = 1.10 ft
 $h_d = 1.20 \text{ ft}$
 $h_c = 7.40 \text{ ft}$
 $h_c/h_b > 0.2 = 6.7$ **lp < 15', drift not req'd**
 Drift height (hd) = 1.20 ft
 Drift width w = 9.61 ft
 Surcharge load: $pd = \gamma \cdot h_d = 19.9 \text{ psf}$
 Balanced Snow load: = 18.3 psf
 38.3 psf

Snow Drift 2- Against roof projections, parapets, etc

Up or downwind fetch lu = 160.0 ft
 Projection height h = 4.0 ft
 Projection width/length lp = 20.0 ft
 Snow density $\gamma = 16.6 \text{ pcf}$
 Balanced snow height hb = 1.10 ft
 $h_d = 2.97 \text{ ft}$
 $h_c = 2.90 \text{ ft}$
 $h_c/h_b > 0.2 = 2.6$ **Therefore, design for drift**
 Drift height (hc) = 2.90 ft
 Drift width w = 23.17 ft
 Surcharge load: $pd = \gamma \cdot h_d = 48.1 \text{ psf}$
 Balanced Snow load: = 18.3 psf
 66.4 psf



Note: If bottom of projection is at least 2 feet above hb then snow drift is not required.

Seismic Loads:

IBC 2021 & 2024

Strength Level Forces

Risk Category : III
 Importance Factor (Ie) : 1.25
 Site Class : D

Ss (0.2 sec) = 0.37 g
 S1 (1.0 sec) = 0.15 g

Sms = 3.690
 Sm1 = 2.550

S_{DS} = 2.460
 S_{D1} = 1.700

Site specific ground motion analysis performed:
 Design Category = D
 Design Category = D

Seismic Design Category = D
 Redundancy Coefficient ρ = 1.30
 Number of Stories: 1

Structure Type: All other building systems
 Horizontal Struct Irregularities: No plan Irregularity
 Vertical Structural Irregularities: No vertical Irregularity

Flexible Diaphragms: No
 Building System: Cantilevered Column Systems detailed to conform to the requirements for:
 Seismic resisting system: Steel ordinary cantilever column system
 System Structural Height Limit: System not permitted for this seismic design category
 Actual Structural Height (hn) = 2.7 ft
 See ASCE7 Section 12.2.5 for exceptions and other system limitations

DESIGN COEFFICIENTS AND FACTORS

Response Modification Coefficient (R) = 1.25
 Over-Strength Factor (Ω_o) = 1.25
 Deflection Amplification Factor (Cd) = 1.25

$T_o = 0.2(S_{d1}/S_{ds}) = 0.138$
 $T_s = S_{d1}/S_{ds} = 0.691$
 Long Period Transition Period (TL) = 6 sec

S_{DS} = 2.460
 S_{D1} = 1.700

Seismic Load Effect (E) = $E_h \pm E_v = \rho Q_E \pm 0.2S_{DS}D = 1.3Q_E \pm 0.492D$ Q_E = horizontal seismic force
 Special Seismic Load Effect (Em) = $E_mh \pm E_v = \Omega_o Q_E \pm 0.2S_{DS}D = 1.25Q_E \pm 0.492D$ D = dead load

ALLOWABLE STORY DRIFT

Structure Type: All other structures

Allowable story drift $\Delta_a = 0.015h_{sx}$ where h_{sx} is the story height below level x

PERMITTED ANALYTICAL PROCEDURES

Index Force Analysis - Method Not Permitted (only applies to Seismic Category A)

Model & Seismic Response Analysis - Permitted (see code for procedure)

Equivalent Lateral-Force (ELF) Analysis - Permitted

Building period coer. (C_T) = 0.020 $C_u = 1.40$
 Approx fundamental period (T_a) = $C_T h_n^x = 0.042$ sec $x = 0.75$ $T_{max} = C_u T_a = 0.059$ sec
 User calculated fundamental period = T = 0.042 sec

Method 2: Seismic response coef. (Cs) = $S_{ds}/R = 2.460$
 need not exceed Cs = $S_{d1}/RT = 40.355$
 but not less than Cs = $0.044S_{ds}I = 0.135$
 USE Cs = 2.460

Design Base Shear $V = 2.460W$

Method 1: Enter S_a =
 Seismic response coef. (Cs) = $S_a/R = 0.000$
 but not less than Cs = $0.044S_{ds}I = 0.135$
 Cs = Method not applicable

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Seismic Loads - cont. :

Strength Level Forces

Seismic Design Category (SDC)= D

Ie = 1.25

Sds = 2.460

CONNECTIONS**Force to connect smaller portions of structure to remainder of structure**

$$F_p = 0.133 S_d s w_p = 0.327 w_p$$

$$\text{or } F_p = 0.05 w_p = 0.05 w_p \quad \text{Use } F_p = 0.33 w_p \quad w_p = \text{weight of smaller portion}$$

Beam, girder or truss connection for resisting horizontal force parallel to member

F_p = no less than 0.05 times dead plus live load vertical reaction

Anchorage of Structural Walls to elements providing lateral support

F_p = not less than $0.2 K_a I_e W_p$

Flexible diaphragm span L_f =Enter L_f to calculate F_p for flexible diaphragm

$$F_p = 0.4 S_d s K_a I_e W_p = 1.23 W_p, \text{ but not less than } 0.25 W_p \text{ (rigid diaphragm)} \quad K_a = 1 \quad F_p = 1.230 W_p$$

w/ anchor adjustment factor

but F_p shall not be less than 5 psf

$h = 2.7$	Flexible Diaphragm:	$F_p = W_p$	
$z = 2.7$	Rigid Diaphragm:	$F_p = 1.230 W_p$	factor = 1.000

MEMBER DESIGN**Bearing Walls and Shear Walls (out of plane force)**

$$F_p = 0.4 S_d s I_e W_w = 1.230 w_w$$

$$\text{but not less than } 0.10 w_w \quad \text{Use } F_p = 1.230 w_w$$

Diaphragms

$$F_p = (\text{Sum } F_i / \text{Sum } W_i) W_{px} + V_{px} = (\text{Sum } F_i / \text{Sum } W_i) W_{px} + V_{px}$$

$$\text{need not exceed } 0.4 S_d s I_e W_{px} + V_{px} = 1.230 W_{px} + V_{px}$$

$$\text{but not less than } 0.2 S_d s I_e W_{px} + V_{px} = 0.615 W_{px} + V_{px}$$

ARCHITECTURAL COMPONENTS SEISMIC COEFFICIENTS

Architectural Component : Egress stairways not part of the building seismic force-resisting system

0

Importance Factor (I_p) : 1.0Comp Resonance ductility factor (C_{ar}) = 1 $h = 2.7$ feetComp Response Modification Factor (R_{po}) = 1.5 $z = 2.7$ feetOver-Strength Factor (Ω_{po}) = 2 $z/h = 1.00$ $H_f = 3.50$

$$R_u = (1.1 R / I_e / \Omega_o)^{.5} = 0.94$$

$$F_p = 0.4 S_d s I_p W_p H_f C_{ar} / R_u / R_{po} = 1.766 W_p$$

use $R_u = 1.3$ since calculated $R_u < 1.3$

$$\text{not greater than } F_p = 1.6 S_d s I_p W_p = 3.936 W_p$$

$$\text{but not less than } F_p = 0.3 S_d s I_p W_p = 0.738 W_p$$

$$\text{use } F_p = 1.766 W_p$$

MECH AND ELEC COMPONENTS SEISMIC COEFFICIENTSSeismic Design Category D & $I_p = 1.0$, therefore

see ASCE7 Section 13.1.4 for exceptions

Mech or Electrical Component : Wet-side HVAC, boilers, furnaces, atmospheric tanks and bins, chillers, water heaters, etc
 plus other mechanical components constructed of high-deformability materials.

Importance Factor (I_p) : 1.0Comp Resonance ductility factor (C_{ar}) = 1 $h = 2.7$ feetComp Response Modification Factor (R_{po}) = 1.5 $z = 2.7$ feetOver-Strength Factor (Ω_{po}) = 2 $z/h = 1.00$ $H_f = 3.50$

$$R_u = (1.1 R / I_e / \Omega_o)^{.5} = 0.94$$

$$F_p = 0.4 S_d s I_p W_p H_f C_{ar} / R_u / R_{po} = 1.766 W_p$$

use $R_u = 1.3$ since calculated $R_u < 1.3$

$$\text{not greater than } F_p = 1.6 S_d s I_p W_p = 3.936 W_p$$

$$\text{but not less than } F_p = 0.3 S_d s I_p W_p = 0.738 W_p$$

$$\text{use } F_p = 1.766 W_p$$

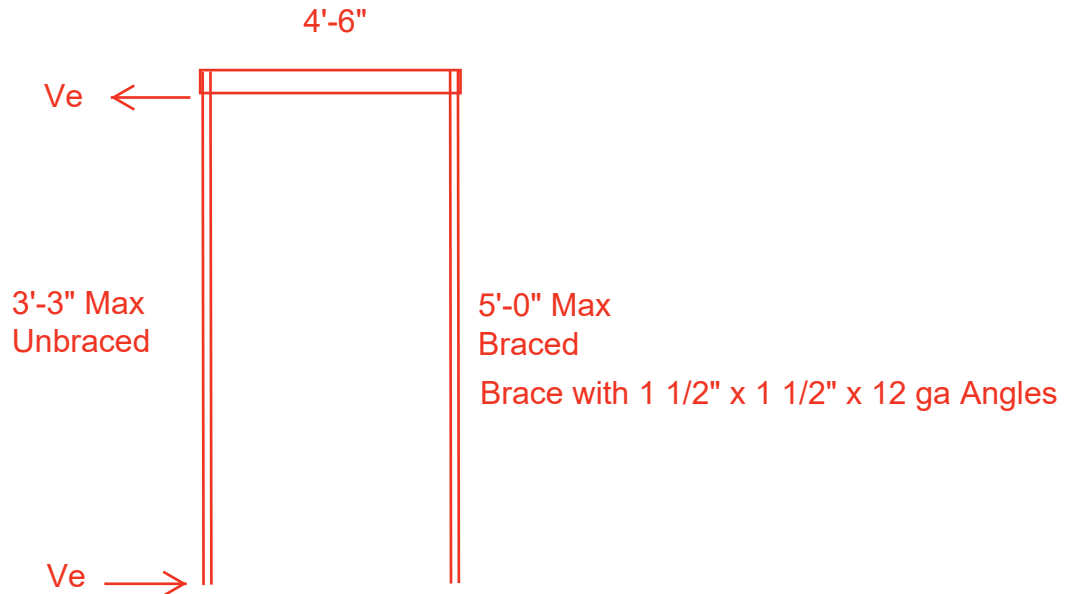


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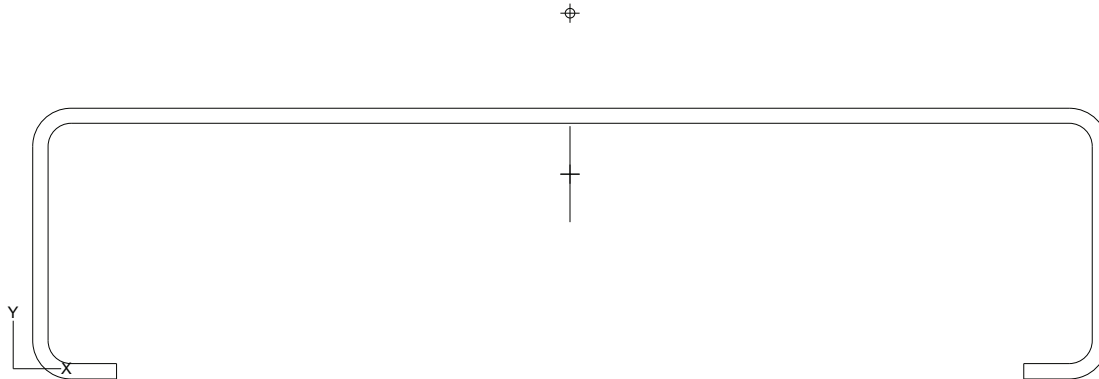
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$$\begin{aligned} D &= 6.5' \times 12' \times 5 \text{ psf} / 8 \text{ legs} = 48.75 \text{ lbs} \\ V_e &= 2 \text{ legs} \times 48.75 \text{ lbs} \times 0.7 = 68.25 \text{ lbs (Nom.) per (2) legs} \\ M_e &= 68.25 \text{ lbs} \times 3.25' \times 12' / 1000 = 2.66175 \text{ k-in} \\ P &= 3' \times 4.5' \times 5 \text{ psf} = 67.5 \text{ lbs (Nom.) Dead Load} \\ P &= 3' \times 4.5' \times 100 \text{ psf} = 1350 \text{ lbs (Nom.) Live Load} \end{aligned}$$

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Printed: 2/13/2026 11:02:18 AM

Item 3: Walking Surface
Trib Width = 4"



Section Inputs

Material: A653 SS Grade 33
No cold work of forming strength increase.
No inelastic reserve strength increase.
Modulus of Elasticity, E 29500 ksi
Yield Strength, Fy 33 ksi
Tensile Strength, Fu 45 ksi
Min Elongation in 2 inches 20 %
Torsion Constant Override, J 0 in⁴
Warping Constant Override, Cw 0 in⁶

Stiffened Channel, Thickness 0.0566 in (54 mil)

Placement of Part from Origin:

X to center of gravity 0 in

Y to center of gravity 0 in

Outside dimensions, Open shape

	Length (in)	Angle (deg)	Radius (in)	Web	k Coef.	Hole Size (in)	Distance (in)
1	0.3100	180.000	0.084900	None	0.000	0.0000	0.1550
2	1.0000	90.000	0.084900	Single	0.000	0.0000	0.5000
3	3.9700	0.000	0.084900	Cee	0.000	0.0000	1.9850
4	1.0000	-90.000	0.084900	Single	0.000	0.0000	0.5000
5	0.3100	-180.000	0.084900	None	0.000	0.0000	0.1550

Full Section Properties

Area	0.34918 in ²	Wt.	0.0011872 k/ft	Flat	6.1693 in
Ix	0.03888 in ⁴	rx	0.3337 in	Ixy	0.00000 in ⁴
Sx(t)	0.15942 in ³	y(t)	0.2439 in	α	-90.000 deg
Sx(b)	0.05141 in ³	y(b)	0.7561 in		
Zx	0.07709 in ³	Height	1.0000 in		
Iy	0.75426 in ⁴	ry	1.4697 in	x _o	0.0000 in
Sy(l)	0.37998 in ³	x(l)	1.9850 in	y _o	0.5945 in
Sy(r)	0.37998 in ³	x(r)	1.9850 in	jx	0.0000 in
Zy	0.46190 in ³	Width	3.9700 in	jy	-2.2063 in

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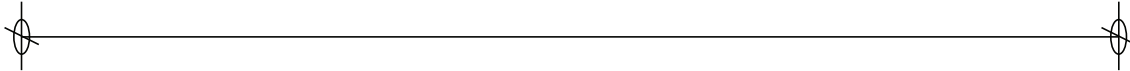
I_1	0.75426 in ⁴	r_1	1.4697 in	Cw	0.11666 in ⁶
I_2	0.03888 in ⁴	r_2	0.3337 in	J	0.00037287 in ⁴
I_c	0.79314 in ⁴	r_c	1.5071 in		
I_o	0.91655 in ⁴	r_o	1.6201 in		

Fully Braced Strength - AISI S100-24, US, ASD

Material Type: A653 SS Grade 33, Fy=33 ksi

Axial		Positive Bending		Positive Bending	
Pao	5.2736 k	Maxo	1.0160 k-in	Mayo	7.5086 k-in
Ae	0.28765 in ²	Ixe	0.03888 in ⁴	Iye	0.75426 in ⁴
Ta	6.9000 k	Sxe(t)	0.15942 in ³	Sye(l)	0.37998 in ³
		Sxe(b)	0.05141 in ³	Sye(r)	0.37998 in ³
Shear		Negative Bending		Negative Bending	
Vay	0.9623 k	Maxo	1.0160 k-in	Mayo	7.5086 k-in
Vax	2.2217 k	Ixe	0.03888 in ⁴	Iye	0.75426 in ⁴
Torsion		Sxe(t)	0.15942 in ³	Sye(l)	0.37998 in ³
Ba	1.5725 k-in ²	Sxe(b)	0.05141 in ³	Sye(r)	0.37998 in ³

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 By: Salvatore Granata
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Analysis Inputs

General

Member Orientation: Horizontal
 Calculate global buckling using specification equations
 Do not include torsion in member checks

Members

Section File				Revision Date and Time			
1 8 - Walking Surface.cfss				2/21/2023 8:27:09 AM			
Material	Area (in ²)	Length (ft)	Weight (k)				
1 A653 SS Grade 33	0.34918	2.0000	0.0023744				
Total		2.0000	0.0023744				
Start Loc. (ft)	End Loc. (ft)	Braced Flange	R	kφ (k)	Lm (ft)	ex (in)	ey (in)
1 0.0000	2.0000	None	0.0000	0.0000	6.0000	0.000	0.000

Supports

Type	Location (ft)	Bearing (in)	Fastened	K
1 XYT	0.0000	2.00	No	1.0000
2 XYT	2.0000	2.00	No	1.0000

Loading: Dead Load

Type	Angle (deg)	Start Loc. (ft)	End Loc. (ft)	Start Magnitude	End Magnitude	
1 Distributed	90.000	0.0000	2.0000	-0.001665	-0.001665	k/ft

Loading: Live Load

Type	Angle (deg)	Start Loc. (ft)	End Loc. (ft)	Start Magnitude	End Magnitude	
1 Distributed	90.000	0.0000	2.0000	-0.033300	-0.033300	k/ft

Analysis: Walking Surface Analysis.cfsa
2 ft Span Simple Beam

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Load Combination: D

Specification: AISI S100-24, US, ASD

Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000

Load Combination: D+L

Specification: AISI S100-24, US, ASD

Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Live Load	1.000
4 Product Load	1.000

Load Combination: D+0.75(L+Lr)

Specification: AISI S100-24, US, ASD

Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Live Load	0.750
4 Product Load	0.750
5 Roof Live Load	0.750

Load Combination: 0.6D+0.6W

Specification: AISI S100-24, US, ASD

Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	0.600
2 Dead Load	0.600
3 Wind Load	0.600

Load Combination: D+0.7E

Specification: AISI S100-24, US, ASD

Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Earthquake Load	0.700

Load Combination: D+0.525E+0.75L+0.1S

Specification: AISI S100-24, US, ASD

Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Earthquake Load	0.525
4 Live Load	0.750
5 Product Load	0.750
6 Snow Load	0.100

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Load Combination: 0.6D+0.7E
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	0.600
2 Dead Load	0.600
3 Earthquake Load	0.700

Member Check - AISI S100-24, US, ASD

Load Combination: D+L

Design Parameters at 1.0000 ft:

Lx	2.0000 ft	Ly	2.0000 ft	Lt	2.0000 ft
Kx	1.0000	Ky	1.0000	Kt	1.0000

Section: 8 - Walking Surface.cfss

Material Type: A653 SS Grade 33, Fy=33 ksi

Cbx	1.1364	Cby	1.0000	ex	0.0000 in
Cmx	1.0000	Cmy	1.0000	ey	0.0000 in

Braced Flange: None	kφ	0 k
Red. Factor, R: 0	Lm	6.0000 ft

Loads:	P (k)	Mx (k-in)	Vy (k)	My (k-in)	Vx (k)
Total	0.0000	0.2169	0.0000	0.0000	0.0000
Applied	0.0000	0.2169	0.0000	0.0000	0.0000
Strength	4.3274	1.0160	0.9623	7.5086	2.2217

Interaction Equations

Eq. H1.2-1 (P, Mx, My) $0.000 + 0.214 + 0.000 = 0.214 \leq 1.0$
Eq. H2-1 (Mx, Vy) $\text{Sqrt}(0.046 + 0.000) = 0.214 \leq 1.0$
Eq. H2-1 (My, Vx) $\text{Sqrt}(0.000 + 0.000) = 0.000 \leq 1.0$

Calculation of Mndy used as=1 from DSM inputs.

Web Crippling Check - AISI S100-24, US, ASD

Load Combination: D+L

Parameters at 0.0000 ft:

Total Load: 0.036152 k on bottom flange

Total Moment: 0.0000 k-in

Bearing: 2.0000 in

Flange fastened to bearing surface: No

Distance from edge of bearing to edge of opposite load: 2.0000 ft

Section: 8 - Walking Surface.cfss

Material Type: A653 SS Grade 33, Fy=33 ksi

Applied Load: 0.03615 k on bottom flange

Applied Moment: 0.0000 k-in

Distance from edge of bearing to end of member: 0.0000 ft

Analysis: Walking Surface Analysis.cfsa
2 ft Span Simple Beam

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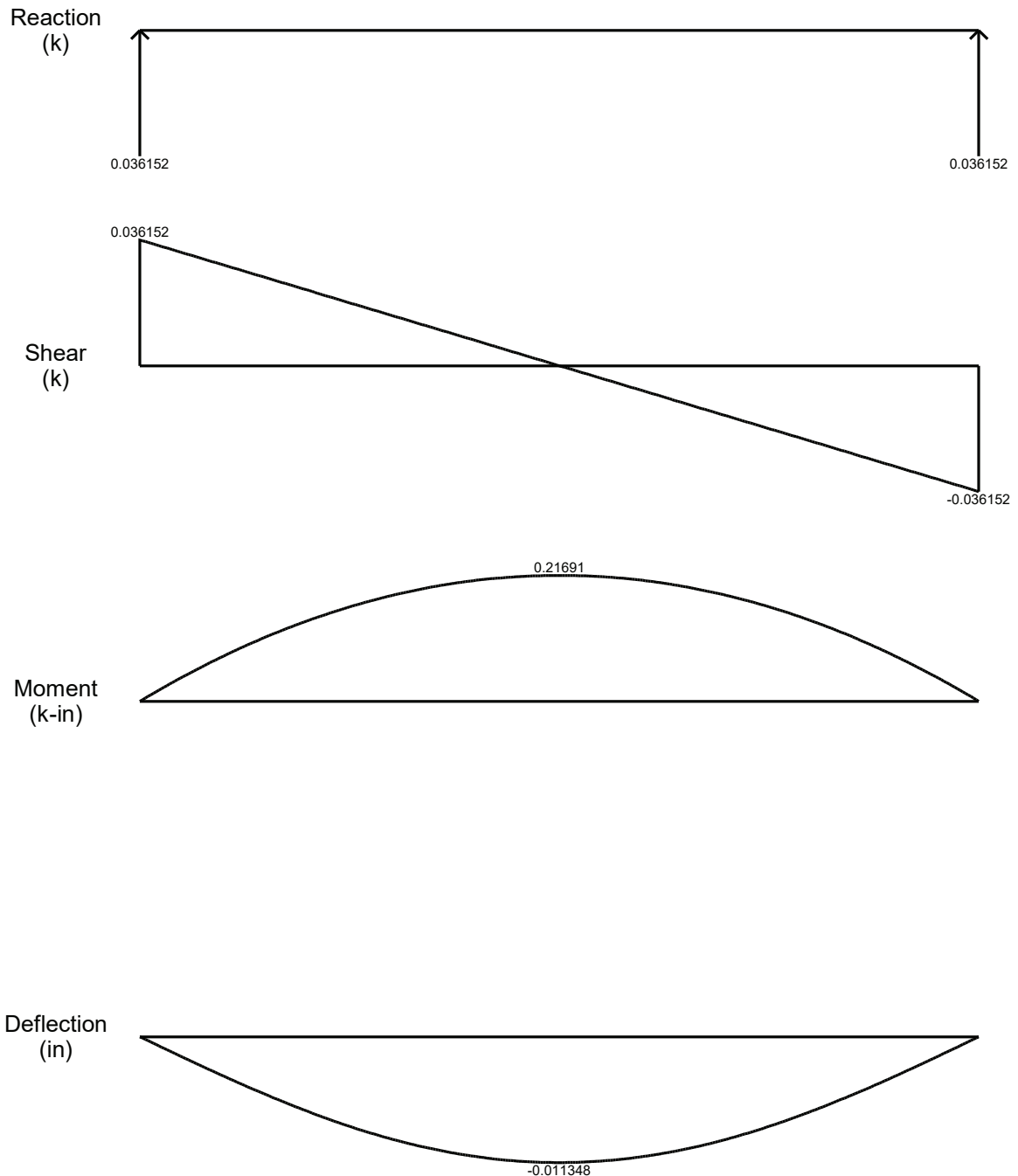
Part	Elem	Calculation Type	Pa (k)	Pay (k)	Notes
1	2	Single, UU-EOF	0.43994	0.43994	N/h>2
1	4	Single, UU-EOF	0.43994	0.43994	N/h>2
		Web Crippling Strength		0.87987	

Web Crippling Check: 0.03615 k <= 0.87987 k
 Moment Check: 0.0000 k-in <= 1.0160 k-in
 Interaction Equations
 Eq. H3-1a (P, M) 0.019 + 0.000 = 0.019 <= 0.665

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Item 3: Walking Surface

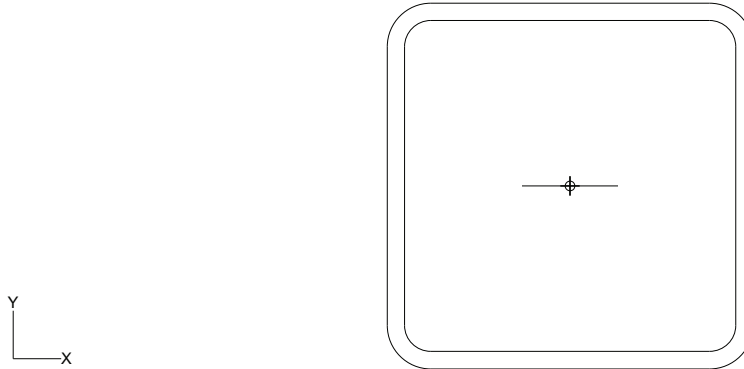
Load Combination: D+L, Y Direction



Allowable Deflection: $L / 240 = 2' * 12" / 240 = 0.10"$ OK

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Item 4: Stair Stinger



Section Inputs

Material: A653 SS Grade 33
No cold work of forming strength increase.
No inelastic reserve strength increase.
Modulus of Elasticity, E 29500 ksi
Yield Strength, Fy 33 ksi
Tensile Strength, Fu 45 ksi
Min Elongation in 2 inches 20 %
Torsion Constant Override, J 0 in⁴
Warping Constant Override, Cw 0 in⁶

Tube, Thickness 0.0713 in (68 mil)

Placement of Part from Origin:

X to center of gravity 0 in

Y to center of gravity 0 in

Outside dimensions, Closed shape

	Length (in)	Angle (deg)	Radius (in)	Web	k Coef.	Hole Size (in)	Distance (in)
1	1.5000	0.000	0.10690	Single	0.000	0.0000	0.7500
2	1.5000	90.000	0.10690	Single	0.000	0.0000	0.7500
3	1.5000	180.000	0.10690	Single	0.000	0.0000	0.7500
4	1.5000	-90.000	0.10690	Single	0.000	0.0000	0.7500

Full Section Properties

Area	0.39002 in ²	Wt.	0.0013261 k/ft	Flat	5.4701 in
Ix	0.12939 in ⁴	rx	0.57599 in	Ixy	0.00000 in ⁴
Sx(t)	0.17253 in ³	y(t)	0.75000 in	α	0.000 deg
Sx(b)	0.17253 in ³	y(b)	0.75000 in		
Zx	0.20555 in ³	Height	1.50000 in		
Iy	0.12939 in ⁴	ry	0.57599 in	x _o	0.00000 in
Sy(l)	0.17253 in ³	x(l)	0.75000 in	y _o	0.00000 in
Sy(r)	0.17253 in ³	x(r)	0.75000 in	jx	0.00000 in
Zy	0.20555 in ³	Width	1.50000 in	jy	0.00000 in

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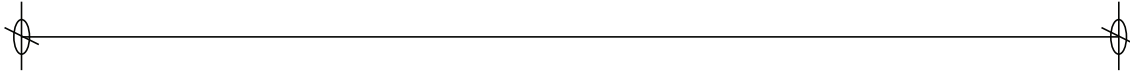
I_1	0.12939 in ⁴	r_1	0.57599 in	Cw	0.00003 in ⁶
I_2	0.12939 in ⁴	r_2	0.57599 in	J	0.21353 in ⁴
I_c	0.25879 in ⁴	r_c	0.81458 in		
I_o	0.25879 in ⁴	r_o	0.81458 in		

Fully Braced Strength - AISI S100-24, US, ASD

Material Type: A653 SS Grade 33, Fy=33 ksi

Axial		Positive Bending		Positive Bending	
Pao	7.1503 k	Maxo	3.4092 k-in	Mayo	3.4092 k-in
Ae	0.39002 in ²	Ixe	0.12939 in ⁴	Iye	0.12939 in ⁴
Ta	7.7069 k	Sxe(t)	0.17253 in ³	Sye(l)	0.17253 in ³
		Sxe(b)	0.17253 in ³	Sye(r)	0.17253 in ³
Shear		Negative Bending		Negative Bending	
Vay	1.9335 k	Maxo	3.4092 k-in	Mayo	3.4092 k-in
Vax	1.9335 k	Ixe	0.12939 in ⁴	Iye	0.12939 in ⁴
Torsion		Sxe(t)	0.17253 in ³	Sye(l)	0.17253 in ³
Ba	0.041099 k-in ²	Sxe(b)	0.17253 in ³	Sye(r)	0.17253 in ³

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Analysis Inputs

General

Member Orientation: Horizontal
 Calculate global buckling using specification equations
 Do not include torsion in member checks

Members

Section File				Revision Date and Time			
1 9 - Stair Stringers.cfss				2/17/2023 6:16:26 PM			
Material	Area (in ²)	Length (ft)	Weight (k)				
1 A653 SS Grade 33	0.39002	4.1000	0.0054368				
Total		4.1000	0.0054368				
Start Loc. (ft)	End Loc. (ft)	Braced Flange	R	kφ (k)	Lm (ft)	ex (in)	ey (in)
1 0.0000	4.1000	None	0.0000	0.0000	4.1000	0.000	0.000

Supports

Type	Location (ft)	Bearing (in)	Fastened	K
1 Y	0.0000	2.00	No	1.0000
2 XT	0.0000	2.00	No	1.0000
3 Y	4.1000	2.00	No	1.0000
4 XT	4.1000	2.00	No	1.0000

Loading: Dead Load

Type	Angle (deg)	Start Loc. (ft)	End Loc. (ft)	Start Magnitude	End Magnitude	
1 Distributed	90.000	0.0000	4.1000	-0.00563	-0.00563	k/ft

Loading: Live Load

Type	Angle (deg)	Start Loc. (ft)	End Loc. (ft)	Start Magnitude	End Magnitude	
1 Distributed	90.000	0.0000	4.1000	-0.11260	-0.11260	k/ft

Analysis: Stair Stringer Analysis.cfsa
4.1 ft Span Simple Beam

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Load Combination: D
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000

Load Combination: D+L
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Live Load	1.000
4 Product Load	1.000

Load Combination: D+0.75(L+Lr)
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Live Load	0.750
4 Product Load	0.750
5 Roof Live Load	0.750

Load Combination: 0.6D+0.6W
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	0.600
2 Dead Load	0.600
3 Wind Load	0.600

Load Combination: D+0.7E
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Earthquake Load	0.700

Load Combination: D+0.525E+0.75L+0.1S
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Earthquake Load	0.525
4 Live Load	0.750
5 Product Load	0.750
6 Snow Load	0.100

Analysis: Stair Stringer Analysis.cfsa
4.1 ft Span Simple Beam

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Load Combination: 0.6D+0.7E
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	0.600
2 Dead Load	0.600
3 Earthquake Load	0.700

Member Check - AISI S100-24, US, ASD

Load Combination: D+L

Design Parameters at 2.0500 ft:

Lx	4.1000 ft	Ly	4.1000 ft	Lt	4.1000 ft
Kx	1.0000	Ky	1.0000	Kt	1.0000

Section: 9 - Stair Stringers.cfss

Material Type: A653 SS Grade 33, Fy=33 ksi

Cbx	1.1364	Cby	1.0000	ex	0.0000 in
Cmx	1.0000	Cmy	1.0000	ey	0.0000 in
Braced Flange: None	kφ	0 k			
Red. Factor, R: 0	Lm	4.1000 ft			

Loads:	P (k)	Mx (k-in)	Vy (k)	My (k-in)	Vx (k)
Total	0.0000	3.0146	0.0000	0.0000	0.0000
Applied	0.0000	3.0146	0.0000	0.0000	0.0000
Strength	5.0583	3.4092	1.9335	3.4092	1.9335

Interaction Equations

Eq. H1.2-1	(P, Mx, My)	$0.000 + 0.884 + 0.000 = 0.884 \leq 1.0$
Eq. H2-1	(Mx, Vy)	$\text{Sqrt}(0.782 + 0.000) = 0.884 \leq 1.0$
Eq. H2-1	(My, Vx)	$\text{Sqrt}(0.000 + 0.000) = 0.000 \leq 1.0$

Web Crippling Check - AISI S100-24, US, ASD

Load Combination: D+L

Parameters at 4.1000 ft:

Total Load: 0.24509 k on bottom flange

Total Moment: 0.0000 k-in

Bearing: 2.0000 in

Flange fastened to bearing surface: No

Distance from edge of bearing to edge of opposite load: 4.1000 ft

Section: 9 - Stair Stringers.cfss

Material Type: A653 SS Grade 33, Fy=33 ksi

Applied Load: 0.2451 k on bottom flange

Applied Moment: 0.0000 k-in

Distance from edge of bearing to end of member: 0.0000 ft

Analysis: Stair Stringer Analysis.cfsa
4.1 ft Span Simple Beam

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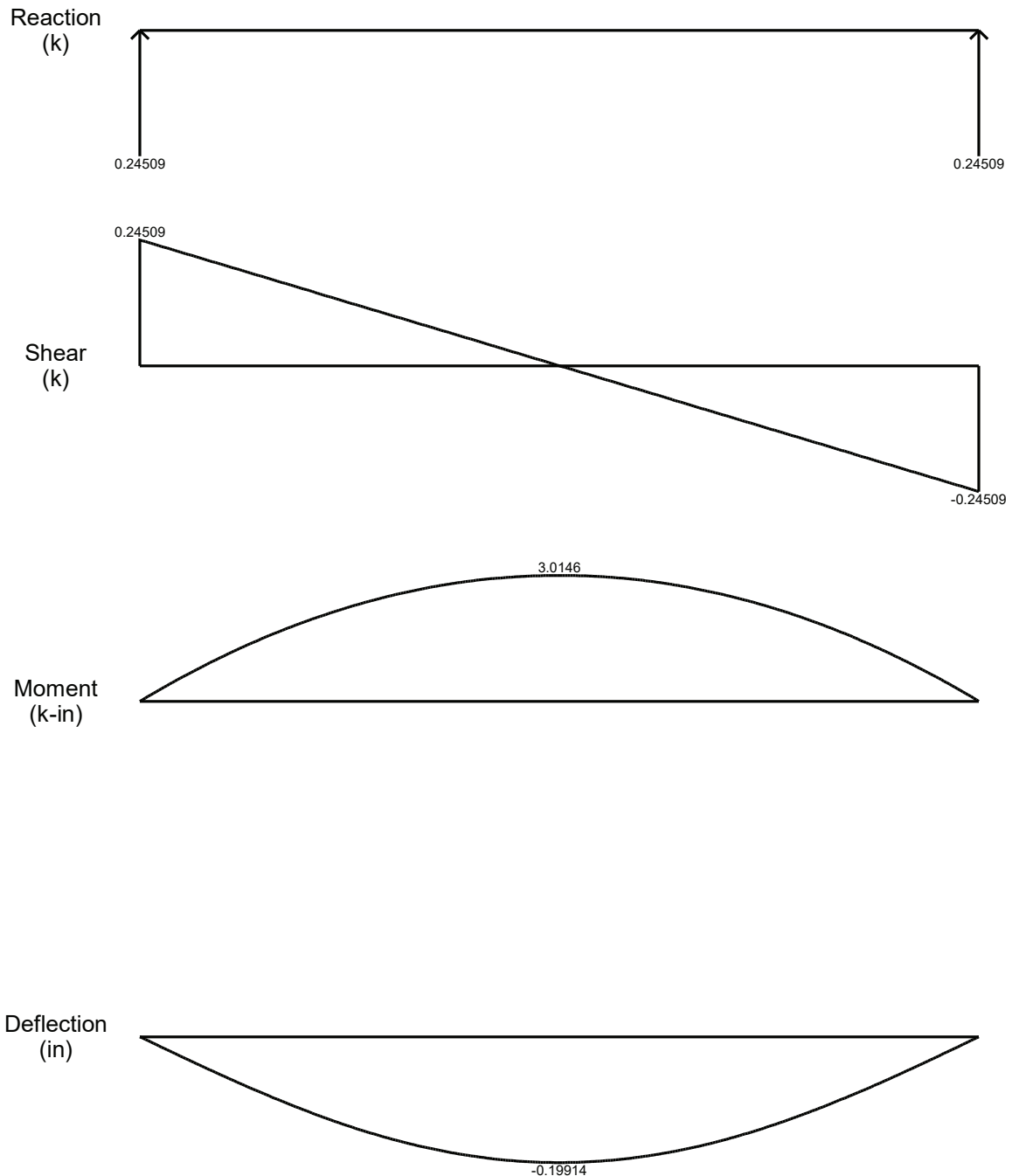
Part	Elem	Calculation Type	Pa (k)	Pay (k)	Notes
1	2	Single, US-EOF	0.7890	0.7890	
1	4	Single, US-EOF	0.7890	0.7890	
		Web Crippling Strength		1.5779	

Web Crippling Check: 0.2451 k <= 1.5779 k
 Moment Check: 0.0000 k-in <= 3.4092 k-in
 Interaction Equations
 Eq. H3-1a (P, M) $0.076 + 0.000 = 0.076 <= 0.782$

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Item 4: Stair Stinger

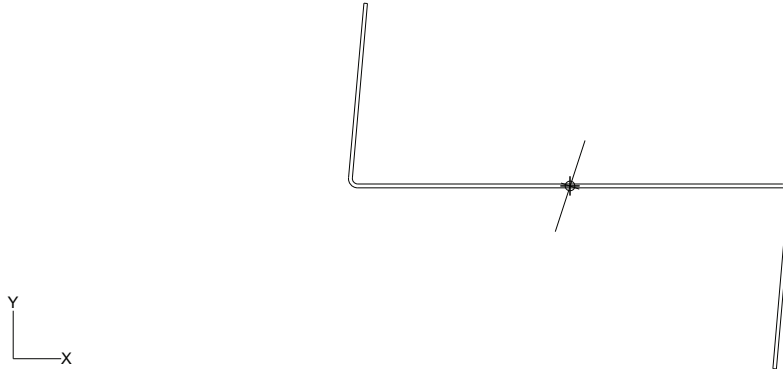
Load Combination: D+L, Y Direction



Allowable Deflection: $L / 240 = 4' * 12" / 240 = 0.20"$ OK

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Item 5: Stair Treads



Section Inputs

Material: A653 SS Grade 33
Apply cold work of forming strength increase.
No inelastic reserve strength increase.
Modulus of Elasticity, E 29500 ksi
Yield Strength, Fy 33 ksi
Tensile Strength, Fu 45 ksi
Min Elongation in 2 inches 20 %
Torsion Constant Override, J 0 in⁴
Warping Constant Override, Cw 0 in⁶

Angle, Thickness 0.1017 in (97 mil)
Placement of Part from Origin:
X to center of gravity 0 in
Y to center of gravity 0 in
Outside dimensions, Open shape

	Length (in)	Angle (deg)	Radius (in)	Web	k Coef.	Hole Size (in)	Distance (in)
1	5.000	-95.000	0.15250	Single	0.000	0.000	2.500
2	12.000	0.000	0.15250	Single	0.000	0.000	6.000
3	5.000	265.000	0.15250	Single	0.000	0.000	2.500

Full Section Properties

Area	2.1931 in ²	Wt.	0.0074566 k/ft	Flat	21.565 in
Ix	8.135 in ⁴	rx	1.9259 in	Ixy	-14.015 in ⁴
Sx(t)	1.6500 in ³	y(t)	4.9301 in	α	71.923 deg
Sx(b)	1.6500 in ³	y(b)	4.9301 in		
Zx	2.5069 in ³	Height	9.8602 in		
Iy	46.496 in ⁴	ry	4.6044 in	x _o	0.0000 in
Sy(l)	7.7794 in ³	x(l)	5.9768 in	y _o	0.0000 in
Sy(r)	7.7794 in ³	x(r)	5.9768 in	jx	0.0000 in
Zy	9.2261 in ³	Width	11.9536 in	jy	0.0000 in

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I_1	51.071 in ⁴	r_1	4.8256 in	Cw	188.18 in ⁶
I_2	3.560 in ⁴	r_2	1.2741 in	J	0.007561 in ⁴
I_c	54.631 in ⁴	r_c	4.9910 in		
I_o	54.631 in ⁴	r_o	4.9910 in		

Fully Braced Strength - AISI S100-24, US, ASD

Material Type: A653 SS Grade 33, Fy=33 ksi

Axial		Positive Bending		Positive Bending	
Pao	17.284 k	Maxo	18.48 k-in	Mayo	101.90 k-in
Ae	0.94274 in ²	Ixe	4.170 in ⁴	Iye	35.432 in ⁴
Ta	44.136 k	Sxe(t)	1.3986 in ³	Sye(l)	6.9710 in ³
		Sxe(b)	0.9353 in ³	Sye(r)	5.1569 in ³
Shear		Negative Bending		Negative Bending	
Vay	11.346 k	Maxo	18.48 k-in	Mayo	101.90 k-in
Vax	9.257 k	Ixe	4.170 in ⁴	Iye	35.432 in ⁴
Torsion		Sxe(t)	0.9353 in ³	Sye(l)	5.1569 in ³
		Sxe(b)	1.3986 in ³	Sye(r)	6.9710 in ³
Ba	164.87 k-in ²				

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Analysis Inputs

General

Member Orientation: Horizontal
 Calculate global buckling using specification equations
 Do not include torsion in member checks

Members

Section File				Revision Date and Time			
1 10 - Stair Tread.cfss				2/23/2023 5:50:56 PM			
Material	Area (in ²)	Length (ft)	Weight (k)				
1 A653 SS Grade 33	2.1931	3.6667	0.027341				
Total		3.6667	0.027341				
Start Loc. (ft)	End Loc. (ft)	Braced Flange	R	kφ (k)	Lm (ft)	ex (in)	ey (in)
1 0.0000	3.6667	None	0.0000	0.0000	3.6667	0.000	0.000

Supports

Type	Location (ft)	Bearing (in)	Fastened	K
1 XYT	0.0000	2.00	No	1.0000
2 XYT	3.6667	2.00	No	1.0000

Loading: Dead Load

Type	Angle (deg)	Start Loc. (ft)	End Loc. (ft)	Start Magnitude	End Magnitude	
1 Distributed	90.000	0.0000	3.6667	-0.00500	-0.00500	k/ft

Loading: Live Load

Type	Angle (deg)	Start Loc. (ft)	End Loc. (ft)	Start Magnitude	End Magnitude	
1 Distributed	90.000	0.0000	3.6667	-0.10000	-0.10000	k/ft

Analysis: Stair Tread Analysis.cfsa
3.6667 ft Span Simple Beam

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Load Combination: D

Specification: AISI S100-24, US, ASD

Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000

Load Combination: D+L

Specification: AISI S100-24, US, ASD

Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Live Load	1.000
4 Product Load	1.000

Load Combination: D+0.75(L+Lr)

Specification: AISI S100-24, US, ASD

Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Live Load	0.750
4 Product Load	0.750
5 Roof Live Load	0.750

Load Combination: 0.6D+0.6W

Specification: AISI S100-24, US, ASD

Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	0.600
2 Dead Load	0.600
3 Wind Load	0.600

Load Combination: D+0.7E

Specification: AISI S100-24, US, ASD

Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Earthquake Load	0.700

Load Combination: D+0.525E+0.75L+0.1S

Specification: AISI S100-24, US, ASD

Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Earthquake Load	0.525
4 Live Load	0.750
5 Product Load	0.750
6 Snow Load	0.100

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Load Combination: 0.6D+0.7E
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	0.600
2 Dead Load	0.600
3 Earthquake Load	0.700

Member Check - AISI S100-24, US, ASD

Load Combination: D+L

Design Parameters at 1.8334 ft:

Lx	3.6667 ft	Ly	3.6667 ft	Lt	3.6667 ft
Kx	1.0000	Ky	1.0000	Kt	1.0000

Section: 10 - Stair Tread.cfss

Material Type: A653 SS Grade 33, Fy=33 ksi

Cbx	1.1364	Cby	1.0000	ex	0.0000 in
Cmx	1.0000	Cmy	1.0000	ey	0.0000 in
Braced Flange:	None	kφ	0 k		
Red. Factor, R:	0	Lm	3.6667 ft		

Loads:	P (k)	Mx (k-in)	Vy (k)	My (k-in)	Vx (k)
Total	0.000	2.27	0.000	0.00	0.000
Applied	0.000	2.27	0.000	0.00	0.000
Strength	16.717	18.48	11.346	101.90	9.257

Interaction Equations

Eq. H1.2-1	(P, Mx, My)	$0.000 + 0.123 + 0.000 = 0.123 \leq 1.0$
Eq. H2-1	(Mx, Vy)	$\text{Sqrt}(0.015 + 0.000) = 0.123 \leq 1.0$
Eq. H2-1	(My, Vx)	$\text{Sqrt}(0.000 + 0.000) = 0.000 \leq 1.0$

Web Crippling Check - AISI S100-24, US, ASD

Load Combination: D+L

Parameters at 0.0000 ft:

Total Load: 0.20617 k on bottom flange

Total Moment: 0.0000 k-in

Bearing: 2.0000 in

Flange fastened to bearing surface: No

Distance from edge of bearing to edge of opposite load: 3.6667 ft

Section: 10 - Stair Tread.cfss

Material Type: A653 SS Grade 33, Fy=33 ksi

Applied Load: 0.2062 k on bottom flange

Applied Moment: 0.000 k-in

Distance from edge of bearing to end of member: 0.0000 ft

Analysis: Stair Tread Analysis.cfsa
3.6667 ft Span Simple Beam

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Part	Elem	Calculation Type	Pa (k)	Pay (k)	Notes
1	1	Single, US-EOF	1.2420	1.2420	$\theta > 90^\circ$
1	3	Single, US-EOF	1.6267	1.6329	
		Web Crippling Strength		2.8749	

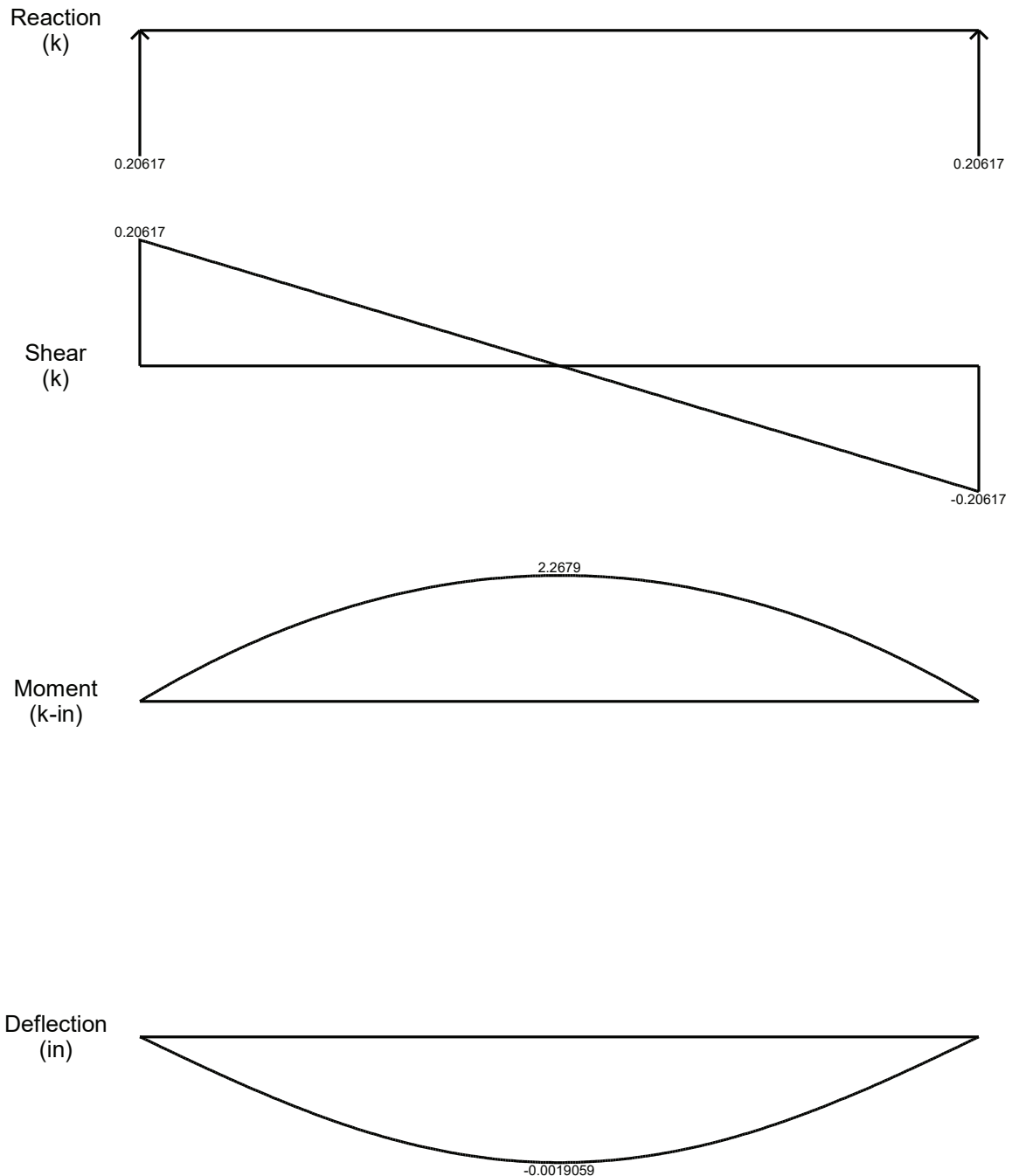
Web Crippling Check: 0.2062 k <= 2.8749 k
 Moment Check: 0.000 k-in <= 18.481 k-in
 Interaction Equations
 Eq. H3-1a (P, M) $0.034 + 0.000 = 0.034 <= 0.665$

Rev. Date: 2/13/2026 11:27:49 AM

Printed: 2/13/2026 11:29:23 AM

Item 6: Stair Treads

Load Combination: D+L, Y Direction



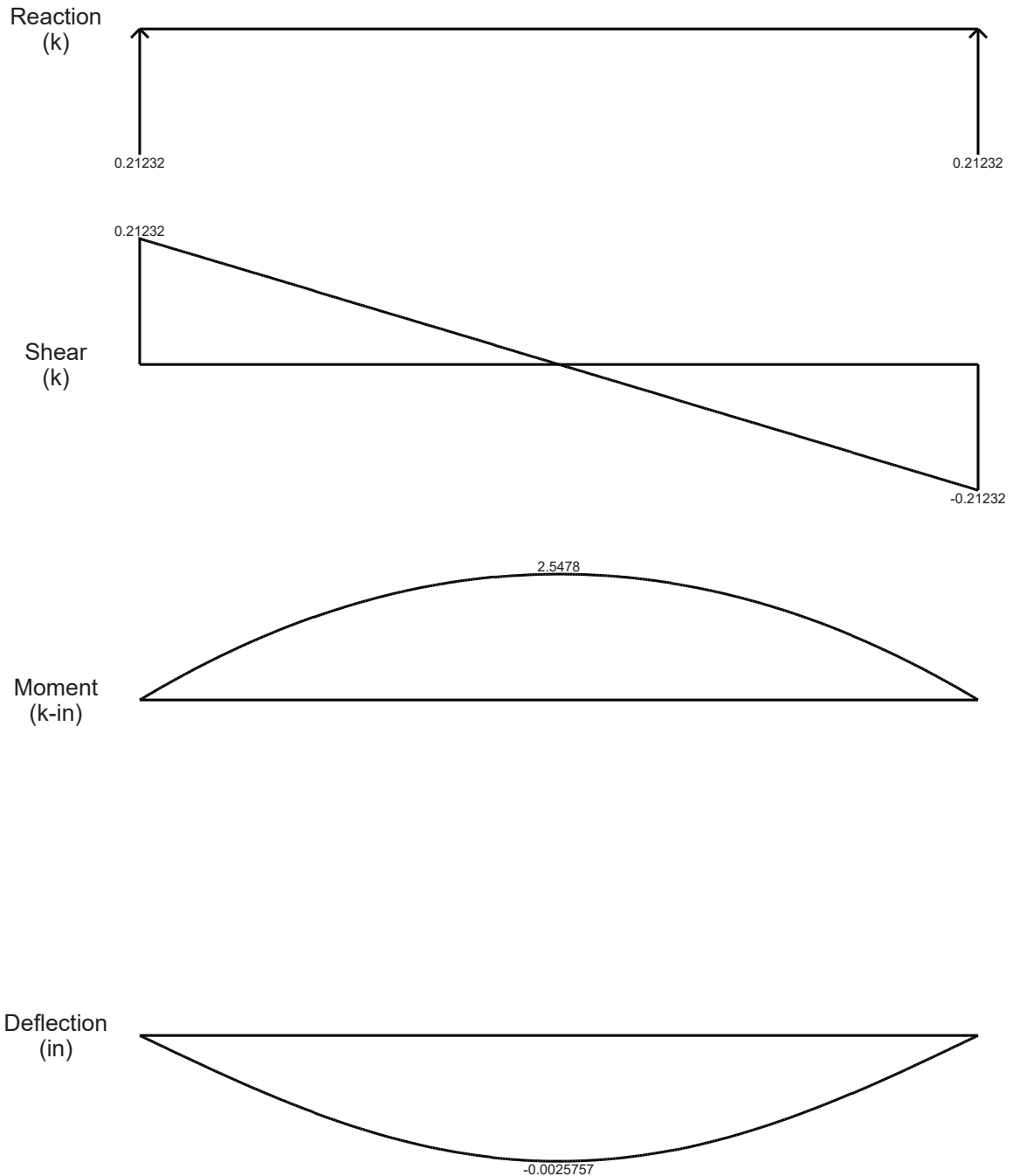
Allowable Deflection: $L / 240 = 4' * 12" / 240 = 0.20"$ OK

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Item 5: Stair Treads

Load Combination: D+L, Y Direction



Allowable Deflection: $L / 240 = 4.5' * 12" / 240 = 0.225"$ OK

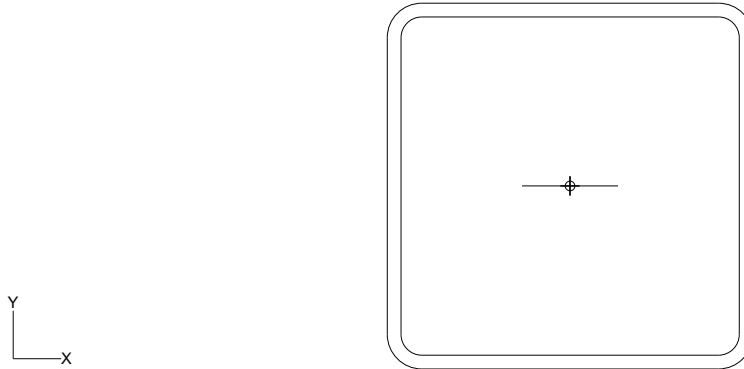
Section: 11 - Item 10 Platform Leg Tube 1.5x1.5-16 Gage.cfss
 Tube 1.5x1.5-16 Gage

Rev. Date: 2/24/2023 1:17:02 PM

By: Sal

Printed: 2/13/2026 11:42:54 AM

Item 10: Platform Leg Seismic Load



Section Inputs

Material: A653 SS Grade 33

No cold work of forming strength increase.

No inelastic reserve strength increase.

Modulus of Elasticity, E 29500 ksi

Yield Strength, Fy 33 ksi

Tensile Strength, Fu 45 ksi

Min Elongation in 2 inches 20 %

Torsion Constant Override, J 0 in⁴

Warping Constant Override, Cw 0 in⁶

Tube, Thickness 0.0566 in (54 mil)

Placement of Part from Origin:

X to center of gravity 0 in

Y to center of gravity 0 in

Outside dimensions, Closed shape

	Length (in)	Angle (deg)	Radius (in)	Web	k Coef.	Hole Size (in)	Distance (in)
1	1.5000	0.000	0.084900	Single	0.000	0.0000	0.7500
2	1.5000	90.000	0.084900	Single	0.000	0.0000	0.7500
3	1.5000	180.000	0.084900	Single	0.000	0.0000	0.7500
4	1.5000	-90.000	0.084900	Single	0.000	0.0000	0.7500

Full Section Properties

Area	0.31579 in ²	Wt.	0.0010737 k/ft	Flat	5.5793 in
Ix	0.10758 in ⁴	rx	0.58367 in	Ixy	0.00000 in ⁴
Sx(t)	0.14344 in ³	y(t)	0.75000 in	α	0.000 deg
Sx(b)	0.14344 in ³	y(b)	0.75000 in		
Zx	0.16880 in ³	Height	1.50000 in		
Iy	0.10758 in ⁴	ry	0.58367 in	x _o	0.00000 in
Sy(l)	0.14344 in ³	x(l)	0.75000 in	y _o	0.00000 in
Sy(r)	0.14344 in ³	x(r)	0.75000 in	jx	0.00000 in
Zy	0.16880 in ³	Width	1.50000 in	jy	0.00000 in

Section: 11 - Item 10 Platform Leg Tube 1.5x1.5-16 Gage.cfss
Tube 1.5x1.5-16 Gage

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By: Sal

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I_1	0.10758 in ⁴	r_1	0.58367 in	Cw	0.00002 in ⁶
I_2	0.10758 in ⁴	r_2	0.58367 in	J	0.17428 in ⁴
I_c	0.21516 in ⁴	r_c	0.82544 in		
I_o	0.21516 in ⁴	r_o	0.82544 in		

Fully Braced Strength - AISI S100-24, US, ASD

Material Type: A653 SS Grade 33, Fy=33 ksi

Axial		Positive Bending		Positive Bending	
Pao	5.7894 k	Maxo	2.8344 k-in	Mayo	2.8344 k-in
Ae	0.31579 in ²	Ixe	0.10758 in ⁴	Iye	0.10758 in ⁴
Ta	6.2401 k	Sxe(t)	0.14344 in ³	Sye(l)	0.14344 in ³
		Sxe(b)	0.14344 in ³	Sye(r)	0.14344 in ³

Shear

Vay		Negative Bending		Negative Bending	
Vay	1.6334 k	Maxo	2.8344 k-in	Mayo	2.8344 k-in
Vax	1.6334 k	Ixe	0.10758 in ⁴	Iye	0.10758 in ⁴
		Sxe(t)	0.14344 in ³	Sye(l)	0.14344 in ³
		Sxe(b)	0.14344 in ³	Sye(r)	0.14344 in ³

Member Check - AISI S100-24, US, ASD

Material Type: A653 SS Grade 33, Fy=33 ksi

Design Parameters:

Lx	3.2500 ft	Ly	3.2500 ft	Lt	0.0000 ft
Kx	1.0000	Ky	1.0000	Kt	1.0000
Cbx	1.0000	Cby	1.0000	ex	0.0000 in
Cmx	1.0000	Cmy	1.0000	ey	0.0000 in
Braced Flange: None		kφ	0 k		
Red. Factor, R: 0		Lm	3.2500 ft		

Loads:	P (k)	Mx (k-in)	Vy (k)	My (k-in)	Vx (k)
Entered	0.0675	2.6618	0.0683	0.0000	0.0000
Applied	0.0675	2.6758	0.0683	0.0000	0.0000
Strength	4.6843	2.8344	1.6334	2.8344	1.6334

Interaction Equations

Eq. H1.2-1	(P, Mx, My)	$0.014 + 0.944 + 0.000 = 0.958 \leq 1.0$
Eq. H2-1	(Mx, Vy)	$\text{Sqrt}(0.891 + 0.002) = 0.945 \leq 1.0$
Eq. H2-1	(My, Vx)	$\text{Sqrt}(0.000 + 0.000) = 0.000 \leq 1.0$

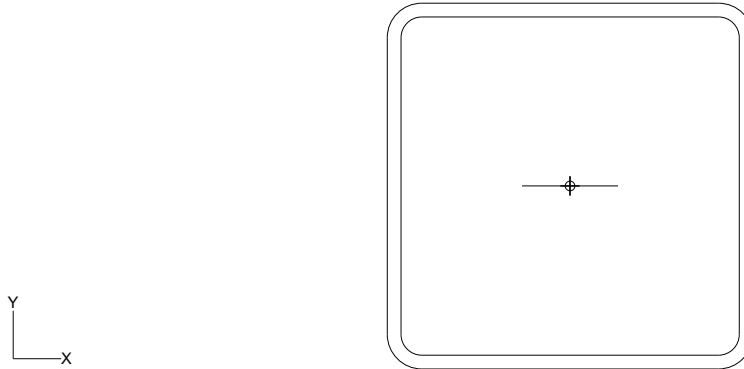
Section: 12 - Item 10 Platform Leg Tube 1.5x1.5-16 Gage - D + L.cfss
 Tube 1.5x1.5-16 Gage

Rev. Date: 2/24/2023 1:17:02 PM

By: Sal

Printed: 2/13/2026 11:49:18 AM

Item 10: Platform Leg D + L



Section Inputs

Material: A653 SS Grade 33

No cold work of forming strength increase.

No inelastic reserve strength increase.

Modulus of Elasticity, E 29500 ksi

Yield Strength, Fy 33 ksi

Tensile Strength, Fu 45 ksi

Min Elongation in 2 inches 20 %

Torsion Constant Override, J 0 in⁴

Warping Constant Override, Cw 0 in⁶

Tube, Thickness 0.0566 in (54 mil)

Placement of Part from Origin:

X to center of gravity 0 in

Y to center of gravity 0 in

Outside dimensions, Closed shape

	Length (in)	Angle (deg)	Radius (in)	Web	k Coef.	Hole Size (in)	Distance (in)
1	1.5000	0.000	0.084900	Single	0.000	0.0000	0.7500
2	1.5000	90.000	0.084900	Single	0.000	0.0000	0.7500
3	1.5000	180.000	0.084900	Single	0.000	0.0000	0.7500
4	1.5000	-90.000	0.084900	Single	0.000	0.0000	0.7500

Full Section Properties

Area	0.31579 in ²	Wt.	0.0010737 k/ft	Flat	5.5793 in
Ix	0.10758 in ⁴	rx	0.58367 in	Ixy	0.00000 in ⁴
Sx(t)	0.14344 in ³	y(t)	0.75000 in	α	0.000 deg
Sx(b)	0.14344 in ³	y(b)	0.75000 in		
Zx	0.16880 in ³	Height	1.50000 in		
Iy	0.10758 in ⁴	ry	0.58367 in	x _o	0.00000 in
Sy(l)	0.14344 in ³	x(l)	0.75000 in	y _o	0.00000 in
Sy(r)	0.14344 in ³	x(r)	0.75000 in	jx	0.00000 in
Zy	0.16880 in ³	Width	1.50000 in	jy	0.00000 in

Section: 12 - Item 10 Platform Leg Tube 1.5x1.5-16 Gage - D + L.cfss
Tube 1.5x1.5-16 Gage

Rev. Date: 2/24/2023 1:17:02 PM

By: Sal

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I_1	0.10758 in ⁴	r_1	0.58367 in	Cw	0.00002 in ⁶
I_2	0.10758 in ⁴	r_2	0.58367 in	J	0.17428 in ⁴
I_c	0.21516 in ⁴	r_c	0.82544 in		
I_o	0.21516 in ⁴	r_o	0.82544 in		

Fully Braced Strength - AISI S100-24, US, ASD

Material Type: A653 SS Grade 33, Fy=33 ksi

Axial		Positive Bending		Positive Bending	
Pao	5.7894 k	Maxo	2.8344 k-in	Mayo	2.8344 k-in
Ae	0.31579 in ²	Ixe	0.10758 in ⁴	Iye	0.10758 in ⁴
Ta	6.2401 k	Sxe(t)	0.14344 in ³	Sye(l)	0.14344 in ³
		Sxe(b)	0.14344 in ³	Sye(r)	0.14344 in ³
Shear		Negative Bending		Negative Bending	
Vay	1.6334 k	Maxo	2.8344 k-in	Mayo	2.8344 k-in
Vax	1.6334 k	Ixe	0.10758 in ⁴	Iye	0.10758 in ⁴
Torsion		Sxe(t)	0.14344 in ³	Sye(l)	0.14344 in ³
Ba	0.028691 k-in ²	Sxe(b)	0.14344 in ³	Sye(r)	0.14344 in ³

Member Check - AISI S100-24, US, ASD

Material Type: A653 SS Grade 33, Fy=33 ksi

Design Parameters:

Lx	3.2500 ft	Ly	3.2500 ft	Lt	0.0000 ft
Kx	1.0000	Ky	1.0000	Kt	1.0000
Cbx	1.0000	Cby	1.0000	ex	0.0000 in
Cmx	1.0000	Cmy	1.0000	ey	0.0000 in
Braced Flange: None		kφ	0 k		
Red. Factor, R: 0		Lm	3.2500 ft		

Loads:	P	Mx	Vy	My	Vx
	(k)	(k-in)	(k)	(k-in)	(k)
Entered	1.4175	0.0000	0.0000	0.0000	0.0000
Applied	1.4175	0.0000	0.0000	0.0000	0.0000
Strength	4.6843	2.8344	1.6334	2.8344	1.6334

Interaction Equations

Eq. H1.2-1	(P, Mx, My)	$0.303 + 0.000 + 0.000 = 0.303 \leq 1.0$
Eq. H2-1	(Mx, Vy)	$\text{Sqrt}(0.000 + 0.000) = 0.000 \leq 1.0$
Eq. H2-1	(My, Vx)	$\text{Sqrt}(0.000 + 0.000) = 0.000 \leq 1.0$

Rev. Date: 2/20/2023 9:36:29 AM
 By: Sal
 Printed: 2/13/2026 11:56:47 AM

Item 2: Main Frame Side Rails
 Trib Width = 2.25'



Section Inputs

Material: A653 SS Grade 33
 Apply cold work of forming strength increase.
 No inelastic reserve strength increase.
 Modulus of Elasticity, E 29500 ksi
 Yield Strength, Fy 33 ksi
 Tensile Strength, Fu 45 ksi
 Min Elongation in 2 inches 20 %
 Torsion Constant Override, J 0 in⁴
 Warping Constant Override, Cw 0 in⁶

Part 1, Thickness 0.0566 in (54 mil)
 Placement of Part from Origin:
 X to center of gravity 0 in
 Y to center of gravity 0 in

Outside dimensions, Open shape

	Length (in)	Angle (deg)	Radius (in)	Web	k Coef.	Hole Size (in)	Distance (in)
1	0.6250	0.000	0.084900	None	0.000	0.0000	0.3125
2	4.0000	90.000	0.084900	None	0.000	0.0000	2.0000
3	0.3564	180.000	0.084900	None	0.000	0.0000	0.0891
4	1.0000	-90.000	0.084900	None	0.000	0.0000	0.5000
5	1.1920	180.000	0.084900	None	0.000	0.0000	0.5960

Full Section Properties

Area	0.38220 in ²	Wt.	0.0012995 k/ft	Flat	6.7527 in
Ix	0.6153 in ⁴	rx	1.2688 in	Ixy	-0.0427 in ⁴
Sx(t)	0.35742 in ³	y(t)	1.7215 in	α	4.282 deg
Sx(b)	0.27005 in ³	y(b)	2.2785 in		
Zx	0.40722 in ³	Height	4.0000 in		
Iy	0.0486 in ⁴	ry	0.3565 in	x _o	0.4042 in
Sy(l)	0.03915 in ³	x(l)	1.2408 in	y _o	1.0135 in
Sy(r)	0.19350 in ³	x(r)	0.2510 in	jx	-0.7803 in
Zy	0.08639 in ³	Width	1.4918 in	jy	-1.3515 in

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I_1	0.6185 in ⁴	r_1	1.2721 in	Cw	0.035342 in ⁶
I_2	0.0454 in ⁴	r_2	0.3446 in	J	0.00040813 in ⁴
I_c	0.6639 in ⁴	r_c	1.3179 in		
I_o	1.1189 in ⁴	r_o	1.7110 in		

Fully Braced Strength - AISI S100-24, US, ASD

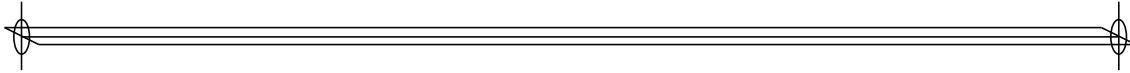
Material Type: A653 SS Grade 33, Fy=33 ksi

Axial		Positive Bending		Positive Bending	
Pao	4.3648 k	Maxo	5.5971 k-in	Mayo	0.7735 k-in
Ae	0.26453 in ²	Ixe	0.61530 in ⁴	Iye	0.04857 in ⁴
Ta	8.0210 k	Sxe(t)	0.35742 in ³	Sye(l)	0.03915 in ³
		Sxe(b)	0.27005 in ³	Sye(r)	0.19350 in ³
Shear		Negative Bending		Negative Bending	
Vay	0.0000 k	Maxo	6.1813 k-in	Mayo	0.6400 k-in
Vax	0.0000 k	Ixe	0.61530 in ⁴	Iye	0.03583 in ⁴
Torsion		Sxe(t)	0.35742 in ³	Sye(l)	0.03239 in ³
Ba	0.45052 k-in ²	Sxe(b)	0.27005 in ³	Sye(r)	0.16117 in ³

Section contains no web elements for vertical shear.
Section contains no web elements for horizontal shear.
Edge stiffener D/w (1.6625) exceeds 0.8.

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Analysis Inputs

General

Member Orientation: Horizontal
 Calculate global buckling using specification equations
 Do not include torsion in member checks

Members

Section File				Revision Date and Time			
1 13 - 16 ga Side Frame - 6 ft Span.cfss				2/20/2023 9:36:29 AM			
Material	Area (in ²)	Length (ft)	Weight (k)				
1 A653 SS Grade 33	0.38220	6.0000	0.0077969				
Total		6.0000	0.0077969				
Start Loc. (ft)	End Loc. (ft)	Braced Flange	R	kφ (k)	Lm (ft)	ex (in)	ey (in)
1 0.0000	6.0000	None	0.0000	0.0000	20.0000	0.000	0.000

Supports

Type	Location (ft)	Bearing (in)	Fastened	K
1 Y	0.0000	2.00	No	1.0000
2 XT	0.0000	2.00	No	0.0000
3 Y	6.0000	2.00	No	1.0000
4 XT	6.0000	2.00	No	1.0000

Loading: Dead Load

Type	Angle (deg)	Start Loc. (ft)	End Loc. (ft)	Start Magnitude	End Magnitude	
1 Distributed	90.000	0.0000	6.0000	-0.01125	-0.01125	k/ft

Loading: Live Load

Type	Angle (deg)	Start Loc. (ft)	End Loc. (ft)	Start Magnitude	End Magnitude	
1 Distributed	90.000	0.0000	6.0000	-0.22500	-0.22500	k/ft

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Load Combination: D
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000

Load Combination: D+L
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Live Load	1.000
4 Product Load	1.000

Load Combination: D+0.75(L+Lr)
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Live Load	0.750
4 Product Load	0.750
5 Roof Live Load	0.750

Load Combination: 0.6D+0.6W
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	0.600
2 Dead Load	0.600
3 Wind Load	0.600

Load Combination: D+0.7E
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Earthquake Load	0.700

Load Combination: D+0.525E+0.75L+0.1S
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Earthquake Load	0.525
4 Live Load	0.750
5 Product Load	0.750
6 Snow Load	0.100

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Load Combination: 0.6D+0.7E
 Specification: AISI S100-24, US, ASD
 Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	0.600
2 Dead Load	0.600
3 Earthquake Load	0.700

Member Check - AISI S100-24, US, ASD

Load Combination: D+L

Design Parameters at 3.0000 ft:

Lx	6.0000 ft	Ly	6.0000 ft	Lt	6.0000 ft
Kx	1.0000	Ky	0.0000	Kt	0.0000

Section: 13 - 16 ga Side Frame - 6 ft Span.cfss

Material Type: A653 SS Grade 33, Fy=33 ksi

Cbx	1.0000	Cby	1.0000	ex	0.0000 in
Cmx	1.0000	Cmy	1.0000	ey	0.0000 in
Braced Flange: None		kφ	0 k		
Red. Factor, R: 0		Lm	20.0000 ft		

Loads:	P (k)	Mx (k-in)	Vy (k)	My (k-in)	Vx (k)
Total	0.0000	12.828	0.0000	0.000	0.0000
Applied	0.0000	12.828	0.0000	0.000	0.0000
Strength	3.9226	5.597	0.0000	0.774	0.0000

Interaction Equations

Eq. H1.2-1	(P, Mx, My)	$0.000 + 2.292 + 0.000 = 2.292$	> 1.0
Eq. H2-1	(Mx, Vy)	$\text{Sqrt}(5.253 + 9.999) = 9.999$	> 1.0
Eq. H2-1	(My, Vx)	$\text{Sqrt}(0.000 + 9.999) = 9.999$	> 1.0

Side Rails Proven
By Testing

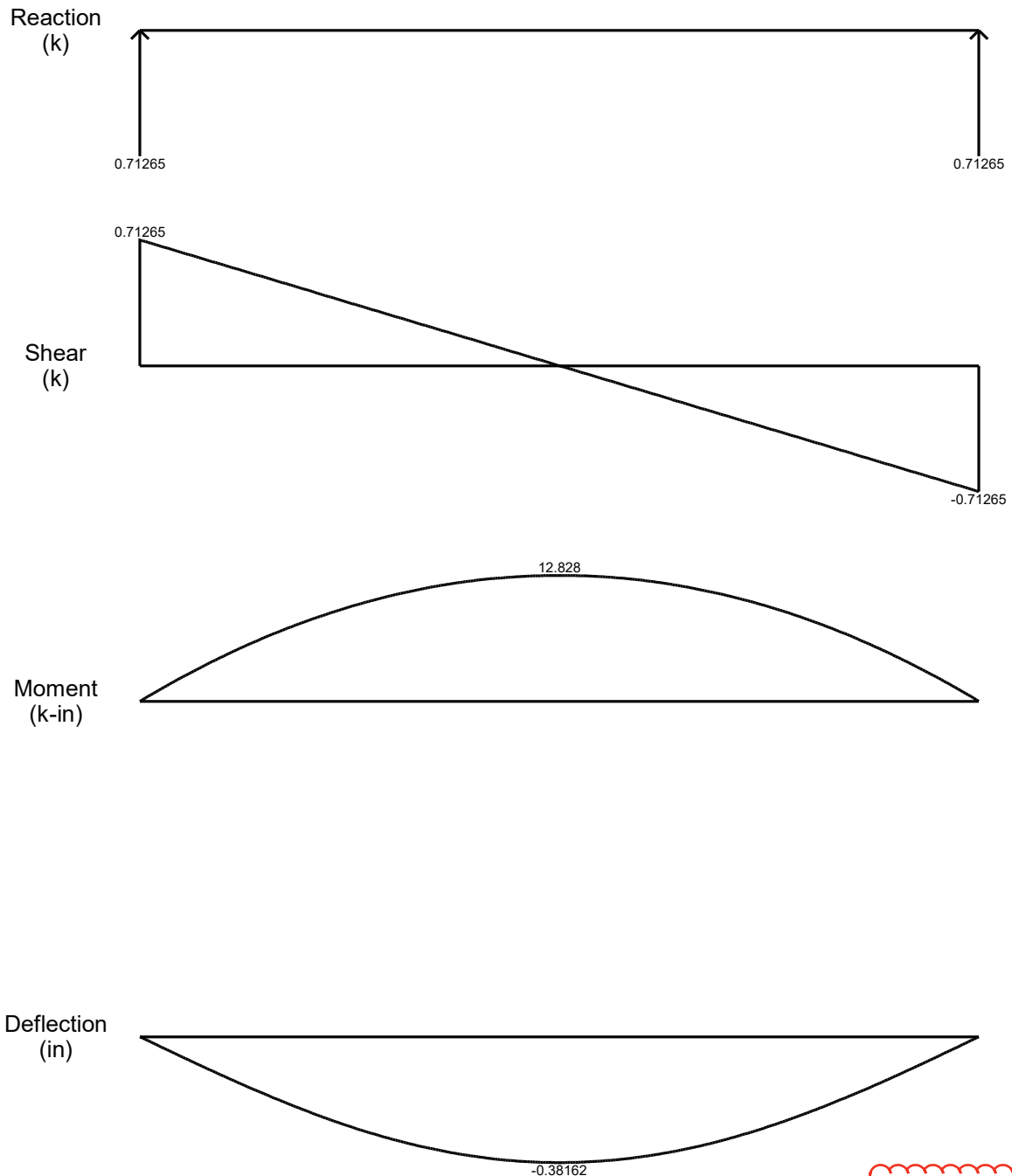
Section contains no web elements for vertical shear.
 Section contains no web elements for horizontal shear.
 Edge stiffener D/w (1.6625) exceeds 0.8.
 Calculation of Mndx used αs=1 from DSM inputs.

Rev. Date: 2/13/2026 11:54:34 AM

Item 2: Main Frame Side Rails

Printed: 2/13/2026 11:56:09 AM

Load Combination: D+L, Y Direction



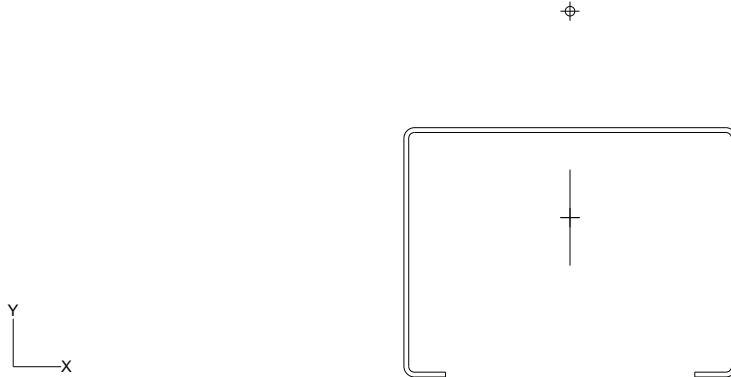
Allowable Deflection: $L / 240 = 6' * 12" / 240 = 0.3"$

Side Rails Proven
By Testing

Section: 14 - Cross-bracing Channel 16 ga x 4 x 3.cfs
Channel 4x2.75-16 Gage

Rev. Date: 2/21/2023 7:52:57 AM
By: Sal
Printed: 2/13/2026 12:02:34 PM

Item 2: Cross-Bracing Channel
Trib Width = 2'-0"
Span 4'-6"



Section Inputs

Material: A653 SS Grade 33
Apply cold work of forming strength increase.
No inelastic reserve strength increase.
Modulus of Elasticity, E 29500 ksi
Yield Strength, Fy 33 ksi
Tensile Strength, Fu 45 ksi
Min Elongation in 2 inches 20 %
Torsion Constant Override, J 0 in⁴
Warping Constant Override, Cw 0 in⁶

Channel, Thickness 0.0566 in (54 mil)

Placement of Part from Origin:

X to center of gravity 0 in

Y to center of gravity 0 in

Outside dimensions, Open shape

	Length (in)	Angle (deg)	Radius (in)	Web	k Coef.	Hole Size (in)	Distance (in)
1	0.5000	180.000	0.069000	Single	0.000	0.0000	0.2500
2	3.0000	90.000	0.069000	Single	0.000	0.0000	1.5000
3	4.0000	0.000	0.069000	Cee	0.000	0.0000	2.0000
4	3.0000	-90.000	0.069000	Single	0.000	0.0000	1.5000
5	0.5000	180.000	0.069000	None	0.000	0.0000	0.2500

Full Section Properties

Area	0.60033 in ²	Wt.	0.0020411 k/ft	Flat	10.607 in
Ix	0.7149 in ⁴	rx	1.0913 in	Ixy	0.0000 in ⁴
Sx(t)	0.65969 in ³	y(t)	1.0837 in	α	-90.000 deg
Sx(b)	0.37309 in ³	y(b)	1.9163 in		
Zx	0.57456 in ³	Height	3.0000 in		
Iy	1.7094 in ⁴	ry	1.6874 in	x _o	0.0000 in
Sy(l)	0.85471 in ³	x(l)	2.0000 in	y _o	2.4879 in
Sy(r)	0.85471 in ³	x(r)	2.0000 in	jx	0.0000 in
Zy	0.95095 in ³	Width	4.0000 in	jy	-3.1305 in

Section: 14 - Cross-bracing Channel 16 ga x 4 x 3.cfss
Channel 4x2.75-16 Gage

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By: Sal

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I_1	1.7094 in ⁴	r_1	1.6874 in	Cw	2.3251 in ⁶
I_2	0.7149 in ⁴	r_2	1.0913 in	J	0.0006411 in ⁴
I_c	2.4243 in ⁴	r_c	2.0096 in		
I_o	6.1402 in ⁴	r_o	3.1981 in		

Fully Braced Strength - AISI S100-24, US, ASD

Material Type: A653 SS Grade 33, Fy=33 ksi

Axial		Positive Bending		Positive Bending	
Pao	7.586 k	Maxo	7.192 k-in	Mayo	13.151 k-in
Ae	0.41380 in ²	Ixe	0.6738 in ⁴	Iye	1.4709 in ⁴
Ta	12.310 k	Sxe(t)	0.58665 in ³	Sye(l)	0.82182 in ³
		Sxe(b)	0.36396 in ³	Sye(r)	0.66550 in ³
Shear		Negative Bending		Negative Bending	
Vay	3.689 k	Maxo	8.770 k-in	Mayo	13.151 k-in
Vax	2.491 k	Ixe	0.7149 in ⁴	Iye	1.4709 in ⁴
Torsion		Sxe(t)	0.65969 in ³	Sye(l)	0.66550 in ³
		Sxe(b)	0.37309 in ³	Sye(r)	0.82182 in ³
Ba		9.1251 k-in ²			

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Analysis Inputs

General

Member Orientation: Horizontal
 Calculate global buckling using specification equations
 Do not include torsion in member checks

Members

Section File Revision Date and Time
 1 14 - Cross-bracing Channel 16 ga x 4 x 3.cfss2/21/2023 7:52:57 AM

Material	Area (in ²)	Length (ft)	Weight (k)
1 A653 SS Grade 33	0.60033	4.5000	0.0091851
Total		4.5000	0.0091851

Start Loc. (ft)	End Loc. (ft)	Braced Flange	R	kφ (k)	Lm (ft)	ex (in)	ey (in)
1 0.0000	4.5000	None	0.0000	0.0000	4.5000	0.000	0.000

Supports

Type	Location (ft)	Bearing (in)	Fastened	K
1 XYT	0.0000	2.00	No	1.0000
2 XYT	4.5000	2.00	No	1.0000

Loading: Dead Load

Type	Angle (deg)	Start Loc. (ft)	End Loc. (ft)	Start Magnitude	End Magnitude
1 Distributed	90.000	0.0000	4.5000	-0.01000	-0.01000 k/ft

Loading: Live Load

Type	Angle (deg)	Start Loc. (ft)	End Loc. (ft)	Start Magnitude	End Magnitude
1 Distributed	90.000	0.0000	4.5000	-0.20000	-0.20000 k/ft

Analysis: Cross-Bracing Analysis.cfsa
4.5 ft Span Simple Beam

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Load Combination: D

Specification: AISI S100-24, US, ASD

Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000

Load Combination: D+L

Specification: AISI S100-24, US, ASD

Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Live Load	1.000
4 Product Load	1.000

Load Combination: D+0.75(L+Lr)

Specification: AISI S100-24, US, ASD

Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Live Load	0.750
4 Product Load	0.750
5 Roof Live Load	0.750

Load Combination: 0.6D+0.6W

Specification: AISI S100-24, US, ASD

Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	0.600
2 Dead Load	0.600
3 Wind Load	0.600

Load Combination: D+0.7E

Specification: AISI S100-24, US, ASD

Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Earthquake Load	0.700

Load Combination: D+0.525E+0.75L+0.1S

Specification: AISI S100-24, US, ASD

Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Earthquake Load	0.525
4 Live Load	0.750
5 Product Load	0.750
6 Snow Load	0.100

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Load Combination: 0.6D+0.7E
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	0.600
2 Dead Load	0.600
3 Earthquake Load	0.700

Member Check - AISI S100-24, US, ASD

Load Combination: D+L

Design Parameters at 2.2500 ft:

Lx	4.5000 ft	Ly	4.5000 ft	Lt	4.5000 ft
Kx	1.0000	Ky	1.0000	Kt	1.0000

Section: 14 - Cross-bracing Channel 16 ga x 4 x 3.cfss

Material Type: A653 SS Grade 33, Fy=33 ksi

Cbx	1.1364	Cby	1.0000	ex	0.0000 in
Cmx	1.0000	Cmy	1.0000	ey	0.0000 in
Braced Flange:	None	kφ	0 k		
Red. Factor, R:	0	Lm	4.5000 ft		

Loads:	P (k)	Mx (k-in)	Vy (k)	My (k-in)	Vx (k)
Total	0.0000	6.441	0.0000	0.000	0.0000
Applied	0.0000	6.441	0.0000	0.000	0.0000
Strength	5.9323	7.192	3.6893	13.055	2.4912

Interaction Equations

Eq. H1.2-1	(P, Mx, My)	$0.000 + 0.896 + 0.000 = 0.896 \leq 1.0$
Eq. H2-1	(Mx, Vy)	$\text{Sqrt}(0.802 + 0.000) = 0.896 \leq 1.0$
Eq. H2-1	(My, Vx)	$\text{Sqrt}(0.000 + 0.000) = 0.000 \leq 1.0$

Calculation of Mndy used as=1 from DSM inputs.

Web Crippling Check - AISI S100-24, US, ASD

Load Combination: D+L

Parameters at 0.0000 ft:

Total Load: 0.47709 k on bottom flange

Total Moment: 0.0000 k-in

Bearing: 2.0000 in

Flange fastened to bearing surface: No

Distance from edge of bearing to edge of opposite load: 4.5000 ft

Section: 14 - Cross-bracing Channel 16 ga x 4 x 3.cfss

Material Type: A653 SS Grade 33, Fy=33 ksi

Applied Load: 0.47709 k on bottom flange

Applied Moment: 0.0000 k-in

Distance from edge of bearing to end of member: 0.0000 ft

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Part	Elem	Calculation Type	Pa (k)	Pay (k)	Notes
1	2	Single, UU-EOF	0.47378	0.47378	
1	4	Single, UU-EOF	0.47378	0.47378	
		Web Crippling Strength		0.94756	

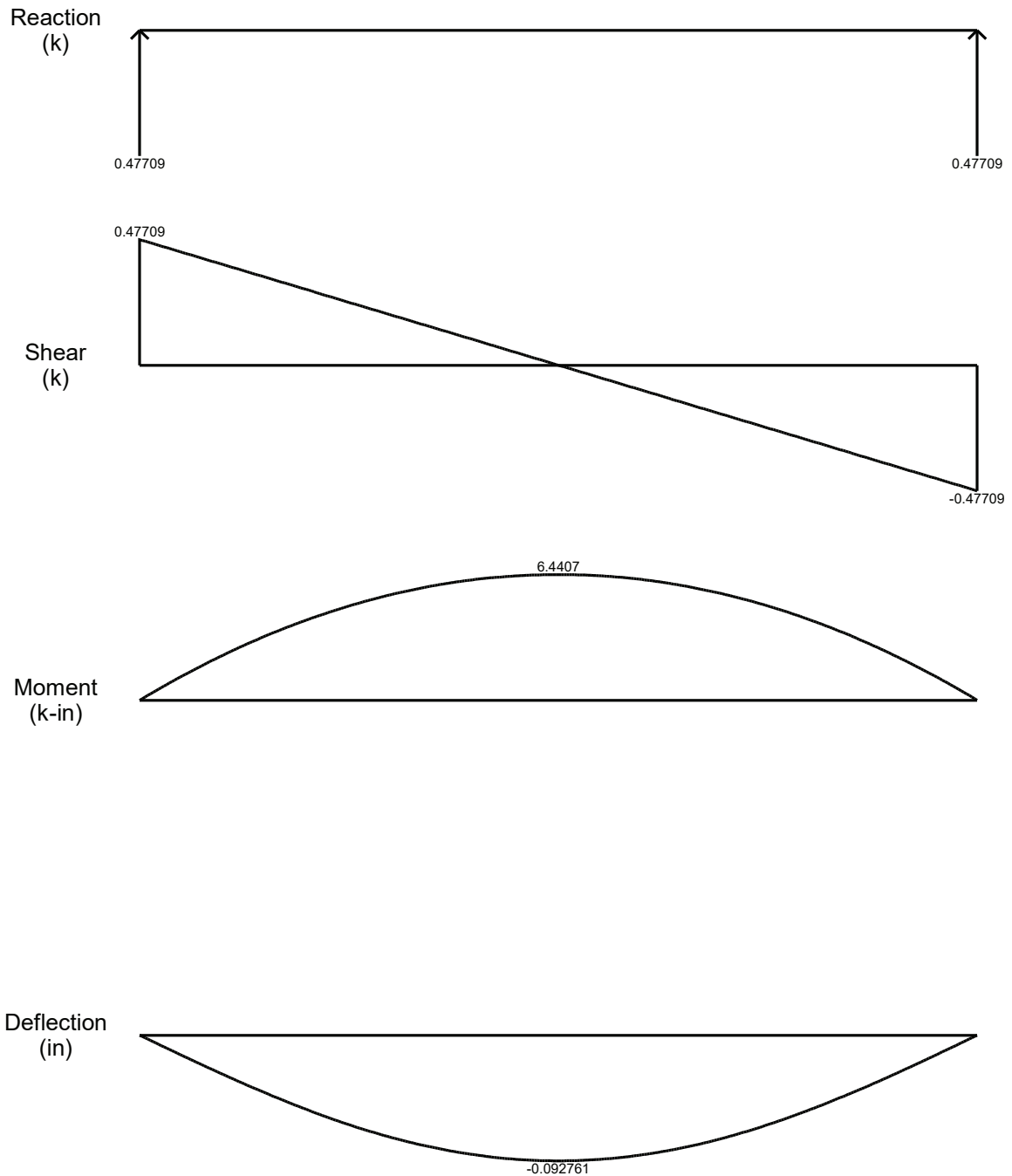
Web Crippling Check: 0.47709 k <= 0.94756 k
Moment Check: 0.0000 k-in <= 7.1919 k-in
Interaction Equations
Eq. H3-1a (P, M) $0.255 + 0.000 = 0.255 <= 0.782$

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Item 2: Cross-Bracing Channel

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Load Combination: D+L, Y Direction

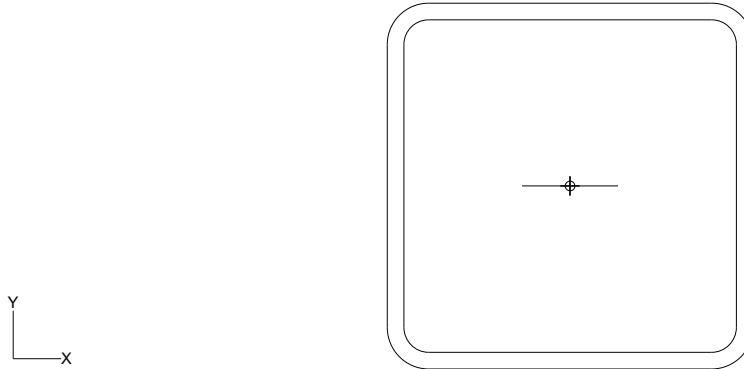


Allowable Deflection: $L / 240 = 4.5' * 12" / 240 = 0.225"$ OK

Section: 15 - Hand Rails Top Rails Section 1 6'.cfss
 Tube 1.25x1.25-16 Gage

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 By: Sal
 Printed: 2/13/2026 12:41:49 PM

Item 24: 6 ft Hand Rails Top Rails



Section Inputs

Material: A653 SS Grade 33
 No cold work of forming strength increase.
 No inelastic reserve strength increase.
 Modulus of Elasticity, E 29500 ksi
 Yield Strength, Fy 33 ksi
 Tensile Strength, Fu 45 ksi
 Min Elongation in 2 inches 20 %
 Torsion Constant Override, J 0 in⁴
 Warping Constant Override, Cw 0 in⁶

Tube, Thickness 0.0566 in (54 mil)

Placement of Part from Origin:

X to center of gravity 0 in

Y to center of gravity 0 in

Outside dimensions, Closed shape

	Length (in)	Angle (deg)	Radius (in)	Web	k Coef.	Hole Size (in)	Distance (in)
1	1.2500	0.000	0.084900	Single	0.000	0.0000	0.6250
2	1.2500	90.000	0.084900	Single	0.000	0.0000	0.6250
3	1.2500	180.000	0.084900	Single	0.000	0.0000	0.6250
4	1.2500	-90.000	0.084900	Single	0.000	0.0000	0.6250

Full Section Properties

Area	0.25919 in ²	Wt.	0.00088123 k/ft	Flat	4.5793 in
Ix	0.06008 in ⁴	rx	0.48147 in	Ixy	0.00000 in ⁴
Sx(t)	0.09613 in ³	y(t)	0.62500 in	α	0.000 deg
Sx(b)	0.09613 in ³	y(b)	0.62500 in		
Zx	0.11421 in ³	Height	1.25000 in		
Iy	0.06008 in ⁴	ry	0.48147 in	x _o	0.00000 in
Sy(l)	0.09613 in ³	x(l)	0.62500 in	y _o	0.00000 in
Sy(r)	0.09613 in ³	x(r)	0.62500 in	jx	0.00000 in
Zy	0.11421 in ³	Width	1.25000 in	jy	0.00000 in

Section: 15 - Hand Rails Top Rails Section 1 6'.cfss
 Tube 1.25x1.25-16 Gage

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I_1	0.06008 in ⁴	r_1	0.48147 in	Cw	0.000009 in ⁶
I_2	0.06008 in ⁴	r_2	0.48147 in	J	0.098740 in ⁴
I_c	0.12017 in ⁴	r_c	0.68090 in		
I_o	0.12017 in ⁴	r_o	0.68090 in		

Fully Braced Strength - AISI S100-24, US, ASD

Material Type: A653 SS Grade 33, Fy=33 ksi

Axial		Positive Bending		Positive Bending	
Pao	4.7517 k	Maxo	1.8996 k-in	Mayo	1.8996 k-in
Ae	0.25919 in ²	Ixe	0.060083 in ⁴	Iye	0.060083 in ⁴
Ta	5.1216 k	Sxe(t)	0.096133 in ³	Sye(l)	0.096133 in ³
		Sxe(b)	0.096133 in ³	Sye(r)	0.096133 in ³
Shear		Negative Bending		Negative Bending	
Vay	1.2978 k	Maxo	1.8996 k-in	Mayo	1.8996 k-in
Vax	1.2978 k	Ixe	0.060083 in ⁴	Iye	0.060083 in ⁴
Torsion		Sxe(t)	0.096133 in ³	Sye(l)	0.096133 in ³
Ba	0.018425 k-in ²	Sxe(b)	0.096133 in ³	Sye(r)	0.096133 in ³

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Analysis Inputs

General

Member Orientation: Horizontal
 Calculate global buckling using specification equations
 Do not include torsion in member checks

Members

Section File Revision Date and Time
 1 15 - Hand Rails Top Rails Section 1 6'.cfss2/20/2023 6:23:44 PM

Material	Area (in ²)	Length (ft)	Weight (k)
1 A653 SS Grade 33	0.25919	6.0000	0.0052874
Total		6.0000	0.0052874

Start Loc. (ft)	End Loc. (ft)	Braced Flange	R	kφ (k)	Lm (ft)	ex (in)	ey (in)
1 0.0000	6.0000	None	0.0000	0.0000	6.0000	0.000	0.000

Supports

Type	Location (ft)	Bearing (in)	Fastened	K
1 XYT	0.0000	2.00	No	1.0000
2 XYT	6.0000	2.00	No	1.0000

Loading: Dead Load

Type	Angle (deg)	Start Loc. (ft)	End Loc. (ft)	Start Magnitude	End Magnitude	
1 Distributed	90.000	0.0000	6.0000	-0.002500	-0.002500	k/ft

Loading: Live Load

Type	Angle (deg)	Start Loc. (ft)	End Loc. (ft)	Start Magnitude	End Magnitude	
1 Distributed	90.000	0.0000	6.0000	-0.050000	-0.050000	k/ft

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Load Combination: D
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000

Load Combination: D+L
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Live Load	1.000
4 Product Load	1.000

Load Combination: D+0.75(L+Lr)
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Live Load	0.750
4 Product Load	0.750
5 Roof Live Load	0.750

Load Combination: 0.6D+0.6W
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	0.600
2 Dead Load	0.600
3 Wind Load	0.600

Load Combination: D+0.7E
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Earthquake Load	0.700

Load Combination: D+0.525E+0.75L+0.1S
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Earthquake Load	0.525
4 Live Load	0.750
5 Product Load	0.750
6 Snow Load	0.100

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Load Combination: 0.6D+0.7E
 Specification: AISI S100-24, US, ASD
 Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	0.600
2 Dead Load	0.600
3 Earthquake Load	0.700

Member Check - AISI S100-24, US, ASD

Load Combination: D+L

Design Parameters at 3.0000 ft:

Lx	6.0000 ft	Ly	6.0000 ft	Lt	6.0000 ft
Kx	1.0000	Ky	1.0000	Kt	1.0000

Section: 15 - Hand Rails Top Rails Section 1 6'.cfss

Material Type: A653 SS Grade 33, Fy=33 ksi

Cbx	1.1364	Cby	1.0000	ex	0.0000 in
Cmx	1.0000	Cmy	1.0000	ey	0.0000 in
Braced Flange: None		kφ	0 k		
Red. Factor, R: 0		Lm	6.0000 ft		

Loads:	P (k)	Mx (k-in)	Vy (k)	My (k-in)	Vx (k)
Total	0.0000	2.8826	0.0000	0.0000	0.0000
Applied	0.0000	2.8826	0.0000	0.0000	0.0000
Strength	1.6441	1.8996	1.2978	1.8996	1.2978

Interaction Equations

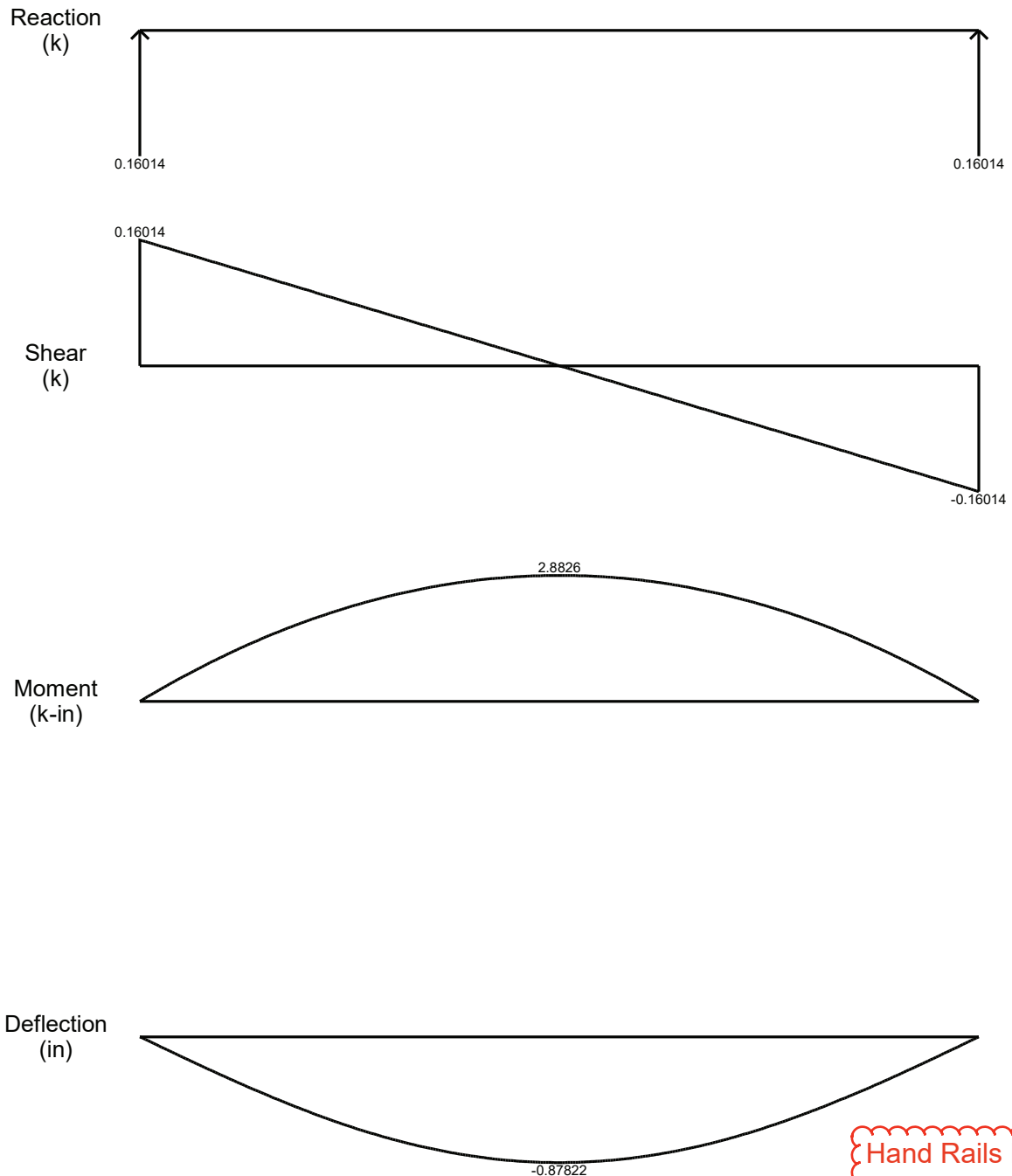
Eq. H1.2-1	(P, Mx, My)	$0.000 + 1.517 + 0.000 = 1.517 > 1.0$
Eq. H2-1	(Mx, Vy)	$\text{Sqrt}(2.303 + 0.000) = 1.517 > 1.0$
Eq. H2-1	(My, Vx)	$\text{Sqrt}(0.000 + 0.000) = 0.000 \leq 1.0$

Hand Rails Proven
 By Testing

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Item 24: 6 ft Hand Rails Top Rails

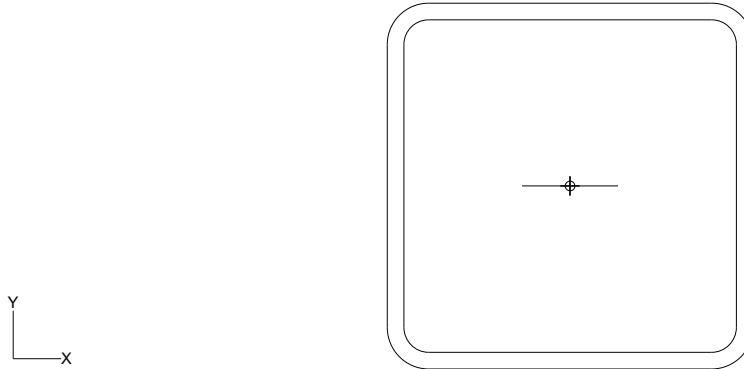
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Load Combination: D+L, Y Direction

Hand Rails Proven
By Testing

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By: Sal
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Item 17 and 24: 5 ft Hand Rails Top Rails



Section Inputs

Material: A653 SS Grade 33
No cold work of forming strength increase.
No inelastic reserve strength increase.
Modulus of Elasticity, E 29500 ksi
Yield Strength, Fy 33 ksi
Tensile Strength, Fu 45 ksi
Min Elongation in 2 inches 20 %
Torsion Constant Override, J 0 in⁴
Warping Constant Override, Cw 0 in⁶

Tube, Thickness 0.0566 in (54 mil)

Placement of Part from Origin:

X to center of gravity 0 in

Y to center of gravity 0 in

Outside dimensions, Closed shape

	Length (in)	Angle (deg)	Radius (in)	Web	k Coef.	Hole Size (in)	Distance (in)
1	1.2500	0.000	0.084900	Single	0.000	0.0000	0.6250
2	1.2500	90.000	0.084900	Single	0.000	0.0000	0.6250
3	1.2500	180.000	0.084900	Single	0.000	0.0000	0.6250
4	1.2500	-90.000	0.084900	Single	0.000	0.0000	0.6250

Full Section Properties

Area	0.25919 in ²	Wt.	0.00088123 k/ft	Flat	4.5793 in
Ix	0.06008 in ⁴	rx	0.48147 in	Ixy	0.00000 in ⁴
Sx(t)	0.09613 in ³	y(t)	0.62500 in	α	0.000 deg
Sx(b)	0.09613 in ³	y(b)	0.62500 in		
Zx	0.11421 in ³	Height	1.25000 in		
Iy	0.06008 in ⁴	ry	0.48147 in	x _o	0.00000 in
Sy(l)	0.09613 in ³	x(l)	0.62500 in	y _o	0.00000 in
Sy(r)	0.09613 in ³	x(r)	0.62500 in	jx	0.00000 in
Zy	0.11421 in ³	Width	1.25000 in	jy	0.00000 in

Section: 16 - Square Handrail Posts Section 3.cfss
 Tube 1.25x1.25-16 Gage

Rev. Date: 2/18/2023 6:48:30 AM

By: Sal

Printed: 2/13/2026 12:37:08 PM

I_1	0.06008 in ⁴	r_1	0.48147 in	Cw	0.000009 in ⁶
I_2	0.06008 in ⁴	r_2	0.48147 in	J	0.098740 in ⁴
I_c	0.12017 in ⁴	r_c	0.68090 in		
I_o	0.12017 in ⁴	r_o	0.68090 in		

Fully Braced Strength - AISI S100-24, US, ASD

Material Type: A653 SS Grade 33, Fy=33 ksi

Axial		Positive Bending		Positive Bending	
Pao	4.7517 k	Maxo	1.8996 k-in	Mayo	1.8996 k-in
Ae	0.25919 in ²	Ixe	0.060083 in ⁴	Iye	0.060083 in ⁴
Ta	5.1216 k	Sxe(t)	0.096133 in ³	Sye(l)	0.096133 in ³
		Sxe(b)	0.096133 in ³	Sye(r)	0.096133 in ³
Shear		Negative Bending		Negative Bending	
Vay	1.2978 k	Maxo	1.8996 k-in	Mayo	1.8996 k-in
Vax	1.2978 k	Ixe	0.060083 in ⁴	Iye	0.060083 in ⁴
Torsion		Sxe(t)	0.096133 in ³	Sye(l)	0.096133 in ³
Ba	0.018425 k-in ²	Sxe(b)	0.096133 in ³	Sye(r)	0.096133 in ³

Rev. Date: 2/13/2026 12:35:55 PM

Printed: 2/13/2026 12:37:10 PM



Analysis Inputs

General

Member Orientation: Horizontal
 Calculate global buckling using specification equations
 Do not include torsion in member checks

Members

Section File				Revision Date and Time				
1	16 - Square Handrail Posts			Section 3.cfss	2/18/2023 6:48:30 AM			
	Material	Area	Length	Weight				
		(in²)	(ft)	(k)				
1	A653 SS Grade 33	0.25919	15.000	0.013218				
	Total		15.000	0.013218				
	Start Loc.	End Loc.	Braced	R	kφ	Lm	ex	ey
	(ft)	(ft)	Flange		(k)	(ft)	(in)	(in)
1	0.000	15.000	None	0.0000	0.0000	15.000	0.000	0.000

Supports

Type	Location (ft)	Bearing (in)	Fastened	K
1 XYT	0.000	2.00	No	1.0000
2 XYT	5.000	2.00	No	1.0000
3 XYT	10.000	2.00	No	1.0000
4 XYT	15.000	2.00	No	1.0000

Loading: Dead Load

Type	Angle (deg)	Start Loc. (ft)	End Loc. (ft)	Start Magnitude	End Magnitude
1 Distributed	90.000	0.000	15.000	-0.002500	-0.002500 k/ft

Loading: Live Load

Type	Angle (deg)	Start Loc. (ft)	End Loc. (ft)	Start Magnitude	End Magnitude
1 Distributed	90.000	0.000	15.000	-0.050000	-0.050000 k/ft

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Load Combination: D
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000

Load Combination: D+L
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Live Load	1.000
4 Product Load	1.000

Load Combination: D+0.75(L+Lr)
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Live Load	0.750
4 Product Load	0.750
5 Roof Live Load	0.750

Load Combination: 0.6D+0.6W
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	0.600
2 Dead Load	0.600
3 Wind Load	0.600

Load Combination: D+0.7E
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Earthquake Load	0.700

Load Combination: D+0.525E+0.75L+0.1S
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	1.000
2 Dead Load	1.000
3 Earthquake Load	0.525
4 Live Load	0.750
5 Product Load	0.750
6 Snow Load	0.100

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Load Combination: 0.6D+0.7E
Specification: AISI S100-24, US, ASD
Inflection Point Bracing: No

Loading	Factor
1 Beam Self Weight	0.600
2 Dead Load	0.600
3 Earthquake Load	0.700

Member Check - AISI S100-24, US, ASD

Load Combination: D+L

Design Parameters at 5.0000 ft, Left side:

Lx	5.0000 ft	Ly	5.0000 ft	Lt	5.0000 ft
Kx	1.0000	Ky	1.0000	Kt	1.0000

Section: 16 - Square Handrail Posts Section 3.cfss

Material Type: A653 SS Grade 33, Fy=33 ksi

Cbx	1.5385	Cby	1.0000	ex	0.0000 in
Cmx	1.0000	Cmy	1.0000	ey	0.0000 in

Braced Flange: None	kφ	0 k
Red. Factor, R: 0	Lm	15.0000 ft

Loads:	P (k)	Mx (k-in)	Vy (k)	My (k-in)	Vx (k)
Total	0.0000	-1.6014	-0.1601	0.0000	0.0000
Applied	0.0000	-1.6014	-0.1601	0.0000	0.0000
Strength	2.2746	1.8996	1.2978	1.8996	1.2978

Interaction Equations

Eq. H1.2-1 (P, Mx, My) $0.000 + 0.843 + 0.000 = 0.843 \leq 1.0$
Eq. H2-1 (Mx, Vy) $\text{Sqrt}(0.711 + 0.015) = 0.852 \leq 1.0$
Eq. H2-1 (My, Vx) $\text{Sqrt}(0.000 + 0.000) = 0.000 \leq 1.0$

Web Crippling Check - AISI S100-24, US, ASD

Load Combination: D+L

Parameters at 5.000 ft:

Total Load: 0.29360 k on bottom flange

Total Moment: -1.6014 k-in

Bearing: 2.0000 in

Flange fastened to bearing surface: No

Distance from edge of bearing to edge of opposite load: 15.000 ft

Section: 16 - Square Handrail Posts Section 3.cfss

Material Type: A653 SS Grade 33, Fy=33 ksi

Applied Load: 0.2936 k on bottom flange

Applied Moment: -1.6014 k-in

Distance from edge of bearing to end of member: 4.9167 ft

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Part	Elem	Calculation Type	Pa (k)	Pay (k)	Notes
1	2	Single, US-IOF	0.8670	0.8670	N/h>2
1	4	Single, US-IOF	0.8670	0.8670	N/h>2
		Web Crippling Strength		1.7340	

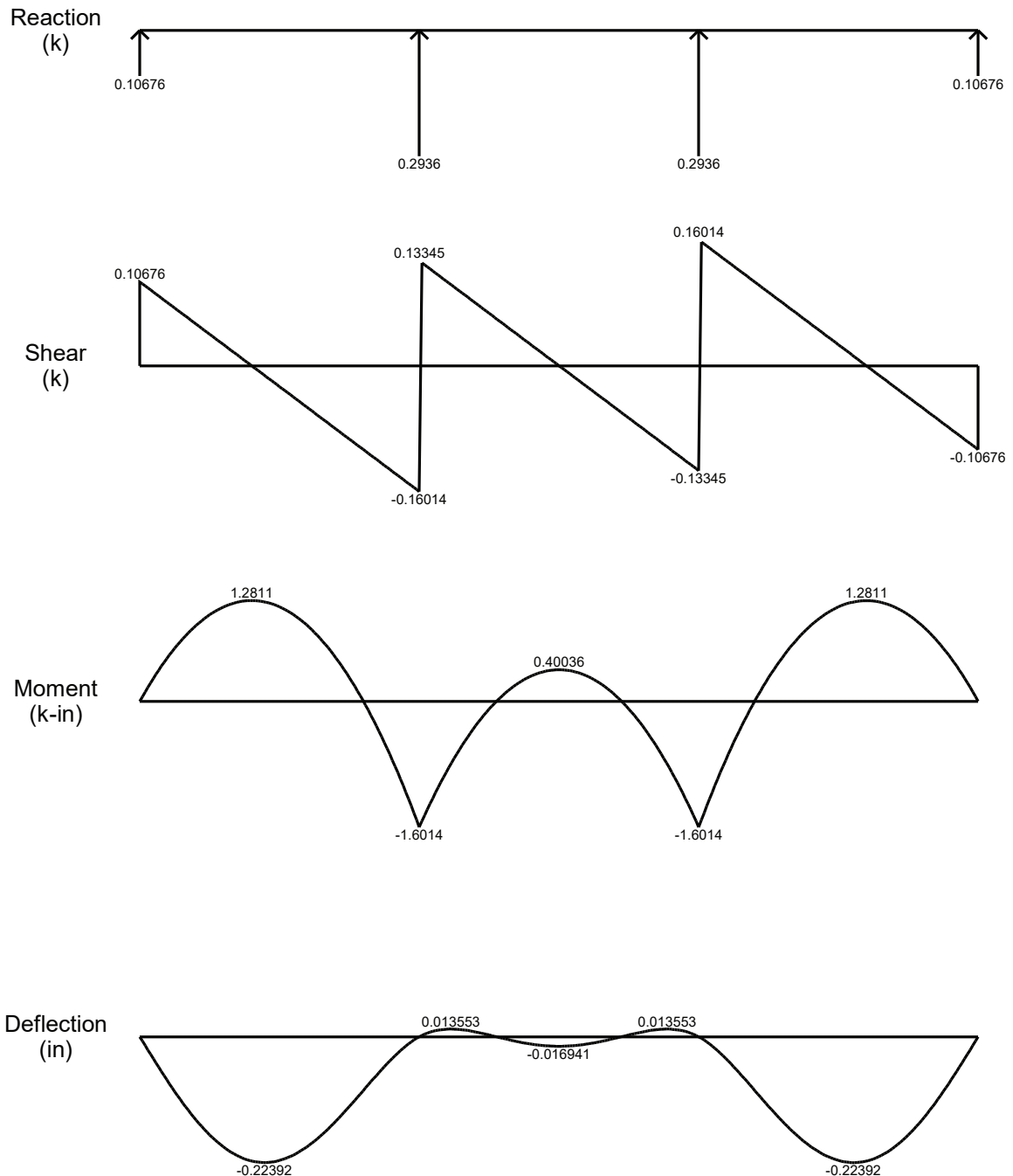
Web Crippling Check: 0.2936 k <= 1.7340 k
Moment Check: 1.6014 k-in <= 1.8996 k-in
Interaction Equations
Eq. H3-1a (P, M) $0.077 + 0.505 = 0.582 <= 0.665$

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Item 17 and 24: 5 ft Hand Rails Top Rails

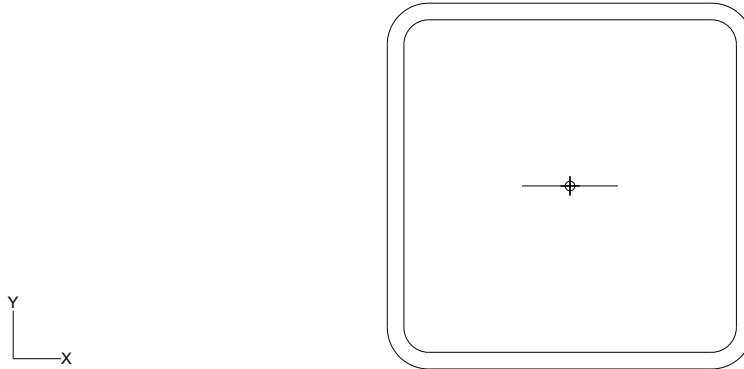
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Load Combination: D+L, Y Direction



Handrail Allowable Deflection $h/12 = 60" / 12 = 5.0"$ per ASTM E985

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By: Sal
Printed: 2/13/2026 12:48:55 PM



Section Inputs

Material: A653 SS Grade 33
No cold work of forming strength increase.
No inelastic reserve strength increase.
Modulus of Elasticity, E 29500 ksi
Yield Strength, Fy 33 ksi
Tensile Strength, Fu 45 ksi
Min Elongation in 2 inches 20 %
Torsion Constant Override, J 0 in⁴
Warping Constant Override, Cw 0 in⁶

Tube, Thickness 0.0566 in (54 mil)

Placement of Part from Origin:

X to center of gravity 0 in

Y to center of gravity 0 in

Outside dimensions, Closed shape

	Length (in)	Angle (deg)	Radius (in)	Web	k Coef.	Hole Size (in)	Distance (in)
1	1.2500	0.000	0.084900	Single	0.000	0.0000	0.6250
2	1.2500	90.000	0.084900	Single	0.000	0.0000	0.6250
3	1.2500	180.000	0.084900	Single	0.000	0.0000	0.6250
4	1.2500	-90.000	0.084900	Single	0.000	0.0000	0.6250

Full Section Properties

Area	0.25919 in ²	Wt.	0.00088123 k/ft	Flat	4.5793 in
Ix	0.06008 in ⁴	rx	0.48147 in	Ixy	0.00000 in ⁴
Sx(t)	0.09613 in ³	y(t)	0.62500 in	α	0.000 deg
Sx(b)	0.09613 in ³	y(b)	0.62500 in		
Zx	0.11421 in ³	Height	1.25000 in		
Iy	0.06008 in ⁴	ry	0.48147 in	x _o	0.00000 in
Sy(l)	0.09613 in ³	x(l)	0.62500 in	y _o	0.00000 in
Sy(r)	0.09613 in ³	x(r)	0.62500 in	jx	0.00000 in
Zy	0.11421 in ³	Width	1.25000 in	jy	0.00000 in

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I_1	0.06008 in ⁴	r_1	0.48147 in	Cw	0.000009 in ⁶
I_2	0.06008 in ⁴	r_2	0.48147 in	J	0.098740 in ⁴
I_c	0.12017 in ⁴	r_c	0.68090 in		
I_o	0.12017 in ⁴	r_o	0.68090 in		

Fully Braced Strength - AISI S100-24, US, ASD

Material Type: A653 SS Grade 33, Fy=33 ksi

Axial		Positive Bending		Positive Bending	
Pao	4.7517 k	Maxo	1.8996 k-in	Mayo	1.8996 k-in
Ae	0.25919 in ²	Ixe	0.060083 in ⁴	Iye	0.060083 in ⁴
Ta	5.1216 k	Sxe(t)	0.096133 in ³	Sye(l)	0.096133 in ³
		Sxe(b)	0.096133 in ³	Sye(r)	0.096133 in ³
Shear		Negative Bending		Negative Bending	
Vay	1.2978 k	Maxo	1.8996 k-in	Mayo	1.8996 k-in
Vax	1.2978 k	Ixe	0.060083 in ⁴	Iye	0.060083 in ⁴
Torsion		Sxe(t)	0.096133 in ³	Sye(l)	0.096133 in ³
Ba	0.018425 k-in ²	Sxe(b)	0.096133 in ³	Sye(r)	0.096133 in ³

Square Handrail Posts

Handrail Post Stress : $M = 3.5' * 12" * 0.2 \text{ kips} = 8.4 \text{ in} - \text{kips}$

$f_b = M / S = 0.82 * 8.4 \text{ in} - \text{kips} / 0.096133 \text{ in}^3 = 72 \text{ ksi}$

$F_b = 0.66 * 50 \text{ ksi} = 33 \text{ ksi}$ Say OK Based on Testing

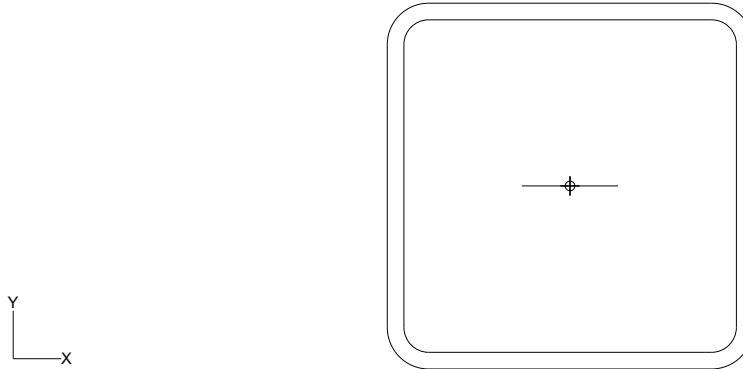
Handrail Deflection :

Deflection : $d = p * L^3 / (3 * E * I)$

Deflection : $d = 0.82 * 200 \text{ lbs} * (42")^3 / (3 * 29,000,000 \text{ psi} * 0.060083 \text{ in}^4) = 2.32" \text{ OK}$

Handrail Allowable Deflection $h/12 = 42" / 12 = 3.5" \text{ per ASTM E985}$

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By: Sal
Printed: 2/13/2026 12:50:51 PM



Section Inputs

Material: A653 SS Grade 33
No cold work of forming strength increase.
No inelastic reserve strength increase.
Modulus of Elasticity, E 29500 ksi
Yield Strength, Fy 33 ksi
Tensile Strength, Fu 45 ksi
Min Elongation in 2 inches 20 %
Torsion Constant Override, J 0 in⁴
Warping Constant Override, Cw 0 in⁶

Tube, Thickness 0.0566 in (54 mil)

Placement of Part from Origin:

X to center of gravity 0 in

Y to center of gravity 0 in

Outside dimensions, Closed shape

	Length (in)	Angle (deg)	Radius (in)	Web	k Coef.	Hole Size (in)	Distance (in)
1	1.2500	0.000	0.084900	Single	0.000	0.0000	0.6250
2	1.2500	90.000	0.084900	Single	0.000	0.0000	0.6250
3	1.2500	180.000	0.084900	Single	0.000	0.0000	0.6250
4	1.2500	-90.000	0.084900	Single	0.000	0.0000	0.6250

Full Section Properties

Area	0.25919 in ²	Wt.	0.00088123 k/ft	Flat	4.5793 in
Ix	0.06008 in ⁴	rx	0.48147 in	Ixy	0.00000 in ⁴
Sx(t)	0.09613 in ³	y(t)	0.62500 in	α	0.000 deg
Sx(b)	0.09613 in ³	y(b)	0.62500 in		
Zx	0.11421 in ³	Height	1.25000 in		
Iy	0.06008 in ⁴	ry	0.48147 in	x _o	0.00000 in
Sy(l)	0.09613 in ³	x(l)	0.62500 in	y _o	0.00000 in
Sy(r)	0.09613 in ³	x(r)	0.62500 in	jx	0.00000 in
Zy	0.11421 in ³	Width	1.25000 in	jy	0.00000 in

Rev. Date: 2/20/2023 6:23:44 PM
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I ₁	0.06008 in ⁴	r ₁	0.48147 in	Cw	0.000009 in ⁶
I ₂	0.06008 in ⁴	r ₂	0.48147 in	J	0.098740 in ⁴
I _c	0.12017 in ⁴	r _c	0.68090 in		
I _o	0.12017 in ⁴	r _o	0.68090 in		

Fully Braced Strength - AISI S100-24, US, ASD

Material Type: A653 SS Grade 33, Fy=33 ksi

Axial		Positive Bending		Positive Bending	
Pao	4.7517 k	Maxo	1.8996 k-in	Mayo	1.8996 k-in
Ae	0.25919 in ²	Ixe	0.060083 in ⁴	Iye	0.060083 in ⁴
Ta	5.1216 k	Sxe(t)	0.096133 in ³	Sye(l)	0.096133 in ³
		Sxe(b)	0.096133 in ³	Sye(r)	0.096133 in ³
Shear		Negative Bending		Negative Bending	
Vay	1.2978 k	Maxo	1.8996 k-in	Mayo	1.8996 k-in
Vax	1.2978 k	Ixe	0.060083 in ⁴	Iye	0.060083 in ⁴
Torsion		Sxe(t)	0.096133 in ³	Sye(l)	0.096133 in ³
Ba	0.018425 k-in ²	Sxe(b)	0.096133 in ³	Sye(r)	0.096133 in ³

Member Check - AISI S100-24, US, ASD

Material Type: A653 SS Grade 33, Fy=33 ksi

Design Parameters:

Lx	2.3330 ft	Ly	2.3330 ft	Lt	0.0000 ft
Kx	1.0000	Ky	1.0000	Kt	1.0000
Cbx	1.0000	Cby	1.0000	ex	0.0000 in
Cmx	1.0000	Cmy	1.0000	ey	0.0000 in
Braced Flange: None		kφ	0 k		
Red. Factor, R: 0		Lm	2.3330 ft		

Loads:	P	Mx	Vy	My	Vx
	(k)	(k-in)	(k)	(k-in)	(k)
Entered	0.7500	1.3720	0.0000	0.0000	0.0000
Applied	0.7500	1.4500	0.0000	0.0000	0.0000
Strength	4.0476	1.8996	1.2978	1.8996	1.2978

Interaction Equations

Eq. H1.2-1	(P, Mx, My)	0.185 + 0.763 + 0.000 = 0.949 <= 1.0
Eq. H2-1	(Mx, Vy)	Sqrt(0.583 + 0.000) = 0.763 <= 1.0
Eq. H2-1	(My, Vx)	Sqrt(0.000 + 0.000) = 0.000 <= 1.0

SPECIFICATIONS

Table 1: Footing Pad Allowable Loads

Pad Identification	Nominal Pad Diameter (in.)	Pad Capacity ¹ 1000 psf soil (lbf)	Pad Capacity ¹ 1500 psf soil (lbf)	Pad Capacity ¹ 3000 psf soil (lbf)
Model #10	10	545	810 ²	1622 ²
Model #16	16		2100 ²	4200 ²

¹ Pad capacity is limited by the lesser of the following criteria; the peak load divided by a safety factor of 3, the soil capacity, load at 3/4-in. deflection

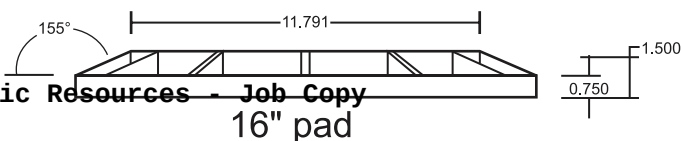
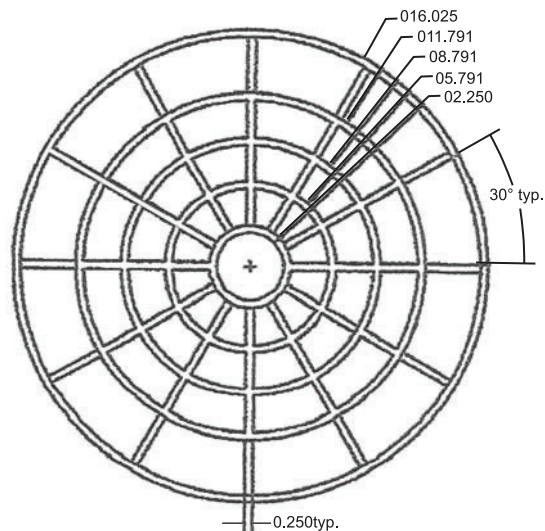
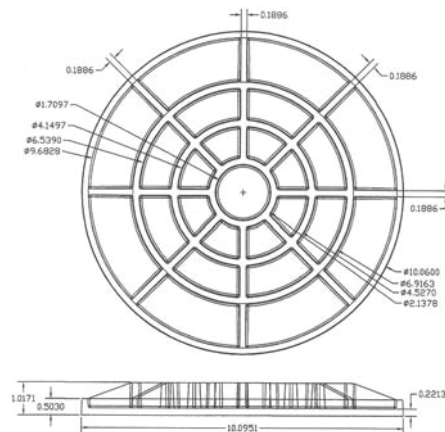
² Pad capacity is limited to soil capacity

Characteristics

Density 1.13g/cm
Stiffness 535.054 psi
Izod Impact 0.95 ft. -lb./in.
Gardner Impact 164 in.-lb.

Benefits:

- UV Stable
- Easy Installation
- Saves time & labor
- Strong and durable
- Eliminate deflection
- Made from 100% recycled materials
- Resists most herbicides and pesticides
- Replaces concrete
- Light weight saves on shipping & handling



For		Date	2/17/2026
At		By	Granata
Description		Checked	
		Page	

ACI 318-19 Section 22.6.5.2

ACI 318-14 Section 22.6.5.2

ACI 318-05 Section 11.12.4.8

Concrete Punching Shear :

Input :

fc' =	3,000	psi
Dead Load =	5	psf
Live Load =	100	psf
Trib : W =	2.2500	ft
Trib : L =	3.00	ft
Ftg Thick: d =	4.00	in
Plate : w =	4.00	in
Con. Wt: w _c =	150	pcf

$$\begin{aligned}
 \text{Trib Area : } A &= W * L = 6.75 \text{ sq. ft} \\
 \text{Dead Load : } D &= A * DL / 1000 = 0.03 \text{ kips} \\
 \text{Live Load : } L &= A * LL / 1000 = 0.68 \text{ kips} \\
 \text{Siesmic Sds} &= 2.460 \\
 \text{Siesmic Vert. Load : } E &= (0.25 * L + D) * 0.2 * Sds = 0.10 \text{ kips} \\
 \text{Total Load : } P &= D + L + 0.7 E = 0.78 \text{ kips} \\
 \text{Total Load : } Pu &= 1.2 * D + 1.6 * L + 1.0 * E = 1.22013 \text{ kips}
 \end{aligned}$$

$$\text{Critical Section : } b_o = 4 * (w + (2 * d / 2)) = 32.0 \text{ in}$$

$$\text{Slab Capacity: } V = \Phi V_n = 0.60 * (4 * \text{sqrt}(fc') * b_o * d) / 1000 = 16.8 \text{ kips} \quad \text{OK}$$

Soil Stress :

ACI 318-19 Section 22.8.3.2

$$\text{Fdn Area : } A_f = (w + (2 * d))^2 = 1.0000 \text{ ft}^2 = 144 \text{ in}^2$$

$$\text{Soil Stress} = P / A_f = 778 \text{ psf} \quad \text{ASD}$$

$$\text{Allowable Soil Stress} = 2600 \text{ psf}$$

Plate Thickness:

$$\begin{aligned}
 \text{Plate Stress : } S &= P / w^2 = 0.05 \text{ ksi} \\
 M &= S * w * ((w - 1.25)/2) = 0.27 \text{ in-kips} \\
 b &= w = 4.00 \text{ in} \\
 \text{Plate Thick: } t &= \text{sqrt}((6 * M) / (b * 0.75 * 36 \text{ ksi})) = 0.122 \text{ in}
 \end{aligned}$$

Use (1) 10 ga Minimum Plate Thickness

(1) 10 ga = 0.1242" Plate Thickness

0.98173 Stress

Concrete One-Way Shear :

ACI 318-19 Section 9.6.3.1

Input :

fc' =	3,000	psi
Dead Load =	5	psf
Live Load =	100	psf
Length (L) =	6.0	ft
Ftg Thick: d =	4.00	in
Con. Wt: w _c =	150	pcf

Assume simply supported slab & 12" width of slab

$$\begin{aligned}
 \text{Controlling Load Combination (w)} &= 1.2 * D + 1.6 * L = 0.17 \text{ klf} \\
 \text{Maximum Shear (Vu)} &= wL/2 = 0.50 \text{ k} \\
 \text{Estimate d} &= 4" - 0.75 - 0.5(.5) = 3.00 \text{ in} \\
 \text{Beam width (bw)} &= 12.00 \text{ in} \\
 \text{Resistance Factor (}\Phi\text{)} &= 0.6 \\
 \text{Nominal Shear Capacity (Vn)} &= \Lambda * \text{sqrt}(fc') * bw * d = 1.97 \text{ k} \\
 \text{Design Shear Capacity (}\Phi V_n\text{)} &= 1.18 \text{ k}
 \end{aligned}$$

$$\text{Verify Capacity: } Vu < \Phi V_n = 0.50 \text{ k} < 1.18 \text{ k} \quad \text{OK}$$

MIAMI-DADE COUNTY

DEPARTMENT OF REGULATORY AND ECONOMIC RESOURCES

<http://www.miamidade.gov/building/home.asp>

4/13/2026 12:35:37 PM

Contact Name: John Hench
Contact Email: JHENCH@CUMMING-GROUP.COM
Tracking Number: 2026011895
Reviews to rework: DERM,FIRE
Comments: Fire & RER Environmental Core Reviews

MIAMI-DADE COUNTY

DEPARTMENT OF REGULATORY AND ECONOMIC RESOURCES

<http://www.miamidade.gov/building/home.asp>

4/14/2026 3:18:07 PM

Contact Name: John Hench

Contact Email: JHENCH@CUMMING-GROUP.COM

Tracking Number: 2026011895

Reviews to rework: DERM,FIRE

Comments: Fire and RER Environmental Core Upload. Files uploaded in the (04.13.2026) submission do not appear to have been added.

MIAMI-DADE COUNTY

DEPARTMENT OF REGULATORY AND ECONOMIC RESOURCES

<http://www.miamidade.gov/building/home.asp>

4/7/2026 10:23:16 AM

Contact Name: John Hench

Contact Email: JHENCH@CUMMING-GROUP.COM

Tracking Number: 2026011895

Reviews to rework: PWIF,DPAD

Comments: Response to Comments submission for Impact Fees & Paving & Drainage.



Salvatore R. Granata. P.E., S.E.

Phone: (678) 367-3151
Cell: (770) 301-5817
Toll Free: (877) 346-5126

11509 Lodgepole Ct
Spanish Fort, AL 36527-6700

Fax: (866)815-4682
sal@srgpe.net
salgranata.com

Digital Certification Validation

Date: March 20, 2026

Project: # 26037 - Gulliver Prep Ramp - Pinecrest, FL

Sealed Documents : Rev 0 Calculations pages 1 thru 66
Rev 0 (5) Drawings pages S-0 thru S-3

This form is intended to provide certification to associate the engineer's seal and signature with the digital signatures on the Adobe PDF design documents listed above submitted for review on the above referenced project. Please note: a separate completed form is required for each digital signature generated for documents not listed above.

Engineer's Digital Certificate Fingerprint ID's

- MD5 Digest **7D AF 5D B1 79 86 D6 F5 83 65 07 E9 2D 15 0D 79**
- SHA1 Digest **BE 73 1C 6E EC 9B F0 4A D5 A3 36 0F C6 38 6D 27 01 BC 25 A1**

If you have any further questions, please do not hesitate to contact me using the information below.

Sincerely,

Salvatore (Sal) Ray Granata, P.E., S.E.
sal@srgpe.net
salgranata.com

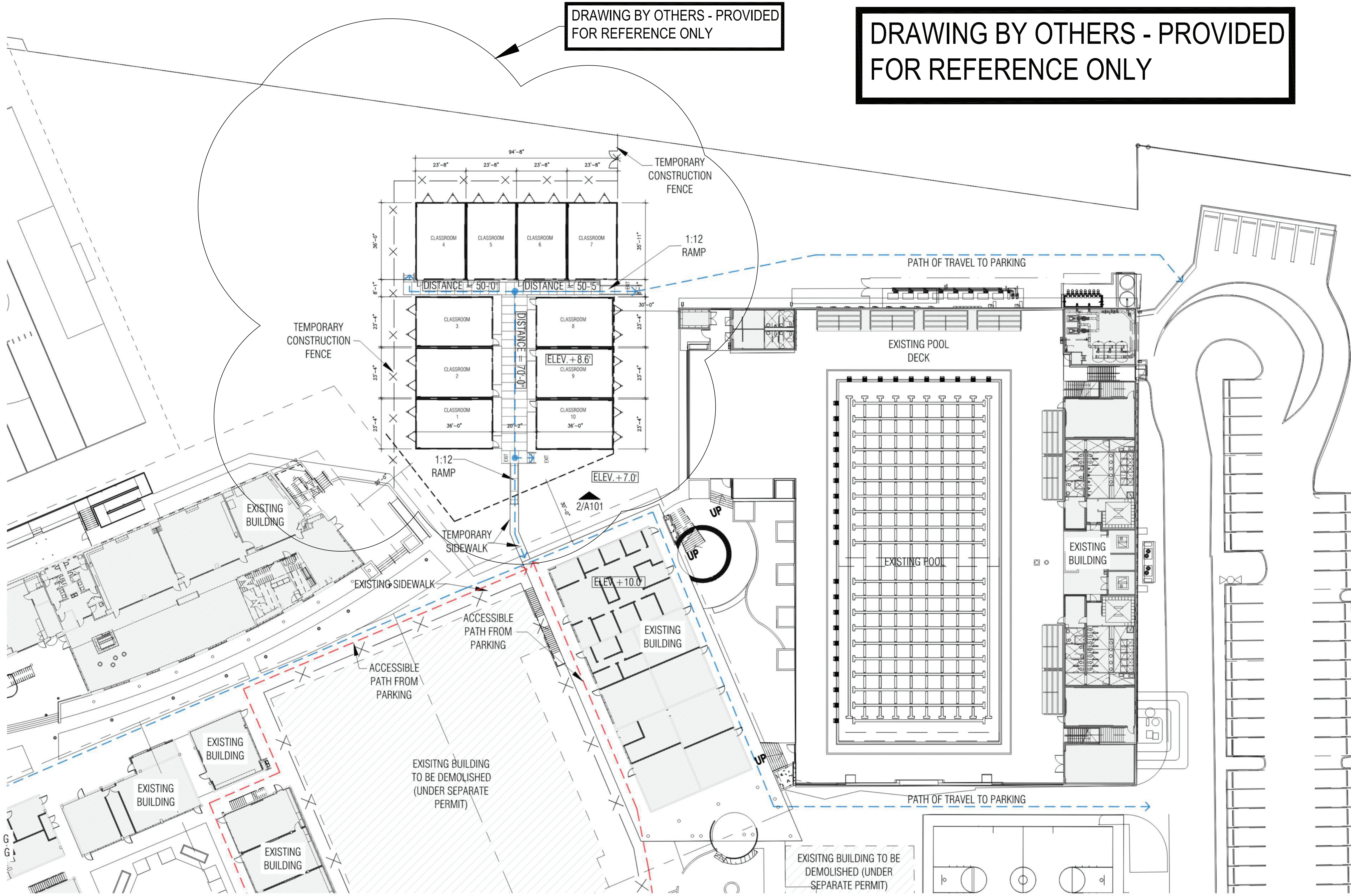
This document has been electronically signed and sealed by Salvatore Ray Granata Professional Engineer of the State of Florida, United States. Printed Copies of this document Are not considered signed and The signature must be verified on any electronic copies.



Salvatore
Ray Granata

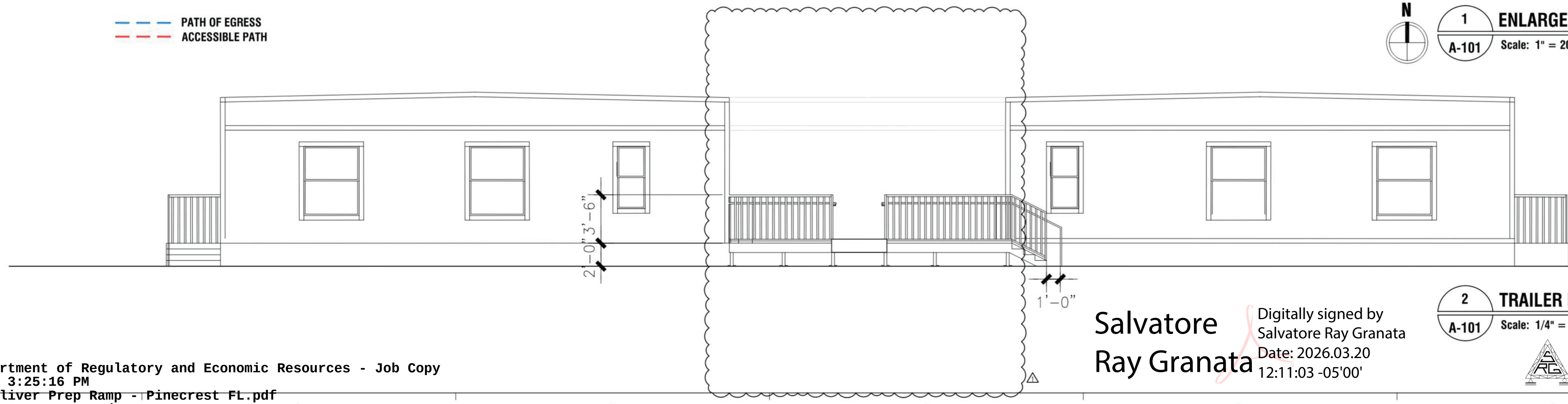
Digitally signed by
Salvatore Ray
Granata

Date: 2026.03.20
12:15:24 -05'00'



--- PATH OF EGRESS
--- ACCESSIBLE PATH

1 ENLARGED TRAILER SITE PLAN
A-101 Scale: 1" = 20'-0"



2 TRAILER ELEVATION
A-101 Scale: 1/4" = 1'-0"

Salvatore Ray Granata
Digitally signed by Salvatore Ray Granata
Date: 2026.03.20 12:11:03 -05'00'



PERMIT SUBMITTAL

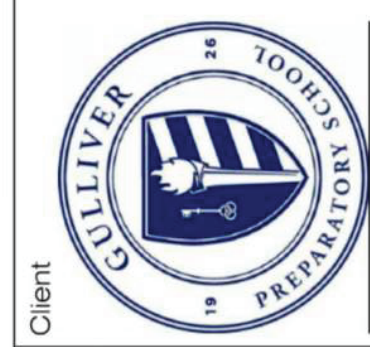
ENLARGED SITE PLAN AND ELEVATION

2025-0568-ASR1 02-18-2026

A-101

M/E/P Engineers
JLRD, CONSULTING ENGINEERS
1450 Centerville Blvd. Suite 350
West Palm Beach, FL 33411
(561) 882-2303 TEL

Project
GulliverPrep
TEMPORARY CLASSROOMS
6575 North Kendall Drive
PINECREST, FL 33156



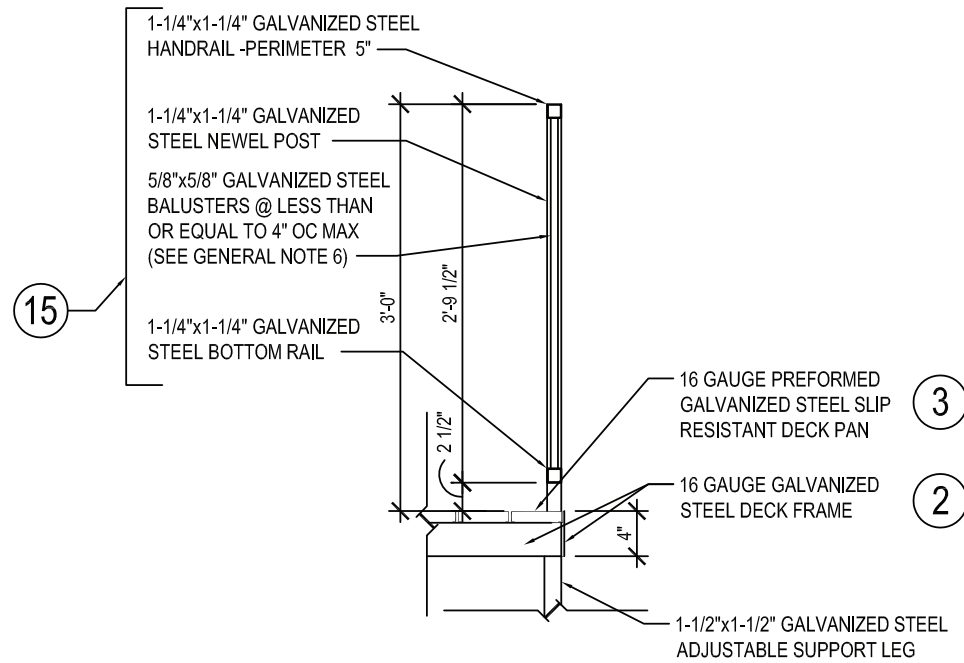
Architect
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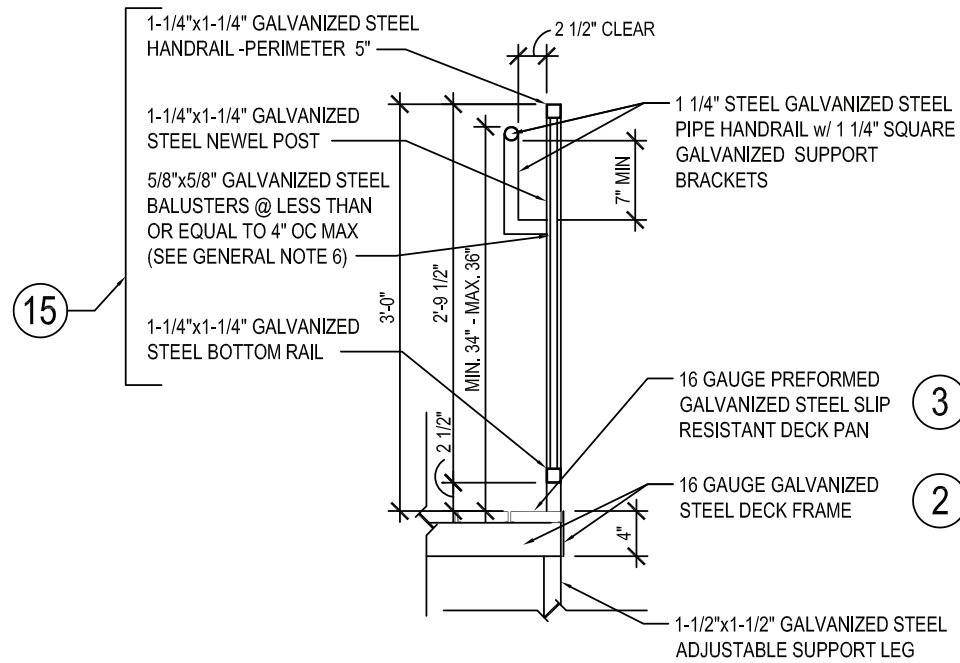
GENERAL NOTES

1. APPLICABLE CODES ARE AS FOLLOWS:
1.1. THE 2023 FLORIDA BUILDING CODE 8th EDITION - IBC 2021 AMMENDED
1.2. THE ADAG 2010 STANDARDS: TITLE II AND III.
1.3. NFPA 101
2. THE DRAWINGS SUBMITTED ARE REPRESENTATIVE OF THE "QUICK-DECK" TEMPORARY MODULAR PLATFORM SYSTEM. THE SYSTEM PROVIDES ACCESS TO TEMPORARY CONSTRUCTION TRAILERS AND/OR TEMPORARY MODULAR BUILDINGS. QUICK-DECK IS A PATENTED (H498610), PRE-ENGINEERED SYSTEM OF DECKING, RAMPS, STAIRS AND RAILINGS. QUICK-DECK IS DESIGNED TO BE TIED TOGETHER IN THE FIELD WITH NO ELECTRICAL POWER OR CONCRETE POURING.
3. ALL STRUCTURAL RAIL AND DECK COMPONENTS WILL BE GALVANIZED STEEL.
4. STRUCTURAL LOADS
4.1. LIVE LOAD MINIMUM 100 PSF.
4.2. HANDRAILS AND GUARDS SHALL BE ABLE TO RESIST A SINGLE CONCENTRATED LOAD OR 200 POUNDS, APPLIED IN ANY DIRECTION AT ANY POINT ALONG THE TOP AND TO TRANSFER THIS LOAD THROUGH THE SUPPORTS TO THE STRUCTURE. THIS LOAD NEED NOT TO BE ASSUMED TO ACT CONCURRENTLY WITH THE OTHER LOADS SPECIFIED.
4.3. INTERMEDIATE RAILS (ALL THOSE EXCEPT HANDRAIL), BALUSTERS AND PANEL FILLERS SHALL BE DESIGNED TO WITHSTAND A HORIZONTALLY APPLIED NORMAL LOAD OF 50 POUNDS ON AN AREA EQUAL TO 1 SQUARE FOOT, INCLUDING OPENINGS AND SPACE BETWEEN RAILS. THIS LOAD NEED NOT TO BE ASSUMED TO ACT CONCURRENTLY WITH THE OTHER LOADS SPECIFIED.
4.4. HANDRAILS ARE DESIGNED FOR A LOAD OF 50 POUNDS PER LINEAL FOOT.
5. ELECTRICAL GROUNDING - BY OTHERS AS REQUIRED BY CODE OR LOCAL REQUIREMENTS
6. STAIR TREADS - ALL STAIR TREADS TO HAVE A CONTRASTING NOSING MAXIMUM 1" FROM TREAD EDGE AND 2" WIDE.
7. HANDRAILS
7.1. 5/8" BALUSTERS @ 4" OC AND 1-1/4" SQUARE NEWEL POSTS RESULT IN THE HANDRAIL OBSTRUCTION BEING LESS THAN 17% LESS THAN THE 20% MAXIMUM OBSTRUCTION REQUIRED BY ANSI A-117.1.
7.2. THE 1-1/4"x1-1/4" SQUARE HANDRAIL COMPLIES WITH THE CODE MINIMUM 1-1/4" DIMENSION AND HAS A PERIMETER OF 5" WHICH IS LESS WITHIN THE CODE REQUIRED PERIMETER RANGE OF 4" TO 6-1/4".
7.3. HANDRAIL EXTENSIONS FOR RAMPS AND STAIRS TO BE 12" MINIMUM AT TOP AND BOTTOM OF RUNS. EXTENSION ANGLE AT BOTTOM OF STAIRS AND LANDING TO FOLLOW THE SLOPE PER CODE REQUIREMENTS.



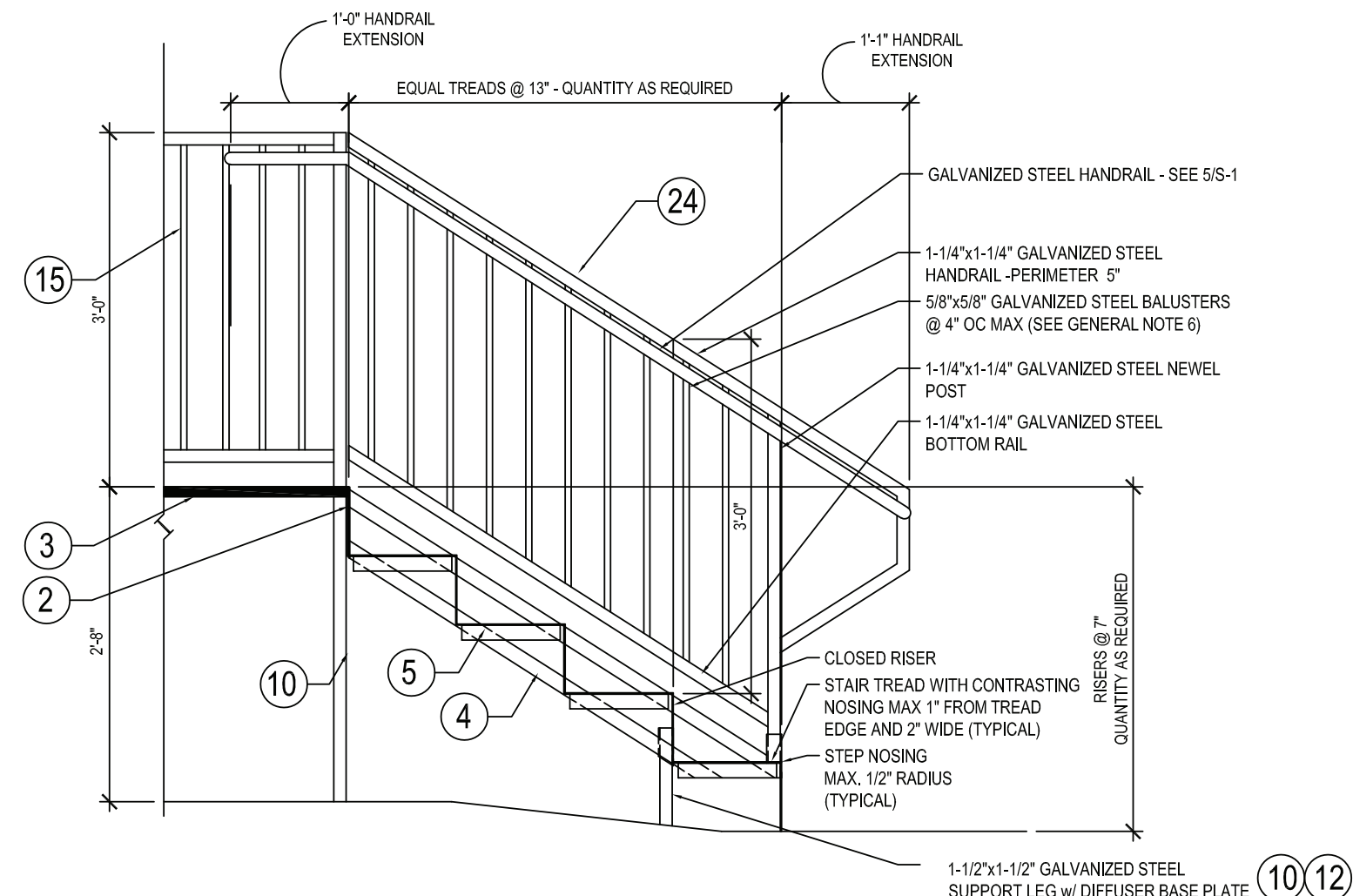
TYPICAL RAIL SECTION

SCALE: 3/4"=1'-0"



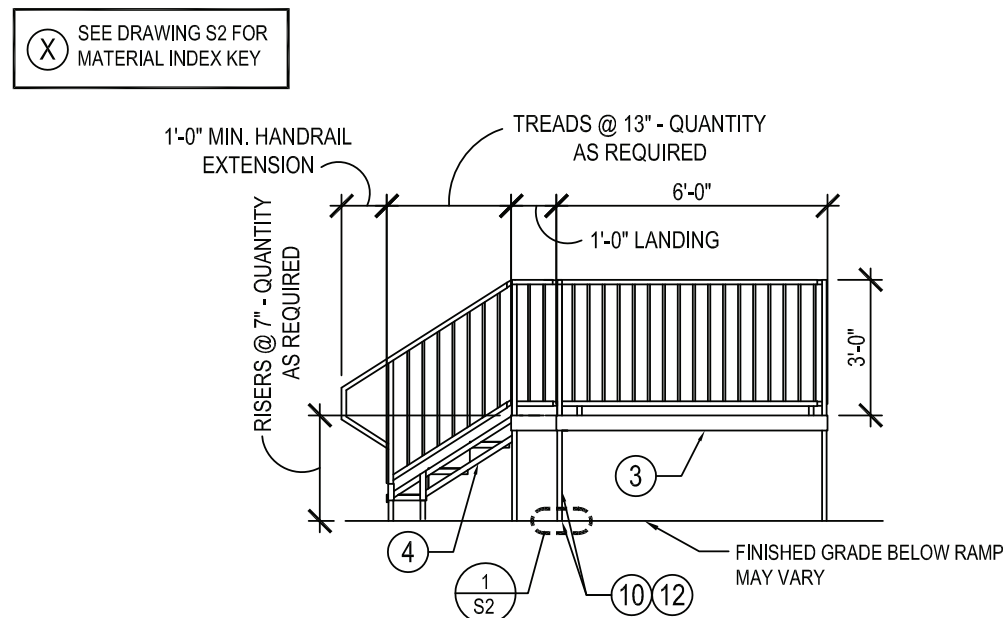
TYPICAL RAIL SECTION

SCALE: 3/4"=1'-0"



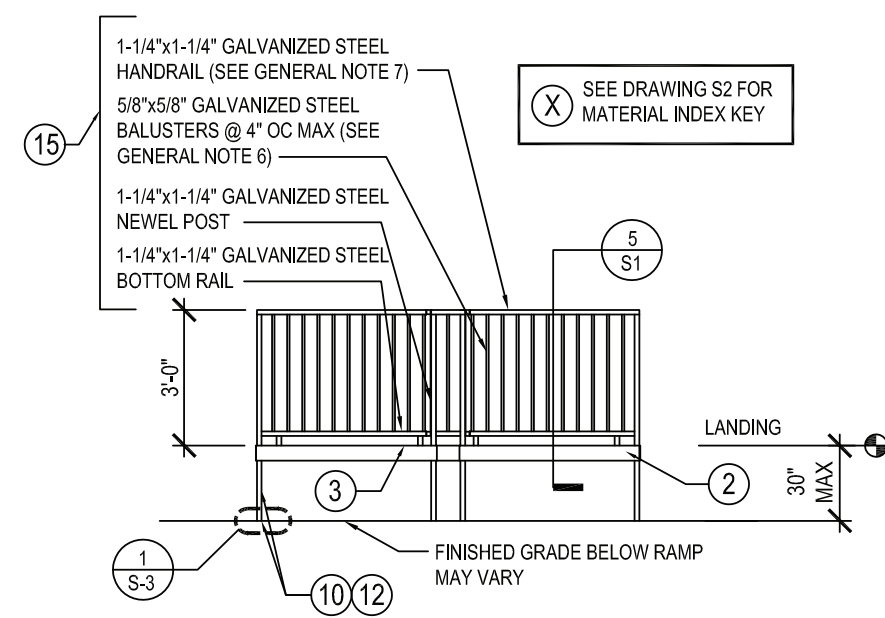
TYP. STAIR SECTION - GENERIC

SCALE: 3/4"=1'-0"



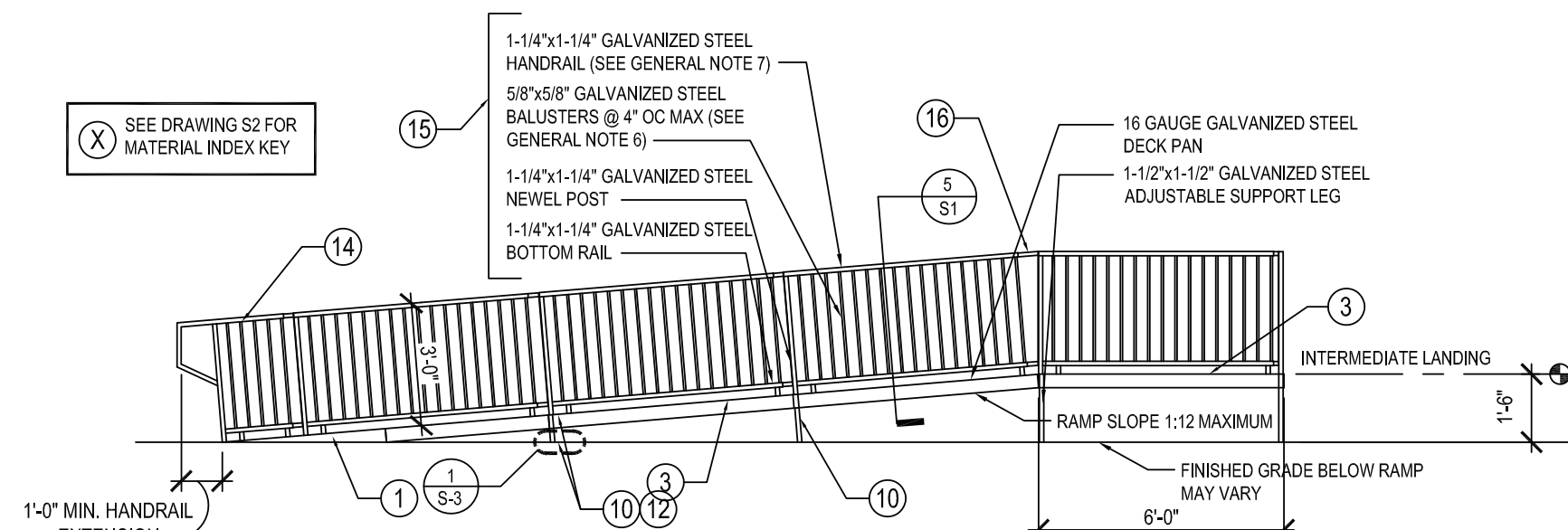
TYPICAL ELEVATION - GENERIC

SCALE: 1/4"=1'-0"



TYPICAL ELEVATION

SCALE: 1/4"=1'-0"



TYPICAL ELEVATION

SCALE: 1/4"=1'-0"

Salvatore Ray Granata

Digitally signed by Salvatore Ray Granata
Date: 2026.03.20 12:12:33 -05'00'

This document has been electronically signed and sealed by Salvatore Ray Granata Professional Engineer of the State of Florida, United States. Printed Copies of this document Are not considered signed and The signature must be verified on any electronic copies.



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Revisions:

Proj. No.:
Date: MARCH 17, 2026

Sheet Name:
Elevations and Details

S-1

MATERIAL INDEX: COMPONENTS AND SPECIFICATIONS

PART	DESCRIPTION	SIZE	MATERIAL	PART	DESCRIPTION	SIZE	MATERIAL	MATERIAL SPECIFICATIONS
1	WEDGE	12 GA	Hot Rolled Steel	18	GUARDRAIL RAM	12 GA	Hot Rolled Steel, 1/4"x.063 Steel Tube	STEEL Tubing ASTM 500. Flat, hot rolled, both clean and galvanize washed.
2	MAINFRAME - SEE 4/S2	16 GA	Hot Rolled Galvanized Steel	19	FRAME SPACER	16 GA	Hot Rolled Galvanized Steel	PLYWOOD 3/4" AP A rated, structural 3/4". Surfaced with a two part epoxy and .030 silica sand.
3	EMBOSSED SURFACE PANEL - SEE 2/S2	16 GA	Hot Rolled Galvanized Steel, 3/8" #16 Standard Expanded Metal	20	BOLT SET	5/8"	3/4" Stainless Steel	FASTENERS 3/8" x1"x2" Grade #2 plated.
4	STRINGER SECTION	12 GA	Hot Rolled Steel Tube 7/16-14 Nut	21	GROUND PAD	1/2"x1/2"	Greenwood	PAINT (RAIL/FRAME) Ela Industrial enamel. Used as protective coating on rails and frames.
5	STAIR TREAD	12 GA	Hot Rolled Steel	22	GROUND SPIKE	1/2" Length	3/4" Steel Rebar w/ weather stop welded to top end of bar	PAINT (SURFACING) Devon 2 part epoxy (#2901)
6	STANCHION	1/2"x1/2"x.063 Steel Tube		23	POST CAP	1/2"x1/2"	Plastic Polymer	WELDS All welds conform to criteria set forth by the American Welding Society.
7	GUARDRAIL STANCHION	1/2"x1/2"x.063 Steel Tube		24	STAIR RAIL	1/2"x1/2"x.063 Steel Tube, 3/8"x.063 Steel Tube, 7/16-14 Nut		
8	STANCHION EXTENSION	1/2"x1/2"x.063 Steel Tube		25	SURFACING CORNER CAP	1/2"x1/2"	16 GA Hot Rolled Steel, 1/4"x.063 Steel Tube	
9	SCREW JACK LEG	1/2"x1/2"x.063 Steel Tube		26				
10	SPEED LEG	1/2"x1/2"x.063 Steel Tube 7/16-14 Nut						
11	TUBE LEG	1/2"x1/2"x.063 Steel Tube						
12	DIFFUSER	12 GA	Hot Rolled Steel					
13	JACK SCREW	1" Diameter Stainless All-Thread Screw						
14	ENTRY RAIL	1/2"x1/2"x.063 Steel Tube, 3/8"x.063 Steel Tube, 7/16-14 Nut						
15	HANDRAIL	1/2"x1/2"x.063 Steel Tube, 3/8"x.063 Steel Tube, 7/16-14 Nut						
16	RAIL RAM	12 GA	Hot Rolled Steel, 1/4"x.063 Steel Tube					
17	GUARDRAIL	1/2"x1/2"x.063 Steel Tube, 1/4"x.063 Steel Tube, 7/16-14 Nut						

SYSTEM COMPONENTS

LEG FOOTINGS
Refer to S-2 for "Foundation Options" and to S-3 for anchoring.

LEGS
Legs come in increments of 12" up to 8 ft. and have 12" of adjustability. Legs are 1 1/2"x1 1/2" tubing with a .063 wall. The legs lock into the frame and adjust to the ground or slope required using a pressure lock system.

SCREW JACK LEGS
Screw Jack Legs are adjustable and require no safety features. Used for decking under 8" high.

FRAMES
Frames are welded together. Multiple frames are bolted together to create a ramp segment and/or deck. Frames come in three sizes: 48"x48", 48"x72", and 72"x72". Parts are pre-galvanized 16 gauge. Cross beam centers are 30" or less.

SPACERS
Spacers bolt to the main frame. Pre-galvanized 16 gauge. Spacers come in two sizes: 6"x48" and 6"x72". Spacers make it possible to separate the leg in a switchback configuration and create odd size landings.

HANDRAILS & GUARDRAILS
Rails come in 2", 4", and 6" lengths. All railing is designed to take 200 lb. lateral load in any direction. Handrails are fastened 36" above the finished ramp/deck surface. Guardrails are fastened 42" above the finished ramp/deck surface. Guardrails are to be installed at all locations where the ramp, landing, or deck is more than 30" above grade.

WEDGE
The wedge is a 12 gauge steel transition piece between the ground and the first mainframe. It has a non-slip surface composed of a two-part epoxy and .030 silica sand.

STAIRS
All steel components. Stair sections are 44" wide. Each riser is 7" high. Each tread is 12" deep. 36" high handrails are provided with a tambo-tongue return at the entrance. All treads are surfaced with a non-slip surfacing. The only tread that is adjustable is the bottom tread. If closures for the step are needed, please specify prior to installation. Ideally, the height of the structure would be a multiple of 7". Thus, a platform height of 42"=6 risers & 5 treads.

BOTTOM RISER
Customer is responsible for the height of the bottom tread of the Quick-Deck stair system. If the height doesn't fall as a multiple of 7" (ex. 14", 21", 28"), you will have to dig away or build up the ground to make the bottom tread 7" high.

TYPICAL STRINGER - PART 4

SCALE: NTS

MAIN FRAME SHOP DRAWING - PART 2

SCALE: NTS

TYPICAL STRINGER HANGER

SCALE: NTS

TYPICAL SURFACE PANEL SHOP DRAWING - PART 3

SCALE: NTS

TYPICAL STRINGER - PART 5

SCALE: NTS

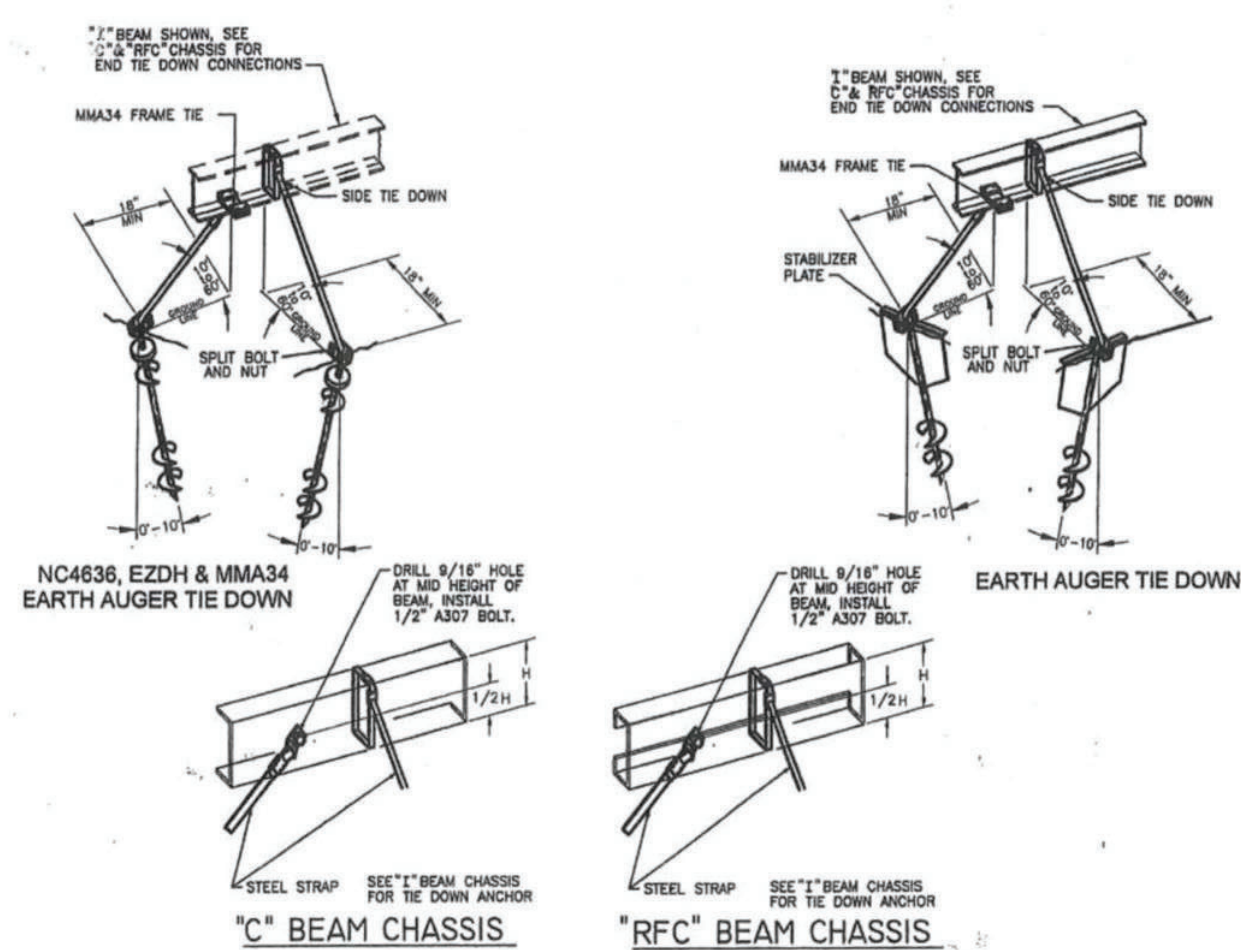
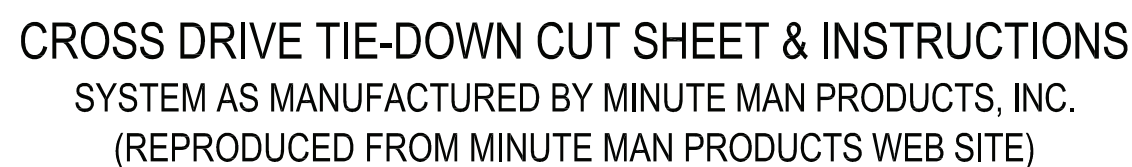
TYPICAL SURFACE PANEL PHOTO - PART 3

SCALE: 1/4"=1'-0"

Salvatore
Ray GranataDigitally signed by
Salvatore Ray
Granata
Date: 2026.03.20
12:13:15 -05'00'

Salvatore R. Granata P.E., S.E.

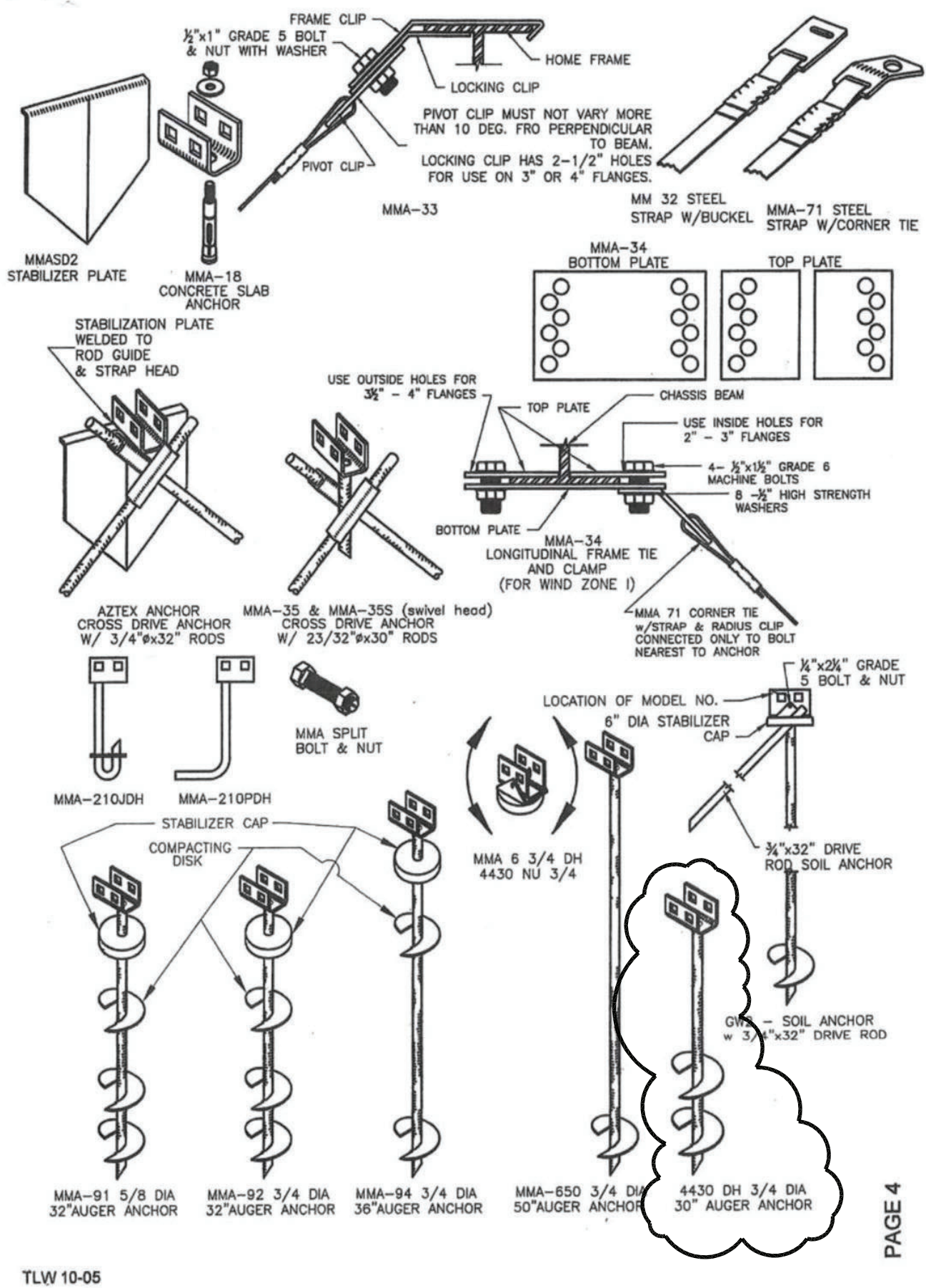
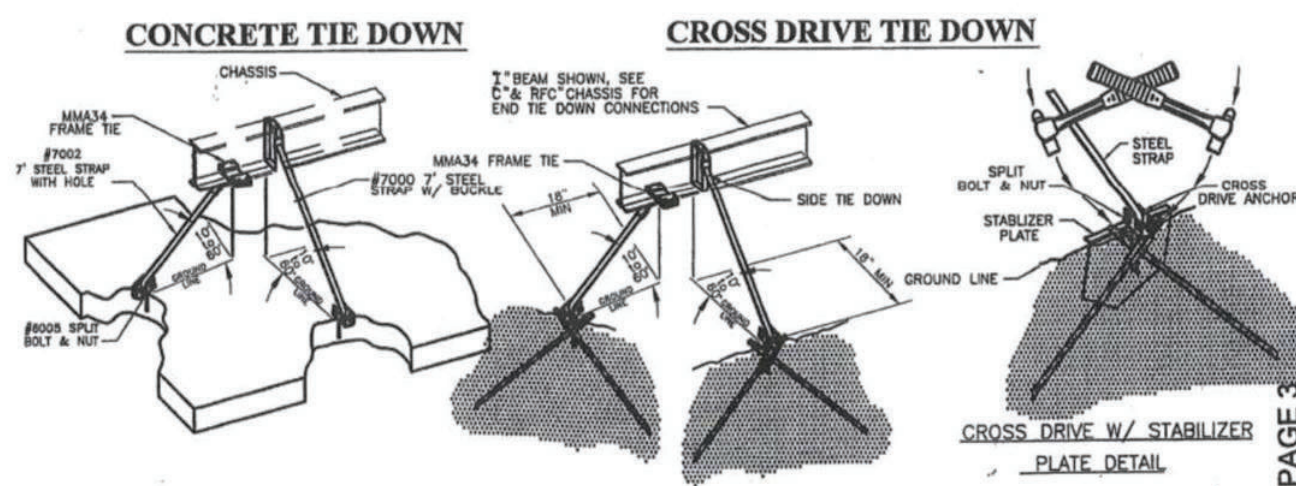
Phone: (678) 367-3151
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Spanish Fort, AL 36527-6700Fax: (666) 615-4682
sal@sgrege.net
salgraweb.com**S-2**



1. THE DRIVE MACHINE IS TURNED AND THE ANCHOR IS TURNED INTO THE GROUND TO A POINT WHERE THE TOP (STABILIZER HEAD PLATE) IS FLUSH OR SLIGHTLY BELOW GROUND LEVEL. THIS INSURES THAT THE E-2 ANCHOR STABILIZER WILL BE AT ITS REQUIRED INSTALLATION POSITION.

2. FOR THE E-2 ANCHOR/STABILIZER TO ACHIEVE FULL POTENTIAL, INSTALL THE ANCHOR VERTICALLY WITH NO HORIZONTAL DEFLECTION. IF AN ANCHOR/STABILIZER IS USED AT AN ANGLE, A GREATER ANGLE MAY BE USED TO START THE ANCHOR TO AVOID CONTACT WITH THE HOME & STRAIGHTENED AS THE ANCHOR IS GROUND SET. THE SPLIT BOLT IS INSERTED, STRAP TENSORED, AND TIGHTENING ADJUSTMENT MADE.

NOTE: WITH MACHINE INSTALLATION, A MINUTE-MAN ADAPTER DESIGNED TO FIT BOTH THE ANCHOR HEAD AND DRIVE MACHINES IS AVAILABLE. INSTALLERS DO NOT NEED ADDITIONAL OR SPECIAL EQUIPMENT FOR E-2 ANCHOR INSTALLATION.

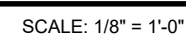
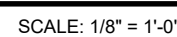


2 SEISMIC/HIGH WIND FOUNDATION ANCHORING
SCALE: 3/4"=1'-0"



S-3

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MODULAR FLOOR PLANS - ELECTRICAL	
2025-0568- ASR1	02-18-2026
E-201	

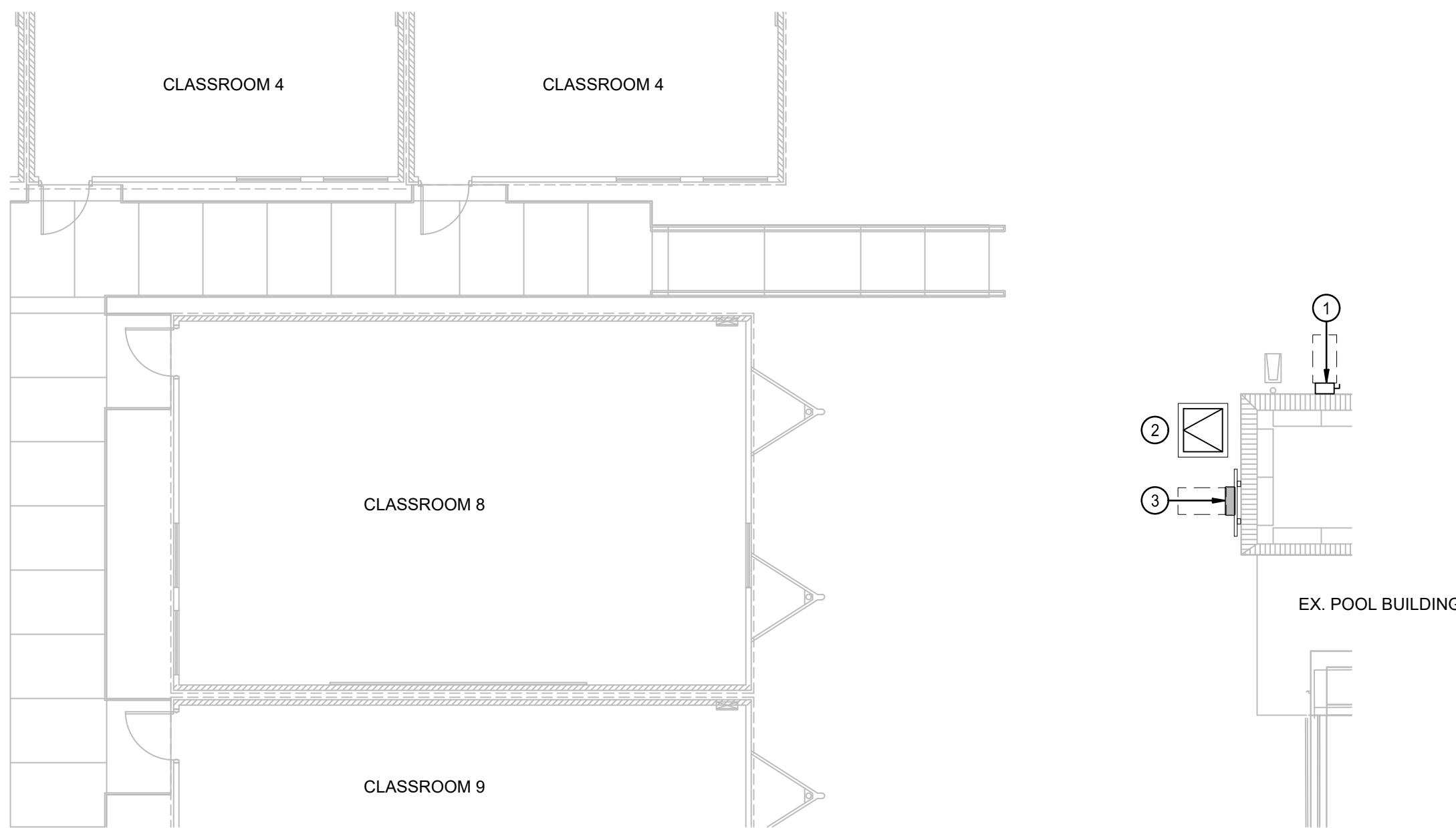


(UL #W-L-1010)

SCALE: NONE

CONDUIT PENETRATION NOTES (GYPSUM)

1. MAXIMUM 3/4" ANNULAR SPACE.
2. INSTALL 3M FIRE BARRIER 2000 SILICONE SEALANT TO COMPLETELY FILL THE ANNULAR SPACE BETWEEN THE PIPE AND THE WALL ASSEMBLY. FILL TO THE FULL THICKNESS OF THE GYPSUM WALL (MINIMUM 1-1/4 INCH SEALANT THICKNESS) PLUS AN ADDITIONAL 1/4 INCH CROWN AROUND THE PERIMETER OF THE CONDUIT.



PARTIAL PLAN - ELECTRICAL SERVICE AREA

SCALE: 1/8" = 1'-0"

PLAN NOTES

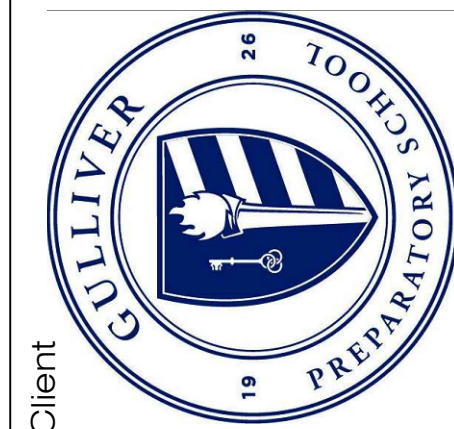
- ① EXISTING PORTABLE MAIN, FUSED DISCONNECT SERVICE RATED IN A NEMA 3R ENCLOSURE.
- ② 'PORTABLE' TRANSFORMER - 480V - 120/208V, 3PH, 4W, 150KVA IN A NEMA 3R ENCLOSURE.
- ③ EXISTING 600A 'PORTABLE' ELECTRICAL PANEL IN A NEMA 3R ENCLOSURE, RACK MOUNTED.

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ELECTRICAL DETAILS

2025-0568- ASR1	02-18-2026
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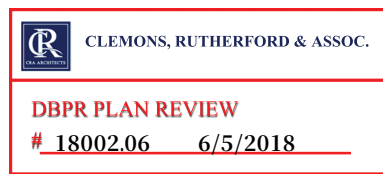
E-400

Engineering Calculations for:

Modular structure 24'x36'

Wind Speed: 186 MPH

ORB Project No. 1004.96



Julio Orbegoso, P.E.
License No. 38769
Date: 5/31/18



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PROJECT #: 1004.96
NAME: MODULAR STRUCTURE 24'x36'
CALCULATED BY: J.O.
DATE: 5/31/18

DESIGN CRITERIA:

1. FLORIDA BUILDING CODE 2017, BTH EDITION
2. ASCE 7-10 MINIMUM DESIGN LOADS FOR BUILDINGS
3. AISI SPECIFICATION FOR COLD FORMED METAL STEEL
4. AISC 14th EDITION
5. WIND SPEED: 136 MPH
6. SSMA TABLES
7. (2) 11'-8" WIDE MODULES
8. Flooring 3/4" armor mega-board or equiv.



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PROJECT #: 1004.96
NAME: MODULAR STRUCTURE 24'x36'
CALCULATED BY: J.O.
DATE: 3/21/18

1. FLOOR JOIST:

TRY 600S162-68 (14 GA.) @2'-0", $F_y = 33 \text{ KSI}$
 $S_x = 1.772 \text{ in}^3$ SPAN = 11'-7½"
 $I_x = 7.089 \text{ in}^4$ $M_{allow} = 35.60 \text{ K-in}$

LOAD D.L. = 10 PSF
 L.L. = 40 PSF TOTAL = 50 PSF

$$W_{D+L} = 50 \times 2 = 100 \text{ PLF}$$

$$M = \frac{0.1(11.63)^2}{8} = 1.69 \text{ K-ft} \times 12 \text{ in/FT}$$

$$M = 20.3 \text{ K-in} < 26.79 \text{ K-in} \dots \text{OK}$$

FROM SSMA TABLE, SINGLE SPAN MAX. DEFLECTION $L/360$

SPAN ALLOW = 12'-5" > 11'-7½" OK

USE FLOOR JOIST 600S162-68 (14 GA.) @2'-0" O.C.

2. WALL STUD:

TRY 362S200-43
 $S_x = 0.448 \text{ in}^3$
 $I_x = 0.836 \text{ in}^4$

FIRST 4'-0" FROM CORNER @12" O.C., HEIGHT = 9'-6"
 WIND COMP. & CLADDING (ZONE 5) $W = 54 \text{ PSF} \times 1 \text{ FT} = 54 \text{ PLF}$

$$M = \frac{54(9.5)^2}{8} = 0.609 \text{ K-ft}$$

$$S_{req} = \frac{0.609 \times 12}{0.6 \times 33} = 0.369 \text{ in}^3 \times 0.75 = 0.28 \text{ in}^3 < 0.448 \text{ in}^3 \dots \text{OK}$$

$$\Delta_{max} = \frac{9.5 \times 12}{300} = 0.38"$$

$$I_{req} = \frac{5 \times 0.054(9.5)^4 \times 1.728}{384 \times 29,000 \times 0.38} = 0.90 \times 0.75 = 0.67 \text{ in}^4 < 0.836 \text{ in}^4 \dots \text{OK}$$

USE STUD 362S200-43 @12" O.C. FOR 4'-0" FROM CORNER



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CALCULATED BY: J.O.
DATE: 3/21/18

3. ROOF:

3.1) PURLIN 600S200-54 (16 GA) @2'-0" O.C. FIRST 4'-0" FROM THE END WALL

$$S_x = 1.106 \text{ in}^3$$

$$I_x = 3.31 \text{ in}^4$$

CLEAR SPAN = 11'-0"

CONTRIB. AREA: $A = 12' \times 2' = 24 \text{ SF}$.

$$\text{WIND ZONE 3 COMP. \& CLADDING} = \frac{(94.7 - 68.8)}{(50 - 20)} (24 - 20) = 3.5 \text{ PSF}$$

$$\text{FOR } A = 24 \text{ SF.} \rightarrow \text{WIND} = 94.7 - 3.5 = 91.2 \text{ PSF}$$

LESS DL = 5 PSF

$$\text{NET UPLIFT} = 86.2 \text{ PSF}$$

$$W = 86.2 \times 2 = 172.4 \text{ PLF}$$

$$M = \frac{172.4(11)^2}{8} = 2.61 \text{ K-ft}$$

$$S_{\text{req}} = \frac{2.61 \times 12}{0.6 \times 33} = 1.58 \text{ in}^3 \times 0.75 = 1.18 \text{ in}^3 > 1.106 \text{ in}^3 \text{ BY } 8\% \text{.....OK}$$

$$\Delta = \frac{L}{300} = \frac{11 \times 12}{300} = 0.44"$$

$$I_{\text{req}} = \frac{5 \times 0.172(11)^4 \times 1,728}{384 \times 29,000 \times 0.44} = 4.44 \times 0.75 = 3.33 \text{ in}^4 \approx 3.32 \text{ in}^4 \text{.....OK}$$

USE PURLIN 600S200-54 @2'-0" FIRST 4'-0" FROM END WALL

3.2) PURLINS @4'-0" FOR WIND ZONE 1

$$A = 4' \times 12' = 48 \text{ SF}$$

USE WIND PRESSURE = 43 PSF

$$W = 43 \times 4 = 172 \text{ PLF}$$

$$M = \frac{172(11)^2}{8} = 2.6 \text{ K-ft}$$

$$S_{\text{req}} = \frac{2.6 \times 12}{0.6 \times 36} = 1.45 \text{ in}^3 \times 0.75 = 1.08 \text{ in}^3 < 1.106 \text{ in}^3 \text{.....OK}$$

USE PURLIN 600S200-54 @4'-0" O.C. FOR WIND ZONE 1.



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DATE: 3/21/18

3.3) ROOF EXTERIOR BEAM

MC8x8.5 $F_y = 36 \text{ KSI}$
 $S_y = 0.431 \text{ in}^3$
 $S_x = 5.82 \text{ in}^3$

SUPPORTS @ 9'-0" O.C.

D.L. = 10 PSF
L.L. = 40 PSF TOTAL = 50 PSF

$$W = 50(12/2) = 300 \text{ PLF}$$

$$M = \frac{0.3(9)^2}{8} = 3.0 \text{ K-ft}$$

$$S_{req} = \frac{3 \times 12}{0.6 \times 36} = 1.7 \text{ in}^3 < 5.82 \text{ in}^3 \text{.....OK}$$

3.4) AT ROOF MATE LINE

D.L. = 10 PSF
L.L. = 20 PSF TOTAL = 30 PSF

$$W = 30(12/2) = 180 \text{ PLF}$$

$$M = \frac{0.18(36)^2}{8} = 29.16 \text{ K-ft}$$

$$S_{req} = \frac{29.16 \times 12}{0.6 \times 36} = 16.2 \text{ in}^3$$

$$\Delta_{max} = \frac{L}{360} = \frac{36 \times 12}{360} = 1.2"$$

$$I_{req} = \frac{5 \times 0.18(36)^4 \times 1,728}{384 \times 29,000 \times 1.2} = 195.5 \text{ in}^4$$

USE MC12x31 ($S_x = 33.7 \text{ in}^3$, $I_x = 202 \text{ in}^4$) AT MATE LINE

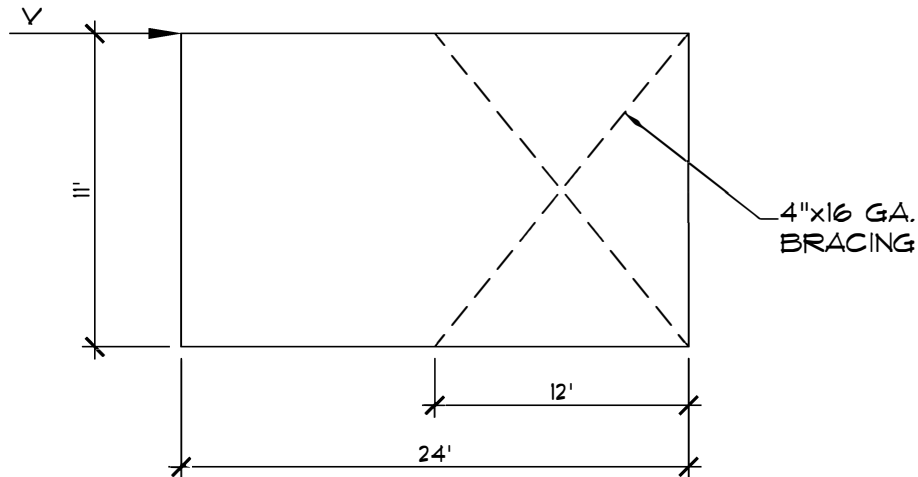


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NAME: MODULAR STRUCTURE 24'x36'
CALCULATED BY: J.O.
DATE: 3/21/18

4. SHEAR WALL



$$V = 33.0 \text{ PSF} \left(\frac{11}{2} \right) \left(\frac{36}{2} \right)$$

$$V = 3,267 \text{ #}$$

$$\frac{\text{u/ft}}{= \frac{3,267}{24}} = 136 \text{ PLF}$$

SHEAR WALL CAPACITY (FBC 2004, TAB. 2211.2(2))

FASTENER SPACING @7" O.C.

$$\text{CAPACITY} = 585 / 2.5 \text{ (SAFETY FACTOR)} = 234 \text{ PLF} > 136 \text{ PLF} \text{OK}$$

OVERTURNING SECTION

$$\text{Tot} = 3,267 \left(\frac{11}{24} \right) = 1,497 \text{ #}$$

TRY $\frac{3}{16}$ " WELDING 3" MIN. LENGTH ($\frac{1}{16}$ " $\rightarrow$ 609 #/in)

$$\text{CAPACITY} = 3(609) = 1,827 \text{ #} (3) = 5,481 \text{ #} > 1,497 \text{ #} \text{ OK}$$

FASTENER TO BOTTOM TRACK

$$\text{SHEAR} = 33 \text{ PSF} \times 11 \times (36/2) = 6,534 \text{ #} / 24' = 272.3 \times 0.75 = 204 \text{ #/FT}$$

$$\#12 \text{ SCREWS @12" O.C. } \rightarrow \text{CAPACITY} = 280 \text{ #/FT} > 204 \text{ #/FT} \text{ OK}$$

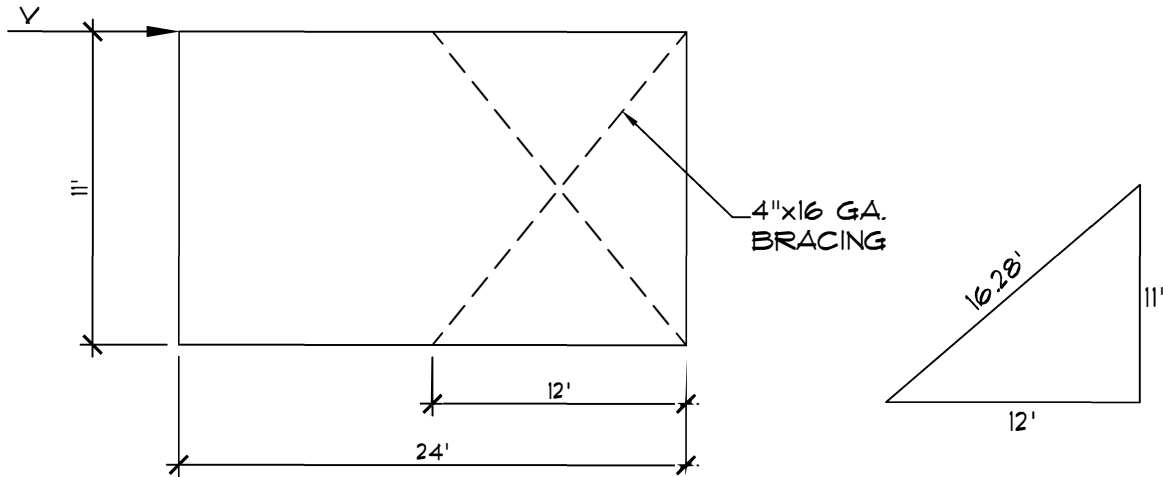


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CALCULATED BY: J.O.
DATE: 3/21/18

5. END WALL DIAGONAL BRACING



(2) PLATES 4"x16 GA.

$$A = 4 \times 0.0598 \times 2 = 0.48 \text{ in}$$

$$\text{DIAGONAL TENSION} = 3,267 \left(\frac{16.28}{12} \right) = 4,432 * = 4.43 \text{ k}$$

$$A_{req} = \frac{4.43}{0.6 \times 33} = 0.22 \text{ in}^2 < 0.48 \text{ in}^2$$

END CONNECTION $\frac{1}{8}$ " MIN. 8" LENGTH EACH SIDE

$$\frac{1}{8} = \frac{2}{16}$$

$$\text{CAPACITY} = 2 \times 609 \times 8 \times 2 = 19,500 * > 4,432 *$$

USE 4"x16 GA. DIAGONAL BRACING WITH $\frac{1}{8}$ " FILLET WELDING x8" MIN. LENGTH



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DATE: 5/31/18

6. FLOOR DECKING

LOAD D.L. = 10 PSF
 L.L. = 40 PSF

TOTAL D + L = 50 PSF

USE MEGABOARD STRUCTURAL CEMENT BOARD OR EQUIV.

CAPACITY = 100 PSF (SUPPORT @2'-0" O.C.) > 50 PSF (D.L. + L.L)

Dade County 175 mph Risk Category II
 Broward County 170 mph Risk Category II
<http://www.atcouncil.org/windspeed/>

Project: **ORB**

Date: **12-17-16**

Risk Category	III	Wind Speed Nominal	185 mph	area 1	80 sf	Bldg width	24
Exposure	C	V asd	145 mph	area 2	150 sf	Bldg length	36
Mean Roof Height	12 feet	Roof Slope	0.25:12	ASD factor	0.6	a=3'-0"	
Top of parapet	12 feet	Roof slope in degree	1.19	Enclosed		Monoslope	

(MWFRS DIRECTIONAL PROCEDURE) Chapter 27.2 Buildings of All Heights

MWFRS Walls direction 1 Normal to width				MWFRS Walls direction 2 Normal to length				MWFRS PARAPETS	
	windward	leeward	sidewalls		windward	leeward	sidewalls	GCpi +	57.5 windward parapet
GCpi +	19.2	-20.0	-29.8	GCpi +	19.2	-23.2	-29.8	GCpi -	-38.3 leeward parapet
GCpi -	33.0	-6.2	-16.0	GCpi -	33.0	-9.4	-16.0		
MWFRS Roof parallel to ridge				MWFRS Roof normal to ridge				MWFRS Roof Overhang	
	horiz distance from windward edge				horiz distance from windward edge			GCpi +	32.0
0	6	-36.2	7.0	0	6	-36.2	7.0	GCpi -	55.0
6	12	-36.2	7.0	6	12	-36.2	7.0		
12	24	-23.2	7.0	12	24	-23.2	7.0		
24		-16.7	7.0	24		-16.7	7.0		
								MWFRS Min. Design Loads	
								Walls	10.0
								Roofs	5.0

(Components & Cladding Low Rise Buildings h<60) Chapter 30.4

Components and Cladding		Predefined Area								area 1		area 2	
		10		20		50		100		80		150	
Zone		+	-	+	-	+	-	+	-	+	-	+	-
1	GCpi +	4.6	-46.3	3.5	-44.1	2.0	-42.6	0.8	-41.5	1.1	-41.8	0.8	-41.5
	GCpi -	18.4	-31.5	17.3	-30.3	15.8	-28.8	14.6	-27.7	15.0	-28.0	14.6	-27.7
2	GCpi +	4.6	-76.0	3.5	-67.9	2.0	-67.2	0.8	-49.1	1.1	-61.7	0.8	-49.1
	GCpi -	18.4	-62.2	17.3	-54.1	15.8	-43.4	14.6	-35.3	15.0	-37.9	14.6	-35.3
3	GCpi +	4.6	-114.3	3.5	-94.7	2.0	-88.8	0.8	-49.1	1.1	-55.4	0.8	-49.1
	GCpi -	18.4	-100.5	17.3	-80.9	15.8	-64.9	14.6	-35.3	15.0	-41.8	14.6	-35.3
4	GCpi +	28.3	-44.2	26.5	-42.4	24.1	-40.0	22.3	-38.2	22.8	-38.8	21.2	-37.1
	GCpi -	40.7	-31.8	38.9	-30.0	36.5	-27.5	34.7	-25.7	35.2	-26.3	33.6	-24.7
5	GCpi +	28.3	-54.5	26.5	-50.9	24.1	-46.0	22.3	-42.4	22.8	-43.6	21.2	-40.2
	GCpi -	40.7	-42.1	38.9	-38.5	36.5	-33.8	34.7	-29.9	35.2	-31.1	33.6	-27.8



Hernando J. Navas, P.E.
Mark S. Johnson, P.S.M.

Of Counsel
Alfonso C. Tello, P.E.,
P.S.M.

**CIVIL ENGINEERS
LAND SURVEYORS**
2844 CORPORATE WAY - MIRAMAR, FLORIDA 33025
PHONE: (954) 435-7010

Luis F. Leon, P.E.
Andrea Etcheverry, P.E.
Michael J. Alley, P.S.M.

April 2, 2026

Mayra De Torres, Engineer 3
Department of Regulatory and Economic Resources,
Water Control Section
Overtown Transit Village
701 NW 1st Court, • Suite 600 • Miami, Florida 33136
mayra.detorres@miamidade.gov
Office phone: 305-372-6638

**RE: Gulliver Temporary Modular Classrooms (M2026011895)
SWMGP-13-80068-P**

Dear Mayra,

The modular classrooms being permitted under the above referenced process number is a temporary structure elevated by several pillars which will only take a negligible amount of above ground stormwater storage from the overall surface water permit (by the vertical pillars). The lowest horizontal structural member is proposed at the minimum FFE by the SWMGP (+9.00' NGVD) and above the 100-Year, 3-Day on-site peak stage of 8.92' NGVD by the same permit. Please refer to the attached exhibit for a graphical representation of the above text. This modular classroom project will not adversely affect the existing surface water management permit.

If you have additional questions or require additional information, please do not hesitate to contact me.

Date:04-02-26

Regards,

Schwebke-Shiskin & Associates, Inc.

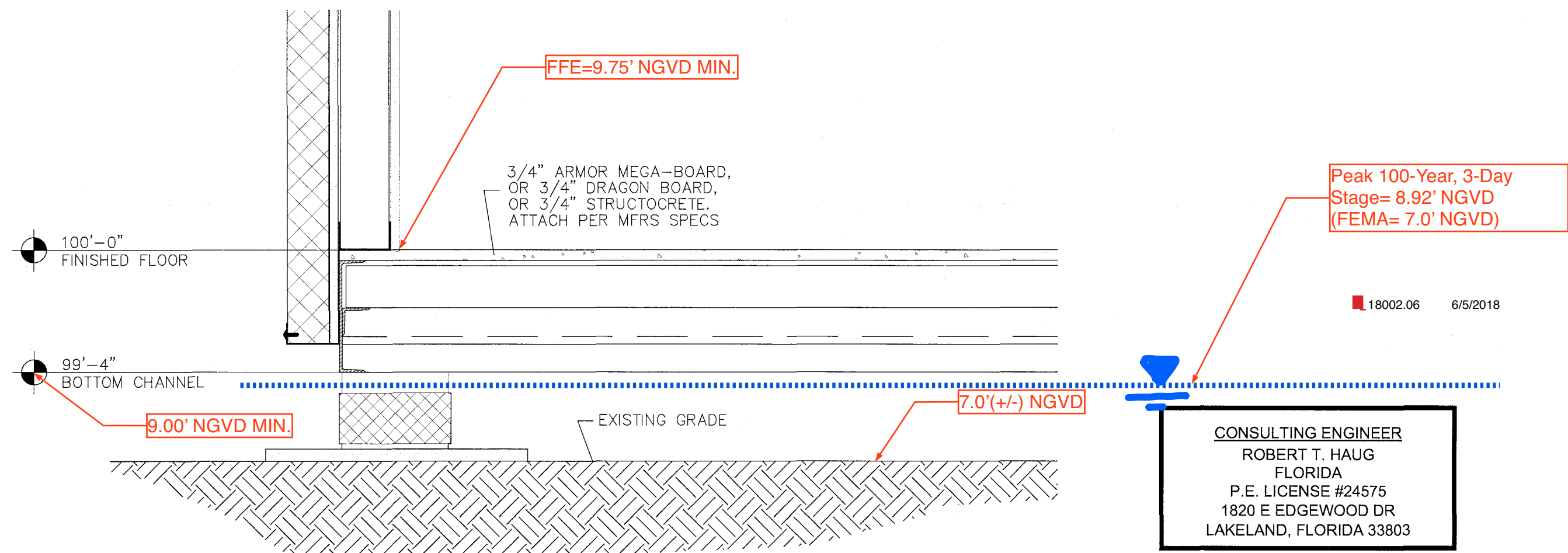
Hernando J. Navas, P.E. No. 50635

Principal Digitally signed by
Hernando J Navas
Date: 2026.04.02
19:49:20-04'00'



THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY HERNANDO J. NAVAS, P.E.
ON THE DATE ADJACENT TO THE SEAL. PRINTED COPIES OF THIS DOCUMENT ARE
NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED
ON ANY ELECTRONIC COPIES.

PRODUCT APPROVAL			
PRODUCT CATEGORY	SUB. CATEGORY	MANUFACTURER	STATE OF FLORIDA APPROVAL #:
DOORS	EXTERIOR	VISION HOLLOW METAL	FL#: 19036.2
DOORS	EXTERIOR	VISION HOLLOW METAL	FL#: 19036.4
FLOORING	SUB-FLOOR	ECTEK INTERNATIONAL	ARMOR-MEGA BOARD



FLOORING @ SIDEWALL
SCALE: 1 1/2" = 1'-0"

CONSULTING ENGINEER
ROBERT T. HAUG
FLORIDA
P.E. LICENSE #24575
1820 E EDGEWOOD DR
LAKELAND, FLORIDA 33803

MOBILE MODULAR
ADDITIONAL APPROVED PRODUCTS
CRA #: 18002.06



MODULAR PLANS DESIGN, CO.

1074 S. FLORIDA AVE. SUITE 201
LAKELAND, FLORIDA
33803-5001
PH: 863-688-1054
FAX: 863-688-7118
MODULARPLANS@TAMPABAY.FL.COM

Handwritten signature and date: 5/29/18

DATE:	05-24-2018
DRAWN:	R.C.S.
JOB #:	MPD-1518
SHEET NO.	SD-1

GulliverPrep

TEMPORARY CLASSROOMS
6575 North Kendall Drive, Pinecrest, FL 33156

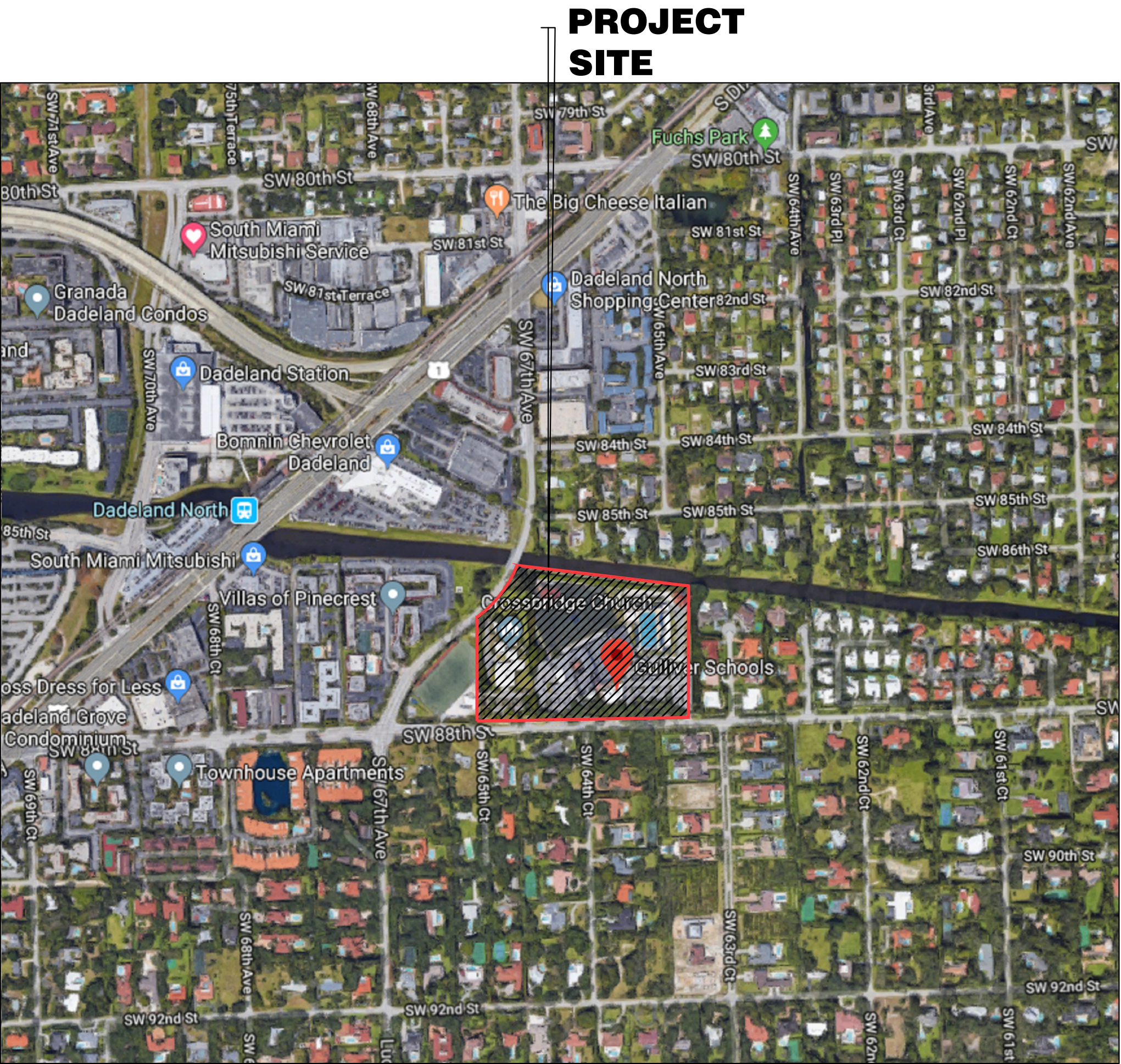
Project No. 2025-0568-ASR1

02-18-2026

PERMIT SUBMITTAL



AERIAL



LOCATION MAP

SCOPE OF WORK

THIS PROJECT INCLUDES THE PLACEMENT OF 10 NEW TEMPORARY CLASSROOMS.

ALL CONSTRUCTION WILL BE IN CONFORMANCE WITH THE CURRENT FLORIDA BUILDING CODE AND THE FLORIDA FIRE PROTECTION CODE.

CONSTRUCTION MANAGER TO COORDINATE ALL REQUIRED PERMITTING, BID PACKAGES AND CLARIFICATIONS OF SCOPE FOR THE PROJECT. PLEASE NOTE ALL DRAWINGS AND SPECIFICATIONS FOR THIS PROJECT WILL BE MADE AVAILABLE VIA THE CONSTRUCTION MANAGER.

PROJECT TEAM

ZYSCOVICH
A Stratus Team Company

100 Biscayne Boulevard, 27th Floor
Miami, Florida 33132
(305) 372-5222 Tel

M/E/P ENGINEERS

JLRD, CONSULTING ENGINEERS
1450 Centerpark Blvd. Suite 350
West Palm Beach, FL 33401
(561) 689-2303 TEL

INDEX OF DRAWINGS			
Current Revision	Current Revision Date	Sheet Number	Sheet Name
GENERAL	03.05.2026	GN-000	COVER SHEET
		GN-100	GENERAL NOTES
ARCHITECTURE	03.05.2026	A-100	OVERALL SITE PLAN
		A-101	ENLARGED SITE PLAN AND ELEVATION
		A-201	ESPRESSO PLAN
ELECTRICAL	03.05.2026	E-101	PARTIAL OVERALL ELECTRICAL PLAN
		E-201	MODULAR FLOOR PLANS - ELECTRICAL
		E-300	RISERS
		E-400	ELECTRICAL DETAILS
		E-500	SPECIFICATIONS



ARCHITECTURAL TAGS

Diagram showing various architectural tags and their corresponding symbols:

- WALL SECTION TAG: XX A-XXX
- DETAIL TAG: 9 A-750 TYP
- ELEVATION CHANGE TAG: TOP BOT
- SURVEY ELEVATION TAG: Elevation 82.0 NGVD
- EXTERIOR ELEVATION TAG: Elevation X/A-XXX Sheet
- REVISION TAG: Revision Type
- REVISION RESPONSE TO COMMENTS BY: CM - Construction Manager, DCP - Design Criteria Professional, BRBD - Boca Raton Building Department

MATERIAL LEGEND

CONCRETE MASONRY	EARTH
CONCRETE	GRAVEL/POROUS FILL
CONCRETE IN SECTION	CORAL ROCK
STUCCO	

ABBREVIATIONS			
ABV. ---	ABOVE	IDIA. ---	INSIDE DIAMETER
A.DRN. ---	AREA DRAIN	INT. ---	INTERIOR
ADJ. ---	ADJACENT / ADJUSTABLE	INV. ---	INVERT
ALUM. ---	ALUMINUM	JT. ---	JOINT
AND. ---	AND	L.F. ---	LINEAR FEET
ARCH. ---	ARCHITECTURAL	L.P. ---	LOW POINT
ASPH. ---	ASPHALT	MAX. ---	MAXIMUM
ASST. ---	ASSISTANT	MECH. ---	MECHANICAL
BLDG. ---	BUILDING	MFR. ---	MANUFACTURER
BLK. ---	BLOCK / BLOCKING	MH. ---	MANHOLE
B.O. ---	BOTTOM OF	MIN. ---	MINIMUM
CAB. ---	CABINET	MISC. ---	MISCELLANEOUS
CB. ---	CATCH BASIN	M.O. ---	MASONRY OPENING
CEM. ---	CEMENT	MTL. ---	METAL
C.PLAS. ---	CEMENT PLASTER	NA. ---	NOT APPLICABLE
C.G. ---	CORNER GUARD	NC. ---	NON-COMBUSTIBLE
C.JNT. ---	CONTROL JOINT	N.I.C. ---	NOT IN CONTRACT
CLR. ---	CLEAR (FINISH TO FINISH)	NO. / NUM. ---	NUMBER
COL. ---	COLUMN	NOM. ---	NORMAL
CLS. ---	CEILING	N.T.S. ---	NOT TO SCALE
CONC. ---	CONCRETE	O.C. ---	ON CENTER
CONT. ---	CONTINUOUS	O.D. ---	OUTSIDE DIAMETER
C.M.U. ---	CONCRETE MASONRY UNIT	OPNG. ---	OPENING
DET. ---	DETAIL	P.L. ---	PROPERTY LINE
DI. ---	DIAMETER	PLAS. ---	PLASTER
DN. ---	DOWN	PLUM. ---	PLUMBING
DWG. ---	DRAWING	P.O.B. ---	POINT OF BEGINNING
EA. ---	EACH	PR. ---	PAIR
ELEC. ---	ELECTRICAL	PTD. ---	PAINTED
ELEV. ---	ELEVATION	R. ---	RISER
E.N.C. ---	EXISTING, NO CHANGE	RAD. ---	RADIUS
E.O.S. ---	EDGE OF SLAB	REF. ---	REFERENCE
EQ. ---	EQUAL	REIN. ---	REINFORCED
EQUIP. ---	EQUIPMENT	REQ. ---	REQUIRED
EXIST. ---	EXISTING	R.O. ---	ROUGH OPENING
EXT. ---	EXTERIOR	SC. ---	SOLID CORE
EXP.JT. ---	EXPANSION JOINT	SCHD. ---	SCHEDULE
FBC. ---	FLORIDA BUILDING CODE (MOST CURRENT)	SHT. ---	SHEET
F.D. ---	FIRE DAMPER	SIM. ---	SIMILAR
F.F.E. ---	FINISH FLOOR ELEVATION	SPECS. ---	SPECIFICATIONS
F.F.P.C. ---	FLORIDA FIRE PROTECTION CODE	SQ. ---	SQUARE
FIXT. ---	FIXTURE	S.S. ---	STAINLESS STEEL
FLASH. ---	FLASHING	STL. ---	STEEL
FL.D. ---	FLOOR DRAIN	STOR. ---	STORAGE
F.D.F. ---	FACE OF FINISH	STRUCT. ---	STRUCTURAL
F.R. ---	FIRE RATED	SUSP. ---	SUSPENDED
GA. ---	GALVE	SY. ---	SYMMETRICAL
GALV. ---	GALVANIZED	T. ---	TEMPERED
GEN. ---	GENERATOR	THK. ---	THICKNESS
GL. ---	GLASS	T.O. ---	TOP OF
H.C. ---	HANDICAP	TYP. ---	TYPICAL
H.C. ---	HOLLOW CORE	UA. ---	UNLIMITED AREA
HWD. ---	HARDWOOD	U.O. ---	UNLESS OTHERWISE NOTED
HM. ---	HOLLOW METAL	VERT. ---	VERTICAL
HORIZ. ---	HORIZONTAL	V.F. ---	VERIFY IN FIELD
HGT. ---	HEIGHT	W. ---	WITH
H.P. ---	HIGH POINT	W/O ---	WITHOUT
		WP. ---	WATERPROOF
		WT. ---	WEIGHT

GENERAL NOTES

- CONTRACTORS SHALL COMPLY WITH THE FLORIDA BUILDING CODE 8TH EDITION (2023) (WITH ALL APPLICABLE REVISIONS), FLORIDA FIRE PREVENTION CODE 8TH EDITION (2023), ALL STATE AND LOCAL ZONING CODES. PERMITS SHALL BE POSTED IN A VISIBLE PLACE AT ALL TIMES. ALL PERMITS, UTILITY AND METER CONNECTION FEES SHALL BE OBTAINED AND PAID FOR BY THE CONTRACTOR.
- ALL WORK, MATERIALS AND EQUIPMENT UTILIZED IN THIS PROJECT SHALL BE NEW AND INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURERS' RECOMMENDATIONS AND SPECIFICATIONS.
- ALL WORK FOR THIS PROJECT SHALL CONFORM TO STANDARDS PUBLISHED BY RECOGNIZED PROFESSIONAL AND INDUSTRY ORGANIZATIONS.
- CONTRACTOR SHALL BE RESPONSIBLE FOR VISITING THE SITE PRIOR TO BIDDING AND FAMILIARIZING HIMSELF/HERSELF WITH ALL EXISTING CONDITIONS AFFECTING THE WORK, INCLUDING BUT NOT LIMITED TO PRIVATE AND PUBLIC UTILITIES, ON AND OFF SITE, ACCESS ROADS AND OTHER SUPPORT FACILITIES.
- CONTRACTOR TO REMOVE, RELOCATE OR RE-ROUTE, AS NECESSARY, ELECTRICAL, TELEPHONE, LOW VOLTAGE OR ANY OTHER UTILITY LINES ENCOUNTERED AND SHALL COORDINATE THIS WORK WITH ALL LOCAL UTILITY COMPANIES.
- CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ARCHITECT OF ANY UNEXPECTED OR UNKNOWN FIELD CONDITIONS, ERRORS, OMISSIONS, OR DISCREPANCIES IN THE DRAWINGS, PROJECT MANUAL, OR CONTRACT DOCUMENTS PRIOR TO PROCEEDING WITH THE WORK OR SHOP FABRICATIONS.
- CONTRACTOR SHALL PREPARE AND MAINTAIN ALL CONSTRUCTIONS AREAS, AS WELL AS SURROUNDING AREAS FREE OF DEBRIS OR HAZARDOUS EQUIPMENT AT ALL TIMES.
- CONTRACTOR SHALL BE RESPONSIBLE FOR THE REPAIR AND/OR THE REPLACEMENT OF ANY ITEMS DAMAGED DURING CONSTRUCTIONS OR CLEAN-UP. CONSTRUCTION PERSONNEL SHALL BE CONFINED TO THE LIMITS OF THE CONSTRUCTION AREA.
- DRAWINGS ARE NOT TO BE SCALED. WRITTEN DIMENSIONS SHALL BE FOLLOWED.
- CONTRACTOR TO SUBMIT SHOP DRAWINGS FOR APPROVAL BEFORE COMMENCING FABRICATION AND/OR INSTALLATION OF ALL APPLICABLE ITEMS FOR CONSTRUCTION. ALL SHOP DRAWING DIMENSIONS SHALL BE FIELD VERIFIED, REVIEWED AND APPROVED BY CONTRACTOR BEFORE SUBMITTAL. SHOP DRAWINGS WHICH ARE INCOMPLETE OR LACKING SUFFICIENT INFORMATION WILL BE RETURNED WITHOUT REVIEW. REFER TO SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
- THE CONTRACTOR SHALL BE AWARE THAT SPECIFIC FIRE-RATED SEPARATION WITHIN THE BUILDING'S CONSTRUCTION ARE REQUIRED BY CODE. THE USE OF SPECIFIC MATERIALS AND COMBINATION OF MATERIALS WITHIN FIRE RATED ASSEMBLIES, AS CALLED FOR ON THE DRAWINGS AND SPECIFICATIONS, ARE FOR THE PURPOSE OF ACHIEVING THOSE REQUIRED FIRE SEPARATIONS. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY THAT ANY CHANGE IN MATERIAL THAT IS REQUESTED BY OR MADE BY THE CONTRACTOR AND/OR ITS SUBCONTRACTORS, FROM THOSE MATERIALS DRAWN OR SPECIFIED, DOES NOT IN ANY WAY AFFECT OR LESSEN THE REQUIRED CONSTRUCTION ASSEMBLY. SPECIFIC SEQUENCES OR METHODS OF ACHIEVING FIRE RATED ASSEMBLIES REQUIRED BY A "U.L. NO." LISTING SHALL BE FOLLOWED WITHOUT DEVIATION, UNLESS OTHERWISE NOTED OR DETAILED.
- THE USE OF FIRE RETARDANT TREATED WOOD IS PROHIBITED. PRESSURE TREATED WOODS SHALL BE USED WHERE WOOD IS IN CONTACT WITH CONCRETE OR MASONRY OR IN CONTACT WITH THE ELEMENTS (EXTERIOR.)
- IN ADDITION TO WALL TYPES SHOWN ON PLANS, THE CONTRACTOR SHALL REFER TO THE ROOM FINISH SCHEDULE, INTERIOR ELEVATIONS, AND DETAILS FOR ADDITIONAL INFORMATION REGARDING FINISHES.
- CONTRACTOR SHALL COORDINATE WITH MECHANICAL AND ELECTRICAL CONTRACTORS THE LOCATION OF ALL ACCESS PANELS SO AS TO ALLOW FOR PROPER EQUIPMENT ACCESSIBILITY, MAINTENANCE AND/OR OPERATION OF THE BUILDING SYSTEMS PRIOR TO THE START OF THE WORK. THE OMISSIONS OF ANY OR ALL ACCESS PANELS DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY FROM PROVIDING AND INSTALLING SUCH PANELS OR DOORS. ACCESS PANELS IN RATED ELEMENTS SHALL HAVE THE SAME FIRE RATING. PROVIDE ACCESS PANELS FOR ALL A/C FIRE DAMPERS WHERE APPLICABLE.
- CONTRACTOR SHALL ALSO BE RESPONSIBLE FOR PROPERLY CLEANING ALL AREAS PRIOR TO FINAL ACCEPTANCE BY THE OWNER INCLUDING BUT NOT LIMITED TO WINDOWS, FLOOR, CARPETS, WALLS, DOORS, EQUIPMENT, ETC.
- UPON COMPLETION OF THIS PROJECT, THE CONTRACTOR SHALL GIVE TO THE OWNER A COMPLETE SET OF AS-BUILT ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND FIRE PROTECTION DRAWINGS ALONG WITH THE WRITTEN GUARANTIES, OPERATION AND MAINTENANCE MANUALS OF ALL EQUIPMENT AND FINISHES INSTALLED. THE CONTRACTOR SHALL MAINTAIN A CURRENT SET OF AS-BUILT DRAWINGS AND SPECIFICATIONS. INFORMATION SHALL BE RECORDED BY CONTRACTOR AS CONSTRUCTION PROGRESSES AND REVIEWED FOR COMPLETENESS AT EACH REQUISITION REQUEST. REFER TO PROJECT MANUAL.
- CONTRACTOR SHALL PROVIDE GUARDRAILS IN COMPLIANCE WITH THE LIFE SAFETY PLANS DURING ALL PHASES OF CONSTRUCTION, IN ACCORDANCE WITH FBC 2023.
- WHERE EXISTING WALLS ARE TO BE PATCHED AND FILLED TO 'LIKE NEW' CONDITION, THEY SHALL BE MADE TO MATCH THICKNESS AND FINISH OF ADJACENT CONSTRUCTION. FINISHES SHALL BE REPLACED TO THE END OF EACH WALL OR NEAREST CONSTRUCTION JOINT.
- CONTRACTOR TO PROVIDE A SAFETY PLAN PRIOR TO START OF CONSTRUCTION TO CLEARLY DELINEATE AREAS FOR CONSTRUCTION, SAFETY BARRIERS, EXITS, AND CONSTRUCTION TRAFFIC DURING THE VARIOUS PHASES AND WHEN CONDITIONS CHANGE.
- IT IS THE RESPONSIBILITY OF THE CM AND EACH OF THEIR SUBCONTRACTORS TO REVIEW ALL DRAWINGS, THE PROJECT MANUAL, ADDENDA, ETC. TO ASSURE COORDINATION OF ALL WORK AFFECTING EACH TRADE.
- THE CM SHALL HAVE THEIR CONSTRUCTION SCHEDULE APPROVED BY THE OWNER REPRESENTATIVE PRIOR TO COMMENCING CONSTRUCTION WORK WITH THE TIME PERIOD SPECIFIED IN THE CONTRACT DOCUMENTS.
- THE STAGING AREA WILL BE LOCATED BY THE OWNER'S REPRESENTATIVE PRIOR TO THE BEGINNING OF CONSTRUCTION.
- THE CM SHALL PROVIDE JOB SITE ACCESS TO THE OWNER AND THEIR REPRESENTATIVES FOR INSPECTION OR OBSERVATION PURPOSES.
- THE CONTRACTOR SHALL PROVIDE ALL NECESSARY REQUIREMENTS FOR NOISE CONTROL DURING CONSTRUCTION PERIOD AND CONFORM WITH APPLICABLE OSHA REQUIREMENTS AND LOCAL ORDINANCES HAVING JURISDICTION.
- ALL COMPONENTS OF THE PRIMARY BUILDING ENVELOPE, INCLUDING BUT NOT LIMITED TO THE FOLLOWING REQUIRE MIAMI-DADE NOTICE OF ACCEPTANCE OR FLORIDA BUILDING CODE APPROVAL: WINDOWS, EXTERIOR DOORS, ROOFING, PANEL WALLS, AND STRUCTURAL COMPONENTS
- THE FOLLOWING ITEMS WILL REQUIRE SPECIALTY ENGINEERED SHOP DRAWING SUBMITTALS FOR APPROVAL:

- LOUVERS
- STOREFRONTS
- PAVERS
- WATERPROOFING
- MILLWORK WITH STRUCTURE
- RAILINGS
- AWNINGS/CANOPIES

- BEFORE INSTALLATION, CONTRACTOR SHALL COORDINATE WITH THE ARCHITECT THE EXACT MOUNTING LOCATIONS OF ALL SWITCHES, WALL RECEPTACLES, PLUGS, FEC AND OTHER WALL MOUNTED FIXTURES AND GRAPHICS/FINISHES. NO SWITCHES, DEVICES, EQUIPMENT, ETC. TO BE INSTALLED IN SPECIALTY FINISH/DISPLAY WALL AREAS.
- REFER TO ENLARGED FLOOR PLAN DRAWINGS FOR ADDITIONAL DIMENSIONS.
- DIMENSIONS ARE TYPICALLY SHOWN FROM OUTSIDE FACE OF CMU WALL TO OUTSIDE FACE OF CMU WALL, OR CENTERLINE OF PARTITION, UNLESS NOTED OTHERWISE CONTRACTOR SHALL NOTE SPECIAL LOCATIONS IN WHICH "CLEAR" DIMENSION IS SPECIFIED ("CLEAR" DIMENSIONS ARE FROM FINISH TO FINISH.)
- ALL EXPOSED AREAS INCLUDING ALL SYSTEMS (IF EXPOSED) ARE TO BE FULLY PAINTED INCLUDING COLOR CODING OF ALL SYSTEMS AND PIPING.
- CONTRACTOR TO PROVIDE ADHERED INSULATION AT UNDERSIDE AND TOP OF ALL EXTERIOR SLAB AREAS TO CLOSE BUILDING ENVELOPE (I.E. ABOVE EXTERIOR DOORS AND BELOW SLAB AT EXTERIOR AREA).
- SLOPE ALL FLOORS TO DRAIN WHERE FLOOR DRAIN IS PROVIDED. ALL SLOPED SLABS AND RECESSED SLAB AREAS ARE TO BE CONSTRUCTED IN ACCORDANCE WITH T.C.A. RECOMMENDATIONS.
- COORDINATE WITH OVERALL SITE PLAN, CIVIL, LANDSCAPE, ELECTRICAL, MECHANICAL, PLUMBING, AND FIRE PROTECTION DRAWINGS FOR ALL SITE CONDITIONS.
- ALL ROOMS WITH FLOOR DRAINS AT ELEVATED LEVELS ARE TO HAVE PROTECTIVE WATERPROOFING MEMBRANE BELOW FINISH WITH ANTI-FRACTURE REINFORCEMENT.

INTERIOR FINISHES NOTES:

- FLAME SPREAD RATING FOR CEILING AND WALLS SHALL BE CLASS B OR HIGHER.
- FLAME SPREAD FOR DECORATIVE MATERIALS AND TRIM MAY BE CLASS C OR HIGHER.
- FLOORS MAY BE CLASS I OR CLASS II.
- ALL WOOD FINISHES TO BE FIRE RETARDANT WOOD.
- ALL FLOORING MATERIAL TO ALLOW FOR A MAXIMUM 1/2" THRESHOLD AND TRANSITION AT EACH SIDE OF THE DOOR

SMOKE DETECTOR NOTES:

- PROVIDE INTERCONNECTED SMOKE ALARMS IN COMPLIANCE WITH FBCE 704.4.3
- FIELD VERIFY ALL LOCATIONS OF EXISTING SMOKE DETECTORS.
- SMOKE DETECTORS SHALL BE NO LESS THAN 36" FROM AC SUPPLY.

MEP SAFETY NOTES:

- CAP WATER SUPPLY AT SOURCE AND CAP WASTE AT SLAB.
- PROTECT FIRE ALARM SYSTEM SPEAKERS AND ASSOCIATED WIRING FROM DUST, PAINT, AND ANY TYPE OF DAMAGE DURING CONSTRUCTION.
- ALL ELECTRICAL OUTLETS TO REMAIN AS EXISTING AND BE PROTECTED DURING RENOVATION.
- PROTECT ALL OPENINGS FROM WATER INFILTRATION DURING CONSTRUCTION.
- REFER TO FIRESTOPPING DETAILS FOR REQUIRED FIRESTOPPING AT PENETRATIONS.



PROPOSED TEMPORARY CLASSROOM LOCATION



PROPOSED TEMPORARY CLASSROOM LOCATION

ADDRESS:

6575 North Kendall Drive, Pinecrest, FL 33156

FOLIO NUMBER:

20-4036-000-0750

LEGAL DESCRIPTION:

36 54 40 9.43AC
W1/2 OF SW1/4 OF SW1/4 LYG S OF
SNAPPER CREEK CANAL
F/A/U 30-4036-000-0750
OR 19123-3193 052000 6 (4)

OCCUPANCY GROUP: EDUCATIONAL - "E"

ZONING AND USE:

ZONING DESIGNATION: PS

USE: PUBLIC SERVICE

SCOPE OF WORK:

ALTERATION LEVEL: LEVEL 1
LEVEL 1 ALTERATIONS INCLUDE THE REMOVAL AND REPLACEMENT OR THE COVERING OF EXISTING MATERIALS, ELEMENTS, EQUIPMENT, OR FIXTURES USING NEW MATERIALS, ELEMENTS, EQUIPMENT, OR FIXTURES THAT SERVE THE SAME PURPOSE.

ALTERATION LEVEL 1 -

- INSTALLATION OF 8 NEW CLASSROOM TRAILERS FOR USE DURING CONSTRUCTION OF NEW SCHOOL BUILDINGS.
- CANOPY AND RAMP (BY SEPARATE PERMIT)
- TRAILERS TO BE CONNECTED TO UTILITIES THAT ARE EXISTING ON SITE (WATER, ELECTRICAL, SEWER)
- TRAILERS SHALL BE CONNECTED TO EXISTING SCHOOL FIRE ALARM NOTIFICATION SYSTEM

NOTE: PERMIT APPLICATION FOR TRAILERS WILL BE LINKED TO CONSTRUCTION MASTER PERMIT FOR SCHOOL CONSTRUCTION. TRAILERS INTENDED USE ARE TO REPLACE CLASSROOMS THAT WILL BE DEMOLISHED AND OUT OF USE DURING CONSTRUCTION.

APPLICABLE CODES AND STANDARDS:

- FLORIDA BUILDING CODE (8th EDITION) 2023
- FLORIDA ACCESSIBILITY CODE (8th EDITION) 2023
- FLORIDA FIRE PROTECTION CODE (8TH EDITION) 2023
- NFPA 1 / 101, (8th EDITION) 2023

DEFERRED SUBMITTALS AND DELEGATED ENG. SYST.:

- NOA'S ARE REQUIRED FOR THE FOLLOWING ITEMS
 - RAILINGS
 - CANOPY STRUCTURES
- ALL ASSEMBLIES WHICH REQUIRE NOA MUST BE APPROVED BY MIAMI DADE COUNTY OR STATE OF FLORIDA COMPLYING WITH 2023 FBCB 107.3.4.1 AND 107.3.5
- SHOP DRAWINGS, CALCULATIONS AND SUBMITTALS ARE REQUIRED ON ALL THE ABOVE MENTIONED TRADES TO BE SUBMITTED TO THE ARCHITECT FOR REVIEW.
- SHOP DRAWING/NOA/PRODUCT DATA TO BE STAMPED - APPROVED/SIGNED AND DATED EACH PAGE BY ARCHITECT. APPROPRIATE ASSEMBLIES; CONNECTIONS, TYPE OF ELEVATION, PRESSURES, AND DETAILS MUST BE SELECTED AND HIGHLIGHTED FROM EACH PRODUCT APPROVAL AND ALSO EACH PRODUCT MUST BE IDENTIFIED IN THE DRAWINGS.
- SHOP DRAWINGS AND SUBSIDIARY PERMITS ASSOCIATED WITH THE MAIN PERMIT WILL BE REVIEWED AND PROCESSED THROUGH SEPARATE PROCEDURES PER 2023 FBC 107.3.5

ASBESTOS NOTE:

THE OWNER HAS NO KNOWN KNOWLEDGE OF ASBESTOS MATERIAL IN THE AREAS OF WORK PROPOSED. REFER TO OWNER'S ASBESTOS REPORT & SURVEY OF THE AREAS OF WORK FOR ADDITIONAL INFORMATION.

ME/P Engineers
JLRD, CONSULTING ENGINEERS
1450 Centerville Blvd, Suite 850
West Palm Beach, FL 33411
(561) 688-2303 TEL

Project
Client
Architect
Seal
Jose Murguio
Reg No. AR-10670

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ALL RIGHTS RESERVED. THESE DRAWINGS AND SPECIFICATIONS ARE INSTRUMENTS OF SERVICE AND SHALL REMAIN THE PROPERTY OF ZYSCOVICH, INC. WHETHER THE PROJECT FOR WHICH THEY WERE PREPARED IS COMPLETED OR NOT. THEY ARE NOT TO BE USED IN ANY MANNER ON OTHER PROJECTS OR EXTENDING TO THIS PROJECT EXCEPT BY THE EXPRESS WRITTEN AND WITH THE APPROPRIATE COMPENSATION TO ZYSCOVICH, INC. ANY UNAUTHORIZED REPRODUCTION OR USE WITHOUT THE WRITTEN CONSENT OF ZYSCOVICH, INC. IS PROHIBITED.

Rev Date Note

GENERAL NOTES

2025-0568-ASR1

02-18-2026

GN-100

Project
Client
Architect
Seal
Jose Murguio
Reg No. AR-10670

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Rev Date Note

GENERAL NOTES

2025-0568-ASR1

02-18-2026

GN-100

MIAMI-DADE COUNTY**DEPARTMENT OF REGULATORY AND ECONOMIC RESOURCES**<http://www.miamidade.gov/building/home.asp>

4/15/2026 3:25:16 PM

Tracking #	Process #	Permit #
2026011895	M2026011895	2026039338

THIS COPY OF PLANS MUST BE AVAILABLE ON BUILDING SITE OR AN INSPECTION WILL NOT BE MADE.				
Process #	Review	Disposition	Reviewer	Date
M2026011895	FIRE	A	FRIAS, JOSE	4/15/2026
M2026011895	IMPACT FEES	N	RICHARDSON, KEITH	4/10/2026
M2026011895	PAVING & DRAINAGE	A	DE TORRES, MAYRA	4/8/2026
M2026011895	RER ENVIRONMENTAL CORE	A	SARDINA, YALENYS	4/15/2026
M2026011895	UPFRONT FEES	A	WEB APPLICATION ID	3/23/2026
M2026011895	WASA	A	GARCIA, MAYTEE	4/8/2026

Disclaimer.

Subject to compliance with all Federal, State, and County Laws, rules and regulations. Miami-Dade County assumes no responsibility for accuracy of or results of these plans.

NOTICE: In addition to the requirements of this permit, there may be additional restrictions applicable to the property that may be found in the public records of this county, and there may be additional permits required from other governmental entities such as water management districts, state agencies or federal agencies.

Stamp Name	Trade	Disposition	Stamp Description
Approved	DERM	A	Approved
Approved	FIRE	A	Approved
Approved As Noted	FIRE	P	Approved as noted. See comments, markups and stamps.
Reference Only.	FIRE	R	Reference only.
Void	FIRE	V	Void



February 12, 2026

Gulliver Prep

6575 SW 88 Street

Pinecrest, Florida 33156

Project Name: Gulliver Prep Modular Classrooms

Project Location: 6575 SW 88 Street; Pinecrest, Florida 33156

Project Description: Code Interpretation (FFPC Section 14.3.5.2)

Existing Building, LLC (EB) has been retained by Gulliver Prep to review the prescriptive fire sprinkler requirements for the Gulliver Prep Modular Classrooms project. The project consists of ten separate modular buildings with a total area of roughly 8,000 square feet. Each modular building will be roughly 800 square feet in area and is intended to serve as a single classroom.

In accordance with Florida Fire Prevention Code (FFPC) Section 14.3.5.2, the extinguishment requirements of FFPC Section 14.3.5.1 may be exempt under certain conditions. The intent of this deliverable is to summarize the applicable code requirements and evaluate compliance with the stated exceptions.

It is the professional opinion of EB that this Code Interpretation provides a strategy that satisfies the applicable requirements for the Florida Fire Prevention Code.

Sincerely,

Existing Building, LLC

2448 Sunset Vista Drive

Spring Hill, FL 34607

1. Building Description

Gulliver Prep is an existing educational campus serving students in Grade 12 and below, located in Pinecrest, Florida. The campus spans approximately 40 acres and includes various academic and athletic facilities; however, the scope of this assessment is limited solely to the ten modular classrooms being installed on the property. The modular classrooms will be provided with new fire alarm devices, and a schematic of the ten modular buildings is shown in Figure 1.

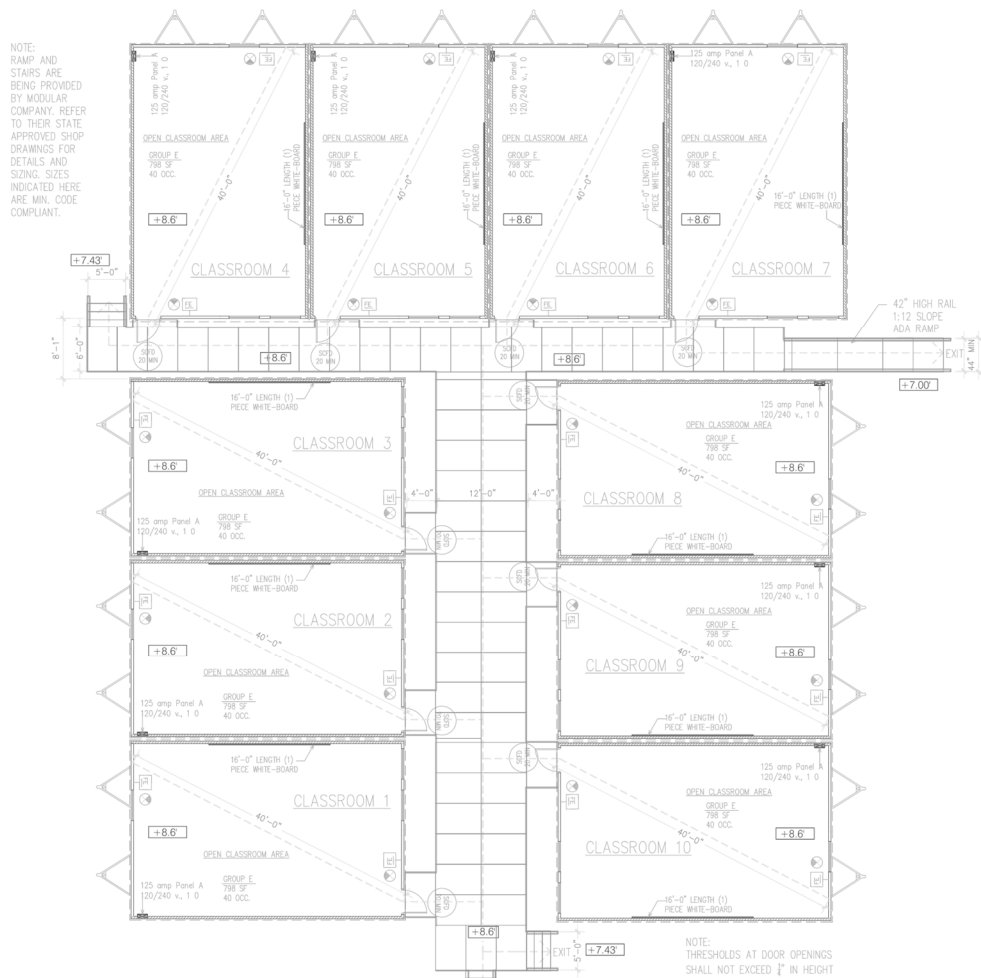


Figure 1: Gulliver Prep Modular Classroom

Based on the proposed use of the modular classrooms, the classrooms meet the classification for permanent buildings in accordance with the FFPC. Accordingly, EB respectfully requests that the buildings be classified as "non-relocatable" under FFPC Section 14.3.5.2.

2. Code Requirements

The FFPC generally categorizes buildings as either “permanent structures” or “temporary structures.” The definitions for each classification are provided below:

3.3.284.9 Permanent Structure.

A building or structure that is intended to remain in place for a period of more than 180 days in any consecutive 12-month period. (SAF-FUN)

3.3.284.10 Temporary Structure.

A building or structure not meeting the definition of *permanent structure*. (See also **3.3.284.9**, *Permanent Structure*.) (SAF-FUN)

Figure 2: FFPC Section 3.3.284

Based on the intended use of the Gulliver Prep modular buildings, the structures meet the definition of a permanent, Educational occupancy. However, the classifications outlined in Chapter 14 of the FFPC do not directly align with these definitions.

FFPC Section 14.3.5 further states that new educational occupancies are required to be protected throughout by an automatic fire sprinkler system, unless otherwise permitted by FFPC Section 14.3.5.2, as shown below:

14.3.5.2

The requirement of **14.3.5.1** shall not apply to any of the following:

- (1) Non-relocatable buildings having an area not exceeding 1000 ft² (93 m²)
- (2) Non-relocatable buildings containing a single classroom
- (3) The requirement of **14.3.5.1** shall not apply to relocatable buildings not exceeding 12,000 ft² (1120 m²) or complying with the allowed square footage and construction type requirements as indicated by the Florida Building Code, whichever is more stringent. Refer to the Florida Building Code, Types of Construction and the State Requirements for Educational Facilities.

Figure 3: FFPC Section 14.3.5.2

Each modular classroom is approximately 800 square feet and is intended to function solely as an individual classroom, satisfying the criteria of both FFPC Section 14.3.5.2(1) and FFPC Section 14.3.5.2(2). In addition, the combined area of the eight modular classrooms is approximately 8,000 square feet, which may independently qualify for the exception outlined in FFPC Section 14.3.5.2(3) applicable to relocatable buildings.

It is also important to note that the base edition of NFPA 101 includes additional building separation requirements for relocatable structures under NFPA 101 Section 14.3.5.2. However, this language has been amended out of the FFPC. Given this Florida-specific amendment relates to relocatable buildings, and considering that the proposed buildings more closely resemble non-relocatable structures, it is the professional opinion of EB that this unamended NFPA 101 requirement does not apply to the proposed project.

3. Code Interpretation

It is the understanding of EB that each modular classroom will remain in place for approximately two years, which exceeds the duration associated with a temporary structure under FFPC Section 3.3.284. Additionally, each unit will be provided with an interlocking deck system intended to support long-term use. This deck system will be fully permitted and would be inconsistent with the factory-built school buildings described in Florida Statute 553.415. Although the modular classrooms and deck systems could theoretically be disassembled, it is the professional opinion of EB that the overall condition more closely aligns with the intent of a non-relocatable building outlined by FFPC Section 14.3.5.2.

It is important to note that automatic fire sprinkler protection is not required in non-relocatable buildings that either (1) have an area not exceeding 1,000 square feet or (2) contain a single classroom. Each modular classroom will be approximately 800 square feet in area and will contain a single classroom, thereby independently meeting both of these exceptions. Additionally, it should be considered that each classroom is provided with an exterior door discharging directly to the outside without requiring travel through a shared corridor. Building separation distances will be further evaluated in accordance with the Florida Building Code.

Based on these factors, EB respectfully requests that each modular classroom be classified as a non-relocatable building meeting the fire sprinkler exceptions provided in FFPC Sections 14.3.5.2(1) and 14.3.5.2(2). It is the professional opinion of EB that this Code Interpretation provides a strategy that satisfies the applicable requirements for the Florida Fire Prevention Code.

Sincerely,

Existing Building, LLC

2448 Sunset Vista Drive

Spring Hill, FL 34607

Prepared By:

Jeffrey Marcic

Jeffrey Marcic

Principal

Reviewed By:

Matthew Foley

Matthew Foley, PE

Principal Fire Protection Engineer

Miami-Dade Department of Regulatory and Economic Resources

CONTACT INFORMATION FOR PERMIT APPLICATION

FIRST NAME <i>(print clearly)</i> John	LAST NAME <i>(print clearly)</i> Hench
MOBILE PHONE 561-267-0762	OFFICE/HOME PHONE
EMAIL <i>(required so you can be notified on the status of your plans)</i> jhench@cumming-group.com	

COMMENTS *(If you are submitting a municipal plan, please provide the municipal process number(s) and ensure the municipal application is in the office set of plans.)*

BL2026-0415 - Village of Pinecrest Building Permit Number. Gulliver Preparatory School, Inc. Temporary Modular Classrooms

Response to Comments submission for Impact Fees & Paving & Drainage

PLANS *(check all that apply)*

Please indicate if plans qualify for the following expedited plan reviews:

- | | | |
|--|---|--|
| ☐ GOV'T PROJECT/DEPT _____ | ☐ GREEN BLDG* <i>(new construction only)</i> | ☐ PACE PROJECT* |
| ☐ AFFORDABLE/WORKFORCE HOUSING* | ☐ ECONOMIC SIGNIFICANCE* | ☐ CONCIERGE |

*(*Pursuant to Ordinance 99-140; Ordinance 05-115; and Ordinance 08-51. Project may have additional requirements.)*

Further, under the penalty of perjury, I, being first duly sworn, depose and say that I have read the foregoing application and that the facts stated herein are accurate and true, including any boxes checked. I further acknowledge that this application and affidavit is subject to penalties of perjury, and acknowledge that Miami-Dade County reserves the right to revoke, cancel, void, or suspend, any permit issued pursuant to any application that contains any materially false or fraudulent statements, and acknowledge that continued operation of the uses after the permit is revoked, canceled, voided, or suspended, may subject me to enforcement penalties allowed by law. I further acknowledge that any application that contains any materially false or fraudulent statements may subject me to additional permitting fees including fees for expedited plan review services.

REQUESTED PLAN REVIEWS *(check all that apply for rework only)*

- | | | | | | |
|--------------------------------------|--|--|---|--|---|
| ☐ ALL | ☐ BLDG/HCAP | ☐ ELEC | ☐ ENRG | ☐ FIRE | ☐ ROOF |
| ☐ LANDSCAPING | ☐ MECH | ☐ PLUM | ☐ PWKS | ☐ PWCC | ☐ SIGN |
| ☐ STRU | ☐ ZNPR | ☐ WASD | ☐ PWIF | ☐ LPGX | ☐ SHOP DRAWING |
| ☐ DERM CORE | ☐ DERM AIR | ☐ DERM AIRPORT | ☐ DERM ASBESTOS | ☐ DERM COASTAL | ☐ DERM FLOOD |
| ☐ DERM GREASE | ☐ DERM INDUSTRIAL | ☒ DERM PAVING & DRAINAGE | ☐ DERM POLLUTION | ☐ DERM PRE-TREATMENT | ☐ DERM SOLID WASTE |
| ☐ DERM TANKS | ☐ DERM TREES | ☐ DERM WATER TREATMENT | ☐ DERM WETLANDS | ☐ PERMIT BY AFFIDAVIT CHECK | ☐ SHORT TERM EVENT AFFIDAVIT CHECK |
| ☐ DOH/HRS | ☐ DERM OSTD | | | | |

OPTIONAL PLAN REVIEWS *(check all that apply)*

- | | | | | |
|-------------------------------|-------------------------------|-------------------------------|-------------------------------|-------------------------------|
| ☐ BLDG | ☐ ELEC | ☐ MECH | ☐ PLUM | ☐ STRU |
|-------------------------------|-------------------------------|-------------------------------|-------------------------------|-------------------------------|

OPR DERM INITIAL REVIEWS *(check all that apply)*

- | | |
|------------------------------------|---|
| ☐ DERM CORE | ☐ DERM SPECIALTY <i>(You will be notified after core review is complete for additional fees)</i> |
|------------------------------------|---|

OPR DERM REWORK *(OPR for specialty only available at PIC)*

- | | | | | | |
|--------------------------------|-------------------------------------|-----------------------------------|--|-------------------------------|--|
| ☐ TREE | ☐ GREASE | ☐ ASBESTOS | ☐ COASTAL | ☐ AIR | ☐ PAVING & DRAINAGE |
| ☐ TANKS | ☐ INDUSTRIAL | ☐ WETLAND | ☐ PRE-TREATMENT | ☐ CORE | ☐ FLOOD |

FOR OFFICE USE ONLY *(to be completed by Permitting Staff)*

APPLICATION DATE	CLERK NAME	ARRIVAL TIME
PROCESS NUMBER	PROCESS NUMBER	PROCESS NUMBER
☐ RE-ISSUE	☐ PLAN REVISION	☐ REWORK
☐ SHOP DRAWING		

Miami-Dade Department of Regulatory and Economic Resources

CONTACT INFORMATION FOR PERMIT APPLICATION

FIRST NAME <i>(print clearly)</i> John	LAST NAME <i>(print clearly)</i> Hench
MOBILE PHONE 561-267-0762	OFFICE/HOME PHONE
EMAIL <i>(required so you can be notified on the status of your plans)</i> jhench@cumming-group.com	

COMMENTS *(If you are submitting a municipal plan, please provide the municipal process number(s) and ensure the municipal application is in the office set of plans.)*
BL2026-0415 - Village of Pinecrest Building Permit Number. Gulliver Preparatory School, Inc. Temporary Modular Classrooms

PLANS *(check all that apply)*

Please indicate if plans qualify for the following expedited plan reviews:

- | | | |
|--|---|--|
| ☐ GOV'T PROJECT/DEPT _____ | ☐ GREEN BLDG* <i>(new construction only)</i> | ☐ PACE PROJECT* |
| ☐ AFFORDABLE/WORKFORCE HOUSING* | ☐ ECONOMIC SIGNIFICANCE* | ☐ CONCIERGE |

*(*Pursuant to Ordinance 99-140; Ordinance 05-115; and Ordinance 08-51. Project may have additional requirements.)*

Further, under the penalty of perjury, I, being first duly sworn, depose and say that I have read the foregoing application and that the facts stated herein are accurate and true, including any boxes checked. I further acknowledge that this application and affidavit is subject to penalties of perjury, and acknowledge that Miami-Dade County reserves the right to revoke, cancel, void, or suspend, any permit issued pursuant to any application that contains any materially false or fraudulent statements, and acknowledge that continued operation of the uses after the permit is revoked, canceled, voided, or suspended, may subject me to enforcement penalties allowed by law. I further acknowledge that any application that contains any materially false or fraudulent statements may subject me to additional permitting fees including fees for expedited plan review services.

REQUESTED PLAN REVIEWS *(check all that apply for rework only)*

- | | | | | | |
|---|--|---|---|--|---|
| ☐ ALL | ☐ BLDG/HCAP | ☐ ELEC | ☐ ENRG | ☒ FIRE | ☐ ROOF |
| ☐ LANDSCAPING | ☐ MECH | ☐ PLUM | ☐ PWKS | ☐ PWCC | ☐ SIGN |
| ☐ STRU | ☐ ZNPR | ☐ WASD | ☐ PWIF | ☐ LPGX | ☐ SHOP DRAWING |
| ☒ DERM CORE | ☐ DERM AIR | ☐ DERM AIRPORT | ☐ DERM ASBESTOS | ☐ DERM COASTAL | ☐ DERM FLOOD |
| ☐ DERM GREASE | ☐ DERM INDUSTRIAL | ☐ DERM PAVING & DRAINAGE | ☐ DERM POLLUTION | ☐ DERM PRE-TREATMENT | ☐ DERM SOLID WASTE |
| ☐ DERM TANKS | ☐ DERM TREES | ☐ DERM WATER TREATMENT | ☐ DERM WETLANDS | ☐ PERMIT BY AFFIDAVIT CHECK | ☐ SHORT TERM EVENT AFFIDAVIT CHECK |
| ☐ DOH/HRS | ☐ DERM OSTD | | | | |

OPTIONAL PLAN REVIEWS *(check all that apply)*

- | | | | | |
|-------------------------------|-------------------------------|-------------------------------|-------------------------------|-------------------------------|
| ☐ BLDG | ☐ ELEC | ☐ MECH | ☐ PLUM | ☐ STRU |
|-------------------------------|-------------------------------|-------------------------------|-------------------------------|-------------------------------|

OPR DERM INITIAL REVIEWS *(check all that apply)*

- | | |
|---|---|
| ☒ DERM CORE | ☐ DERM SPECIALTY <i>(You will be notified after core review is complete for additional fees)</i> |
|---|---|

OPR DERM REWORK *(OPR for specialty only available at PIC)*

- | | | | | | |
|--------------------------------|-------------------------------------|-----------------------------------|--|-------------------------------|--|
| ☐ TREE | ☐ GREASE | ☐ ASBESTOS | ☐ COASTAL | ☐ AIR | ☐ PAVING & DRAINAGE |
| ☐ TANKS | ☐ INDUSTRIAL | ☐ WETLAND | ☐ PRE-TREATMENT | ☐ CORE | ☐ FLOOD |

FOR OFFICE USE ONLY *(to be completed by Permitting Staff)*

APPLICATION DATE	CLERK NAME	ARRIVAL TIME
PROCESS NUMBER	PROCESS NUMBER	PROCESS NUMBER
☐ RE-ISSUE	☐ PLAN REVISION	☐ REWORK
☐ SHOP DRAWING		

NOTE: ALL SHEETS MUST BE REVIEWED

MIAMI-DADE COUNTY DEPARTMENT OF REGULATORY AND ECONOMIC RESOURCES

Herbert S. Saffir Permitting and Inspection Center

11805 SW 26th Street (Coral Way) • Miami, Florida 33175-2474 • (786) 315-2000

APPLICATION FOR MUNICIPAL PERMIT APPLICANTS THAT REQUIRE PLAN REVIEW FROM MIAMI-DADE FIRE RESCUE AND/OR ENVIRONMENTAL SERVICES

PROVIDE MUNICIPAL PROCESS NUMBER HERE					
LOCATION OF IMPROVEMENTS	Job Address <u>6575 SW 88 ST</u>			CONTRACTOR INFORMATION	Contractor No. <u>CGC 1517443</u>
	Folio <u>20-4036-000-0750</u>				Last four (4) digits of Qualifier No. _____
	Lot _____ Block _____				Contractor Name <u>Green Label Construction, Inc.</u>
	Subdivision _____ PBpg _____				Qualifier Name <u>Julio Zaldivar Jr.</u>
	Metes and bounds _____				Address <u>7105 SW 47 ST #403</u>
					City <u>Miami</u> State <u>FL</u> Zip <u>33155</u>
TYPE OF IMPROVEMENTS	☒ New Construction on Vacant Land ☐ Alteration Interior ☐ Alteration Exterior ☐ Relocation of Structure ☐ Enclosure ☐ Repair ☐ Repair Due to Fire		☐ Demolish ☐ Shell Only ☐ Addition Attached ☐ Addition Detached ☐ Re-Roof ☐ Foundation Only ☐ Tent		Current use of property <u>School</u>
					Description of Work <u>Temporary Modular Classrooms</u>
PERMIT TYPE	☒ MBLD* Category <u>01</u> ☐ MELE _____ ☐ MPLU _____ ☐ MLPG _____ ☐ MMEC _____ ☐ FIRE _____	REVIEW STATUS	☐ Chg. Contractor ☐ Re-Issue ☐ Re-Stamp ☐ Revision ☐ Not Applicable for Fire	PROPERTY OWNER'S INFO	Owner <u>Gulliver Preparatory School, Inc.</u>
					Address <u>6575 SW 88 ST</u>
PERMIT CONTACT	Name <u>John Hench</u>			ARCHITECT / ENGINEER	Owner _____
	Address <u>545 NW 26 ST, Suite 610</u>				Address _____
	City <u>Miami</u> State <u>FL</u> Zip <u>33127</u>				City _____ State _____ Zip _____
	Phone _____				Phone _____
FIRE SPECIAL REQUEST PLAN REVIEW (SRI)	I am requesting a Special Request Plan Review (SRI) to be scheduled as soon as possible. There is a minimum charge of one-hour. Please contact the Fire Department for current rate.				
	1 st Request: _____ Date: _____ 2 nd Request: _____ Date: _____ 3 rd Request: _____ Date: _____				
If the applicant is a known named violator with: unpaid civil penalties; unpaid administrative costs of hearing; unpaid County investigative, enforcement, testing, or monitoring costs; or unpaid liens, any or all of which are owed to Miami-Dade County pursuant to the provisions of the Code of Miami-Dade County, Florida, a hold on the review may be placed on this application.					



BUILDING PERMIT CATEGORIES

CATEGORY	DESCRIPTION	PERMIT TYPE
BUILDING		
01	GENERAL BUILDING—COMMERCIAL	MBLD
02	SUB—GENERAL BUILDING—RESIDENTIAL	MBLD
08	CANVAS AWNING	MBLD
10	COMMUNICATION TOWER	MBLD
15	DEMOLITION	MBLD
29	METAL AWNING & STORM SHUTTER	MBLD
48	SCREEN ENCLOSURES	MBLD
51	MURAL SIGNS (NON-ELECTRICAL)	MBLD
55	SWIMMING POOL	MBLD
56	TENNIS COURTS (SURFACE PAVING)	MBLD
86	TRAILER TIE DOWN	MBLD
88	WALK-IN COOLER	MBLD
91	MARINAS	MBLD
92	LOW SLOPE APPLICATIONS (GRAVEL, SMOOTH MODIFIED, SINGLE PLY)	MBLD
95	SHINGLES (ASPHALT, FIBERGLASS)	MBLD
96	SHINGLES (METAL ROOFS/WOOD SHINGLES & SHAKE)	MBLD
97	STAGE 2 VAPOR RECOVERY SYSTEM	MBLD
99	SOIL IMPROVEMENT	MBLD
0100	BULK STORAGE PROPANE TANK	MBLD
0101	REMOVABLE STORM PANELS	MBLD
0107	TILE ROOF	MBLD
0110	WATER MAIN	MBLD
0111	SITE PLAN	MBLD
0112	INDOOR EVENT/EXHIBIT	MBLD
0114	STATE, FEDERAL GOVERNMENT & SCHOOL BOARD PROPERTIES REVIEW	MBLD
ELECTRICAL		
04	FIRE ALARM SPECIALTY	MELE
16	SPECIALTY WIRING	MELE
34	SOLAR PHOTOVOLTAIC	MELE
38	GENERATORS	MELE
40	BUILDING PUBLIC RADIO ENHANCEMENT SYSTEM	MELE
PLUMBING		
0020	SEWER CONNECTION TO PUBLIC SYSTEM (THIS CATEGORY IS USED WHEN NO BUILDING PERMIT EXIST)	MPLU
0024	INTERCEPTOR/GREASE TRAPS (REPLACEMENT OR INSTALLATION THAT IS NOT PART OF A BUILDING PERMIT)	MPLU
LPGX		
01	LIQUEFIED PETROLEUM GAS	MLPG
02	MISCELLANEOUS	MLPG
04	LIQUEFIED PETROL. GAS/STATE	MLPG
MECHANICAL		
09	ABOVE/BELOW GROUND TANKS/PUMPS & POLLUTANT STORAGE SYSTEM	MMEC
38	COMMERCIAL HOODS	MMEC
43	FIRE CHEMICAL	MMEC
46	SPRAY BOOTHS	MMEC
48	SMOKE CONTROL	MMEC
52	RESIDENTIAL ELEVATOR	MMEC
52	FIRE SPRINKLER	FIRE

Miami - Dade County Building Department

Gulliver Preparatory School, Inc. - Temporary Modular Classrooms

Narrative Response to Comments for M Process # M2026011895

Impact Fees:

Comment: WHAT IS THE REASON OF THE TEMPORARY TRAILERS? PROVIDE BUILDING PERMIT FOR THE CONSTRUCTION RELATED TO THE TRAILERS. BEVERLY.GUTIERREZ@MIAMIDADE.GOV.

Response: The temporary trailers are required for the displaced classroom space from the building being demolished under MDC Permit # 2026034121 approved and issued on (03.23.2026). We will also be submitting in May of 2026 another permit for the new construction permit. To avoid impacting the schools' operations and maintaining our schedule, it is prudent for the team to demolish the existing building over the summer and have classroom space for the school year. There is no increase in enrollment. We completed a similar submission in 2022 under M process #M2022012341.

Paving & Drainage:

Comment: This facility is served by the Surface Water Management General Permit (SWMGP)-13-80068-P. Approved plans do not match proposed. Please submit an application for modifications in accordance with Rule 62-330.315, F.A.C. For detailed guidance on the permitting process or to confirm exemption applicability, please contact the DERM Water Control Section at (305) 372 6681 or via dermwatercontrol@miamidade.gov. Please be advised that the approval of this process is contingent upon the approval of the SWMGP.

Response: Per email correspondence with Mayra De Torres from (03.25.2026) a letter from the Civil Engineer is being submitted noting the units are elevated and not installed to the ground.

Thank you.

Miami - Dade County Building Department

Gulliver Preparatory School, Inc. - Temporary Modular Classrooms

Narrative Response to Comments for M Process # M2026011895

Fire:

Comment: THE ANALYSIS TO EXEMPT THE FIRE SPRINKLER INSTALLATION REQUIREMENT OF NFPA

101: 14.3.5.1 BASED OF EXEMPTIONS REFERENCED IN SECTIONS 14.3.5.2 (1) AND 14.3.5.2 (2) REQUIRES DISCUSSION THROUGH A MEETING INCLUDING OWNER REPRESENTATIVE, DESIGN PROFESSIONAL, FIRE PLANS PROCESSOR, AND CAPT. RAUL RODRIGUEZ.

PLEASE SUBMIT THE MEETING REQUEST TO THE FOLLOWING EMAIL ADDRESS:

MDFR-FEB-MEETINGS@MIAMIDADE.GOV

IF ASSISTANCE IS NEEDED, PLEASE CONTACT THE FIRE ENGINEERING BUREAU AT (786) 315-2771 (OPTION 1)..

Response: Per meeting held Friday (04.10.2026) with Captain Raul Rodriguez and Fire Plans Reviewer Jose Frias at 9:00 AM, the design team has added the requested note from our submitted Life Safety Consultant letter to plan sheet A – 201 noting the fire sprinkler exemption.

RER Environmental Core:

Comment: Water & Sewer verification form is required from the Water & Sewer Utility Company serving the property. (Also referred to as Plumbing Section Form).

Response: Verification form has been uploaded directly to the permitting portal by WASA and review has been approved.

Thank you.

PERMITTING HOURS

8:00 AM TO 2:00 PM

MONDAY - FRIDAY

CONTACT INFORMATION

T: 305.234.2121 F: 305.234.2133

building@pinecrest-fl.gov

INSPECTIONS: 305.234.2111

INSURANCE RENEWALS: renewals@pinecrest-fl.gov

**VILLAGE OF PINECREST**

Building & Planning Department

12645 Pinecrest Parkway,
Pinecrest, Florida 33156**MASTER PERMIT #** BL2026-0415**SUB PERMIT #** _____**INSTRUCTIONS**

STEP 1. APPLICATION WHICH MUST BE SIGNED BY THE PROPERTY OWNER & QUALIFIER; BOTH SIGNATURES MUST BE NOTARIZED.

PERMITS/PLANS DROP OFF CLOSING @ 2:00 P.M.

STEP 2. SUBMIT THE COMPLETED APPLICATION WITH ALL NECESSARY DOCUMENTS TO THE BUILDING AND PLANNING DEPARTMENT FOR PROCESSING. REFER TO PERMIT CHECKLIST ONLINE AT:

[HTTPS://WWW.PINECREST-FL.GOV/Government/Building/Apply-for-a-Permit/All-Permits.](https://www.pinecrest-fl.gov/Government/Building/Apply-for-a-Permit/All-Permits)

STEP 3. PROJECTS OVER \$2,500 REQUIRE RECORDED NOTICE OF COMMENCEMENT POSTED AT JOBSITE PRIOR TO COMMENCEMENT OF CONSTRUCTION.

APPLICATION

JOB ADDRESS: 6575 SW 88 ST, Pinecrest, FL 33156 FOLIO #: 20-4036-0000-750

DESCRIPTION OF WORK: Temporary Modular Classrooms FLOOD ZONE: X AH7 AE7 AE10 AE11

VALUE OF WORK: \$100,000.00 PROJECT SQ. FT.: 8,000 NO. OF FLOORS: 1

CONTACT: John Hench EMAIL: jhench@cumming-group.com PHONE: 561-267-0762

PERMIT TYPE	(✓)	PERMIT CHANGE	(✓)	IMPROVEMENT TYPE	(✓)	CATEGORY	(✓)
BUILDING	✓	REVISION		NEW CONSTRUCTION	✓	COMMERCIAL	✓
ELECTRICAL		RENEWAL		ALTERATION		RESIDENTIAL	
MECHANICAL		CHG OF CONTRACTOR		ADDITION		APPRAISED BLDG. VALUE	
PLUMBING		CHG OF ARCH/EGR		DEMOLITION		\$	
ZONING							

PROPERTY OWNER

NAME	Gulliver Preparatory School, Inc.
ADDRESS	6575 SW 88 ST, Pinecrest, FL 33156
PHONE	305-669-5476
EMAIL	

CONTRACTOR

COMPANY	Green Label Constuction, Inc.
QUALIFIER	Julio Zaldivar Jr.
ADDRESS	7105 SW 47th Street, #403 Miami, FL 33155
LICENSE #	CGC1517443
PHONE	305-262-1339
EMAIL	julio@greenlabelconstruction.com

ARCHITECT

NAME	Zyscovich/Stratus
LICENSE #	AR10670
PHONE	305-372-2222
EMAIL	info@zyscovich.com

ENGINEER

NAME	
LICENSE #	
PHONE	
EMAIL	

OFFICE USE ONLY☐ PERMIT CLERK REVIEW COMPLETE.

UPFRONT FEE AMOUNT PAID \$ _____ (if applicable)

FEES TYPE	AMOUNT	FEES TYPE	AMOUNT	SECTION	BY	DATE
SCAN FEE (\$3/PG)		ZONING/PLANNING		ZONING/PLANNING		
BUILDING FEES		SOLID WASTE		ELECTRICAL		
STORMWATER REVIEW		MUNICIPAL		MECHANICAL		
MIAMI-DADE COUNTY		STORM DRAINAGE		PLUMBING		
FL DBR		IMPACT ADMIN		PUBLIC WORKS		
FL DCA		SIDEWALK		STRUCTURAL		
FINES		CONCURRENCY		STORMWATER		
CERT. OF OCCUPANCY		EXPEDITE- REWORK		BUILDING		
				BUILDING OFFICIAL		
TOTAL DUE:				300 Copy		

Miami Dade County Department of Regulatory and Economic Resources
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 VoP Permit Application.pdf

Examiner Date Time Stamp Trade Stamp Name
 FRIAS, JOSE 4/1/2026 2:09:57 PM FIRE Reference Only.

CONDITIONS OF APPROVAL

☐ **CONDITION DESCRIPTION:**

IMPORTANT NOTICES

1. DO NOT BEGIN ANY WORK WITHOUT HAVING RECEIVED YOUR VALIDATED PERMIT AND PERMIT CARD. APPLYING FOR A PERMIT DOES NOT GRANT THE RIGHT TO BEGIN CONSTRUCTION. HOURS OF CONSTRUCTION ARE LIMITED TO MONDAY THROUGH FRIDAY FROM 7:00 A.M. TO 6:30 P.M., SATURDAYS FROM 8:00 A.M. TO 4:00 P.M. NO CONSTRUCTION ON SUNDAYS OR STATE HOLIDAYS. NO INSPECTIONS WILL BE CONDUCTED ON WEEKENDS OR HOLIDAYS.
2. ALL CONSTRUCTION AND/OR DEMOLITION AREAS MUST BE MAINTAINED IN A CLEAN, NEAT AND SANITARY CONDITION FREE FROM CONSTRUCTION DEBRIS.
3. STREETS AND NEIGHBORING PROPERTIES SHALL BE KEPT FREE FROM DIRT AND DEBRIS.
4. SWALES MUST BE PROTECTED FROM BEING DAMAGED BY EQUIPMENT OR VEHICLES.
5. CONSTRUCTION TRAILERS ARE PROHIBITED ON SINGLE FAMILY RESIDENTIAL CONSTRUCTION SITES. OTHER CONSTRUCTION MAY HAVE A TRAILER, WHICH REQUIRES A SEPARATE PERMIT.
6. PORTABLE TOILETS FOR A CONSTRUCTION SITE REQUIRE A SEPARATE PERMIT.
7. DO NOT DISCHARGE WATER INTO THE RIGHT OF WAY OR STORM DRAINS WITHOUT APPROVAL FROM THE BUILDING AND PLANNING DEPARTMENT.
8. EQUIPMENT AND MATERIALS SHALL BE STORED WITHIN YOUR PROPERTY, NOT ON PUBLIC RIGHT OF WAY.
9. FLORIDA DEPARTMENT OF HEALTH APPROVAL IS REQUIRED FOR APPLICATIONS INVOLVING SEPTIC TANKS. DEPARTMENT OF ENVIRONMENTAL RESOURCES MANAGEMENT (DERM) AND/OR MIAMI DADE WATER AND SEWER DEPARTMENT (MDWASD) APPROVAL IS REQUIRED FOR APPLICATIONS INVOLVING SEWERS AND WATER.
10. THE ISSUANCE OF A DEVELOPMENT PERMIT BY THE VILLAGE DOES NOT IN ANY WAY CREATE ANY RIGHT ON THE PART OF AN APPLICANT TO OBTAIN A PERMIT FROM A STATE OR FEDERAL AGENCY AND DOES NOT CREATE ANY LIABILITY ON THE PART OF THE VILLAGE FOR ISSUANCE OF THE PERMIT IF THE APPLICANT FAILS TO OBTAIN REQUISITE APPROVALS OR FULFILL THE OBLIGATIONS IMPOSED BY A STATE OR FEDERAL AGENCY OR UNDERTAKES ACTIONS THAT RESULT IN A VIOLATION OF STATE OR FEDERAL LAW. IN ADDITION, ALL APPLICABLE STATE AND FEDERAL PERMITS SHALL BE OBTAINED BY THE APPLICANT BEFORE COMMENCEMENT OF THE DEVELOPMENT.

AFFIDAVIT

APPLICATION IS HEREBY MADE TO OBTAIN A PERMIT TO DO WORK AND INSTALLATION AS INDICATED. I, THE OWNER OF THE PROPERTY, CERTIFY THAT ALL WORK WILL BE PERFORMED TO MEET THE STANDARDS OF ALL LAWS REGARDING CONSTRUCTION IN THE VILLAGE OF PINECREST. I UNDERSTAND THAT SEPARATE PERMITS ARE REQUIRED FOR ELECTRICAL, PLUMBING, POOL, EXTERIOR DOOR, MECHANICAL, WINDOW, FENCE, DRIVEWAY, ROOFING, SHUTTERS AND SIGNS AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL AGENCIES.

I, THE OWNER OF THE PROPERTY, HAVE DISCLOSED ALL INFORMATION RELATED TO ANY WORK AT THE PROPERTY PERFORMED IN THE PRIOR ONE (1) YEAR TO THE BUILDING OFFICIAL. FURTHER, I AM FULLY AWARE THAT IF THE CUMULATIVE COST OF WORK TO MY HOME OR BUSINESS UNDER THIS AND ANY OTHER PERMIT DURING THE ONE (1) YEAR PERIOD BEGINNING ON THE DATE OF THE FIRST IMPROVEMENT OR REPAIR TO THE STRUCTURE SUBSEQUENT TO JANUARY 1, 2015 EXCEEDS FIFTY PERCENT (50%) OF THE MARKET VALUE OF THE STRUCTURE, THE ENTIRE STRUCTURE MUST MEET THE PRESENT FEDERAL FLOOD CRITERIA FOR FINISHED FLOOR ELEVATION PLUS 1 FOOT. I AM ALSO FULLY AWARE THAT IF THE WORK AREA TO MY HOME OR BUSINESS UNDER THIS AND ANY OTHER PERMIT EQUALS OR EXCEEDS FIFTY PERCENT (50%) OF THE BUILDING AREA, THEN THE ALTERATION MUST CONFORM TO THE CURRENT CODE REQUIREMENTS OF THE FLORIDA BUILDING CODE, EXISTING BUILDINGS SECTION 604 ALTERATION LEVEL 3.

I, THE OWNER OF THE PROPERTY, UNDERSTAND THAT A PERMIT APPLICATION IS SUBJECT TO DENIAL AND A VALIDATED PERMIT OR PERMIT CARD IS SUBJECT TO REVOCATION OR MODIFICATION BASED UPON APPLICABLE DEEDS, COVENANTS, DECLARATIONS, EASEMENTS AND ANY OTHER LEGAL RESTRICTION. BY ISSUING A PERMIT, THE VILLAGE MAKES NO REPRESENTATION AS TO THE EXISTENCE OR VALIDITY OF ANY PROPERTY RESTRICTION.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU ARE SPENDING MORE THAN \$2,500 OR INTEND TO OBTAIN FINANCING, YOU MAY WISH TO CONSULT WITH YOUR ATTORNEY OR LENDER BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT. THE NOTICE OF COMMENCEMENT MUST BE RECORDED AT: 22 N.W. 1ST STREET, 1ST FLOOR (305) 275-1155 EXTENSION 6. ONCE RECORDED, THE NOTICE OF COMMENCEMENT MUST BE POSTED AT THE JOB SITE IN ACCORDANCE WITH SECTION 713.35 OF FLORIDA STATUTES

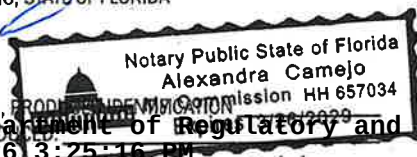
STATE OF FLORIDA, COUNTY OF MIAMI-DADE

SIGNATURE OF OWNER

PRINT NAME SWORN TO AND SUBSCRIBED BEFORE ME THIS 12 DAY
OF March, 2026

SIGNATURE OF NOTARY PUBLIC, STATE OF FLORIDA

SEAL:



PERSONALLY KNOWN ☒ OR, PRODUCED IDENTIFICATION
Miami Dade County Department of Regulatory and Economic Resources
2026011895 - 4/15/2026 3:25:16 PM
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FRIAS, JOSE 4/1/2026 2:09:57 PM FIRE Reference Only.

STATE OF FLORIDA, COUNTY OF MIAMI-DADE

SIGNATURE OF CONTRACTOR QUALIFIER

PRINT NAME SWORN TO AND SUBSCRIBED BEFORE ME THIS 12th DAY OF
march, 2026

SIGNATURE OF NOTARY PUBLIC, STATE OF FLORIDA

SEAL:

