

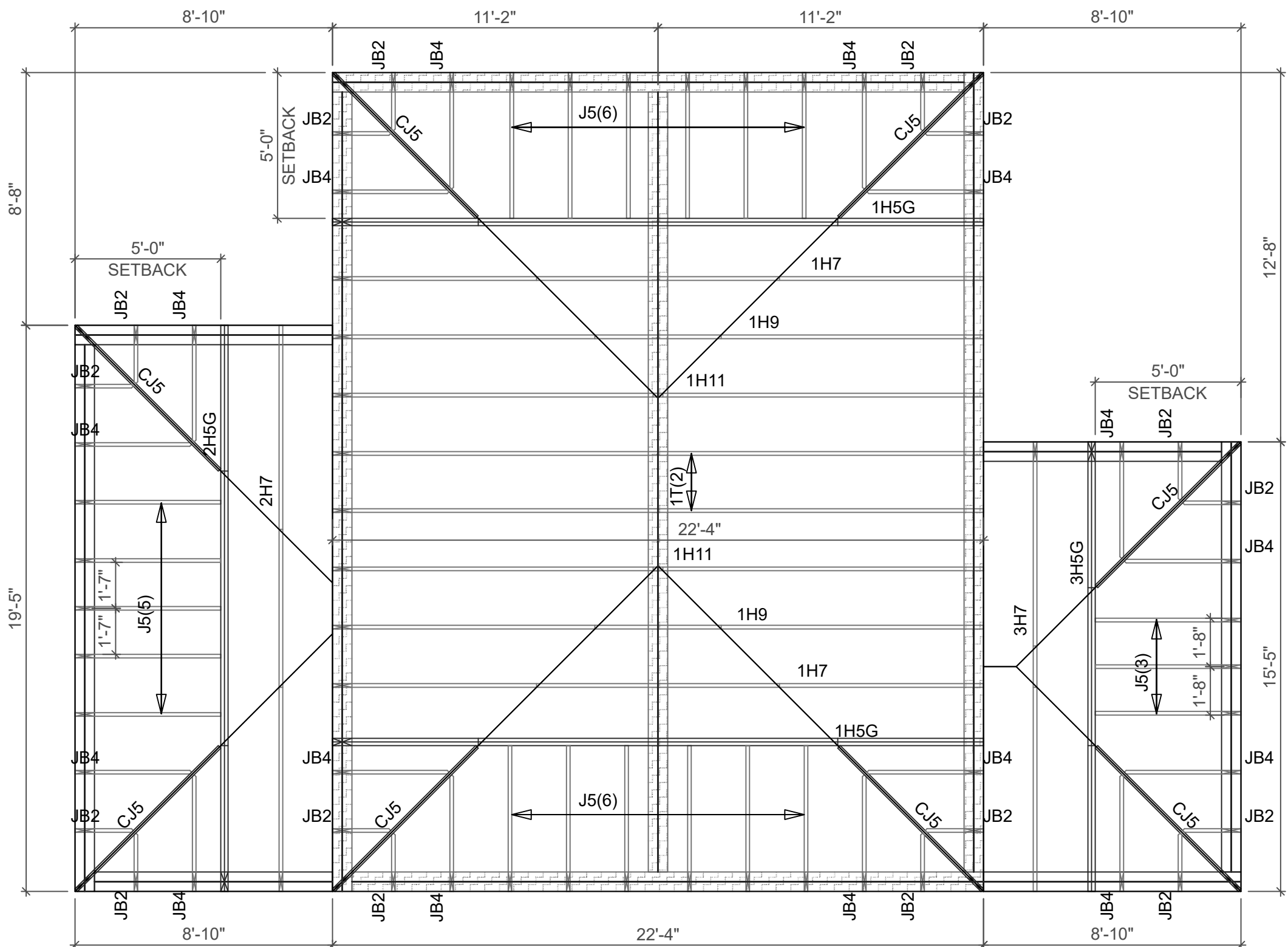


1930 N Commerce Parkway, Suite 1 / Weston, Florida 33326

info@conengineers.com

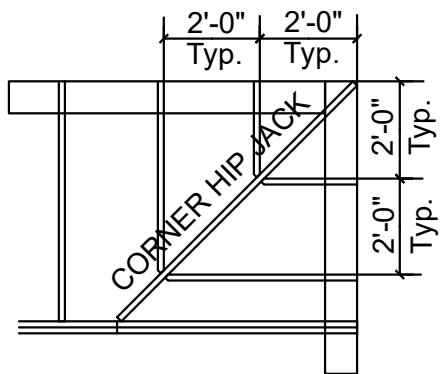
PINECREST CONSTRUCTION SUBMITTAL FORM

SUBMITTAL N°	SUBMITTAL NAME	PROJECT ID	PROJECT NAME			
5	TRUSSES	2025-017	GARY MATZNER PARK CONSTRUCTION			
	PROJECT LOCATION :	6610 NORTH KENDALL DRIVE / PINECREST, FLORIDA 33156				
PROJECT MANAGER		DATE OF SUBMISSION				
MAURICIO MANCO		Friday, November 21, 2025				
SUBMITTED TO	SUBMITTAL TYPE					
DISTRIBUTION LIST	X	SHOP DRAWING	PRODUCT DATA	COMP. FORM / QUALITY REC.	QUALITY SYSTEM DOC	SPECIFICATIONS
VILLAGE OF PINECREST	X	ENGINEERING PACKAGE				
DESCRIPTION OF SUBMITTAL						
This submittal is being submitted for approval by the Village of Pinecrest and contains the information regarding the TRUSSES to be installed in the park.  JUAN CARLOS PEREZ						
ATTACHMENTS						
PDF FORMAT ARCHIVES: 2025 11 21 - 0 SMT 05 - MARTINEZ Trusses Complete Package						
APPROVAL COMMENTS						
WJA COMMENT: CONTRACTOR SHALL CONSTRUCT THE DECORATIVE RAFTER AS INDICATED ON SHEET S-303 DETAIL H. CONTRACTOR SHALL COORDINATE DECORATIVE RAFTER STYLE TO COMPLY WITH THE VILLAGE OF PINECREST'S APPROVED ARCHITECTURAL GUIDELINES/PREFERENCES.				STAMP ☐ NO EXCEPTION TAKEN ☒ APPROVED AS NOTED ☐ REJECTED ☐ SUBMIT SPECIFIED ITEM ☐ REVISE AND RESUBMIT Checking is only for general conformance with the design concept of the project and general compliance with the information given in the contract documents. Any action shown is subject to the requirements of the plans and specifications. Contractor is responsible for: Dimensions which shall be confirmed and correlated at the job site; fabrication processes and techniques of construction; coordination of the work, coordination of the work of other trades and satisfactory performance of the work. Wannemacher Jensen Architects, Inc. Date: NOVEMBER 25 2025 By: David Pinto		
CLIENT POINT OF CONTRACT		REVIEW STATUS				
		APPROVED	APPROVED AS NOTED	DISAPPROVED - RESUBMIT	INCOMPLETE - RESUBMIT	
		OTHER (DESCRIBE) -->				
PREPARED BY	PREPARER TITLE	DATE	APPROVED BY	APPROVING TITLE	DATE	
JUAN CARLOS PEREZ	CIVIL ENGINEER	November 21, 2025	JUAN CARLOS PEREZ	PROJECT MANAGER	November 21, 2025	

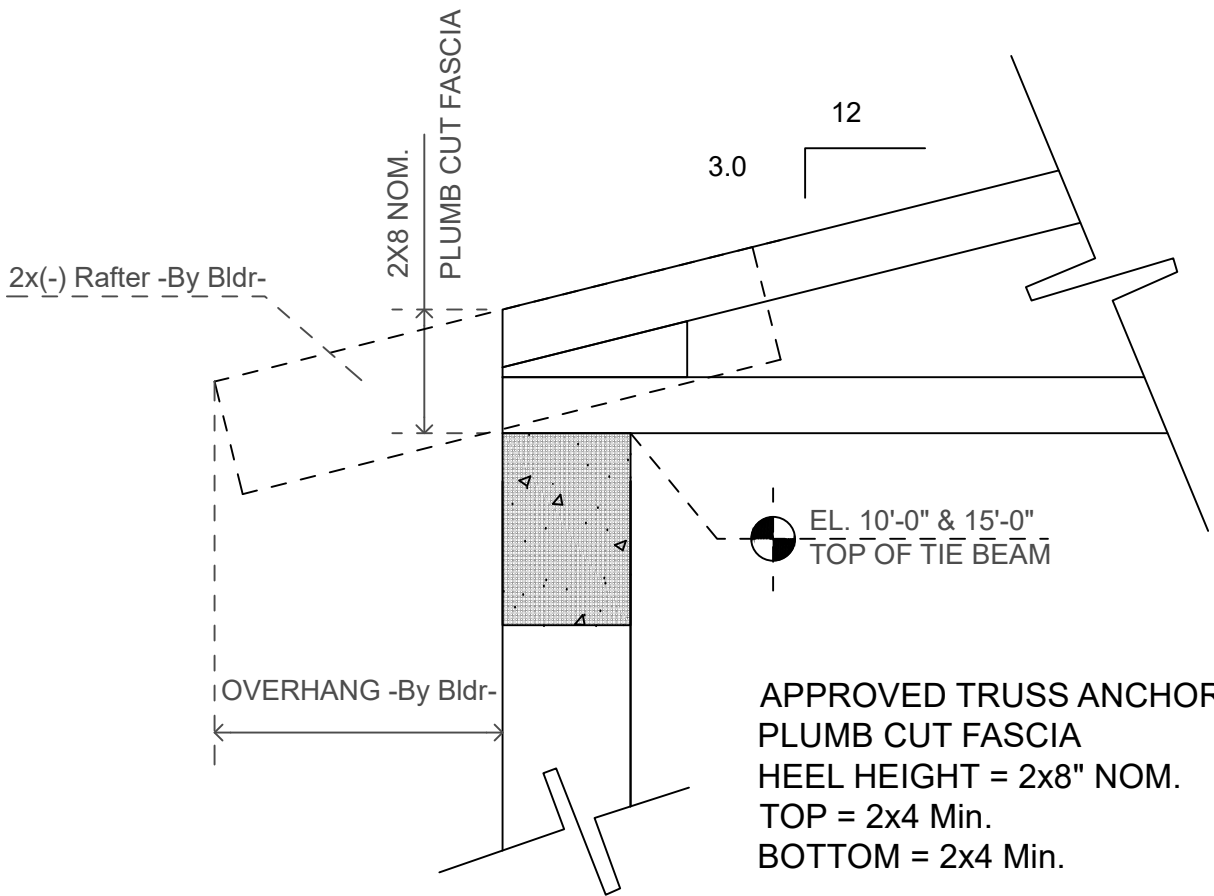


ROOF TRUSS LAYOUT
Scale: 1/4": 1'-0"

HATCH LEGEND	
	EL. 10'-0" TOP OF TIE BEAM
	EL. 15'-0" TOP OF TIE BEAM



Typ. OVERHANG CORNER SET
Scale: N.T.S.



Typ. ROOF TRUSS END DETAIL
Scale: 1": 1'-0"

HANGER LIST					
NOTE: IN ORDER FOR HANGER TO ACHIEVE SPEC. LOAD CAPACITY ALL NAIL HOLES MUST BE FILLED ON CONNECTOR.					
QTY	MODEL	GRAVITY LOAD	UPLIFT	MFR.	TRUSS IDENTIFICATION
0	HUS-26	3,230	1,320	SIMPSON	N/A
0	HUS-26-2	1,320	1,165	SIMPSON	N/A
0	HUS-28-2	1,980	1,320	SIMPSON	N/A
0	LUS-26	1,060	1,165	SIMPSON	N/A
0	LUS-26-2	1,060	1,165	SIMPSON	N/A
0	LUS-26-2	1,265	1,060	SIMPSON	N/A
0	HGUS-26	5,230	1,040	SIMPSON	N/A
0	HGUS-26-2	5,230	2,155	SIMPSON	N/A
0	HGUS-26-3	5,230	2,155	SIMPSON	N/A
0	THJU-26 2x4	1,915	780	SIMPSON	N/A
0	THJU-26 2x5	2,350	1,310	SIMPSON	N/A
0	SUR-26 BY BLDG	1,055	675	SIMPSON	N/A
0	SUL-26 BY BLDG	1,055	675	SIMPSON	N/A
0	SUR-26	1,055	675	SIMPSON	N/A
0	SUL-26	1,055	675	SIMPSON	N/A
0	HHUS-46	3,415	1,320	SIMPSON	N/A
0	GTWS-2T BY BLDG	8,720	8,435	USP	N/A
0	GTWS-3T BY BLDG	11,100	12,490	USP	N/A
0	MTW-16 STRAP BY BLDG	N/A	965	USP	N/A

THE GRAVITY AND UPLIFT REACTIONS SHOWN ON ENG. & LAYOUT ARE MAXIMUM VALUES BASED ON WORST CASE SCENARIOS, AND ARE USED IN CALCULATING THE TRUSS ITSELF.

NOTE: SEE TRUSS CALCULATION CUT SHEETS FOR SPECIFIC JACK & CORNER JACK CONNECTIONS

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS.

REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

APPROVED BY:	DATE:
Print Name: _____	_____
Signature: _____	_____

TRUSS REACTIONS LIST			
TRUSS ID	REACTIONS		
1H5G (2M)	R= 933 lbs U= -342 lbs	R= 2805 lbs U= -1043 lbs	R= 933 lbs U= -342 lbs
2H5G (2M)	R= 2094 lbs U= -765 lbs	R= 2904 lbs U= -765 lbs	
3H5G (2M)	R= 1678 lbs U= -612 lbs	R= 1678 lbs U= -612 lbs	

REVIEW OF STRUCTURAL SUBMITTAL PREPARED BY SPECIALTY ENGINEER

NO EXCEPTION TAKEN	☒
MAKE CORRECTIONS INDICATED	☐
REJECTED/RESUBMIT	☐

DATE: November 25 2025 REVIEWED BY: Y.R

SCOPE OF REVIEW CONSISTS OF:
1. That the specified structural submittal has been furnished.
2. That the submittal has been signed and sealed by a Florida Registered Professional Engineer.
3. That the specialty engineer has understood our intent and used the specified criteria (No detailed check of calculations was performed)
4. That the configuration set forth in the structural submittal is consistent with that on the structural construction documents. (No check of dimensions was performed except at interface areas).

DDA ENGINEERS, P.A.
4930 SW 74th Court
Miami, FL 33155
(305) 666-0711

TRUSS LOADING

FBC 8th Ed. 2023

ROOF TRUSSES	FLOOR TRUSSES
Top Chord Live: 30	Top Chord Live: 0
Top Chord Dead: 15	Top Chord Dead: 0
Bottom Chord Live: 0	Bottom Chord Live: 0
Bottom Chord Dead: 10	Bottom Chord Dead: 0
Total Roof Load: 55	Total Floor Load: 0
Stress Increase: 1.25	Stress Increase: 0
Wind Standard: ASCE 7-22	Building Exposure: C
Wind Speed: 175	Wind usage: Residential - CAT II

NOTE: LABELING TRUSSES IS A SERVICE, NOT A REQUIREMENT. THEREFORE, IT IS THE RESPONSIBILITY OF THE BUILDER TO USE THE ENGINEERING DRAWINGS WHEN ERECTING MARKINGS AND ENG. DRAWINGS THE ENG. DRAWINGS PREVAILS.

NOTE: UPLIFT REACTIONS SHOWN ON TRUSS DESIGNS AND PLACEMENT PLANS ARE TO BE USED IN DESIGN OF TRUSS TO TRUSS CONNECTIONS AND FOR REFERENCE. THE UPLIFT VALUE OF CONNECTIONS OF THE TRUSSES TO THE SUPPORTING STRUCTURE ARE THE RESPONSIBILITY OF THE ARCH. AND/OR ENG. OF RECORD. REFER TO APRVD PERMIT DOCUMENTS FOR EXACT VALUES.

NOTE: THIS TRUSS LAYOUT IS UNDER NO CIRCUMSTANCE TO BE DIGITALLY ALTERED IN ANY WAY. ALL REVISIONS MADE TO THIS LAYOUT ARE TO BE CLEARLY MARKED AND NOTATED. ANY DRAWINGS THAT HAVE BEEN DIGITALLY ALTERED WILL BE NULL AND VOID. ANY ERRORS RESULTING FROM THIS WILL BE SOLE RESPONSIBILITY OF THE PURCHASER.

FIELD BRACING IS NOT THE RESPONSIBILITY OF THE TRUSS FABRICATOR, truss designer, or plate manufacturer. Persons erecting trusses are cautioned to seek professional advice regarding temporary and erection bracing which is always required to prevent toppling and dominoing during erection, and permanent bracing which may be required in specific applications. Trusses shall be erected and fastened in a straight and plumb position. Where no direct top chord sheathing is applied, trusses must be braced at 24" on center maximum. Where no direct bottom chord sheathing is applied, trusses must be braced at 10'-0" on center maximum. Trusses must be handled with extreme care during erection to prevent damage or personal injury. Refer to truss engineering for connection and bracing requirements. These calculations are supplied in order for the ENGINEER OF RECORD to adequately provide for connection and integration of the roof assembly to the supporting structure. Designers of supporting connections are SOLELY responsible for the integrity of their product. Trusses remain our property until paid for in full. Truss layouts and engineering may not be reproduced in part or in full under any circumstances.

V.I.F. = VERIFY IN FIELD

REVISIONS

No	DATE	NAME
1		
2		
3		
4		
5		
6		

IMPORTANT NOTE:
ALL DIMENSION TO BE VERIFIED IN FIELD BY G.C. BEFORE FABRICATION

ALL TRUSS SPACING @ 24" O.C. UNLESS OTHERWISE SPECIFIED

ROOF TRUSS PLACEMENT

DO NOT CUT OR ALTER TRUSSES! MARTINEZ TRUSS COMPANY WILL NOT HONOR CLAIMS OR BACK CHARGES UNLESS AUTHORIZED BY OUT OFFICE IN WRITING!

ATTENTION BUILDERS:
ALL DIMENSIONS, OVERHANGS, PITCHES, ETC. TO BE CHECKED BEFORE ORDERING TRUSSES, IT IS YOUR RESPONSIBILITY TO CHECK OUR TRUSS PLAN.

EXISTING CONDITIONS TO BE VERIFIED BY BUILDER IN ORDER FOR NEW TRUSSES TO MATCH EXISTING TRUSS CONDITIONS.

JOSE MARTINEZ
FL Reg. Eng. No. 031509
24 EAST 5th Street #1D,
HIALEAH, FL. 33010



CC25-0804.05 J. MARTINEZ 9280 N.W. South River Dr. Modley, FL 33166
Office: (305) 883-6261
Fax: (305) 885-2925

CONENGINEERS BUILDERS LLC

Date: 10/13/2025	Owner:	SHEET NO. 1
Drw by: JL		
Job Name: GARY MATZNER PARK		DRAWING NO. 25052803
Job Address: 6610 N KENDALL DR ,Pincrest,FL,		



MARTINEZ TRUSS CO., INC.

9280 NW SOUTH RIVER DR. - MEDLEY, FL 33166

Office: (305)883-6261

Fax: (305)885-2925

info@MartinezTruss.com

Order #: 25052803

Customer Information

Name:

CONENGINEERS BUILDERS LLC

Address:

1930 N Commerce Pkwy

City, State, Zip:

Weston, FL 33326

Job Information

Name:

GARY MATZNER PARK

Address:

6610 N KENDALL DR

City, State, Zip:

Pinecrest, FL

Building Code: FBC 8th Ed. 2023

Roof Loading:

TC LL: 30 psf

TC DL: 15 psf

BC LL: 0 psf

BC DL: 10 psf

Wind Loading:

Wind Standard: ASCE 7-22

Wind Speed: 175

WindLoadType: MWFRS

Wind Usage: Residential - CAT II

Project Engineer of Record

This cover sheet provided as per Florida Statute 61G15-31.003 in lieu of signing and sealing each individual sheet. An Index sheet of truss designs is attached which is numbered and with the identification and date of each drawing.

Engineer of Truss Design Package

JOSE MARTINEZ

FL Reg. Eng. No 031509

24 EAST 5th St. #1D

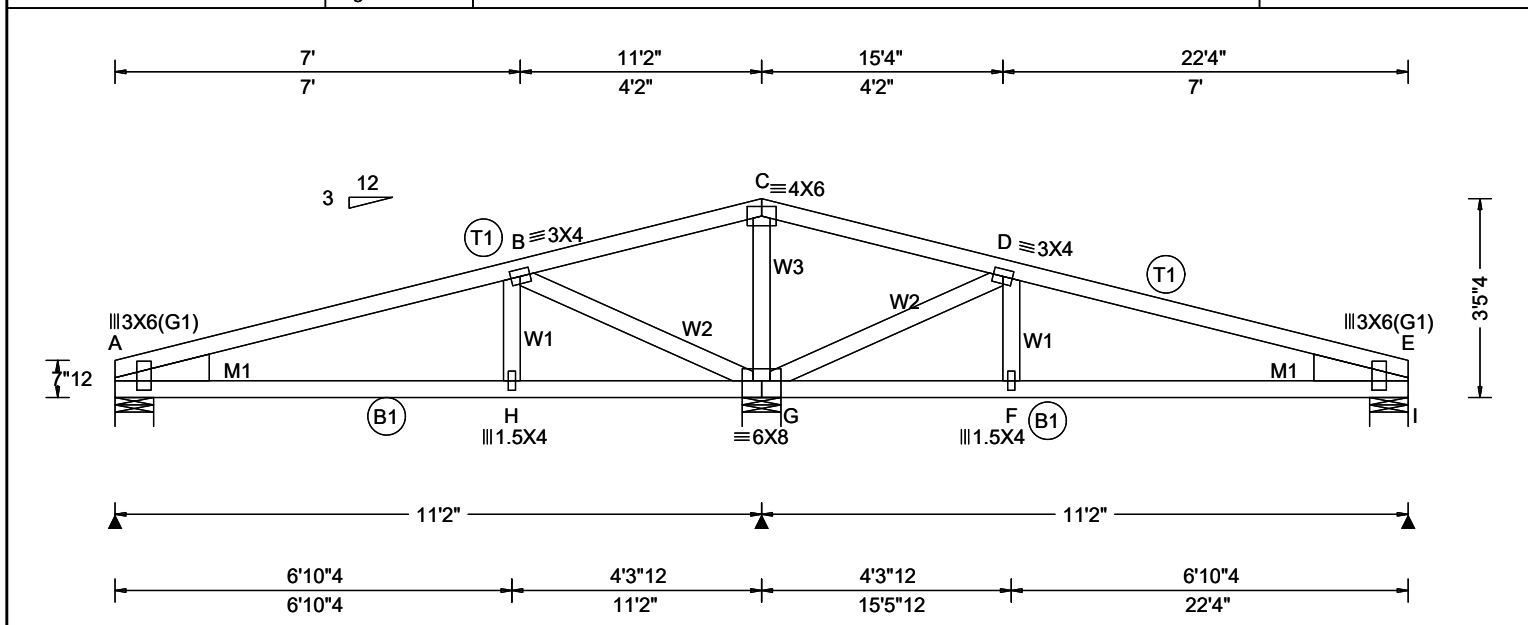
HIALEAH, FL 33010



#	SEQN	Item Desc	Date
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2	209771	1H11	10/15/2025
3	209768	1H9	10/15/2025
4	209765	1H7	10/15/2025
5	209803	1H5G	10/15/2025
6	209806	2H7	10/15/2025
7	209800	2H5G	10/15/2025
8	209809	3H7	10/15/2025
9	209812	3H5G	10/15/2025
10	209792	CJ5	10/15/2025
11	209783	J5	10/15/2025
12	209786	JB4	10/15/2025
13	209789	JB2	10/15/2025

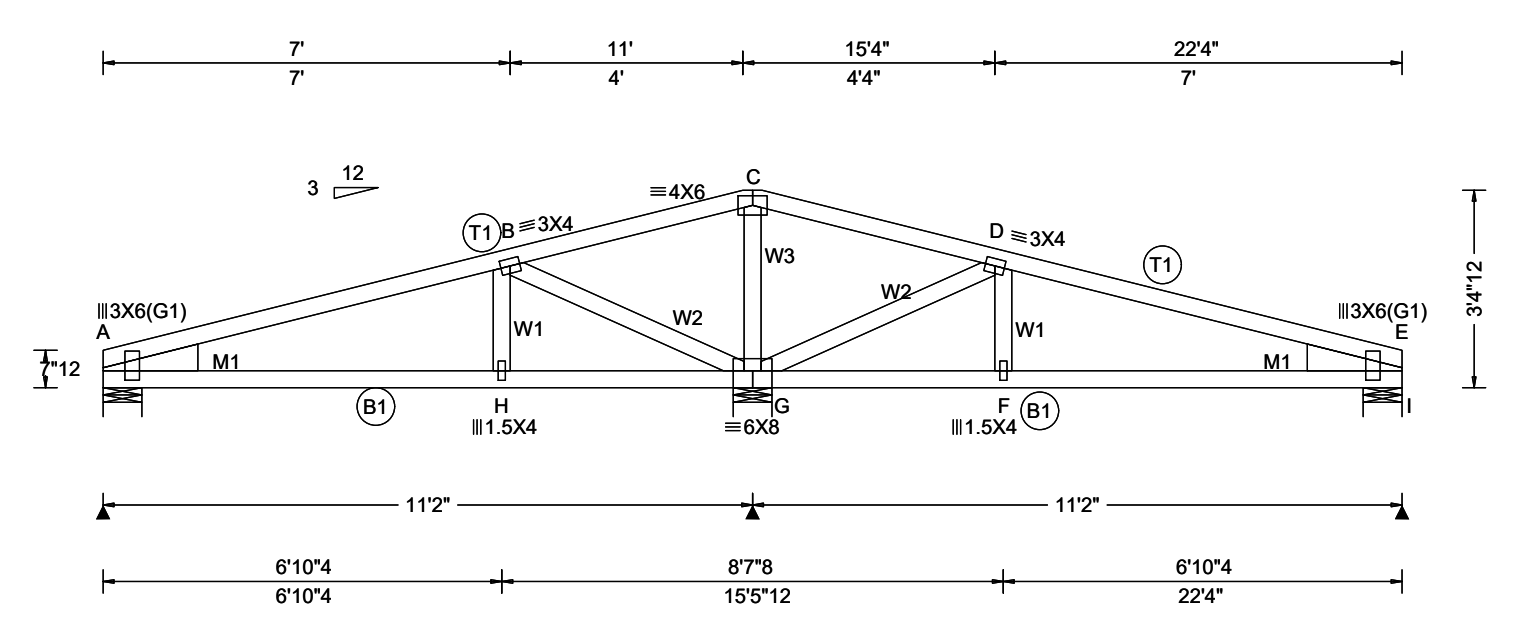
#	SEQN	Item Desc	Date
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#	SEQN	Item Desc	Date
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Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)		Defl/CSI Criteria		▲ Maximum Reactions (lbs)						
TCLL:	30.00	Wind Std: ASCE 7-22	Pg: NA	Ct: NA	CAT: NA	PP Deflection in loc L/defl L/#	Gravity		Non-Gravity				
TCDL:	15.00	Speed: 175 mph	Pf: NA		Ce: NA	VERT(LL): 0.010 F 999 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL:	0.00	Enclosure: Closed	Lu: NA	Cs: NA		VERT(CL): 0.022 F 999 240	A	538	/-	/-	/243	/188	/54
BCDL:	10.00	Risk Category: II	Snow Duration: NA			HORZ(LL): 0.012 E - -	G	1461	/-	/-	/696	/514	/-
Des Ld:	55.00	EXP: C Kzt: NA				HORZ(TL): 0.022 E - -	I	538	/-	/-	/249	/188	/-
NCBCLL:	10.00	Mean Height: 16.37 ft	Building Code:			Creep Factor: 2.0	Wind reactions based on MWFRS						
Soffit:	2.00	TCDL: 6.0 psf	FBC 8th Ed. 2023 Res. HVHZ			Max TC CSI: 0.741	A		Brg Wid = 8.0		Min Req = 1.5 (Truss)		
Load Duration:	1.25	BCDL: 6.0 psf	TPI Std: 2014			Max BC CSI: 0.476	G		Brg Wid = 8.0		Min Req = 1.7		
Spacing:	24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes			Max Web CSI: 0.440	I		Brg Wid = 8.0		Min Req = 1.5 (Truss)		
		C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)			Mfg Specified Camber:	Bearings A, G, & I are a rigid surface.						
		Loc. from endwall: not in 9.00 ft	Plate Type(s):				Maximum Top Chord Forces Per Ply (lbs)						
		GCpi: 0.18	WAVE			VIEW Ver: 24.01.03.0909.08	Chords	Tens.Comp.		Chords	Tens. Comp.		
		Wind Duration: 1.60					A - B	409	-824	C - D	432	- 74	
Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2; Lt Stub Wedge: 2x6 SP #2;Rt Stub Wedge: 2x6 SP #2; Wind Wind loads based on MWFRS with additional C&C member design. Wind loading based on both gable and hip roof types.							B - C	432	- 82	D - E	411	- 824	
						Maximum Bot Chord Forces Per Ply (lbs)							
						Chords	Tens.Comp.		Chords	Tens. Comp.			
						A - H	724	-309	G - F	716	-308		
						H - G	716	-311	F - E	724	-306		
						Maximum Web Forces Per Ply (lbs)							
						Webs	Tens.Comp.		Webs	Tens. Comp.			
						H - B	264	0	G - D	627	-1132		
						B - G	627	-1132	D - F	264	0		
						C - G	224	-413					

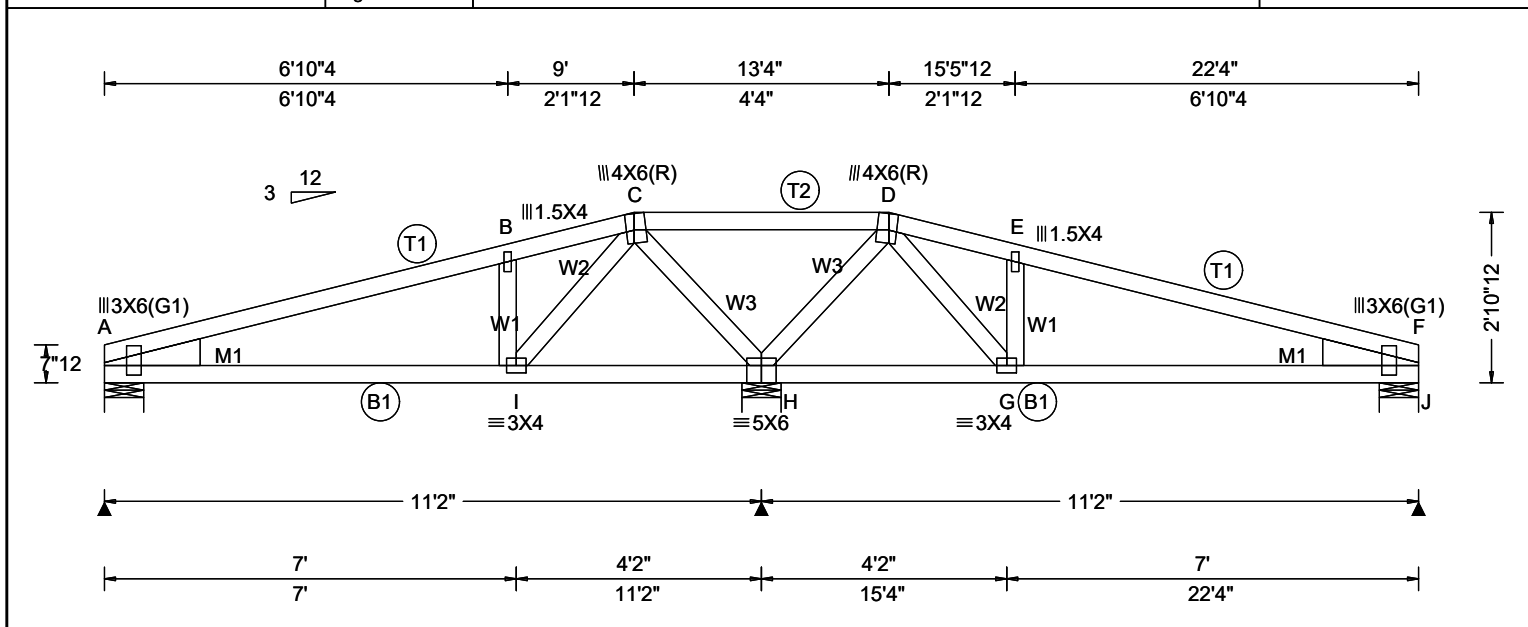
****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL			
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	A	538	/-	/-	/243	/187	/54
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): -0.014 F 999 360	G	1461	/-	/-	/686	/502	/-
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.022 F 999 240	I	538	/-	/-	/249	/187	/-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.018 E - -	Wind reactions based on MWFRS						
	EXP: C Kzt: NA		HORZ(TL): 0.023 E - -	A	Brg Wid = 8.0		Min Req = 1.5 (Truss)			
Des Ld: 55.00	Mean Height: 16.35 ft	Building Code:	Creep Factor: 2.0	G	Brg Wid = 8.0		Min Req = 1.7			
NCBCLL: 10.00	TCDL: 6.0 psf	FBC 8th Ed. 2023 Res. HVHZ	Max TC CSI: 0.741	I	Brg Wid = 8.0		Min Req = 1.5 (Truss)			
Soffit: 2.00	BCDL: 6.0 psf	TPI Std: 2014	Max BC CSI: 0.476	Bearings A, G, & I are a rigid surface.						
Load Duration: 1.25	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.440	Maximum Top Chord Forces Per Ply (lbs)						
Spacing: 24.0 "	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	Mfg Specified Camber:	Chords	Tens.Comp.	Chords	Tens.	Comp.		
	Loc. from endwall: not in 9.00 ft	Plate Type(s):								
	GCpi: 0.18	WAVE	VIEW Ver: 24.01.03.0909.08							
	Wind Duration: 1.60									

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2; Lt Stub Wedge: 2x6 SP #2;Rt Stub Wedge: 2x6 SP #2;	Maximum Bot Chord Forces Per Ply (lbs)			
	Chords	Tens.	Comp.	Chords Tens. Comp.
Wind Wind loads based on MWFRS with additional C&C member design. Wind loading based on both gable and hip roof types.	A - H	724	-306	G - F 716 -306
	H - G	716	-308	F - E 724 -304
	Maximum Web Forces Per Ply (lbs)			
	Webs	Tens.	Comp.	Webs Tens. Comp.
	H - B	264	0	C - G 208 -413
	B - G	627	-1132	D - F 264 0
	G - D	627	-1132	

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	GravityNon-Gravity LocR+ / R- / Rh / Rw / U / RL
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): 0.017 E 999 360	A 546 -/- /- /249 /190 /43
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.020 E 999 240	H 1439 -/- /- /680 /511 -/-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.012 F - -	J 546 -/- /- /254 /190 -/-
Des Ld: 55.00	EXP: C Kzt: NA		HORZ(TL): 0.016 F - -	Wind reactions based on MWFRS
NCBCLL: 10.00	Mean Height: 16.10 ft	Building Code:	Creep Factor: 2.0	A Brg Wid = 8.0 Min Req = 1.5 (Truss)
Soffit: 2.00	TCDL: 6.0 psf	FBC 8th Ed. 2023 Res. HVHZ	Max TC CSI: 0.705	H Brg Wid = 8.0 Min Req = 1.7
Load Duration: 1.25	BCDL: 6.0 psf	TPI Std: 2014	Max BC CSI: 0.470	J Brg Wid = 8.0 Min Req = 1.5 (Truss)
Spacing: 24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.203	Bearings A, H, & J are a rigid surface.
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	Mfg Specified Camber:	Maximum Top Chord Forces Per Ply (lbs)
	Loc. from endwall: not in 9.00 ft	Plate Type(s):		ChordsTens.Comp. ChordsTens. Comp.
	GCpi: 0.18	WAVE	VIEW Ver: 24.01.03.0909.08	A - B 459 -858 D - E 537 -791
	Wind Duration: 1.60			B - C 540 -791 E - F 457 -858
Lumber				C - D 511 -248
Top chord: 2x4 SP #2;				Maximum Bot Chord Forces Per Ply (lbs)
Bot chord: 2x4 SP #2;				ChordsTens.Comp. ChordsTens. Comp.
Webs: 2x4 SP #2;				A - I 759 -372 H - G 168 -60
Lt Stub Wedge: 2x6 SP #2;Rt Stub Wedge: 2x6 SP #2;				I - H 168 -60 G - F 759 -376
Wind				Maximum Web Forces Per Ply (lbs)
Wind loads based on MWFRS with additional C&C member design.				WebsTens.Comp. WebsTens. Comp.
Wind loading based on both gable and hip roof types.				B - I 454 -560 H - D 720 -915
				I - C 899 -493 D - G 899 -493
				C - H 719 -915 G - E 454 -560

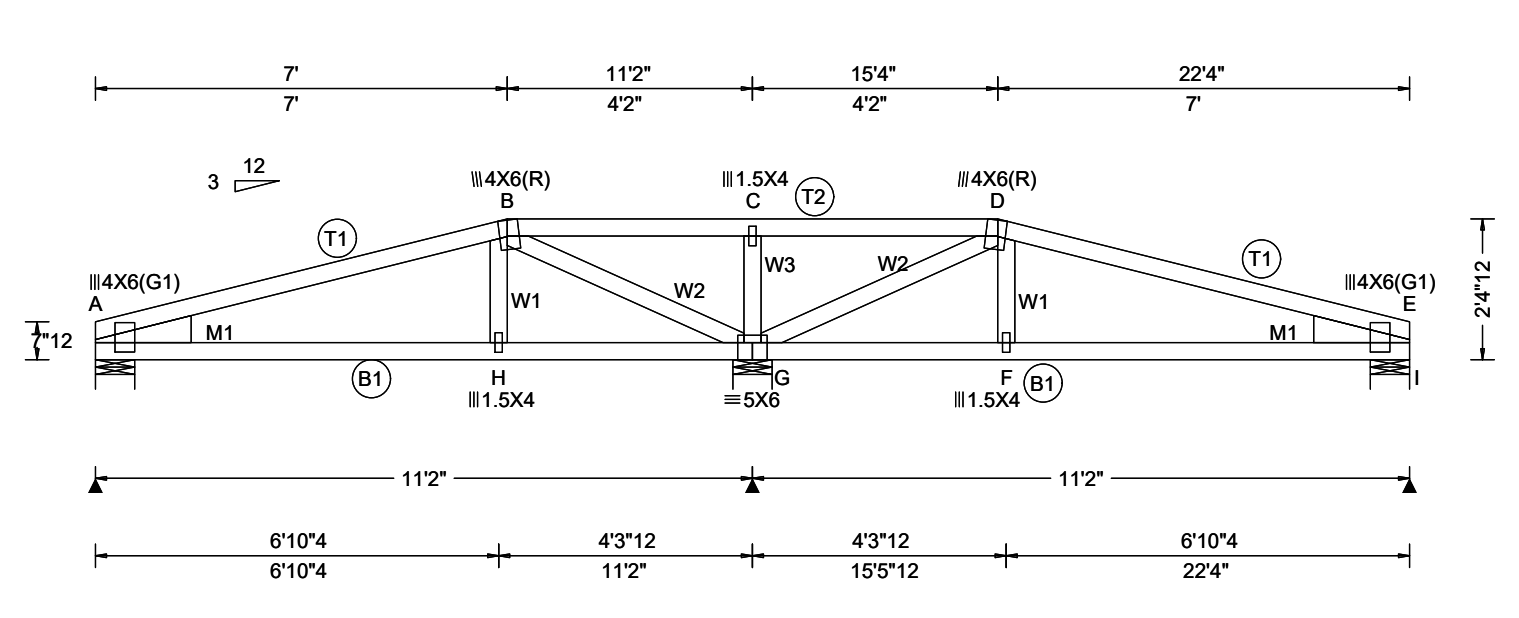
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): 0.008 F 999 360	A	608	/-	/-	/284	/246	/33
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.014 F 999 240	G	1261	/-	/-	/604	/445	/-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.016 E - -	I	608	/-	/-	/284	/246	/-
Des Ld: 55.00	EXP: C Kzt: NA	Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	HORZ(TL): 0.029 E - -	Wind reactions based on MWFRS						
NCBCLL: 10.00	Mean Height: 15.85 ft		Creep Factor: 2.0	A	Brg Wid = 8.0 Min Req = 1.5 (Truss)					
Soffit: 2.00	TCDL: 6.0 psf		Max TC CSI: 0.867	G	Brg Wid = 8.0 Min Req = 1.5					
Load Duration: 1.25	BCDL: 6.0 psf		Max BC CSI: 0.595	I	Brg Wid = 8.0 Min Req = 1.5 (Truss)					
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2		Max Web CSI: 0.341	Bearings A, G, & I are a rigid surface.						
	C&C Dist a: 3.00 ft		Mfg Specified Camber:	Maximum Top Chord Forces Per Ply (lbs)						
	Loc. from endwall: not in 4.50 ft			Chords	Tens.Comp.	Chords	Tens. Comp.			
	GCpi: 0.18			A - B	623 -921	C - D	182 -35			
	Wind Duration: 1.60		VIEW Ver: 24.01.03.0909.08	B - C	182 -35	D - E	621 -921			
				Maximum Bot Chord Forces Per Ply (lbs)						
				Chords	Tens.Comp.	Chords	Tens. Comp.			

Lumber

Top chord: 2x4 SP #1; T2 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;
Lt Stub Wedge: 2x6 SP #2;Rt Stub Wedge: 2x6 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

A - B	623	-921	C - D	182	-35
B - C	182	-35	D - E	621	-921

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - H	803	-513	G - F	790	-519
H - G	790	-515	F - E	803	-517

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
H - B	267	0	C - G	683	-474
B - G	589	-877	D - F	267	0
G - D	590	-877			

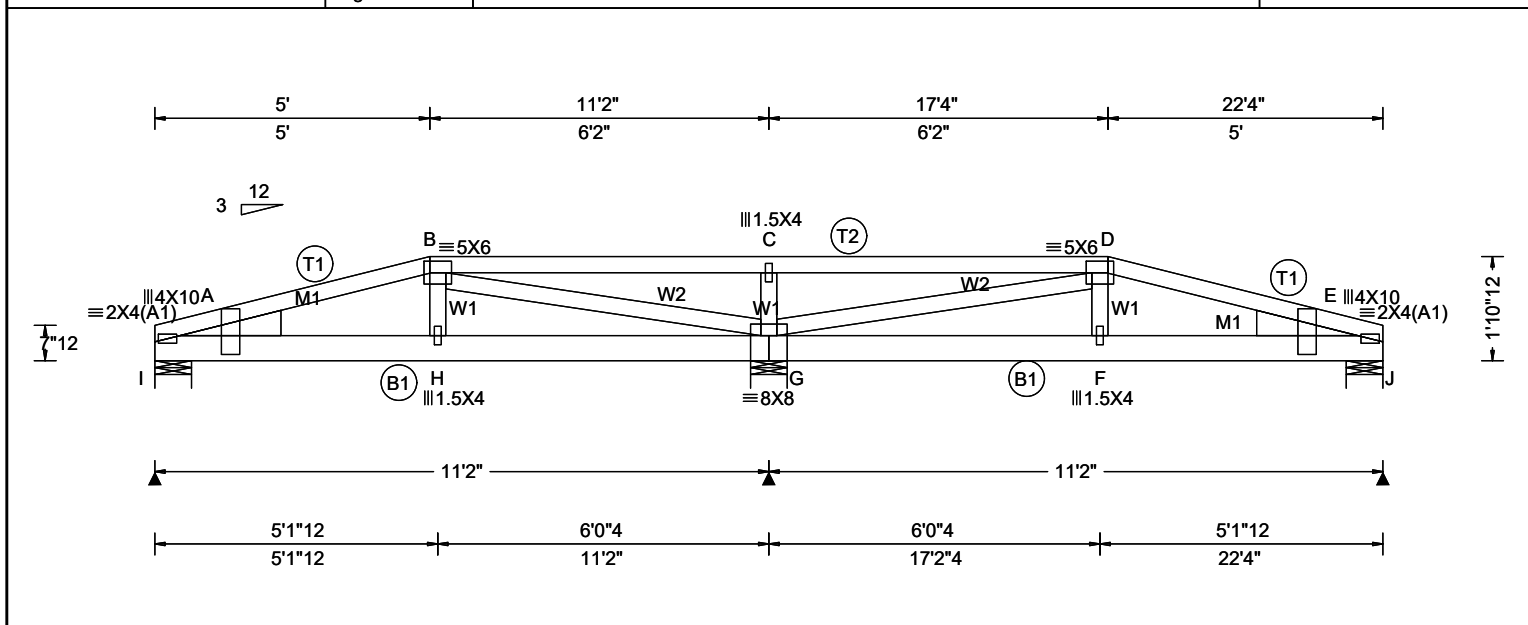
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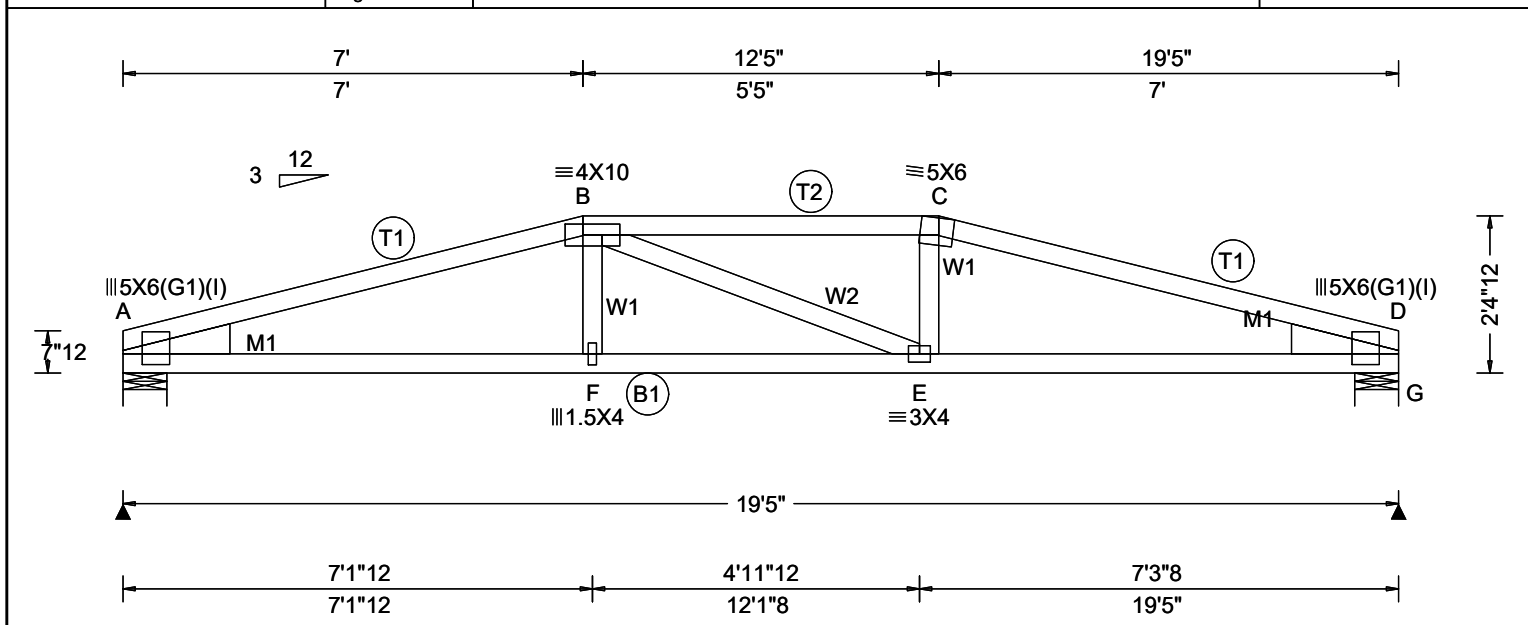
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): 0.053 H 999 360	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.057 H 999 240	I 933 -/- -/- -/- /342 -/-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.016 E - -	G 2805 -/- -/- -/- /1043 -/-
Des Ld: 55.00	EXP: C Kzt: NA		HORZ(TL): 0.018 E - -	J 933 -/- -/- -/- /342 -/-
NCBCLL: 0.00	Mean Height: 15.60 ft		Creep Factor: 2.0	Wind reactions based on MWFRS
Soffit: 2.00	TCDL: 6.0 psf	Building Code:	Max TC CSI: 0.866	I Brg Wid = 8.0 Min Req = 1.5 (Truss)
Load Duration: 1.25	BCDL: 6.0 psf	FBC 8th Ed. 2023 Res. HVHZ	Max BC CSI: 0.468	G Brg Wid = 8.0 Min Req = 1.7
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max Web CSI: 0.331	J Brg Wid = 8.0 Min Req = 1.5 (Truss)
	C&C Dist a: 3.00 ft	Rep Fac: Varies by Ld Case	Mfg Specified Camber:	Bearings I, G, & E are a rigid surface.
	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)		
	GCpi: 0.18	Plate Type(s):		
	Wind Duration: 1.60	WAVE	VIEW Ver: 24.01.03.0909.08	
Lumber		Wind		
Top chord: 2x4 SP #2;		Wind loads and reactions based on MWFRS.		
Bot chord: 2x6 SP #2;		Wind loading based on both gable and hip roof types.		
Webs: 2x4 SP #2;				
Lt Wedge: 2x6 SP #2;Rt Wedge: 2x6 SP #2;				
Nailnote				
Nail Schedule:0.128"x3", min. nails				
Top Chord: 1 Row @12.00" o.c.				
Bot Chord: 1 Row @12.00" o.c.				
Webs : 1 Row @ 4" o.c.				
Use equal spacing between rows and stagger nails				
in each row to avoid splitting.				
(1) 1/2" bolts may be used for				
(2) 0.128"x3", min. nails on				
The Bottom Chord Only.				
Special Loads				
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)				
TC: From 91 plf at 0.00 to 91 plf at 5.00				
TC: From 45 plf at 5.00 to 45 plf at 17.33				
TC: From 91 plf at 17.33 to 91 plf at 22.33				
BC: From 20 plf at 0.00 to 20 plf at 4.96				
BC: From 10 plf at 4.96 to 10 plf at 17.38				
BC: From 20 plf at 17.38 to 20 plf at 22.33				
TC: 119 lb Conc. Load at 4.96,17.38				
TC: 208 lb Conc. Load at 6.15, 8.15,10.15,12.18				
14.18,16.18				
BC: 396 lb Conc. Load at 4.96,17.38				
BC: 98 lb Conc. Load at 6.15, 8.15,10.15,12.18				
14.18,16.18				

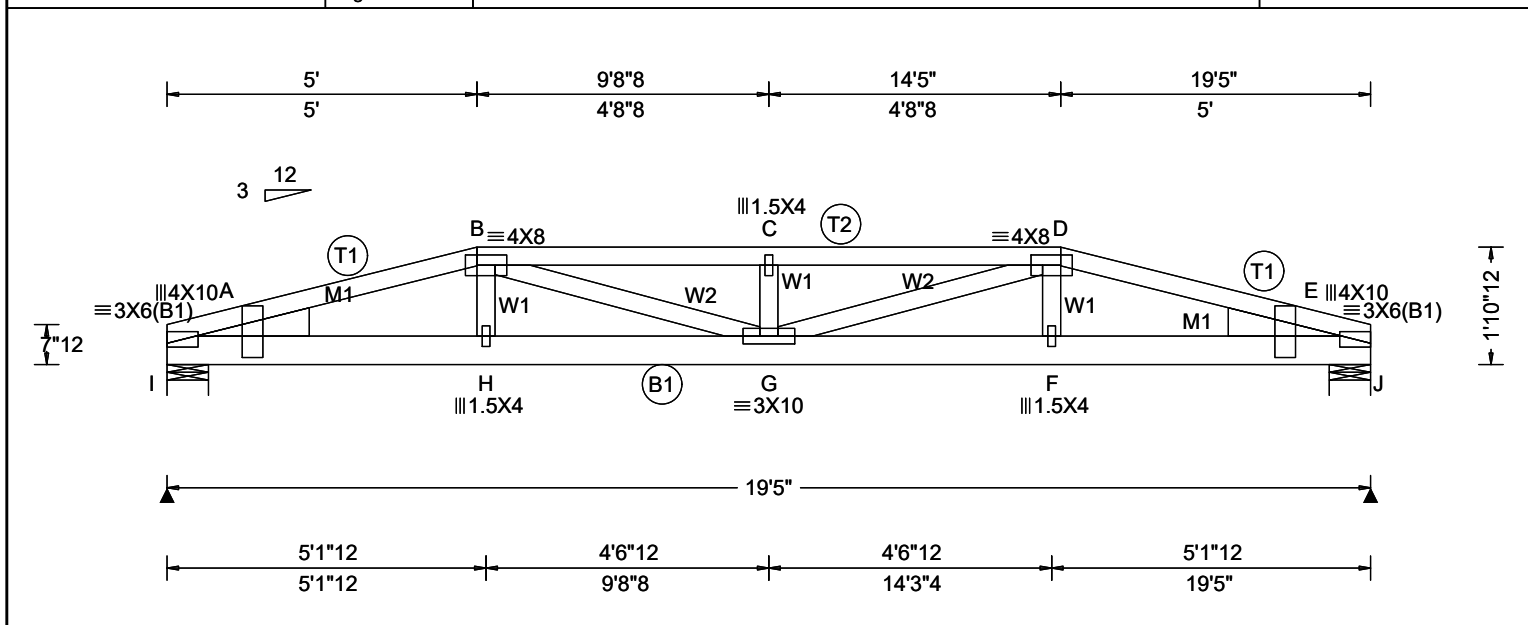
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria	▲ Maximum Reactions (lbs)					
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA	Ct: NA	CAT: NA	PP Deflection in loc L/defl L/#	Gravity			Non-Gravity		
TCDL: 15.00	Speed: 175 mph	Pf: NA		Ce: NA	VERT(LL): 0.136 E 999 360	Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00	Enclosure: Closed	Lu: NA	Cs: NA		VERT(CL): 0.251 E 928 240	A	1077	/- /-	/510	/387	/32
BCDL: 10.00	Risk Category: II	Snow Duration: NA			HORZ(LL): 0.038 D - -	G	1077	/- /-	/510	/387	/-
	EXP: C Kzt: NA				HORZ(TL): 0.071 D - -	Wind reactions based on MWFRS					
Des Ld: 55.00	Mean Height: 15.00 ft				Creep Factor: 2.0	A Brg Wid = 8.0 Min Req = 1.5 (Truss)					
NCBCLL: 10.00	TCDL: 6.0 psf	Building Code:			Max TC CSI: 0.879	G Brg Wid = 8.0 Min Req = 1.5 (Truss)					
Soffit: 2.00	BCDL: 6.0 psf	FBC 8th Ed. 2023 Res. HVHZ			Max BC CSI: 0.715	Bearings A & G are a rigid surface.					
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014			Max Web CSI: 0.050	Maximum Top Chord Forces Per Ply (lbs)					
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes			Mfg Specified Camber:	Chords	Tens.Comp.	Chords	Tens. Comp.		
	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)				A - B	2163 -2592	C - D	2166	-2592	
	GCpi: 0.18	Plate Type(s):				B - C	2157 -2436				
	Wind Duration: 1.60	WAVE			VIEW Ver: 24.01.03.0909.08						

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2; Lt Stub Wedge: 2x6 SP #2;Rt Stub Wedge: 2x6 SP #2;	Plating Notes (I) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.	Wind Wind loads based on MWFRS with additional C&C member design. Wind loading based on both gable and hip roof types.
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA	Ct: NA	CAT: NA	PP Deflection in loc L/defl L/#	Gravity		Non-Gravity				
TCDL: 15.00	Speed: 175 mph	Pf: NA		Ce: NA	VERT(LL): 0.212 C 999 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00	Enclosure: Closed	Lu: NA	Cs: NA		VERT(CL): 0.389 C 589 240	I	2094	/-	/-	/-	/765	/-
BCDL: 10.00	Risk Category: II	Snow Duration: NA			HORZ(LL): 0.027 B - -	J	2094	/-	/-	/-	/765	/-
	EXP: C Kzt: NA				HORZ(TL): 0.049 B - -	Wind reactions based on MWFRS						
Des Ld: 55.00	Mean Height: 15.00 ft				Creep Factor: 2.0	I	Brg Wid = 8.0	Min Req = 1.5	(Truss)			
NCBCLL: 0.00	TCDL: 6.0 psf	Building Code:			Max TC CSI: 0.609	J	Brg Wid = 8.0	Min Req = 1.5	(Truss)			
Soffit: 2.00	BCDL: 6.0 psf	FBC 8th Ed. 2023 Res. HVHZ			Max BC CSI: 0.785	Bearings I & E are a rigid surface.						
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014			Max Web CSI: 0.225	Maximum Top Chord Forces Per Ply (lbs)						
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Varies by Ld Case			Mfg Specified Camber:	Chords	Tens.Comp.	Chords	Tens.	Comp.		
	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)				A - B	1061 -2892	C - D	1380	-3750		
	GCpi: 0.18	Plate Type(s):				B - C	1380 -3750	D - E	1061	-2892		
	Wind Duration: 1.60	WAVE			VIEW Ver: 24.01.03.0909.08							

Lumber Top chord: 2x4 SP #2; Bot chord: 2x6 SP #2; Webs: 2x4 SP #2; Lt Wedge: 2x6 SP #2;Rt Wedge: 2x6 SP #2; Nailnote Nail Schedule:0.128"x3", min. nails Top Chord: 1 Row @12.00" o.c. Bot Chord: 1 Row @12.00" o.c. Webs : 1 Row @ 4" o.c. Use equal spacing between rows and stagger nails in each row to avoid splitting. (1) 1/2" bolts may be used for (2) 0.128"x3", min. nails on The Bottom Chord Only. Special Loads ----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC: From 91 plf at 0.00 to 91 plf at 5.00 TC: From 45 plf at 5.00 to 45 plf at 14.42 TC: From 91 plf at 14.42 to 91 plf at 19.42 BC: From 20 plf at 0.00 to 20 plf at 4.96 BC: From 10 plf at 4.96 to 10 plf at 14.46 BC: From 20 plf at 14.46 to 20 plf at 19.42 TC: 119 lb Conc. Load at 4.96,14.46 TC: 208 lb Conc. Load at 6.09, 8.09, 9.71,11.33 13.33 BC: 396 lb Conc. Load at 4.96,14.46 BC: 98 lb Conc. Load at 6.09, 8.09, 9.71,11.33 13.33	Wind Wind loads and reactions based on MWFRS. Wind loading based on both gable and hip roof types.
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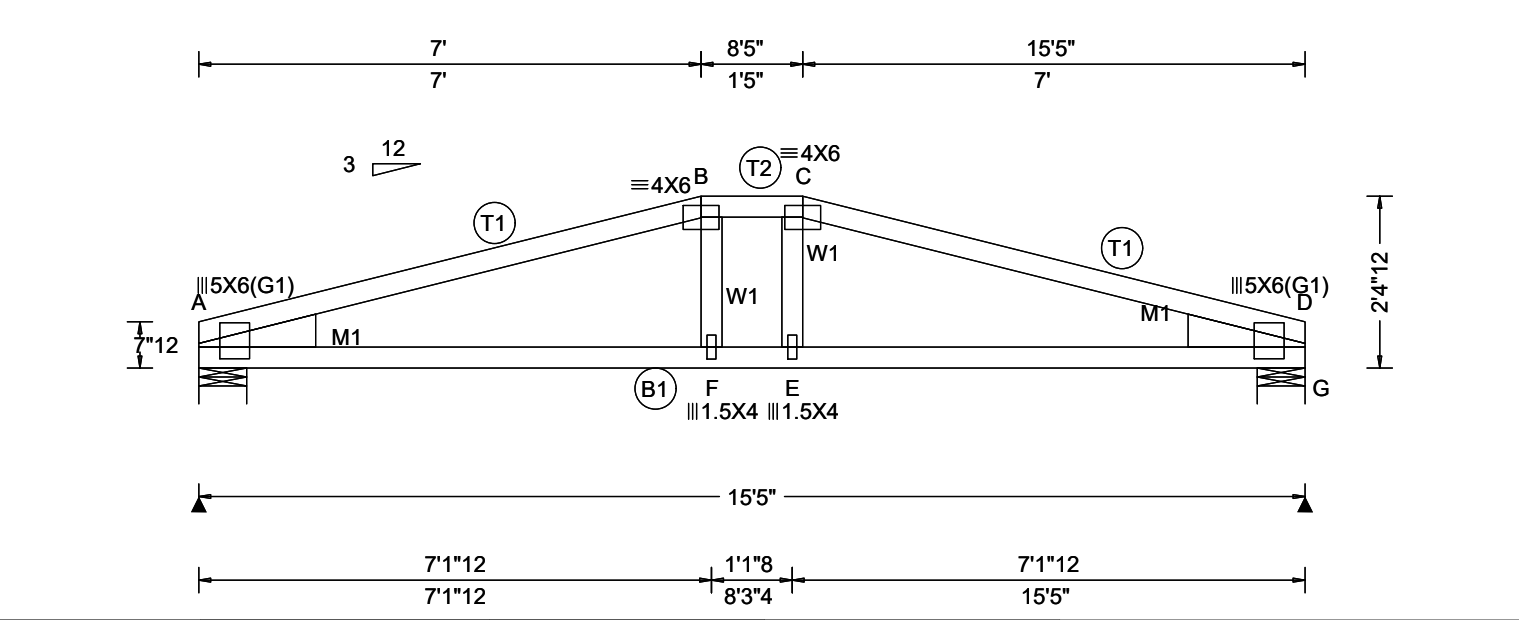
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria		▲ Maximum Reactions (lbs)						
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#		Gravity			Non-Gravity			
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.063 E 999 360		Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.127 E 999 240		A	855	/-	/-	/406	/306	/32
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.022 D - -		G	855	/-	/-	/406	/306	/-
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.040 D - -		Wind reactions based on MWFRS						
NCBCLL: 10.00		Mean Height: 15.00 ft					Creep Factor: 2.0		A Brg Wid = 8.0 Min Req = 1.5 (Truss)						
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.991		G Brg Wid = 8.0 Min Req = 1.5 (Truss)						
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.660		Bearings A & G are a rigid surface.						
Spacing: 24.0 "		MWFRS Parallel Dist: 0 to h/2					Max Web CSI: 0.053		Maximum Top Chord Forces Per Ply (lbs)						
		C&C Dist a: 3.00 ft					Mfg Specified Camber:		Chords	Tens.Comp.	Chords	Tens. Comp.			
		Loc. from endwall: not in 4.50 ft					VIEW Ver: 24.01.03.0909.08		A - B	1455 - 1849	C - D	1455 - 1849			
		GCpi: 0.18							B - C	1480 - 1713					
		Wind Duration: 1.60													

Lumber				Maximum Bot Chord Forces Per Ply (lbs)			
				Chords	Tens.Comp.	Chords	Tens. Comp.
Top chord: 2x4 SP #2;				A - F	1709 - 1302	E - D	1709 - 1307
Bot chord: 2x4 SP #2;				F - E	1713 - 1300		
Webs: 2x4 SP #2;							
Lt Stub Wedge: 2x6 SP #2; Rt Stub Wedge: 2x6 SP #2;							
Wind				Maximum Web Forces Per Ply (lbs)			
				Webs	Tens.Comp.	Webs	Tens. Comp.
Wind loads based on MWFRS with additional C&C member design.				B - F	234 - 84	E - C	234 - 84
Wind loading based on both gable and hip roof types.							

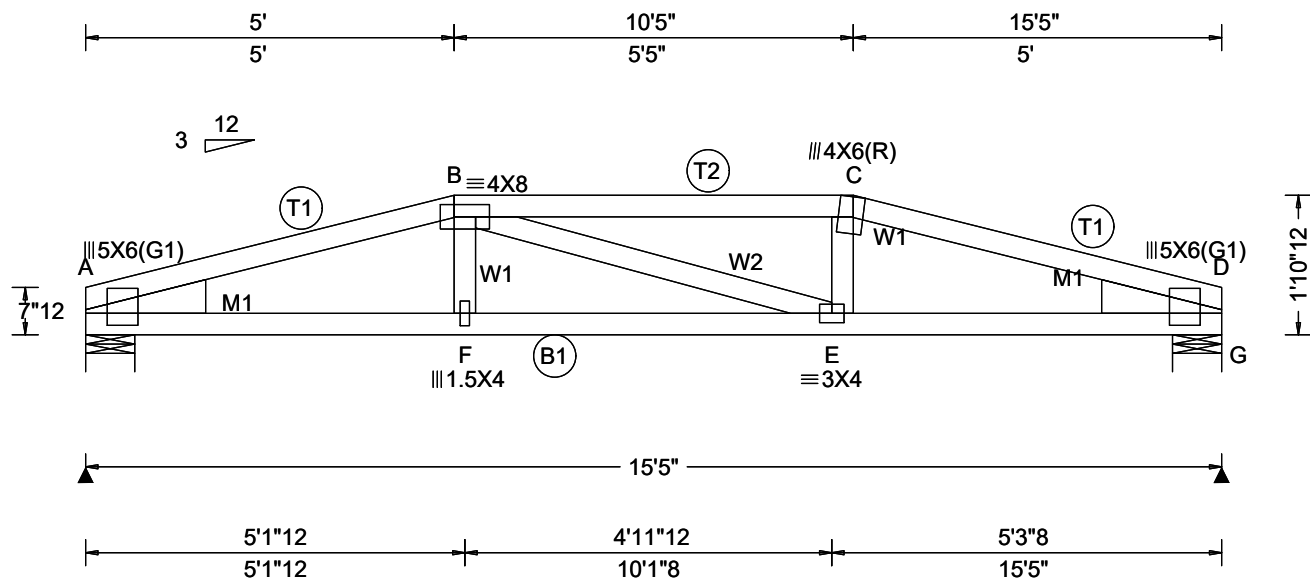
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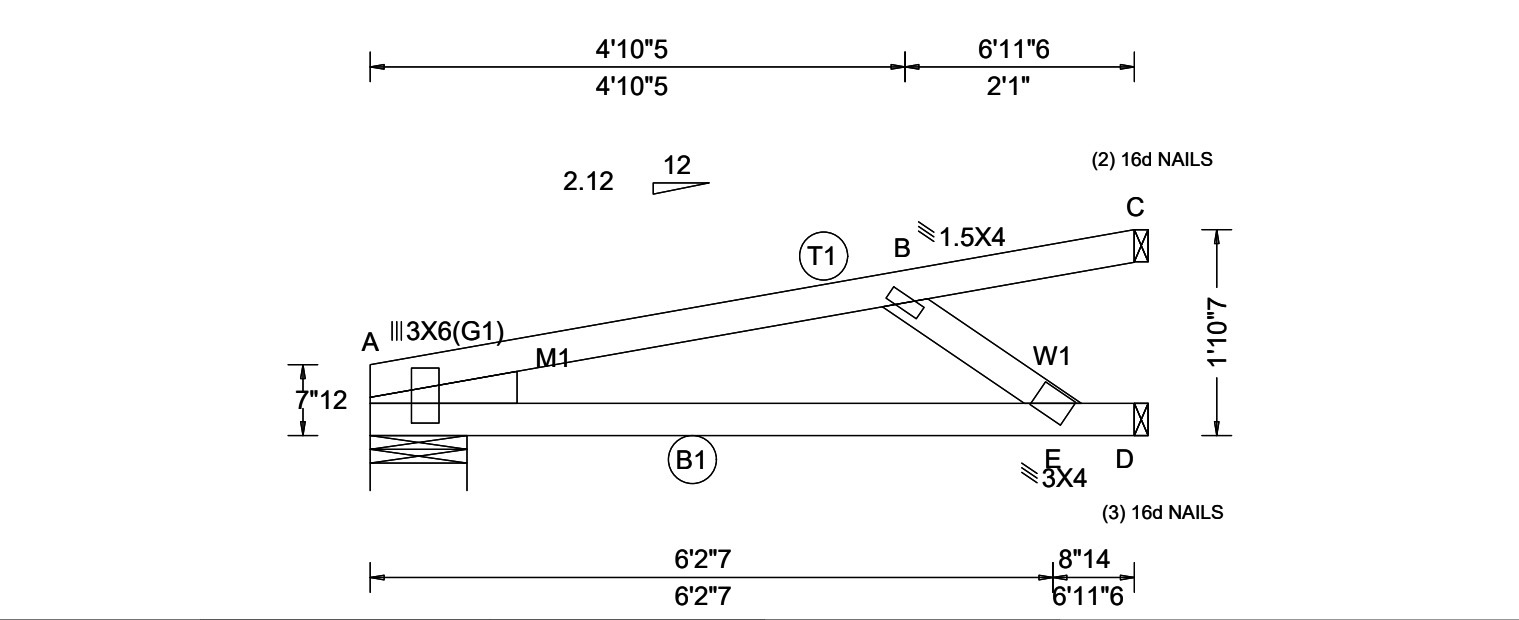
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): 0.200 E 924 360	A	1678	/-	/-	/-	/612	/-
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.213 E 870 240	G	1678	/-	/-	/-	/612	/-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.047 D - -	Wind reactions based on MWFRS						
Des Ld: 55.00	EXP: C Kzt: NA	Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	HORZ(TL): 0.050 D - -	A Brg Wid = 8.0 Min Req = 1.5 (Truss)						
NCBCLL: 0.00	Mean Height: 15.00 ft		Creep Factor: 2.0	G Brg Wid = 8.0 Min Req = 1.5 (Truss)						
Soffit: 2.00	TCDL: 6.0 psf		Max TC CSI: 0.868	Bearings A & G are a rigid surface.						
Load Duration: 1.25	BCDL: 6.0 psf		Max BC CSI: 0.962	Maximum Top Chord Forces Per Ply (lbs)						
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2		Max Web CSI: 0.052	Chords	Tens.Comp.		Chords	Tens. Comp.		
	C&C Dist a: 3.00 ft		Mfg Specified Camber:	A - B	861 - 2338		C - D	862 - 2336		
	Loc. from endwall: not in 4.50 ft		VIEW Ver: 24.01.03.0909.08	B - C	818 - 2264					
	GCpi: 0.18									
	Wind Duration: 1.60									

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2; Lt Stub Wedge: 2x6 SP #2; Rt Stub Wedge: 2x6 SP #2;	Nailnote Nail Schedule: 0.128"x3", min. nails Top Chord: 1 Row @ 12.00" o.c. Bot Chord: 1 Row @ 12.00" o.c. Webs : 1 Row @ 4" o.c. Use equal spacing between rows and stagger nails in each row to avoid splitting.	Special Loads ----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC: From 91 plf at 0.00 to 91 plf at 5.00 TC: From 45 plf at 5.00 to 45 plf at 10.42 TC: From 91 plf at 10.42 to 91 plf at 15.42 BC: From 20 plf at 0.00 to 20 plf at 4.96 BC: From 10 plf at 4.96 to 10 plf at 10.46 BC: From 20 plf at 10.46 to 20 plf at 15.42 TC: 119 lb Conc. Load at 4.96, 10.46 TC: 208 lb Conc. Load at 6.09, 7.71, 9.33 BC: 396 lb Conc. Load at 4.96, 10.46 BC: 98 lb Conc. Load at 6.09, 7.71, 9.33	Wind Wind loads and reactions based on MWFRS. Wind loading based on both gable and hip roof types.
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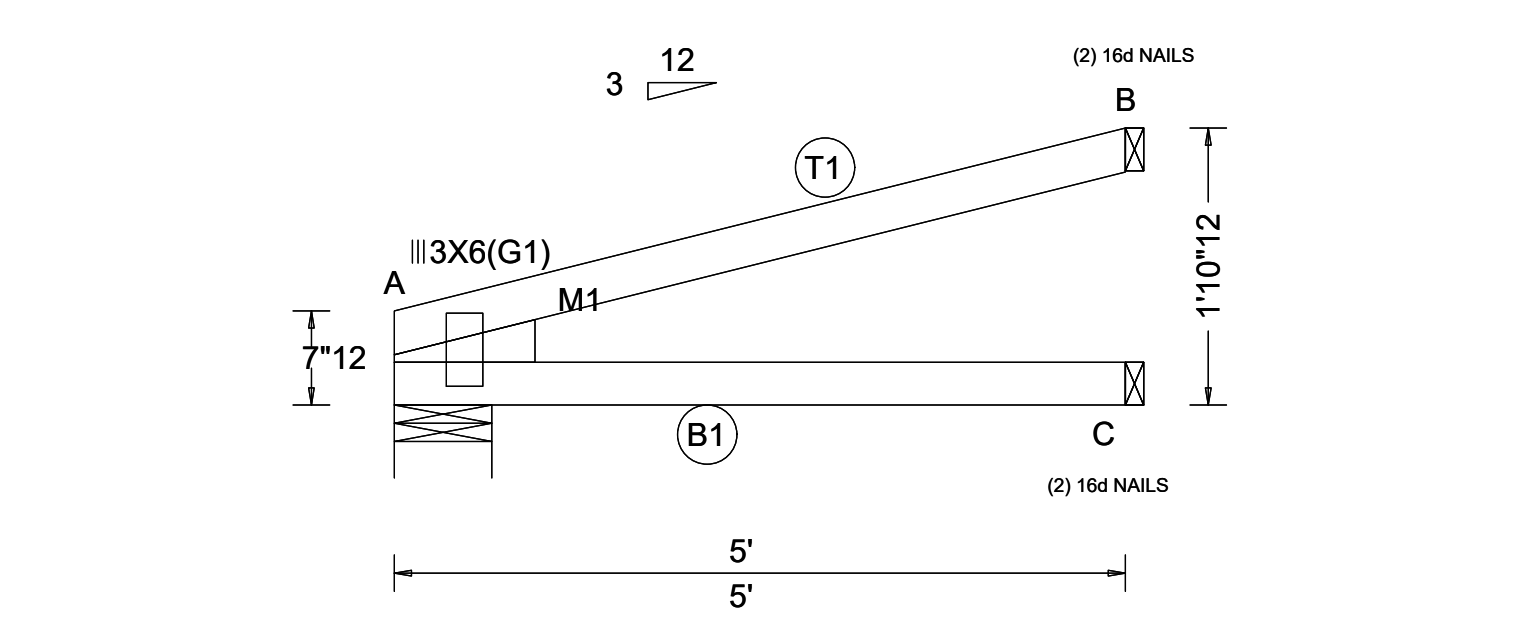
Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria		▲ Maximum Reactions (lbs)						
TCLL:	30.00	Wind Std:	ASCE 7-22	Pg: NA	Ct: NA	CAT: NA	PP Deflection in	loc L/defl L/#	Gravity			Non-Gravity			
TCDL:	15.00	Speed:	175 mph	Pf: NA		Ce: NA	VERT(LL):	0.066 E 999 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL):	0.125 E 664 240	A	252	/-	/-	/-	/86	/-
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA		HORZ(LL):	0.016 B - -	D	396	/-	/0	/-	/131	/0
		EXP:	C Kzt: NA				HORZ(TL):	0.030 B - -	C	119	/-	/-	/-	/56	/-
Des Ld:	55.00	Mean Height:	15.59 ft	Building Code:			Creep Factor:	2.0	Wind reactions based on MWFRS						
NCBCLL:	10.00	TCDL:	6.0 psf	FBC 8th Ed. 2023 Res.	HVHZ		Max TC CSI:	0.528	A	Brg Wid = 10.6 Min Req = 1.5 (Truss)					
Soffit:	2.00	BCDL:	6.0 psf	TPI Std:	2014		Max BC CSI:	0.905	D	Brg Wid = 1.5 Min Req = -					
Load Duration:	1.25	MWFRS Parallel Dist:	0 to h/2	Rep Fac:	Varies by Ld Case		Max Web CSI:	0.185	C	Brg Wid = 1.5 Min Req = -					
Spacing:	24.0 "	C&C Dist a:	3.00 ft	FT/RT:	20(0)/10(0)		Mfg Specified Camber:		Bearing A is a rigid surface.						
		Loc. from endwall:	Any	Plate Type(s):					Maximum Top Chord Forces Per Ply (lbs)						
		GCpi:	0.18						Chords	Tens.Comp.		Chords	Tens. Comp.		
		Wind Duration:	1.60	WAVE			VIEW Ver:	24.01.03.0909.08							

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;
Lt Stub Wedge: 2x4 SP #2;

Special Loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 2 plf at 0.00 to 2 plf at 6.95
BC: From 2 plf at 0.00 to 2 plf at 6.95
TC: 170 lb Conc. Load at 2.83
TC: 334 lb Conc. Load at 5.66
BC: 79 lb Conc. Load at 2.83
BC: 157 lb Conc. Load at 5.66

Wind
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

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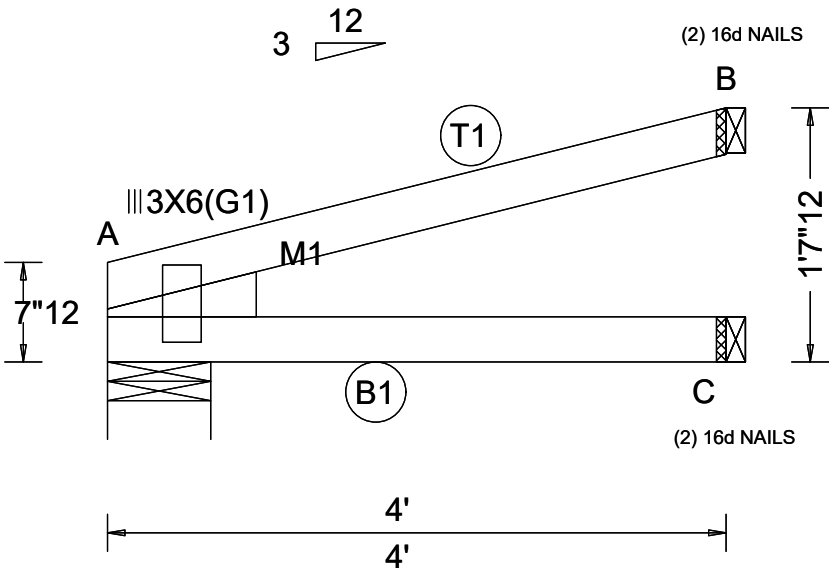


Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)						
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity Non-Gravity						
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): NA			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): NA			A	277	/-	/-	/131	/83	/80
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.006 A - -			C	98	/-	/-	/54	/-	/-
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.011 A - -			B	208	/-	/-	/79	/124	/-
NCBCLL: 10.00		Mean Height: 15.00 ft					Creep Factor: 2.0			Wind reactions based on MWFRS						
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.857			A	Brg Wid = 8.0		Min Req = 1.5 (Truss)			
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.407			C	Brg Wid = 1.5		Min Req = -			
Spacing: 24.0 "		MWFRS Parallel Dist: 0 to h/2					Max Web CSI: 0.000			B	Brg Wid = 1.5		Min Req = -			
		C&C Dist a: 3.00 ft					Mfg Specified Camber:			Bearing A is a rigid surface.						
		Loc. from endwall: not in 4.50 ft					VIEW Ver: 24.01.03.0909.08			Maximum Top Chord Forces Per Ply (lbs)						
		GCpi: 0.18								Chords Tens.Comp.						
		Wind Duration: 1.60														

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Lt Stub Wedge: 2x4 SP #2;

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

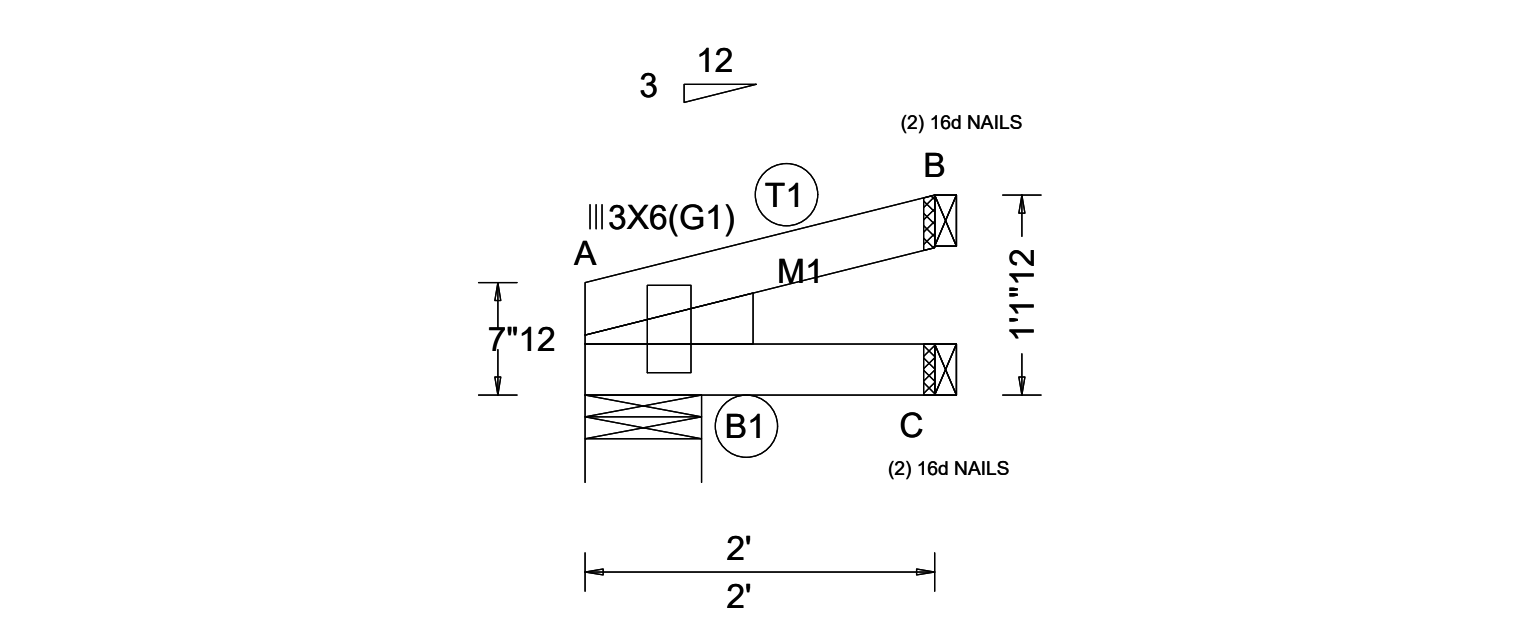
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): NA	A	222	/-	/-	/105	/68	/65
	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA	C	78	/-	/-	/43	/-	/-
	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.003 A - -	B	167	/-	/-	/63	/103	/-
	EXP: C Kzt: NA		HORZ(TL): 0.006 A - -	Wind reactions based on MWFRS						
	Mean Height: 15.48 ft	Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Creep Factor: 2.0	A	Brg Wid = 8.0		Min Req = 1.5 (Truss)			
	TCDL: 6.0 psf		Max TC CSI: 0.754	C	Brg Wid = 1.5		Min Req = -			
	BCDL: 6.0 psf		Max BC CSI: 0.358	B	Brg Wid = 1.5		Min Req = -			
	MWFRS Parallel Dist: 0 to h/2		Max Web CSI: 0.000	Bearing A is a rigid surface.						
	C&C Dist a: 3.00 ft		Mfg Specified Camber:	Maximum Top Chord Forces Per Ply (lbs)						
	Loc. from endwall: Any				Chords		Tens.Comp.			
GCpi: 0.18										
Wind Duration: 1.60			VIEW Ver: 24.01.03.0909.08							

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Lt Stub Wedge: 2x4 SP #2; Wind Wind loads based on MWFRS with additional C&C member design. Wind loading based on both gable and hip roof types.	Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. A - C 2 -2

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity			Non-Gravity			
	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): NA	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA	A	111	/-	/-	/52	/30	/32
	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.001 B - -	C	39	/-	/-	/21	/-	/-
	EXP: C Kzt: NA		HORZ(TL): 0.001 A - -	B	85	/-	/-	/32	/53	/-
	Mean Height: 15.23 ft		Creep Factor: 2.0	Wind reactions based on MWFRS						
	TCDL: 6.0 psf	Building Code:	Max TC CSI: 0.186	A	Brg Wid = 8.0		Min Req = 1.5 (Truss)			
	BCDL: 6.0 psf	FBC 8th Ed. 2023 Res. HVHZ	Max BC CSI: 0.087	C	Brg Wid = 1.5		Min Req = -			
	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max Web CSI: 0.000	B	Brg Wid = 1.5		Min Req = -			
	C&C Dist a: 3.00 ft	Rep Fac: Yes	Mfg Specified Camber:	Bearing A is a rigid surface.						
Loc. from endwall: Any	FT/RT:20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)							
GCpi: 0.18	Plate Type(s):		Chords			Tens.Comp.				
Wind Duration: 1.60	WAVE	VIEW Ver: 24.01.03.0909.08								

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Lt Stub Wedge: 2x4 SP #2; Wind Wind loads based on MWFRS with additional C&C member design. Wind loading based on both gable and hip roof types.	
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GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary lateral restraint and diagonal bracing. Follow the recommendations for handling, installing and temporary restraining and bracing of trusses. Refer to **BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses***** for more detailed information.

Truss Design Drawings may specify locations of permanent lateral restraint or reinforcement for individual truss members. Refer to the **BCSI-B3***** for more information. All other permanent bracing design is the responsibility of the building designer.

NOTAS GENERALES

Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de restricción lateral y arrioste diagonal temporales. Use las recomendaciones de manejo, instalación, restricción y arrioste temporal de los trusses. Vea el folleto **BCSI - Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arrioste de los Trusses de Madera Conectados con Placas de Metal***** para información más detallada.

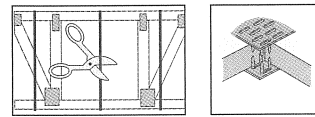
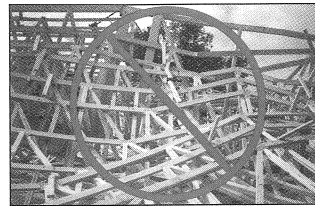
Los dibujos de diseño de los trusses pueden especificar las localizaciones de restricción lateral permanente o refuerzo en los miembros individuales del truss. Vea la hoja resumen **BCSI-B3***** para más información. El resto de los diseños de arriostres permanentes son la responsabilidad del diseñador del edificio.

WARNING! The consequences of improper handling, erecting, installing, restraining and bracing can result in a collapse of the structure, or worse, serious personal injury or death.

¡ADVERTENCIA! El resultado de un manejo, levantamiento, instalación, restricción y arrioste incorrecto puede ser la caída de la estructura o aún peor, heridos o muertos.

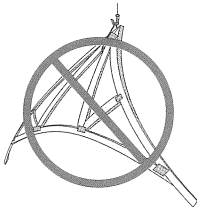
CAUTION! Banding and truss plates have sharp edges. Wear gloves when handling and safety glasses when cutting banding.

¡CAUTELA! Chapas de metal tienen bordes afilados. Lleve guantes y lentes protectores cuando corte las ataduras.

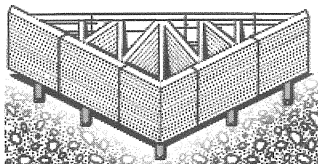


HANDLING — MANEJO

NOTICE Avoid lateral bending. Evite la flexión lateral.



NOTICE The contractor is responsible for properly receiving, unloading and storing the trusses at the jobsite. Unload trusses to smooth surface to prevent damage. El contratista tiene la responsabilidad de recibir, descargar y almacenar adecuadamente los trusses en la obra. Descargue los trusses en la tierra lisa para prevenir el daño.



Trusses may be unloaded directly on the ground at the time of delivery or stored temporarily in contact with the ground after delivery. If trusses are to be stored horizontally for more than one week, place blocking of sufficient height beneath the stack of trusses at 8' (2.4 m) to 10' (3 m) on-center (o.c.).

Los trusses pueden ser descargados directamente en el suelo en aquel momento de entrega o almacenados temporalmente en contacto con el suelo después de entrega. Si los trusses estarán guardados horizontalmente para más de una semana, ponga bloqueando de altura suficiente detrás de la pila de los trusses a 8 hasta 10 pies en centro (o.c.).

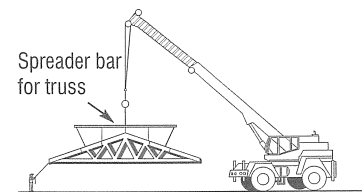
For trusses stored for more than one week, cover bundles to protect from the environment.

Para trusses guardados por más de una semana, cubra los paquetes para protegerlos del ambiente.

Refer to **BCSI***** for more detailed information pertaining to handling and jobsite storage of trusses.

Vea el folleto **BCSI***** para información más detallada sobre el manejo y almacenado de los trusses en área de trabajo.

CAUTION! Use special care in windy weather or near power lines and airports.



Use proper rigging and hoisting equipment.

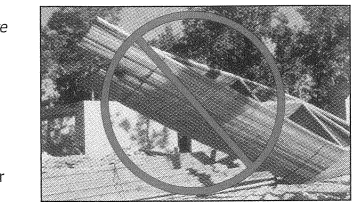
¡CAUTELA! Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.

Use equipo apropiado para levantar e improvisar.



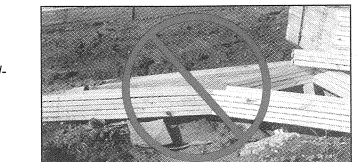
DO NOT store unbraced bundles upright.

NO almacene verticalmente los trusses sueltos.



DO NOT store on uneven ground.

NO almacene en tierra desigual.



HOISTING AND PLACEMENT OF TRUSS BUNDLES

RECOMENDACIONES PARA LEVANTAR PAQUETES DE TRUSSES

DON'T overload the crane.

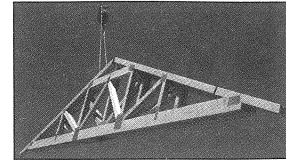
NO sobrecargue la grúa.

NEVER use banding to lift a bundle.

NUNCA use las ataduras para levantar un paquete.

A single lift point may be used for bundles of top chord pitch trusses up to 45' (13.7 m) and parallel chord trusses of 30' (9.1 m) or less. Use at least two lift points for bundles with trusses up to 60' (18.3 m). Use at least 3 lift points for bundles with trusses greater than 60' (18.3 m).

Puede usar un solo lugar de levantar para paquetes de trusses de la cuerda superior hasta 45' y trusses de cuerdas paralelas de 30' o menos. Puede usar dos puntos de levantar para paquetes más de 60 pies. Use por lo menos tres puntos de levantar para paquetes más de 60 pies.



WARNING! Do not over load supporting structure with truss bundle.

¡ADVERTENCIA! No sobrecargue la estructura apoyada con el paquete de trusses.

Place truss bundles in stable position.

Puse paquetes de trusses en una posición estable.

MECHANICAL HOISTING RECOMMENDATIONS FOR SINGLE TRUSSES

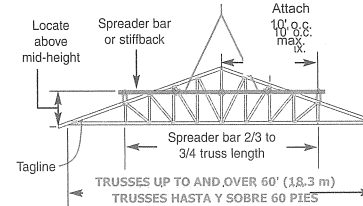
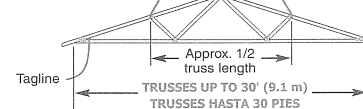
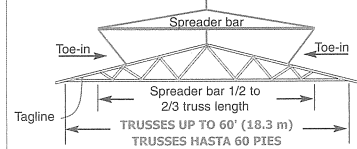
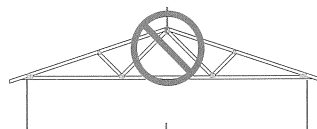
RECOMENDACIONES PARA LEVANTAR TRUSSES INDIVIDUALES

Hold each truss in position with the erection equipment until top chord temporary lateral restraint is installed and the truss is fastened to the bearing points.

Sostenga cada truss en posición con equipo de grúa hasta que la restricción lateral temporal de la cuerda superior esté instalada y el truss está asegurado en los soportes.

NOTICE Using a single pick-point at the peak can damage the truss.

El uso de un solo lugar en el pico para levantar puede hacer daño al truss.

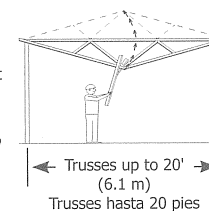


INSTALLATION OF SINGLE TRUSSES BY HAND

RECOMENDACIONES DE LEVANTAMIENTO DE TRUSSES INDIVIDUALES POR LA MANO

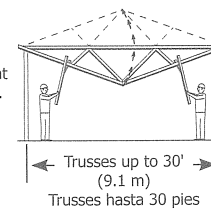
Trusses 20' (6.1 m) or less, support near peak.

Soporte cerca al pico los trusses de 20 pies o menos.



Trusses 30' (9.1 m) or less, support at quarter points.

Soporte de los cuartos de tramo los trusses de 30 pies o menos.



TEMPORARY RESTRAINT & BRACING

RESTRICCIÓN Y ARRIOSTRE TEMPORAL

NOTICE Refer to **BCSI-B2***** for more information.

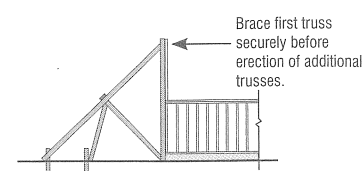
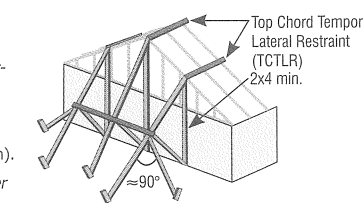
Vea el resumen **BCSI-B2***** para más información.

Locate ground braces for first truss directly in line with all rows of top chord temporary lateral restraint (see table in the next column).

Coloque los arriostres de tierra para el primer truss directamente en línea con cada una de las filas de restricción lateral temporal de la cuerda superior (vea la tabla en la próxima columna).



DO NOT walk on unbraced trusses. **NO** camine en trusses sueltos.



STEPS TO SETTING TRUSSES
LAS MEDIDAS DE LA INSTALACIÓN DE LOS TRUSSES

- 1) Install ground bracing. 2) Set first truss and attach securely to ground bracing. 3) Set next 4 trusses with short member temporary lateral restraint (see below). 4) Install top chord diagonal bracing (see below). 5) Install web member plane diagonal bracing to stabilize the first five trusses (see below). 6) Install bottom chord temporary lateral restraint and diagonal bracing (see below). 7) Repeat process on groups of four trusses until all trusses are set.

NOTICE Refer to BCSI-B2*** for more information.
Vea el resumen BCSI-B2*** para más información.

RESTRAINT/BRACING FOR ALL PLANES OF TRUSSES
RESTRICCIÓN/ARRIOSTRE PARA TODOS PLANOS DE TRUSSES

- This restraint & bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses (PCTs). See top of next column for temporary restraint and bracing of PCTs.
Este método de restricción y arriostre es para todo trusses excepto trusses de cuerdas paralelas (PCTs) 3x2 y 4x2. Vea la parte superior de la columna para la restricción y arriostre temporal de PCTs.

1) TOP CHORD — CUERDA SUPERIOR

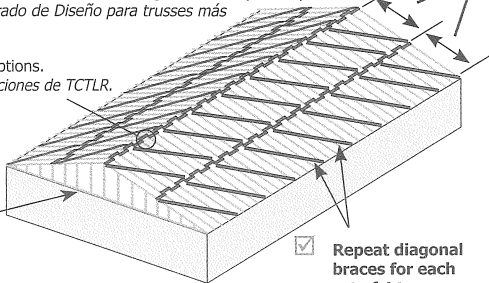
Table with 2 columns: Truss Span / Longitud de Tramo, Top Chord Temporary Lateral Restraint (TCLTR) Spacing / Espaciamiento del Arriostre Temporal de la Cuerda Superior. Rows show spacing requirements for spans up to 30', 30'-45', 45'-60', and 60'-80'.

*Consult a Registered Design Professional for trusses longer than 60' (18.3 m).
*Consulte a un Profesional Registrado de Diseño para trusses más de 60 pies.

- See BCSI-B2*** for TCLTR options.
Vea el BCSI-B2*** para las opciones de TCLTR.

NOTICE Refer to BCSI-B3*** for Gable End Frame restraint/bracing/ reinforcement information.

Para información sobre restricción/arriostre/refuerzo para Armazones Hastiales vea el resumen BCSI-B3***



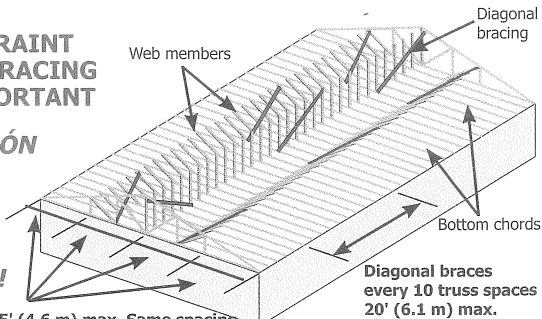
Note: Ground bracing not shown for clarity.

- Repeat diagonal braces for each set of 4 trusses.
Repita los arriostres diagonales para cada grupo de 4 trusses.

2) WEB MEMBER PLANE — PLANO DE LOS MIEMBROS SECUNDARIOS

NOTICE
LATERAL RESTRAINT & DIAGONAL BRACING ARE VERY IMPORTANT

LA RESTRICCIÓN LATERAL Y EL ARRIOSTRE DIAGONAL SON MUY IMPORTANTES!



10' (3 m) - 15' (4.6 m) max. Same spacing as bottom chord lateral restraint

Note: Some chord and web members not shown for clarity.

3) BOTTOM CHORD — CUERDA INFERIOR

Lateral Restraints - 2x4x12' or greater lapped over two trusses.

Bottom chords

10' (3 m) - 15' (4.6 m) max.

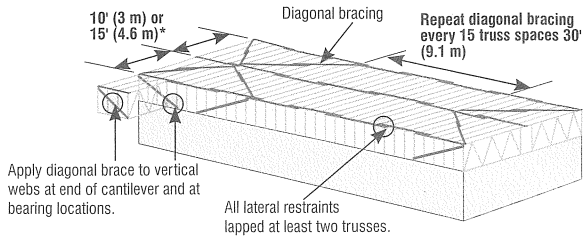
Diagonal braces every 10 truss spaces 20' (6.1 m) max.

Note: Some chord and web members not shown for clarity.

RESTRAINT & BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES
RESTRICCIÓN Y ARRIOSTRE PARA TRUSSES DE CUERDAS PARALELAS 3X2 Y 4X2

NOTICE Refer to BCSI-B7*** for more information.

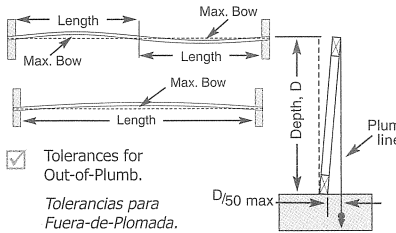
Vea el resumen BCSI-B7*** para más información.



*Top chord temporary lateral restraint spacing shall be 10' (3 m) o.c. max. for 3x2 chords and 15' (4.6 m) o.c. for 4x2 chords.

INSTALLING — INSTALACIÓN

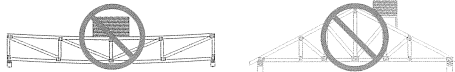
- Tolerances for Out-of-Plane.
Tolerancias para Fuera-de-Plano.



- Tolerances for Out-of-Plumb.
Tolerancias para Fuera-de-Plomada.

CONSTRUCTION LOADING
CARGA DE CONSTRUCCIÓN

- DO NOT proceed with construction until all lateral restraint and bracing is securely and properly in place.
NO proceda con la construcción hasta que todas las restricciones laterales y los arriostres estén colocados en forma apropiada y segura.
- DO NOT exceed maximum stack heights. Refer to BCSI-B4*** for more information.
NO exceda las alturas máximas de montón. Vea el resumen BCSI-B4*** para más información.



- DO NOT overload small groups or single trusses.
NO sobrecargue pequeños grupos o trusses individuales.
- NEVER stack materials near a peak or at mid-span.
NUNCA amontone los materiales cerca de un pico.
- Place loads over as many trusses as possible.
Coloque las cargas sobre tantos trusses como sea posible.
- Position loads over load bearing walls.
Coloque las cargas sobre las paredes soportantes.

ALTERATIONS — ALTERACIONES

NOTICE Refer to BCSI-B5***
Vea el resumen BCSI-B5***

- DO NOT cut, alter, or drill any structural member of a truss unless specifically permitted by the truss design drawing.
NO corte, altere o perforo ningún miembro estructural de un truss, a menos que esté específicamente permitido en el dibujo del diseño del truss.

NOTICE Trusses that have been overloaded during construction or altered without the Truss Manufacturer's prior approval may render the Truss Manufacturer's limited warranty null and void.
Trusses que se han sobrecargado durante la construcción o han sido alterados sin la autorización previa del Fabricante de Trusses, pueden hacer nulo y sin efecto la garantía limitada del Fabricante de Trusses.

***Contact the Component Manufacturer for more information or consult a Registered Design Professional for assistance.
To view a non-printing PDF of this document, visit www.sbcindustry.com/b1.

NOTE: The truss manufacturer and truss designer rely on the presumption that the contractor and crane operator (if applicable) are professionals with the capability to undertake the work they have agreed to do on any given project. If the contractor believes it needs assistance in some aspect of the construction project, it should seek assistance from a competent party. The methods and procedures outlined in this document are intended to ensure that the overall construction techniques employed will put the trusses into place SAFELY. These recommendations for handling, installing, restraining and bracing trusses are based upon the collective experience of leading personnel involved with truss design, manufacture and installation, but must, due to the nature of responsibilities involved, be presented only as a GUIDE for use by a qualified building designer or contractor. It is not intended that these recommendations be interpreted as superior to the building designer's design specification for handling, installing, restraining and bracing trusses and it does not preclude the use of other equivalent methods for restraining/bracing and providing stability for the walls, columns, floors, roofs and all the interrelated structural building components as determined by the contractor. Thus, SBCA and TPI expressly disclaim any responsibility for damages arising from the use, application, or reliance on the recommendations and information contained herein.



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