

UPLIFT CONNECTORS TO TRUSSES AND GIRDER TRUSSES IS THE RESPONSIBILITY OF THE ENGINEER, ARCHITECT, OR GC. NOT THE FABRICATOR. DO NOT CUT OR ALTER TRUSSES BEFORE NOTIFYING AND/OR APPROVAL IN WRITING BY FORT DALLAS TRUSS CO.

ALL NECESSARY REPAIRS TO TRUSSES MUST BE REPORTED TO FORT DALLAS TRUSS CO. WITHIN 48 HOURS OF DISCOVERY. TRUSS LAYOUT IS A PLACEMENT PLAN ONLY.

REVISION OF THIS SHOP DRAWING BY PROFESSIONAL ENGINEER IS IN COMPLIANCE OF POLICY NO. 5 METRO DADE COUNTY BUILDING AND ZONING DEPARTMENT EXCEPT FOR ALL BUILDING COMPONENTS NOT FABRICATED BY FORT DALLAS TRUSS CO. REFER TO ARCHITECT OR ENGINEER FOR COMPLETE CORRELATION OF THE INFORMATION PROVIDED IN THIS SHOP DRAWING WITH ALL OTHER TRADES, STRUCTURAL BEAMS, WALLS AND FOUNDATION REQUIREMENTS OF THE FBC.

FOR TRUSSES HAVING AN OVERALL LENGTH OF 35 FEET (10.7m) OR 6 FEET (1.8m) OVERALL HEIGHT. ERECTION SHALL BE SUPERVISED BY EITHER A REGESTIRED PROFESSIONAL ENGINEER OR REGISTERED ARCHITECT RETAINED BY THE CONTRACTOR.

ALL TRUSSES SPACE 24" O.C. UNLESS OTHERWISE SPECIFIED.

NOTE: LABELING TRUSSES IS A SERVICE, NOT A REQUIREMENT. ENGINEERED DRAWINGS SUPERSEDES LABELING OF TRUSSES. IT IS THE RESPONSIBILITY OF THE BUILDER TO UTILIZE ENGINEERED DRAWINGS WHEN ERECTING THE TRUSSES.

****ATTENTION BUILDER****

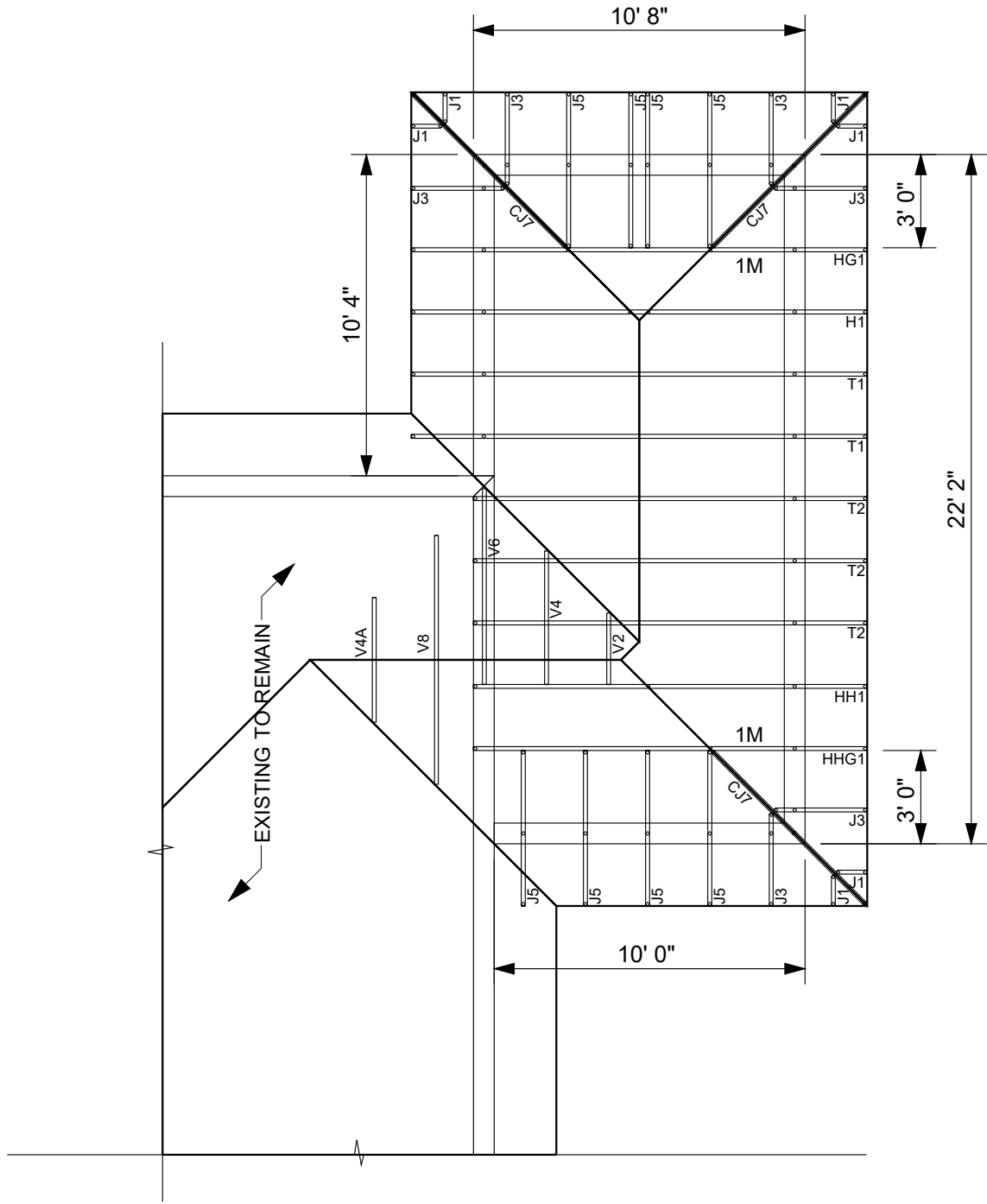
ALL DIMENSIONS,
OVERHANGS, PITCH, ETC
TO BE CHECKED
BEFORE ORDERING TRUSSES
IT IS YOUR RESPONSIBILITY
TO CHECK OUR TRUSS PLAN

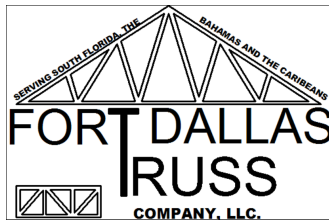
JOSE MARTINEZ, P.E.
Consulting Engineer Civil/Structural
24 E. 5th Street
Hialeah, FL 33010
(Florida P.E. 031509)

STATEMENT in my professional
judgement and to best of my
knowledge and belief, these truss
specifications comply with
applicable building codes.



DATE:	4	3	2	1	DRAWN BY: 03-19-25 XA
APP:					CHECKED BY: 03-20-25 TS
					DRAWING NUMBER: 25C17





Fort Dallas Truss Company, LLC.
7035 SW 44th ST
Miami, FL 33155
Tel: (305) 667-6797
Fax: (305) 667-0592

Re : 25C17: MAZZINI & MELINE

Site Information: Project Customer: TRI-LEVEL BUILDERS INC.

Lot/Block:

Site Name: 10300 SW 70 AVE

Site Address:

MIAMI St: FL Zip:

Job Name : MAZZINI & MELINE RESIDENCE

Subdivision :

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2020/TPI2014 Design Program: 8.82 Jan 17 2025

Roof Load: 60.0 psf Floor Load: N/A

Wind Code: ASCE7-16 C-C for members and forces & MWFRS for reactions

Wind Speed: 175 mph

This package includes 15 individual, dated Truss Design Drawings and 0 Additional Drawings.

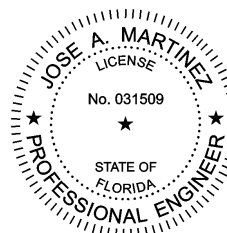
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Date	Truss ID#	Seal#
1	05/20/25	CJ7	A0126232
2	05/20/25	H1	A0126233
3	05/20/25	HG1	A0126234
4	05/20/25	HH1	A0126235
5	05/20/25	HHG1	A0126236
6	05/20/25	J1	A0126237
7	05/20/25	J3	A0126238
8	05/20/25	J5	A0126239
9	05/20/25	T1	A0126240
10	05/20/25	T2	A0126241
11	05/20/25	V2	A0126242
12	05/20/25	V4	A0126243
13	05/20/25	V4A	A0126244
14	05/20/25	V6	A0126245
15	05/20/25	V8	A0126246

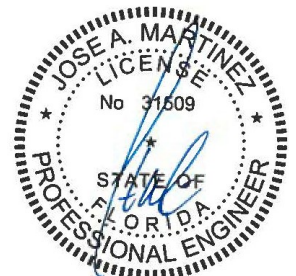
The truss drawing(s) referenced above have been prepared under my direct supervision based on the parameters by Arch. or ENG of Record using MiTek 20/20 software.

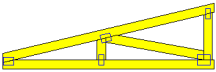
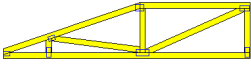
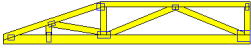
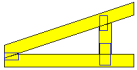
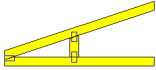
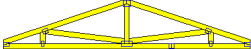
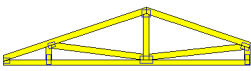
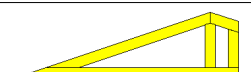
Truss Design Engineer's Name: Jose Martinez, P.E.
24 E 5th ST, Hialeah, FL 33010 (Florida PE# 031509)

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility for the truss components shown.



Digitally signed by
JOSE A MARTINEZ
Date: 2025.06.17
08:42:55 -04'00'



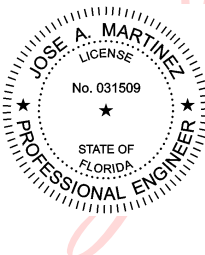
Fort Dallas Truss Co. LLC. 7035 SW 44 St. Miami, FL. 33155 (305) 667-6797 Fax: (305) 667-0592			To: TRI-LEVEL BUILDERS INC. 14347 SW 158 PATH MIAMI, FL					Delivery	
Project: MAZZINI & MELINE Block No: Model: Lot No:			Deliver To: 10300 SW 70 AVE MIAMI, FL					Job Number: 25C17 Page: 1 Date: 06/03/25 16:51:55	
Contact:	Site:	Office:						Account No: 000001019 Designer: / Salesperson: MARIA VALDES Quote Number: P.O. Number:	
Name:		(786) 299-6064							
Phone:									
Fax:									
Profile:	Qty:	Truss Id:	Span:	Truss Type:	Slope	LOH	ROH		
	3	CJ7	07-00-02 2 X 4 / 2 X 4	DIAGONAL HIP	2.83			OA Hght: 02-01-14	1
	1	H1	14-08-00 2 X 4 / 2 X 4	HIP	4.00			OA Hght: 02-10-01	2
	1	HG1	14-08-00 2 X 4 / 2 X 6	HIP GIRDER	4.00			OA Hght: 02-02-01	3
	1	HH1	12-08-00 2 X 4 / 2 X 4	HALF HIP	4.00			OA Hght: 02-10-01	4
	1	HHG1	12-08-00 2 X 4 / 2 X 6	HALF HIP	4.00			OA Hght: 02-02-01	5
	6	J1	01-00-00 2 X 4 / 2 X 4	CORNER JACK	4.00			OA Hght: 00-10-01	6
	6	J3	03-00-00 2 X 4 / 2 X 4	CORNER JACK	4.00			OA Hght: 01-06-01	7
	8	J5	05-00-00 2 X 4 / 2 X 4	JACK-OPEN	4.00			OA Hght: 02-02-01	8
	2	T1	14-08-00 2 X 4 / 2 X 4	COMMON	4.00			OA Hght: 02-11-06	9
	3	T2	12-08-00 2 X 4 / 2 X 4	COMMON	4.00			OA Hght: 02-11-06	10
	1	V2	02-03-08 / 2 X 6	VALLEY	0.00			OA Hght: 00-05-08	11
	1	V4	04-03-08 2 X 4 / 2 X 4	VALLEY	4.00			OA Hght: 01-02-00	12
	1	V4A	04-00-00 2 X 4 / 2 X 4	VALLEY	4.00			OA Hght: 00-08-00	13
	1	V6	06-03-08 2 X 4 / 2 X 4	VALLEY	4.00			OA Hght: 01-10-00	14
	1	V8	08-00-00 2 X 4 / 2 X 4	VALLEY	4.00			OA Hght: 01-04-00	15

Fort Dallas Truss Co. LLC. 7035 SW 44 St. Miami, FL. 33155 (305) 667-6797 Fax: (305) 667-0592			To: TRI-LEVEL BUILDERS INC. 14347 SW 158 PATH MIAMI, FL		Delivery	
Project: MAZZINI & MELINE Block No: Model: Lot No:					Job Number: 25C17	
Contact: Site: Office:			Deliver To: 10300 SW 70 AVE MIAMI, FL		Page: 2	
Name:					Date: 06/03/25 16:51:56	
Phone:					Account No: 000001019	
Fax:					Designer: /	
					Salesperson: MARIA VALDES	
					Quote Number:	
					P.O. Number:	

The truss drawing(s) referenced above have been prepared under my direct supervision based on the parameters by Arch. or ENG of Record using MiTek 20/20 software.

Truss Design Engineer's Name: Jose Martinez, P.E.
24 E 5th ST, Hialeah, FL 33010 (Florida PE# 031509)

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility for the truss components shown.



Digitally signed
by JOSE A
MARTINEZ
Date:
2025.06.17
08:43:29 -04'00'



Engineers Name

Date

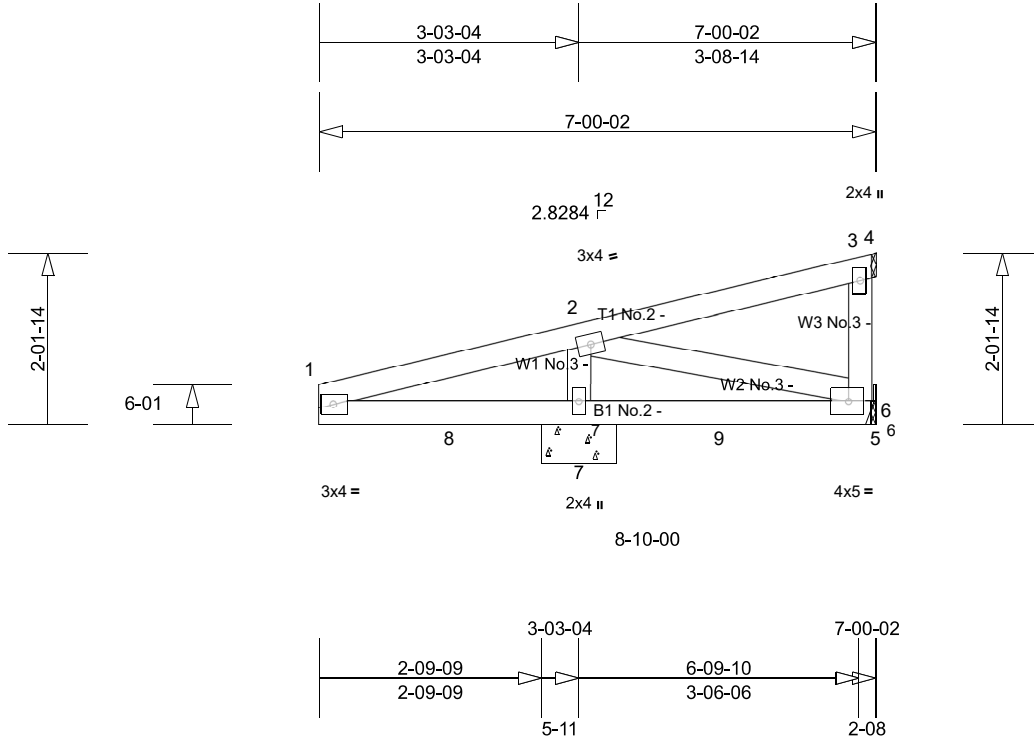
Job 25C17	Truss CJ7	Truss Type Diagonal Hip Girder	Qty 3	Ply 1	TRI-LEVEL BUILDER INC. Job Reference (optional)	A0126232
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FORT DALLAS TRUSS CO. LLC., MIAMI, FL., XUAN

Run: 8.82 S Jan 17 2025 Print: 8.820 S Jan 17 2025 MiTek Industries, Inc. Tue Jun 03 16:53:10

Page: 1

ID:FqjE7aQOHT2HAKeKL0T2UQzZMAc-C?ExvnV50ipdg8oNM_3P4oLJu7K7n1MeMfrVkUz9xv8



Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	30.0	Plate Grip DOL	1.00	TC	0.34	Vert(LL)	-0.01	6-7	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.33	BC	0.42	Vert(CT)	-0.04	6-7	>999	180		
BCLL	0.0	Rep Stress Incr	NO	WB	0.23	Horz(CT)	0.00	6	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-P							Weight: 30 lb	FT = 0%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6'-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6'-0-0 oc bracing.

REACTIONS (lb/size) 6=280/ Mechanical, (min. 1-08), 7=444/11-05, (min. 1-08)

Max Horiz 7=134 (LC 6)
Max Uplift 6=-211 (LC 8), 7=-521 (LC 6)
Max Grav 6=346 (LC 26), 7=607 (LC 27)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-640/665
BOT CHORD 1-8=-620/654, 7-8=-620/654, 7-9=-620/520, 6-9=-620/520
WEBS 2-7=-517/495, 2-6=-540/644

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCDL=5.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone; cantilever left exposed ; Lumber DOL=1.33 plate grip DOL=1.33
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- Plates checked for a plus or minus 0 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 211 lb uplift at joint 6 and 521 lb uplift at joint 7.
- This truss has been designed for a moving concentrated load of 200.0lb dead located at all mid panels and at all panel points along the Bottom Chord, nonconcurrent with any other live loads.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

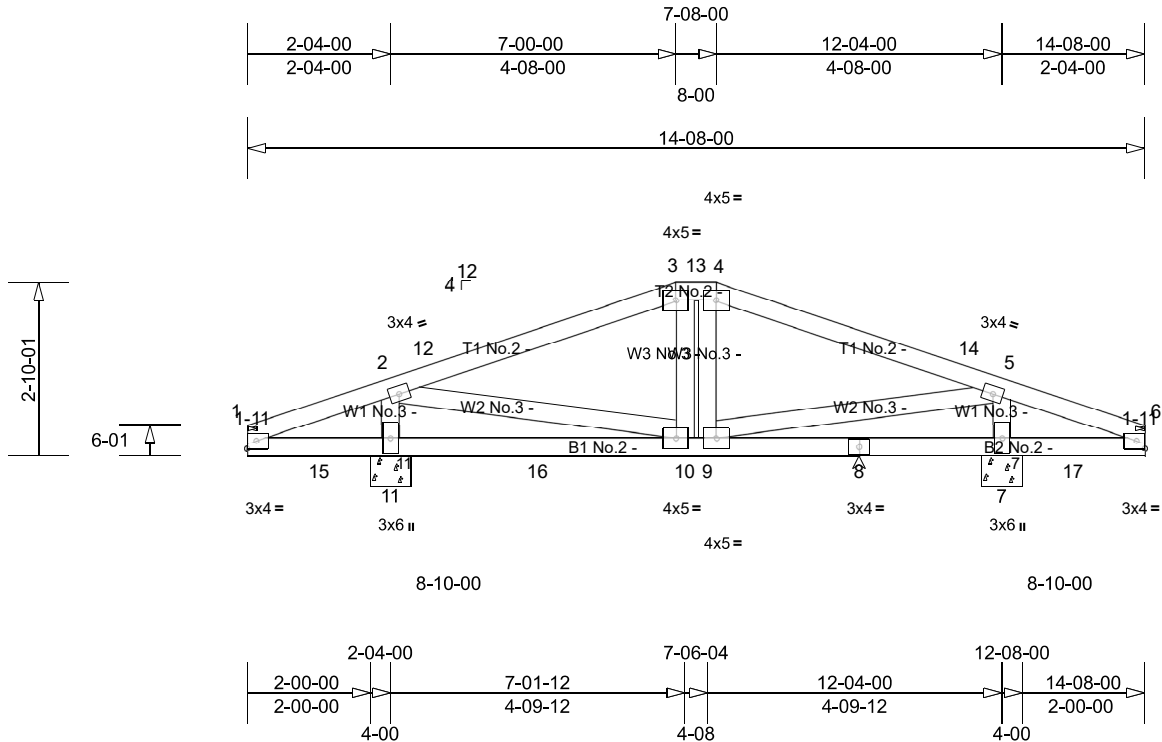
LOAD CASE(S) Standard

- Dead + Roof Live (balanced): Lumber Increase=1.33, Plate Increase=1.00

Trapezoidal Loads (lb/ft)

Vert: 1=0 (F=50, B=50)-to-2=-82 (F=9, B=9), 2=-82 (F=9, B=9)-to-3=-170 (F=-35, B=-35), 3=-110 (F=-35, B=-35)-to-4=-115 (F=-38, B=-38), 1=0 (F=10, B=10)-to-8=-8 (F=6, B=6), 8=-8 (F=6, B=6)-to-7=-16 (F=2, B=2), 7=-16 (F=2, B=2)-to-9=-25 (F=-3, B=-3), 9=-25 (F=-3, B=-3)-to-6=-34 (F=-7, B=-7), 6=-34 (F=-7, B=-7)-to-5=-35 (F=-8, B=-8)

Job 25C17	Truss H1	Truss Type Hip	Qty 1	Ply 1	TRI-LEVEL BUILDER INC. Job Reference (optional)	A0126233
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Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	30.0	Plate Grip DOL	1.00	TC	0.52	Vert(LL)	0.01	10	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.33	BC	0.44	Vert(CT)	-0.07	10-11	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.29	Horz(CT)	0.00	7	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-S							Weight: 68 lb	FT = 0%

LUMBER		BRACING	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing, Except:
WEBS	2x4 SP No.3		10-0-0 oc bracing: 9-10.

REACTIONS (lb/size) 7=880/8-00, (min. 1-08), 11=880/8-00, (min. 1-08)
Max Horiz 11=-93 (LC 15)
Max Uplift 7=-550 (LC 7), 11=-550 (LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-217/559, 2-12=-612/471, 3-12=-531/485, 3-13=-505/535, 4-13=-505/535, 4-14=-531/485, 5-14=-612/471, 5-6=-217/559
BOT CHORD 1-15=-484/208, 11-15=-484/208, 11-16=-484/208, 10-16=-484/208, 9-10=-242/499, 8-9=-484/208, 7-8=-484/208, 7-17=-484/208, 6-17=-484/208
WEBS 2-11=-782/810, 2-10=-467/658, 5-7=-782/810, 5-9=-467/658

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCDL=5.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 3-0-0, Exterior(2R) 3-0-0 to 11-8-0, Exterior(2E) 11-8-0 to 14-8-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
 - Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
 - Provide adequate drainage to prevent water ponding.
 - Plates checked for a plus or minus 0 degree rotation about its center.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 550 lb uplift at joint 11 and 550 lb uplift at joint 7.
 - This truss has been designed for a moving concentrated load of 200.0lb dead located at all mid panels and at all panel points along the Bottom Chord, nonconcurrent with any other live loads.

LOAD CASE(S) Standard

Job 25C17	Truss HG1	Truss Type Hip Girder	Qty 1	Ply 1	TRI-LEVEL BUILDER INC. Job Reference (optional)	A0126234
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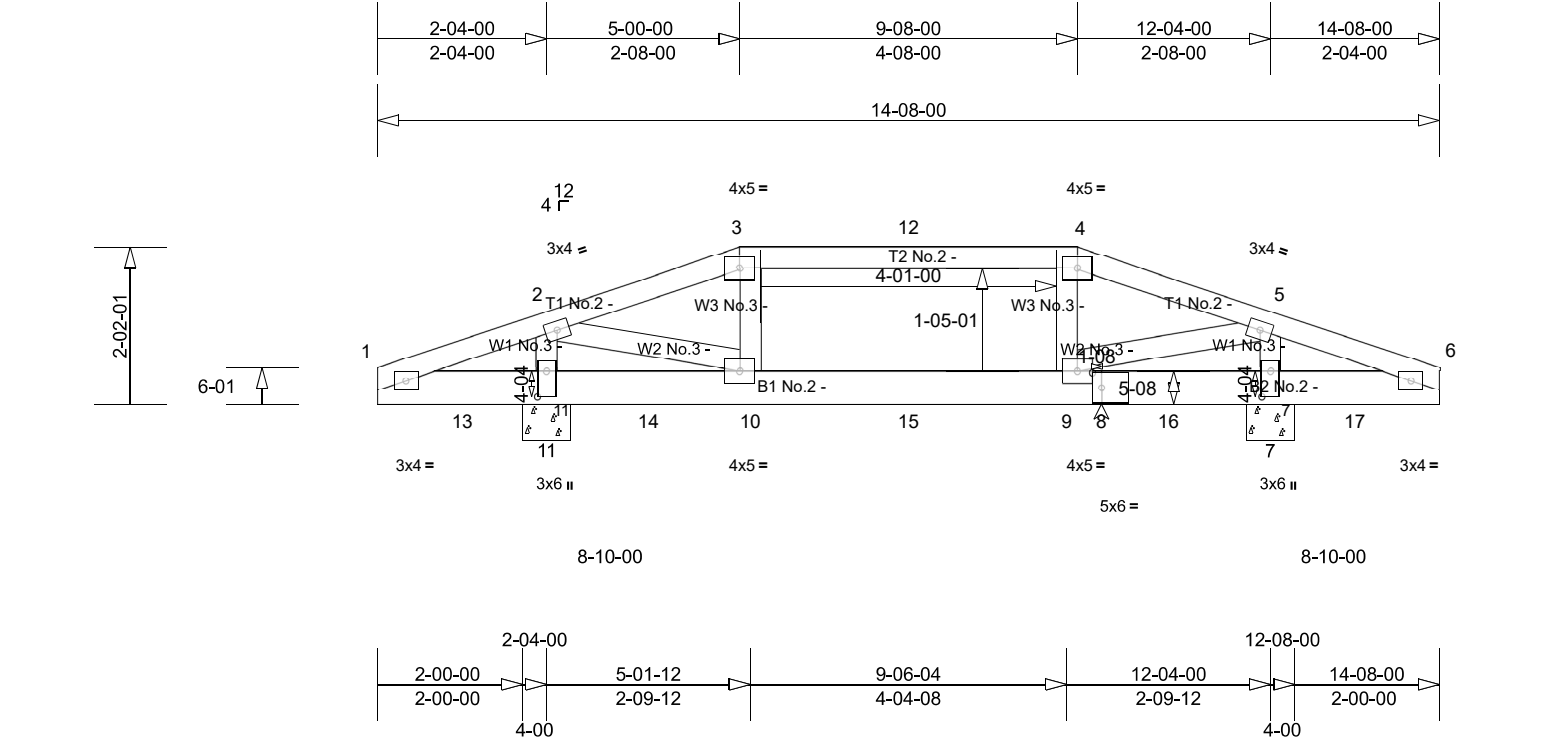


Plate Offsets (X, Y): [7:4-04,1-08], [8:1-08,2-08], [11:4-04,1-08]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	30.0	Plate Grip DOL	1.00	TC	0.88	Vert(LL)	0.04	9-10	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.33	BC	0.37	Vert(CT)	-0.05	9-10	>999	180		
BCLL	0.0	Rep Stress Incr	NO	WB	0.54	Horz(CT)	0.01	7	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-S							Weight: 71 lb	FT = 0%

LUMBER		BRACING	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 3-0-7 oc purlins.
BOT CHORD	2x6 SP No.2	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
WEBS	2x4 SP No.3		9-1-12 oc bracing: 9-10.

REACTIONS (lb/size) 7=1392/8-00, (min. 1-10), 11=1392/8-00, (min. 1-10)
Max Horiz 11=66 (LC 8)
Max Uplift 7=-864 (LC 5), 11=-864 (LC 4)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-189/570, 2-3=-1425/764, 3-12=-1311/744, 4-12=-1311/744, 4-5=-1425/764, 5-6=-187/570
BOT CHORD 1-13=-511/188, 11-13=-511/188, 11-14=-511/140, 10-14=-511/140, 10-15=-640/1311, 9-15=-640/1311, 8-9=-511/186,
8-16=-511/186, 7-16=-511/186, 7-17=-511/186, 6-17=-511/186
WEBS 2-11=-1188/731, 2-10=-832/1510, 5-7=-1188/730, 5-9=-831/1510

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCDL=5.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed ; Lumber DOL=1.33 plate grip DOL=1.33
 - Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
 - Provide adequate drainage to prevent water ponding.
 - Plates checked for a plus or minus 0 degree rotation about its center.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 864 lb uplift at joint 11 and 864 lb uplift at joint 7.
 - This truss has been designed for a moving concentrated load of 200.0lb dead located at all mid panels and at all panel points along the Bottom Chord, nonconcurrent with any other live loads.
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 450 lb down and 187 lb up at 9-8-0, and 450 lb down and 187 lb up at 5-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Dead + Roof Live (balanced): Lumber Increase=1.33, Plate Increase=1.00
Uniform Loads (lb/ft)
Vert: 1-3=-100, 3-4=-158 (F=-58), 4-6=-100, 1-10=-20, 9-10=-32 (F=-12), 6-9=-20
Concentrated Loads (lb)
Vert: 10=-350, 9=-350

Job 25C17	Truss HH1	Truss Type Half Hip	Qty 1	Ply 1	TRI-LEVEL BUILDER INC. Job Reference (optional)	A0126235
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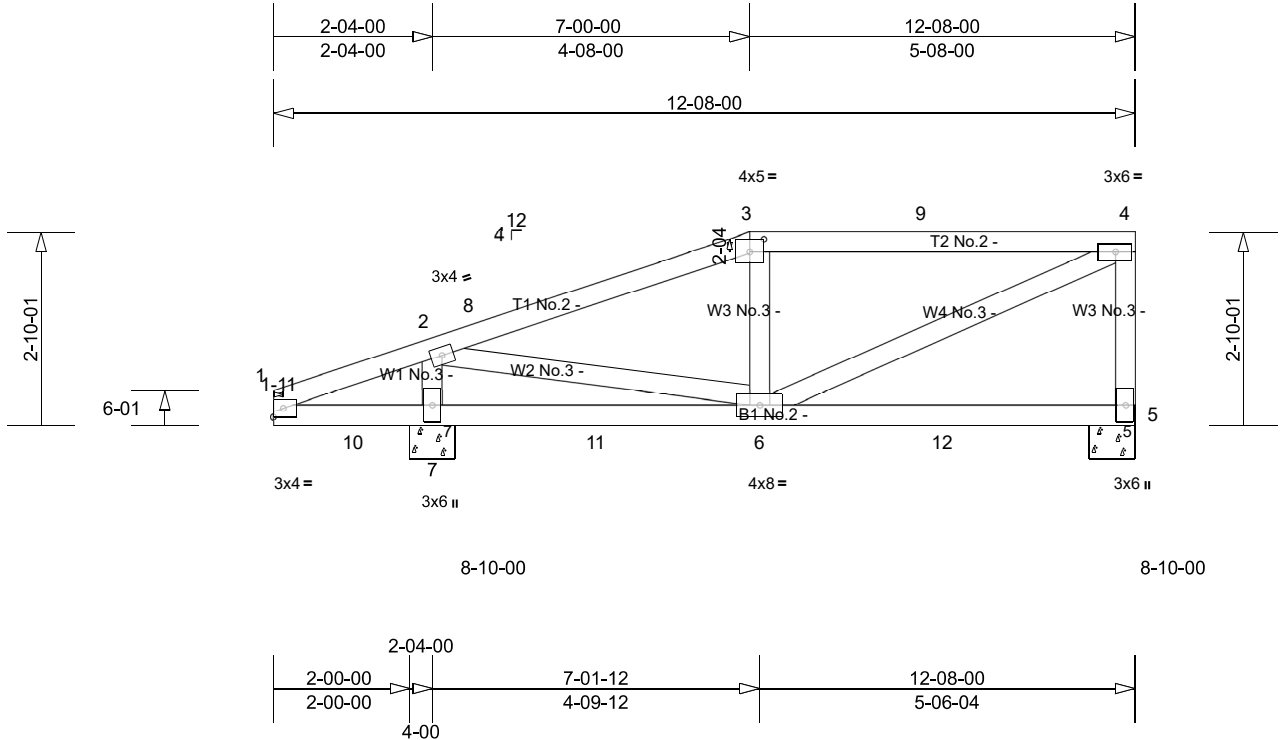


Plate Offsets (X, Y): [3:2-08,2-04]																
Loading		(psf)	Spacing		2-00-00	CSI		DEFL		in	(loc)	I/defl	L/d	PLATES	GRIP	
TCLL (roof)		30.0	Plate Grip DOL		1.00	TC		0.71	Vert(LL)		-0.02	5-6	>999	360	MT20	244/190
TCDL		20.0	Lumber DOL		1.33	BC		0.55	Vert(CT)		-0.12	5-6	>999	180		
BCLL		0.0	Rep Stress Incr		YES	WB		0.39	Horz(CT)		0.00	5	n/a	n/a		
BCDL		10.0	Code		FBC2020/TPI2014	Matrix-S									Weight: 61 lb	FT = 0%

LUMBER			BRACING		
TOP CHORD	2x4 SP No.2		TOP CHORD	Structural wood sheathing directly applied or 5-7-3 oc purlins, except end verticals.	
BOT CHORD	2x4 SP No.2		BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.	
WEBS	2x4 SP No.3				

REACTIONS (lb/size) 5=579/8-00, (min. 1-08), 7=923/8-00, (min. 1-08)
Max Horiz 7=216 (LC 6)
Max Uplift 5=-329 (LC 6), 7=-577 (LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-237/565, 2-8=-721/527, 3-8=-645/540, 3-9=-595/581, 4-9=-595/581, 4-5=-525/526
BOT CHORD 1-10=-491/230, 7-10=-491/230, 7-11=-491/106, 6-11=-491/106
WEBS 3-6=-302/436, 4-6=-583/588, 2-7=-827/854, 2-6=-538/734

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCDL=5.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 3-0-0, Exterior(2R) 3-0-0 to 9-6-4, Exterior(2E) 9-6-4 to 12-6-4 zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
 - Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
 - Provide adequate drainage to prevent water ponding.
 - Plates checked for a plus or minus 0 degree rotation about its center.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 329 lb uplift at joint 5 and 577 lb uplift at joint 7.
 - This truss has been designed for a moving concentrated load of 200.0lb dead located at all mid panels and at all panel points along the Bottom Chord, nonconcurrent with any other live loads.

LOAD CASE(S) Standard

Job 25C17	Truss HHG1	Truss Type Half Hip Girder	Qty 1	Ply 1	TRI-LEVEL BUILDER INC. Job Reference (optional)	A0126236
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FORT DALLAS TRUSS CO. LLC., MIAMI, FL., XUAN

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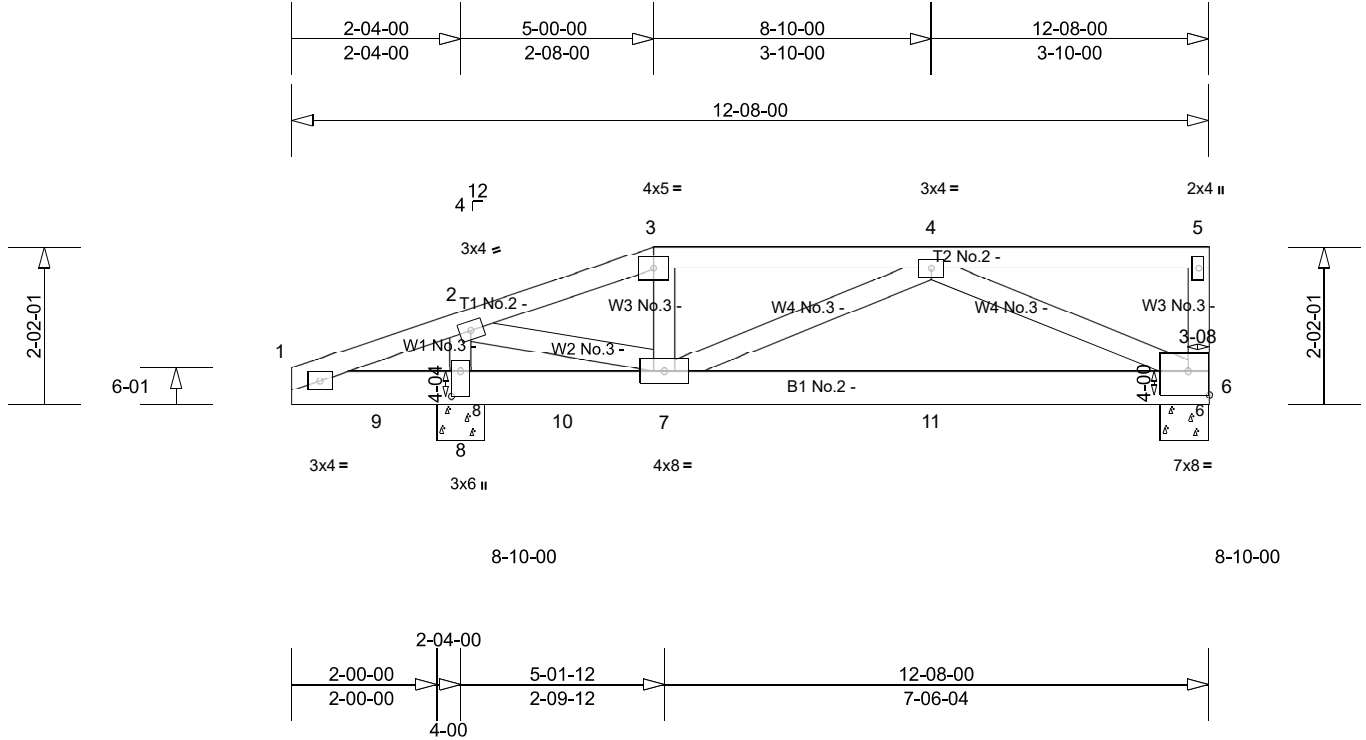


Plate Offsets (X, Y): [6:Edge,3-00], [8:4-04,1-08]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	30.0	Plate Grip DOL	1.00	TC	0.51	Vert(LL)	0.04	6-7	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.33	BC	0.57	Vert(CT)	-0.11	6-7	>999	180		
BCLL	0.0	Rep Stress Incr	NO	WB	0.53	Horz(CT)	0.01	6	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-S							Weight: 69 lb	FT = 0%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-8-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 6=1007/8-00, (min. 1-08), 8=1370/8-00, (min. 1-10)
Max Horiz 8=153 (LC 4)
Max Uplift 6=-578 (LC 5), 8=-847 (LC 4)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-160/564, 2-3=-1400/686, 3-4=-1288/669, 5-6=-251/203
BOT CHORD 1-9=-504/165, 8-9=-504/165, 8-10=-504/54, 7-10=-504/54, 7-11=-897/1381, 6-11=-897/1381
WEBS 3-7=0/258, 4-7=-117/263, 4-6=-1448/969, 2-8=-1215/682, 2-7=-704/1478

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCDL=5.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone; cantilever left exposed ; Lumber DOL=1.33 plate grip DOL=1.33
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- Provide adequate drainage to prevent water ponding.
- Plates checked for a plus or minus 0 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 578 lb uplift at joint 6 and 847 lb uplift at joint 8.
- This truss has been designed for a moving concentrated load of 200.0lb dead located at all mid panels and at all panel points along the Bottom Chord, nonconcurrent with any other live loads.
- Girder carries hip end with 0-0-0 right side setback, 5-0-0 left side setback, and 5-0-0 end setback.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 450 lb down and 187 lb up at 5-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S)

- Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.33, Plate Increase=1.00
Uniform Loads (lb/ft)
Vert: 1-3=-100, 3-5=-158 (F=-58), 1-7=-20, 6-7=-32 (F=-12)
Concentrated Loads (lb)
Vert: 7=-350

Job	Truss	Truss Type	Qty	Ply	TRI-LEVEL BUILDER INC.
25C17	J1	Corner Jack	6	1	A0126237
					Job Reference (optional)

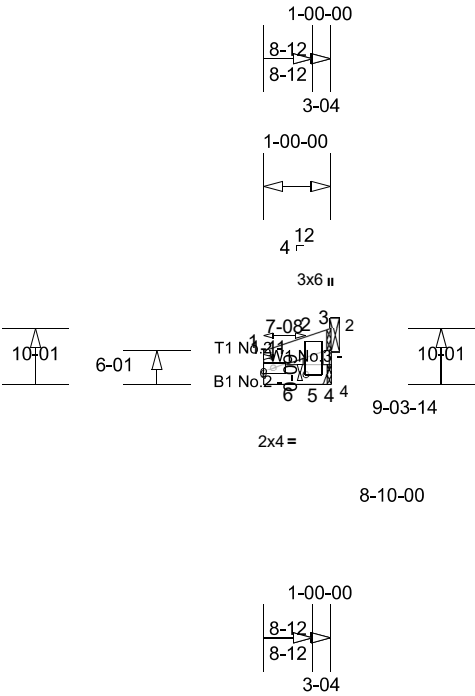


Plate Offsets (X, Y): [2:0-09,7-08]													
Loading		(psf)	Spacing	2-00-00	CSI	DEFL		in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)		30.0	Plate Grip DOL	1.00	TC	0.12	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL		20.0	Lumber DOL	1.33	BC	0.13	Vert(CT)	0.00	5	>999	180		
BCLL		0.0	Rep Stress Incr	YES	WB	0.06	Horz(CT)	n/a	-	n/a	n/a		
BCDL		10.0	Code	FBC2020/TPI2014	Matrix-S							Weight: 3 lb	FT = 0%

LUMBER			BRACING		
TOP CHORD	2x4 SP No.2		TOP CHORD		Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD	2x4 SP No.2		BOT CHORD		Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS	2x4 SP No.3				

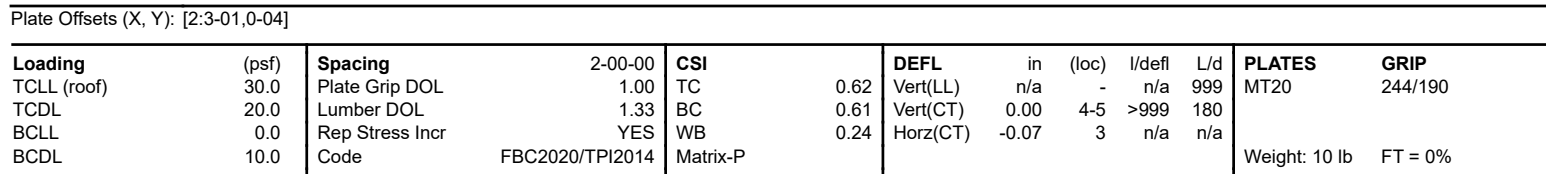
REACTIONS (lb/size) 2=0/ Mechanical, (min. 1-08), 4=104/ Mechanical, (min. 1-08)
Max Horiz 2=507 (LC 21), 4=507 (LC 21)
Max Uplift 4=-102 (LC 6)
Max Grav 4=260 (LC 19)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-223/606
BOT CHORD 1-6=-507/219, 5-6=-507/219, 4-5=-507/219
WEBS 2-5=-344/158

- NOTES**
- Wind: ASCE 7-16; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCDL=5.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
 - Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
 - Plates checked for a plus or minus 0 degree rotation about its center.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - Refer to girder(s) for truss to truss connections.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 102 lb uplift at joint 4.
 - Non Standard bearing condition. Review required.
 - This truss has been designed for a moving concentrated load of 200.0lb dead located at all mid panels and at all panel points along the Bottom Chord, nonconcurrent with any other live loads.
 - Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

LOAD CASE(S) Standard

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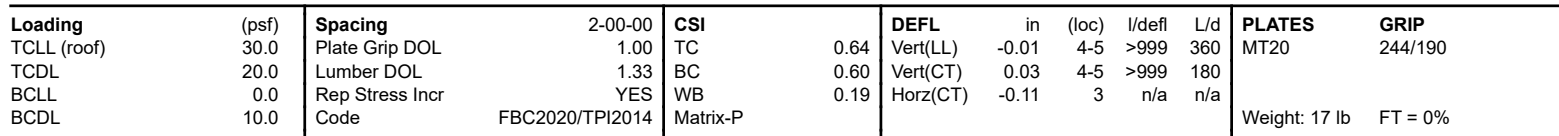


REACTIONS	(lb/size)	3=-262/ Mechanical, (min. 1-08), 4=-243/ Mechanical, (min. 1-08), 5=857/8-00, (min. 1-10)
	Max Horiz	5=100 (LC 6)
	Max Uplift	3=-490 (LC 21), 4=-534 (LC 21), 5=-641 (LC 6)
	Max Grav	3=181 (LC 6), 4=175 (LC 6), 5=1401 (LC 21)
FORCES		(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS		2-5=-691/669

- NOTES**
- 1) Wind: ASCE 7-16; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCDL=5.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
 - 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
 - 3) Plates checked for a plus or minus 0 degree rotation about its center.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 490 lb uplift at joint 3, 534 lb uplift at joint 4 and 641 lb uplift at joint 5.
 - 7) This truss has been designed for a moving concentrated load of 200.0lb dead located at all mid panels and at all panel points along the Bottom Chord, nonconcurrent with any other live loads.

LOAD CASE(S) Standard

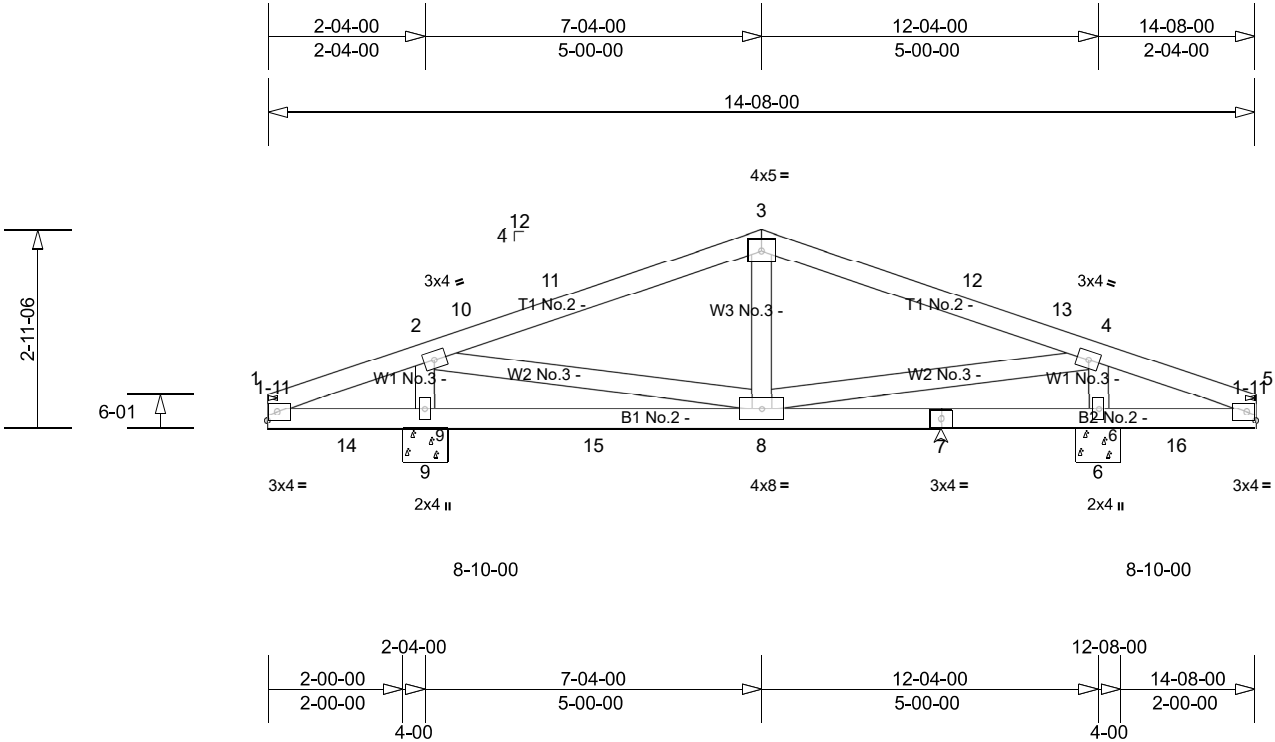
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REACTIONS	(lb/size)	3=57/ Mechanical, (min. 1-08), 4=-27/ Mechanical, (min. 1-08), 5=562/8-00, (min. 1-08)
	Max Horiz	5=157 (LC 6)
	Max Uplift	3=-84 (LC 10), 4=-96 (LC 21), 5=-384 (LC 6)
	Max Grav	3=57 (LC 1), 4=197 (LC 23), 5=660 (LC 21)
FORCES		(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS		2-5=-401/529

- LOAD CASE(S)** Standard

Job	Truss	Truss Type	Qty	Ply	TRI-LEVEL BUILDER INC.	A0126240
25C17	T1	Common	2	1	Job Reference (optional)	



Job 25C17	Truss T2	Truss Type Common	Qty 3	Ply 1	TRI-LEVEL BUILDER INC. Job Reference (optional)	A0126241
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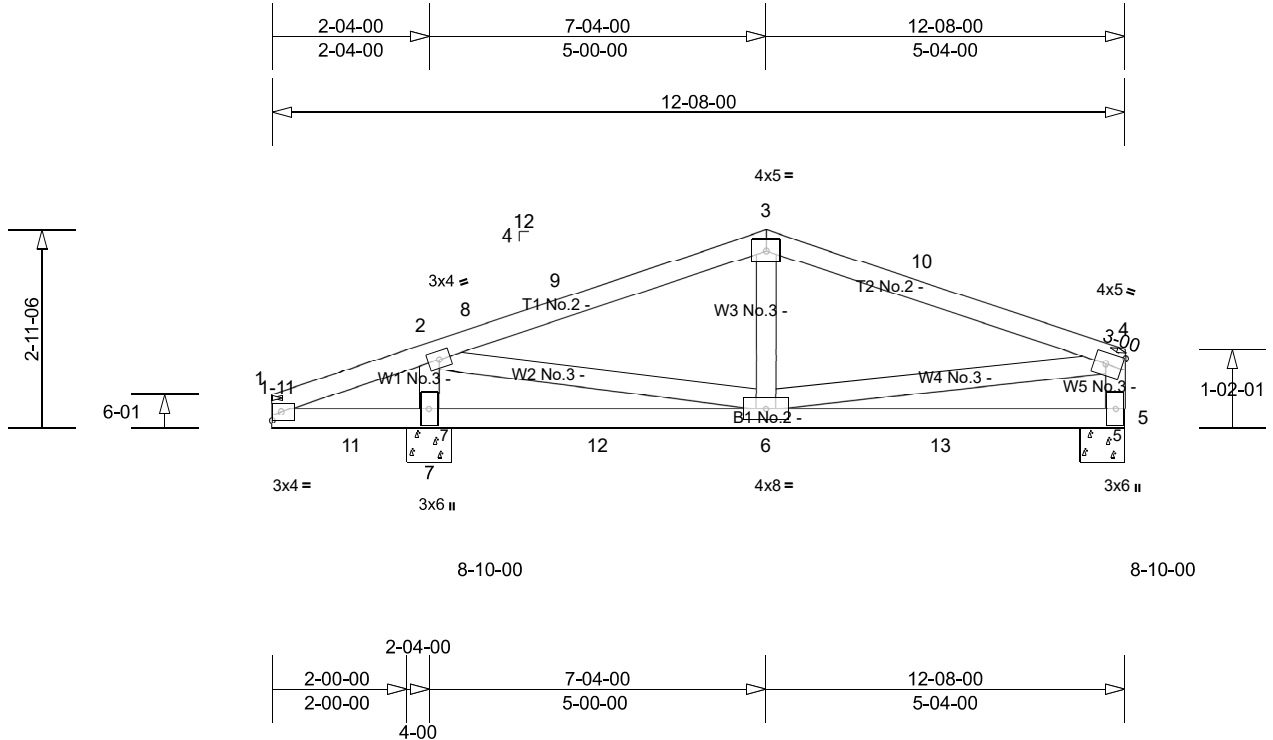


Plate Offsets (X, Y): [4:Edge,2-00]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	30.0	Plate Grip DOL	1.00	TC	0.69	Vert(LL)	-0.02	5-6	>999	360	MT20	244/190
TCDL	20.0	Lumber DOL	1.33	BC	0.54	Vert(CT)	-0.10	5-6	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.31	Horz(CT)	0.00	5	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-S							Weight: 59 lb	FT = 0%

LUMBER			BRACING		
TOP CHORD	2x4 SP No.2		TOP CHORD	Structural wood sheathing directly applied or 5-8-3 oc purlins, except end verticals.	
BOT CHORD	2x4 SP No.2		BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.	
WEBS	2x4 SP No.3				

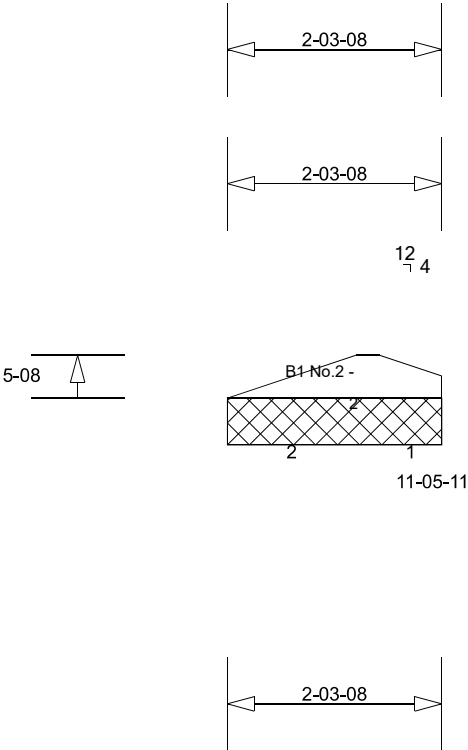
REACTIONS (lb/size) 5=579/8-00, (min. 1-08), 7=923/8-00, (min. 1-08)
Max Horiz 7=141 (LC 10)
Max Uplift 5=-281 (LC 7), 7=-557 (LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-219/552, 2-8=-702/603, 8-9=-639/608, 3-9=-619/617, 3-10=-606/611, 4-10=-701/603, 4-5=-527/510
BOT CHORD 1-11=-477/210, 7-11=-477/210, 7-12=-477/109, 6-12=-477/109
WEBS 2-6=-594/695, 2-7=-828/874, 4-6=-301/410

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCDL=5.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior (1) 3-0-0 to 4-4-0, Exterior(2R) 4-4-0 to 9-6-4, Exterior(2E) 9-6-4 to 12-6-4 zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
 - Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
 - Plates checked for a plus or minus 0 degree rotation about its center.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 281 lb uplift at joint 5 and 557 lb uplift at joint 7.
 - This truss has been designed for a moving concentrated load of 200.0lb dead located at all mid panels and at all panel points along the Bottom Chord, nonconcurrent with any other live loads.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	TRI-LEVEL BUILDER INC.
25C17	V2	Valley	1	1	A0126242
					Job Reference (optional)



Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	30.0	Plate Grip DOL	0.90	TC	0.00	Vert(LL)	n/a	-	n/a	999		
TCDL	20.0	Plate Metal DOL	0.90	BC	0.01	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0	Lumber DOL	0.90	WB	0.00	Horiz(TL)	n/a	-	n/a	n/a		
BCDL	10.0	Rep Stress Incr	YES	Matrix-P								
		Code	FBC2020/TPI2014								Weight: 5 lb	FT = 0%

LUMBER
TOP CHORD
BOT CHORD 2x6 SP No.2

BRACING
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=13/2-03-08, (min. 1-08), 2=13/2-03-08, (min. 1-08)
Max Grav 1=25 (LC 3), 2=25 (LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES**
- 1) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
 - 3) Gable requires continuous bottom chord bearing.
 - 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 - 5) Gable studs spaced at 6-0-0 oc.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) This truss has been designed for a moving concentrated load of 200.0lb dead located at all mid panels and at all panel points along the Bottom Chord, nonconcurrent with any other live loads.

LOAD CASE(S) Standard

Job 25C17	Truss V4	Truss Type Valley	Qty 1	Ply 1	TRI-LEVEL BUILDER INC. Job Reference (optional)	A0126243
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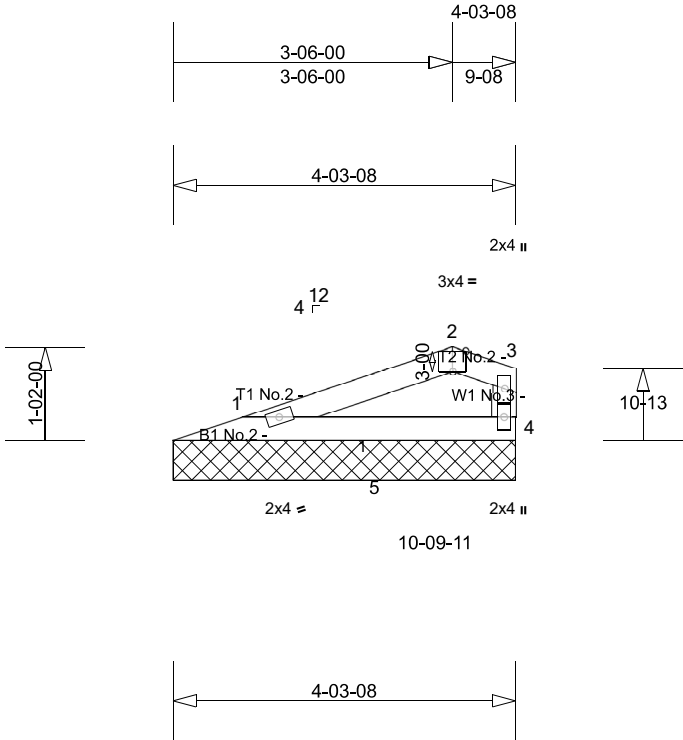


Plate Offsets (X, Y): [2:2-00,Edge]												
Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	30.0	Plate Grip DOL	1.00	TC	0.35	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	20.0	Lumber DOL	1.33	BC	0.34	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	4	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-R							Weight: 12 lb	FT = 0%

LUMBER			BRACING		
TOP CHORD	2x4 SP No.2		TOP CHORD	Structural wood sheathing directly applied or 4-3-8 oc purlins, except end verticals.	
BOT CHORD	2x4 SP No.2		BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.	
WEBS	2x4 SP No.3				

REACTIONS (lb/size) 1=195/4-03-08, (min. 1-08), 4=195/4-03-08, (min. 1-08)
 Max Horiz 1=63 (LC 10)
 Max Uplift 1=-97 (LC 6), 4=-97 (LC 6)
 Max Grav 1=297 (LC 24), 4=297 (LC 25)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCDL=5.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
 - Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
 - Plates checked for a plus or minus 0 degree rotation about its center.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 97 lb uplift at joint 1 and 97 lb uplift at joint 4.
 - This truss has been designed for a moving concentrated load of 200.0lb dead located at all mid panels and at all panel points along the Bottom Chord, nonconcurrent with any other live loads.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	TRI-LEVEL BUILDER INC.	A0126244
25C17	V4A	Valley	1	1	Job Reference (optional)	

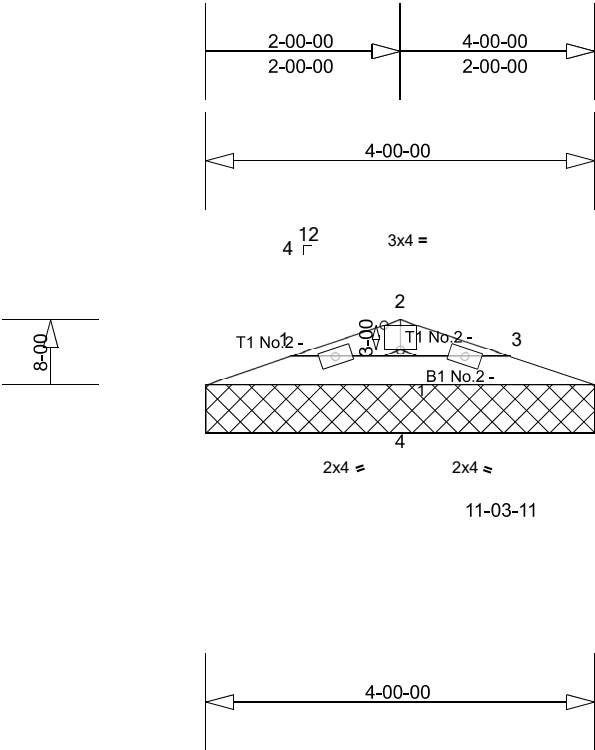


Plate Offsets (X, Y): [2:2-00,Edge]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	30.0	Plate Grip DOL	1.00	TC	0.09	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	20.0	Lumber DOL	1.33	BC	0.26	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	3	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-P							Weight: 9 lb	FT = 0%

LUMBER		BRACING	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 4-0-0 oc purlins.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

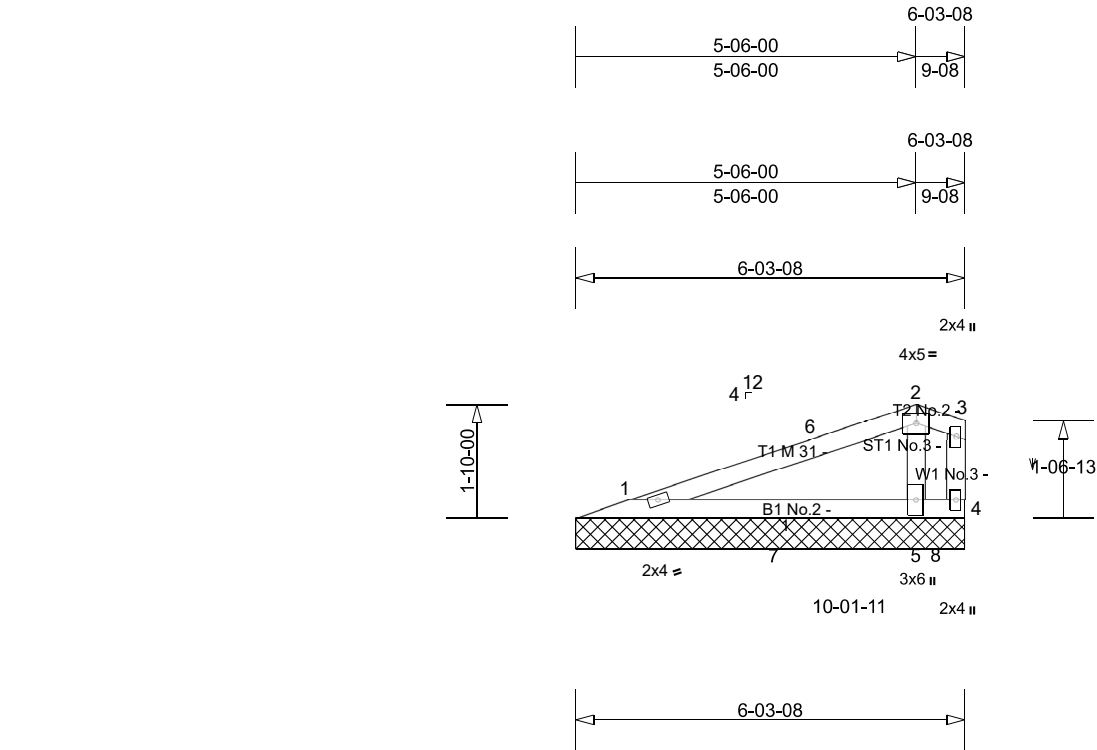
REACTIONS (lb/size) 1=132/4-00-00, (min. 1-08), 3=132/4-00-00, (min. 1-08)
 Max Horiz 1=13 (LC 14)
 Max Uplift 1=-63 (LC 6), 3=-63 (LC 7)
 Max Grav 1=266 (LC 24), 3=266 (LC 25)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCDL=5.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
 - Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
 - Plates checked for a plus or minus 0 degree rotation about its center.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 63 lb uplift at joint 1 and 63 lb uplift at joint 3.
 - This truss has been designed for a moving concentrated load of 200.0lb dead located at all mid panels and at all panel points along the Bottom Chord, nonconcurrent with any other live loads.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	TRI-LEVEL BUILDER INC.
25C17	V6	Valley	1	1	A0126245
					Job Reference (optional)



Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	30.0	Plate Grip DOL	1.00	TC	0.39	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	20.0	Lumber DOL	1.33	BC	0.42	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr	YES	WB	0.17	Horiz(TL)	n/a	-	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-P							Weight: 21 lb	FT = 0%

LUMBER			BRACING		
TOP CHORD	2x4 SP M 31	*Except* T2:2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.	
BOT CHORD	2x4 SP No.2		BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.	
WEBS	2x4 SP No.3				
OTHERS	2x4 SP No.3				

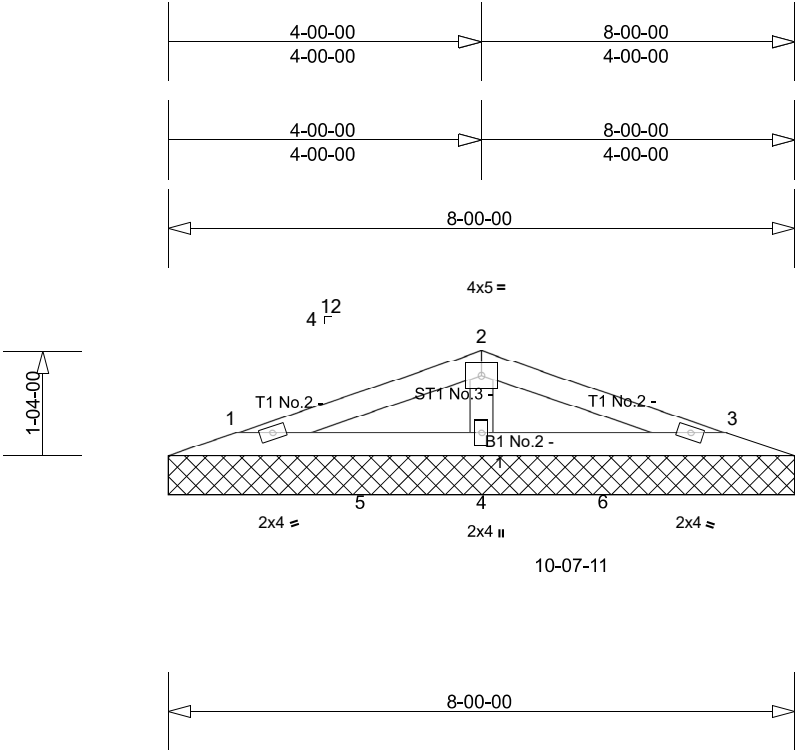
REACTIONS (lb/size) 1=266/6-03-08, (min. 1-08), 4=-33/6-03-08, (min. 1-08), 5=397/6-03-08, (min. 1-08)
Max Horiz 1=120 (LC 10)
Max Uplift 1=-137 (LC 6), 4=-287 (LC 23), 5=-136 (LC 6)
Max Grav 1=328 (LC 25), 4=147 (LC 27), 5=607 (LC 23)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-5=-262/466

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCDL=5.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-10-13 to 3-10-13, Exterior(2R) 3-10-13 to 5-6-0, Exterior(2E) 5-6-0 to 6-1-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
 - Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
 - Plates checked for a plus or minus 0 degree rotation about its center.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 137 lb uplift at joint 1, 287 lb uplift at joint 4 and 136 lb uplift at joint 5.
 - This truss has been designed for a moving concentrated load of 200.0lb dead located at all mid panels and at all panel points along the Bottom Chord, nonconcurrent with any other live loads.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	TRI-LEVEL BUILDER INC.	A0126246
25C17	V8	Valley	1	1	Job Reference (optional)	



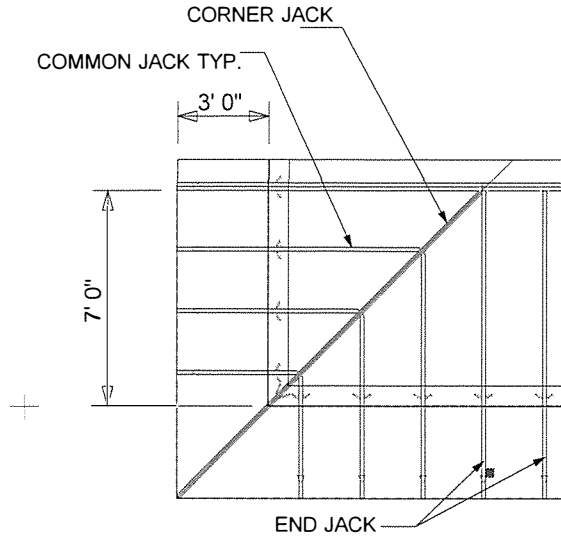
Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	30.0	Plate Grip DOL	1.00	TC	0.39	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	20.0	Lumber DOL	1.33	BC	0.30	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr	YES	WB	0.14	Horiz(TL)	n/a	-	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-P							Weight: 23 lb	FT = 0%

LUMBER			BRACING		
TOP CHORD	2x4 SP No.2		TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.	
BOT CHORD	2x4 SP No.2		BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.	
OTHERS	2x4 SP No.3				
REACTIONS (lb/size)					
	1=184/8-00-00, (min. 1-08), 3=184/8-00-00, (min. 1-08), 4=377/8-00-00, (min. 1-08)				
	Max Horiz 1=-38 (LC 11)				
	Max Uplift 1=-109 (LC 6), 3=-113 (LC 7), 4=-138 (LC 6)				
	Max Grav 1=287 (LC 25), 3=287 (LC 27), 4=397 (LC 26)				
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.					
WEBS 2-4=-299/379					

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCDL=5.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
 - Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
 - Plates checked for a plus or minus 0 degree rotation about its center.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 109 lb uplift at joint 1, 113 lb uplift at joint 3 and 138 lb uplift at joint 4.
 - This truss has been designed for a moving concentrated load of 200.0lb dead located at all mid panels and at all panel points along the Bottom Chord, nonconcurrent with any other live loads.

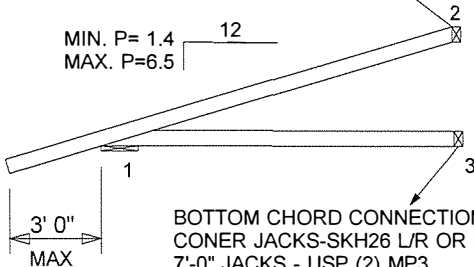
LOAD CASE(S) Standard

7'-0" CORNET SET



PARTIAL ROOF LAYOUT

TOP CHORD CONNECTION
 CONER JACKS - USP (2) MP3
 7'-0" JACKS - USP (2) MP3
 5'-0" JACKS - USP (2) MP3
 3'-0" JACKS - (2) 8d NAILS
 1'-0" JACKS - (2) 8d NAILS



BOTTOM CHORD CONNECTION
 CONER JACKS-SKH26 L/R OR HJC26
 7'-0" JACKS - USP (2) MP3
 5'-0" JACKS - USP (2) MP3
 3'-0" JACKS - (2) 8d NAILS
 1'-0" JACKS - (2) 8d NAILS

Nail design loads:
 (ACCORDING TO NATIONAL DESIGN SPEC.)
 FOR GROUP II SPECIES(S.P.)
 8d COMMON NAILS = 78 lbs (SHEAR)
 10d COMMON NAILS = 94 lbs (SHEAR)

USP HANGERS DESIGN LOADS
 MP3 HURRICANE CLIP 455 lbs (UPLIFT 160%) 445lbs (REACTION 125%) W/ (6)
 10d x 1-1/2" FL17244.19
 SKH26 L/R = 1135 lbs UPLIFT/840 lbs (UPLIFT 160%) REACTION W/ (12)
 NAILS FL17243.10
 HJC26 = 2345 lbs UPLIFT (UPLIFT 160%) 2750 lbs REACTION W/ (28) NAILS
 FL17236.10

MAXIMUM ALLOWABLE LOADS:

		JACK SIZE			
T.C.	CORNER JACK	7'-0"	5'-0"	3'-0"	1'-0"
UPLIFT (160%)	740 lbs	740 lbs	NAILS	NAILS	NAILS
ROOF (125 %)	480 lbs	480 lbs	NAILS	NAILS	NAILS
B.C.	CORNER JACK	7'-0"	5'-0"	3'-0"	1'-0"
UPLIFT (160%)	740 lbs	740 lbs	NAILS	NAILS	NAILS
ROOF (125 %)	480 lbs	480 lbs	NAILS	NAILS	NAILS

COMMON JACK

NOTE:
 THIS DESIGN HAS BEEN CHECKED FOR 180 MPH
 WINF LOAD. WALL HGT. 30' MAX PROVIDE
 FOR UPLIFT AT BEARING WALL CONNECTIONS
 925 lbs LUMBER STRESS INCREASE 1.33

PLATE STRESS INCREASE:

MINIMUM GRADE OF LUMBER				LOADING (PDF)		FLORIDA BUILDING CODE 2023
7'-0"	2x4	No. 2	SP	TOP	LL 30.0	
3'-0"	2x4	No. 2	SP	BOTTOM	10.0	
WEB	2x4	No. 3	SP	SPACING 24" O.C.		

TRUSS CONNECTORS



HHC – Designed to support hip/hip truss/rafter. Contact MiTek when using in multi-ply applications

HJHC – Allows for hip/hip support and hip/jack/hip installations

HJC / HTHJ – Used to simultaneously hang a combination of hips and jacks off girder trusses. These hangers fit both left-hand and right-hand applications. An open back design allows for retrofit installations

Materials: HHC/HJC/HJHC – 12 gauge, HTHJ –18 gauge

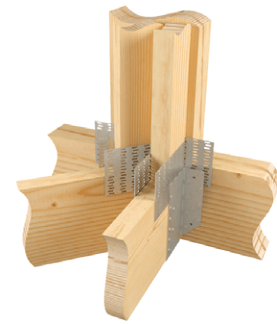
Finish: G90 galvanizing

Options: See HJC Specialty Options Chart below

Codes: See chart for code references

Installation:

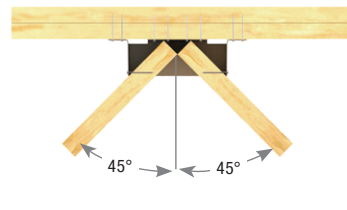
- Use all specified fasteners. See Product Notes, page 18.



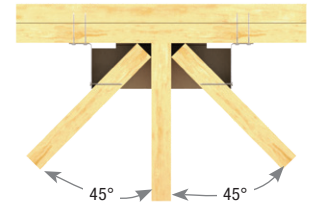
Typical HJC/HTHJ installation



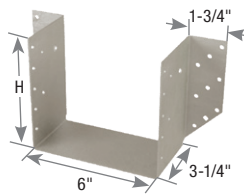
Typical HJC/HTHJ installation top view



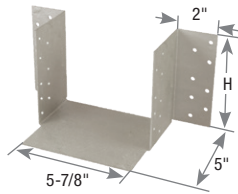
Typical HHC installation top view



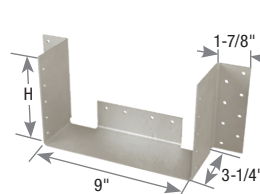
Typical HJHC installation top view



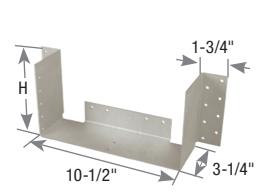
HJC



HTHJ



HHC



HJHC

Description	MiTek USP Stock No.	Ref. No.	Steel Gauge	H (in)	Fastener Schedule ³					DF/SP Allowable Loads (Lbs.) ²				S-P-F Allowable Loads (Lbs.) ²				Code Ref.
					Supporting Member		Supported Member			Allowable Loads (Lbs.) ²				Allowable Loads (Lbs.) ²				
							per Hip	per Jack		Floor	Roof	Uplift ¹	Floor	Roof	Uplift ¹			
Qty	Type	Qty	Qty	Type	100%	115%	125%	160%	100%	115%	125%	160%						
2 x 6 right / left	HJC26	LTHJA26, THJA26, THJU26	12	5-3/8	16	16d	5	7	10d	2750	3055	3265	2345	2420	2685	2750	1905	IBC, FL, LA
2 x 8 right / left	HJC28	--	12	7-1/8	20	16d	6	8	10d	3385	3385	3385	2345	2760	2760	2760	1910	
2 x 6 terminal	HHC26	LTHJA26, THJA26	12	5-7/16	20	16d	5	--	10d	3100	3505	3505	2130	2725	2800	2800	1870	--
2 x 8 terminal	HHC28	--	12	7-3/16	24	16d	6	--	10d	3505	3505	3505	2410	2805	2805	2805	1930	
2 x 6 terminal	HJHC26	--	12	5-7/16	20	16d	5	2	10d	3100	3505	3505	2410	2725	2815	2815	1935	
2 x 8 terminal	HJHC28	--	12	7-3/16	24	16d	6	2	10d	3505	3505	3505	2410	2820	2820	2820	1940	
2 x 6 terminal	HTHJ26-18	--	18	5	16	16d	7	5	16d	2295	2605	2695	1790	1985	2110	2110	1225	

1) Uplift loads have been increased 60% for wind or seismic loads; no further increase shall be permitted.

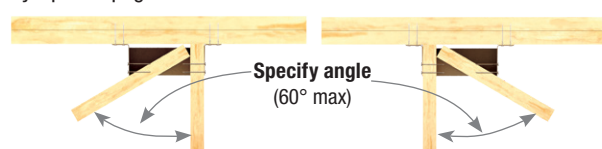
2) Loading published for total load of hip / jack connection.

3) **NAILS:** 10d nails are 0.148" dia. x 3" long, 16d nails are 0.162" dia. x 3-1/2" long.

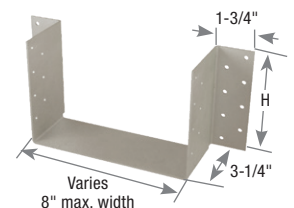
New products or updated product information are designated in **blue font**.

HJC Specialty Options Chart – Refer to Specialty Options pages 320-321 for additional details.

Option	Hip Truss Skew
Range	30° to 60°
Allowable Loads	100% of table load
Ordering	Add SK, angle of hip required, to product number. Ex. HJC26_SK55



Typical HJC (skewed) installation with alternate skew angle top view



HJC (skewed)

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Business & Professional Regulation



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Product Approval

USER: Public User

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► OFFICE OF THE
SECRETARY

FL #

FL17236-R9

Application Type

Revision

Code Version

2023

Application Status

Approved

*Approved by DBPR. Approvals by DBPR shall be reviewed and ratified by the POC and/or the Commission if necessary.

Comments

Archived

☐

Product Manufacturer

MiTek Inc.

Address/Phone/Email

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Chesterfield, MO 63017
(800) 328-5934
ewalden@mii.com

Authorized Signature

Erin Walden
ewalden@mii.com

Technical Representative

Erin M. Walden

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Quality Assurance Representative

Erin M. Walden

Address/Phone/Email	16023 Swingley Ridge Road Chesterfield, MO 63017 (952) 898-8635 ewalden@mii.com	
Category	Structural Components	
Subcategory	Wood Connectors	
Compliance Method	Evaluation Report from a Product Evaluation Entity	
Evaluation Entity	ICC Evaluation Service, LLC	
Quality Assurance Entity	ICC-ES Evaluation Service, LLC	
Quality Assurance Contract Expiration Date	10/31/2024	
Validated By	ICC Evaluation Service, LLC.	
Certificate of Independence	FL17236_R9_COI_ICC-ES Certification of Independence for Evaluation.pdf	
Referenced Standard and Year (of Standard)	<u>Standard</u>	<u>Year</u>
	ANSI/AWC NDS	2018
	ASTM D7147	2011
Equivalence of Product Standards Certified By		
Sections from the Code		
Product Approval Method	Method 1 Option C	
Date Submitted	12/13/2023	
Date Validated	12/14/2023	
Date Pending FBC Approval		
Date Approved	12/16/2023	

Summary of Products

FL #	Model, Number or Name	Description
17236.1	BN264, BN284	BN Breakfast Nook Hanger
Limits of Use Approved for use in HVHZ: No Approved for use outside HVHZ: Yes Impact Resistant: N/A Design Pressure: N/A Other:		Installation Instructions FL17236 R9 II ESR-3448 exp 1024.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL17236 R9 AE ESR-3448 exp 1024.pdf
17236.2	DSC4L/R, LDSC4L/R	DSC/LDSC Drag Strut Connector
Limits of Use Approved for use in HVHZ: Yes Approved for use outside HVHZ: Yes Impact Resistant: N/A Design Pressure: N/A Other:		Installation Instructions FL17236 R9 II ESR-3448 exp 1024.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL17236 R9 AE ESR-3448 exp 1024.pdf
17236.3	FTC1, FTC1F, FTC2, FTC2F	FTC Floor Truss Clip
Limits of Use Approved for use in HVHZ: Yes Approved for use outside HVHZ: Yes Impact Resistant: N/A Design Pressure: N/A Other:		Installation Instructions FL17236 R9 II ESR-3448 exp 1024.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL17236 R9 AE ESR-3448 exp 1024.pdf
17236.4	FTC32	FTC Floor Truss Clip
Limits of Use Approved for use in HVHZ: No Approved for use outside HVHZ: Yes Impact Resistant: N/A Design Pressure: N/A Other:		Installation Instructions FL17236 R9 II ESR-3448 exp 1024.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL17236 R9 AE ESR-3448 exp 1024.pdf
17236.5	GT2T2B, GT2T2BH, GT2T3B, GT2T4B, GT2T6B, GT2T6BH, GT2T8B, GT3T3B, GT3T3BH, GT3T4B, GT3T4BH, GT3T6B, GT3T6BH, GT3T8B, GT3T8BH,	GT Girder Truss Hanger

	GT4T4B, GT4T4BH, GT4T6B, GT4T6BH, GT4T8B, GT5T8BH	
Limits of Use Approved for use in HVHZ: Yes Approved for use outside HVHZ: Yes Impact Resistant: N/A Design Pressure: N/A Other:		Installation Instructions FL17236 R9 II ESR-3448 exp 1024.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL17236 R9 AE ESR-3448 exp 1024.pdf
17236.6	HCPRS	HCPRS Hurricane/Seismic Anchor
Limits of Use Approved for use in HVHZ: No Approved for use outside HVHZ: Yes Impact Resistant: N/A Design Pressure: N/A Other:		Installation Instructions FL17236 R9 II ESR-3448 exp 1024.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL17236 R9 AE ESR-3448 exp 1024.pdf
17236.7	HGA10	HGA Hurricane Gusset Angle
Limits of Use Approved for use in HVHZ: Yes Approved for use outside HVHZ: Yes Impact Resistant: N/A Design Pressure: N/A Other:		Installation Instructions FL17236 R9 II ESR-3448 exp 1024.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL17236 R9 AE ESR-3448 exp 1024.pdf
17236.8	HHCP2	HHCP Hurricane/Seismic Anchor
Limits of Use Approved for use in HVHZ: No Approved for use outside HVHZ: Yes Impact Resistant: N/A Design Pressure: N/A Other:		Installation Instructions FL17236 R9 II ESR-3448 exp 1024.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL17236 R9 AE ESR-3448 exp 1024.pdf
17236.9	HHCP4	HHCP Hurricane/Seismic Anchor
Limits of Use Approved for use in HVHZ: Yes Approved for use outside HVHZ: Yes Impact Resistant: N/A Design Pressure: N/A Other:		Installation Instructions FL17236 R9 II ESR-3448 exp 1024.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL17236 R9 AE ESR-3448 exp 1024.pdf
17236.10	HJC26, HJC28	HJC Hip/Jack Connector

Limits of Use Approved for use in HVHZ: Yes Approved for use outside HVHZ: Yes Impact Resistant: N/A Design Pressure: N/A Other:		Installation Instructions FL17236 R9 II ESR-3448 exp 1024.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL17236 R9 AE ESR-3448 exp 1024.pdf
17236.11	RT3A, RT4, RT5, RT6, RT7, RT7A, RT7AT, RT10, RT15	RT Hurricane Tie
Limits of Use Approved for use in HVHZ: No Approved for use outside HVHZ: Yes Impact Resistant: N/A Design Pressure: N/A Other:		Installation Instructions FL17236 R9 II ESR-3448 exp 1024.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL17236 R9 AE ESR-3448 exp 1024.pdf
17236.12	RT8A, RT16-2, RT16A, RT16AR, RT20	RT Hurricane Tie
Limits of Use Approved for use in HVHZ: Yes Approved for use outside HVHZ: Yes Impact Resistant: N/A Design Pressure: N/A Other:		Installation Instructions FL17236 R9 II ESR-3448 exp 1024.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL17236 R9 AE ESR-3448 exp 1024.pdf
17236.13	SBP4, SBP6	SBP Supplementary Bearing Plate
Limits of Use Approved for use in HVHZ: Yes Approved for use outside HVHZ: Yes Impact Resistant: N/A Design Pressure: N/A Other:		Installation Instructions FL17236 R9 II ESR-3448 exp 1024.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL17236 R9 AE ESR-3448 exp 1024.pdf
17236.14	STC24, STC26, STC28	STC Scissor Truss Clip
Limits of Use Approved for use in HVHZ: No Approved for use outside HVHZ: Yes Impact Resistant: N/A Design Pressure: N/A Other:		Installation Instructions FL17236 R9 II ESR-3448 exp 1024.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL17236 R9 AE ESR-3448 exp 1024.pdf
17236.15	TSP	TSP Stud Plate Tie

Limits of Use**Approved for use in HVHZ:** Yes**Approved for use outside HVHZ:** Yes**Impact Resistant:** N/A**Design Pressure:** N/A**Other:****Installation Instructions**[FL17236_R9_II_ESR-3448_exp_1024.pdf](#)

Verified By: ICC Evaluation Service, LLC

Created by Independent Third Party:

Evaluation Reports[FL17236_R9_AE_ESR-3448_exp_1024.pdf](#)[Back](#)[Next](#)

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Product Approval Accepts:

GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary lateral restraint and diagonal bracing. Follow the recommendations for handling, installing and temporary restraining of trusses. Refer to **BCS1 – Guide to Good Practice for Handling, Installing, Restraining, Bracing of Metal Plate Connected Wood Trusses***** for more detailed information.

Truss Design Drawings may specify locations of permanent lateral restraint or reinforcement for individual truss members. Refer to the **BCS1-B3***** for more information. All other permanent bracing design is the responsibility of the building designer.

NOTAS GENERALES

Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de restricción lateral y arriostramiento diagonal temporales. Use las recomendaciones de manejo, instalación, restricción y arriostramiento temporales de los trusses. Vea el folleto **BCS1 – Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arriostramiento de los Trusses de Placa de Metal***** para información más detallada.

Los dibujos de diseño de los trusses pueden especificar las localizaciones de restricción lateral permanente o refuerzo en los miembros individuales del truss. Vea la **BCS1-B3***** para más información. El resto de los diseños de arriostramientos permanentes son la responsabilidad del diseñador del edificio.

WARNING The consequences of improper handling, erecting, installing, restraining and bracing can result in a collapse of the structure, or worse, serious personal injury or death.

ADVERTENCIA! El resultado de un manejo, levantamiento, instalación, restricción y arriostramiento incorrecto puede ser la caída de la estructura o aún peor, heridos o muertos.

CAUTION Exercise care when removing banding and handling trusses to avoid damaging trusses and prevent injury. Wear personal protective equipment for the eyes, feet, hands and head when working with trusses.

CAUTELA! Utilice cautela al quitar las ataduras o los pedazos de metal de sujetar para evitar daño a los trusses y prevenir la herida personal. Lleve el equipo protector personal para ojos, pies, manos y cabeza cuando trabaja con trusses.

HANDLING – MANEJO

NOTICE Avoid lateral bending.

Evite la flexión lateral.

NOTICE The contractor is responsible for properly receiving, unloading and storing the trusses at the jobsite. Unload trusses to smooth surface to prevent damage.

El contratista tiene la responsabilidad de recibir, descargar y almacenar adecuadamente los trusses en la obra. Descargue los trusses en la tierra lisa para prevenir el daño.



Trusses may be unloaded directly on the ground at the time of delivery or stored temporarily in contact with the ground after delivery. If trusses are to be stored for more than one week, place blocking of sufficient height beneath the stack of trusses at 8' (2.4 m) to 10' (3 m) on-center (o.c.).

Los trusses pueden ser descargados directamente en el suelo en aquel momento de entrega o almacenados temporalmente en contacto con el suelo después de entrega. Si los trusses están bloqueados para más de una semana, ponga bloqueos de altura suficiente debajo de la pila de los trusses a 8' hasta 10' pies en centro (o.c.). For trusses stored for more than one week, cover bundles to protect from the environment.

Para trusses guardados por más de una semana, cubra los paquetes para protegerlos del ambiente. Refer to **BCS1***** for more detailed information pertaining to handling and jobsite storage of trusses.

Vea el folleto **BCS1***** para información más detallada sobre el manejo y almacenamiento de los trusses en áreas de trabajo.

HOISTING AND PLACEMENT OF TRUSS BUNDLES

RECOMENDACIONES PARA LEVANTAR PAQUETES DE TRUSSES

- DON'T** overload the crane.
- NO** sobrecargue la grúa.
- NEVER** use bending to lift a bundle.
- NUNCA** use las ataduras para levantar un paquete.

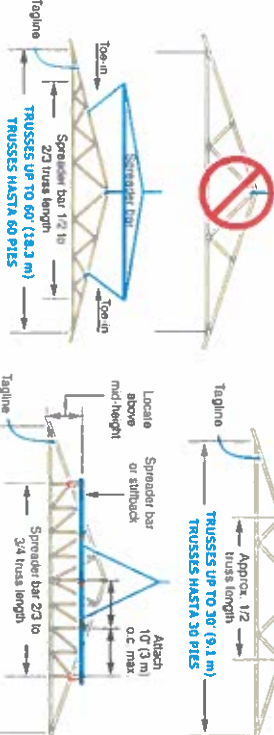
A single lift point may be used for bundles of top chord pitch trusses up to 45' (13.7 m) and parallel chord trusses up to 30' (9.1 m). Use at least two lift points for bundles of top chord pitch trusses up to 60' (18.3 m) and parallel chord trusses up to 45' (13.7 m). Use at least three lift points for bundles of top chord pitch trusses >60' (18.3m) and parallel chord trusses >45' (13.7 m).

Puede usar un solo lugar de levantar para paquetes de trusses de la cuerda superior hasta 45' y trusses de cuerdas paralelas de 30' o menos. Use por lo menos dos puntos de levantar con grupos de trusses de cuerda superior inclinada hasta 60' y trusses de cuerdas paralelas hasta 45'. Use por lo menos dos puntos de levantar con grupos de trusses de cuerda superior inclinada más de 60' y trusses de cuerdas paralelas más de 45'.

MECHANICAL HOISTING RECOMMENDATIONS FOR SINGLE TRUSSES

NOTICE Using a single pick point at the peak can damage the truss.

El uso de un solo lugar en el pico para levantar puede hacer daño al truss.



Hold each truss in position with the erection equipment until top chord temporary lateral restraint is installed and the truss is fastened to the bearing points.

Sostenga cada truss en posición con equipo de grúa hasta que la restricción lateral temporal de la cuerda superior esté instalado y el truss está asegurado en los soportes.

INSTALLATION OF SINGLE TRUSSES BY HAND

RECOMENDACIONES DE LEVANTAMIENTO DE TRUSSES INDIVIDUALES POR LA MANO



TEMPORARY RESTRAINT & BRACING

RESTRICCIÓN Y ARRIOSTRE TEMPORAL

NOTICE Refer to **BCS1-B2***** for more information.

Vea el resumen **BCS1-B2***** para más información.

Locate ground braces for first truss directly in line with all rows of top chord temporary lateral restraint (see table in the next column).

Coloque los arriostradores de tierra para el primer truss directamente en línea con cada una de las filas de restricción lateral temporal de la cuerda superior (vea la tabla en la próxima columna).

DON'T walk on unbraced trusses.

NO camine en trusses sueltos.

STEPS TO SETTING TRUSSES

LAS MEDIDAS DE LA INSTALACIÓN DE LOS TRUSSES

- 1) Install ground bracing.
- 2) Set first truss and attach securely to ground bracing.
- 3) Set next 4 trusses with short member temporary lateral restraint (see below).
- 4) Install top chord diagonal bracing (see below).
- 5) Install web member plane diagonal bracing to stabilize the first five trusses (see below).
- 6) Install bottom chord temporary lateral restraint and diagonal bracing (see below).
- 7) Repeat process with groups of four trusses until all trusses are set.

1) Instale los arriostradores de tierra. 2) Instale el primero truss y ate seguramente al arriostrador de tierra. 3) Instale los próximos 4 trusses con restricción lateral temporal de miembro corto (vea abajo). 4) Instale el arriostrador diagonal de la cuerda superior (vea abajo). 5) Instale arriostrador diagonal para los planos de los miembros secundarios para estabilizar los primeros cinco trusses (vea abajo). 6) Instale la restricción lateral temporal y arriostrador diagonal para la cuerda inferior (vea abajo). 7) Repita este procedimiento en grupos de cuatro trusses hasta que todos los trusses estén instalados.

NOTICE Refer to **BCS1-B2***** for more information.

Vea el resumen **BCS1-B2***** para más información.

RESTRAINT/BRACING FOR ALL PLANES OF TRUSSES

RESTRICCIÓN/ARRIOSTRE PARA TODOS PLANOS DE TRUSSES

This restraint and bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses (PCTs). See top of next column for temporary restraint and bracing of PCTs.

Este método de restricción y arriostramiento es para todos trusses excepto trusses de cuerdas paralelas (PCTs) 3x2 y 4x2. Vea la parte superior de la columna para la restricción y arriostramiento temporal de PCTs.

Truss Span	Top Chord Temporary Lateral Restraint (TCLTR) Spacing
Longitud de Tramo	Espaciamiento del Arriostramiento Temporal de la Cuerda Superior
Up to 30' (9.1 m)	10' (3 m) o.c. max.
30' (9.1 m) – 45' (13.7 m)	8' (2.4 m) o.c. max.
45' (13.7 m) – 60' (18.3 m)	6' (1.8 m) o.c. max.
60' (18.3 m) – 80' (24.4 m)	4' (1.2 m) o.c. max.

*Consult a Registered Design Professional for trusses longer than 60' (18.3 m). * Consulte a un Profesional Registrado de Diseño para trusses más de 60' pies.

See **BCS1-B2***** for TCLTR options.

Vea el **BCS1-B2***** para las opciones de TCLTR.

NOTICE Refer to **BCS1-B3***** for Gable End Frame restraint/bracing/ reinforcement information.

Para información sobre restricción/ arriostramiento para Armazones Gablestiles vea el resumen **BCS1-B3*****.

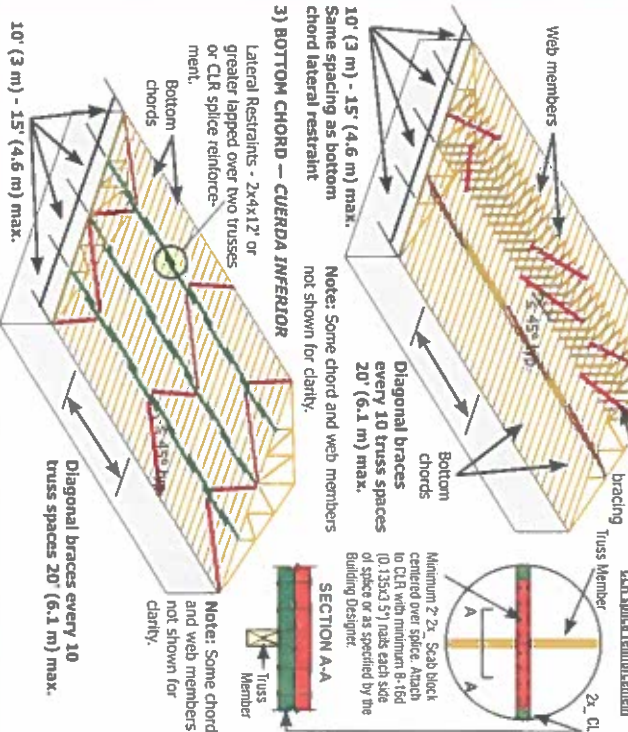
Note: Ground bracing not shown for clarity.

Repetir diagonal braces for each set of 4 trusses.

Repla los arriostradores diagonales para cada grupo de 4 trusses.

NOTICE LATERAL RESTRAINT & DIAGONAL BRACING ARE VERY IMPORTANT

¡LA RESTRICCIÓN LATERAL Y EL ARRIOSTRE DIAGONAL SON MUY IMPORTANTES!



RESTRAINT & BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES

RESTRICCIÓN Y ARRIOSTRE PARA TRUSSES DE CUERDAS PARALELAS 3X2 Y 4X2

NOTICE Refer to **BCS1-B7***** for more information.

Vea el resumen **BCS1-B7***** para más información.

Apply diagonal brace to vertical webs at end of cantilever and at bearing locations.

*Top chord temporary lateral restraint spacing shall be 10' (3 m) o.c. max. for 3x2 chords and 15' (4.6 m) o.c. for 4x2 chords.

INSTALLING – INSTALACIÓN

Out-of-Plane Tolerances for Out-of-Plane Tolerancias para Fuera-de-Plano.		
		
D/50	D (ft.)	Max. Bow
1/4"	1'	3/4"
(6 mm)	(0.3 m)	(18 mm)
1/2"	2'	7/8"
(13 mm)	(0.6 m)	(45 mm)
3/4"	3'	16.7"
(19 mm)	(0.9 m)	(51 mm)
1"	4'	18.8"
(25 mm)	(1.2 m)	(51 mm)
1 1/4"	5'	20.8"
(32 mm)	(1.5 m)	(53 mm)
1 1/2"	6'	22.9"
(38 mm)	(1.8 m)	(58 mm)
1 3/8"	7'	25.0"
(35 mm)	(2.1 m)	(64 mm)
1 1/2"	8'	28.0"
(38 mm)	(2.4 m)	(71 mm)