GENERAL NOTES

1) A SPECIAL INSPECTION BY A FLORIDA CERTIFIED ENGINEER OR SPECIAL INSPECTOR WILL BE REQUIRED FOR REINFORCED MASONRY CONSTRUCTION, SOIL COMPACTION, ROOF TRUSSES OVER 35' LONG OR 6'-0" HIGH + STRUCTURAL STEEL CONNECTIONS + PILE INSTALLATION.

2) THE USE OF A SCALE TO OBTAIN DIMENSIONS NOT SHOWN ON THE PLANS IS STRICTLY FORBIDDEN. THE ENGINEER WILL NOT BE RESPONSIBLE FOR ERRORS RESULTING FROM SUCH ACTIONS.

3) CONTRACTOR TO LOCATE ALL EXISTING UTILITIES PRIOR TO EXCAVATION.

4) ALL SPECIFIED MATERIALS AND CONNECTORS CAN BE SUBSTITUTED WITH EQUAL OR BETTER, WITH THE APPROVAL OF ENGINEER OF RECORD.

5) PRIOR TO CONSTRUCTION PROVIDE TERMITE PROTECTION TREATMENT TO SOIL IN ACCORDANCE WITH 'NATIONAL PEST CONTROL ASSOCIATION' STANDARDS

POISON SOIL UNDER NEW STRUCTURE FOR WEEDS AND TERMITES AND IN ACCORDANCE WITH LOCAL CODES.

UPON COMPLETION OF THE APPLICATION OF THE TERMITE PROTECTIVE TREATMENT, A CERTIFICATE OF COMPLIANCE SHALL BE ISSUED TO THE BUILDING DEPARTMENT BY THE LICENSED PEST CONTROL COMPANY THAT CONTAINS THE FOLLOWING STATEMENT: "THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES.
TREATMENT IS IN ACCORDANCE WITH RULES AND LAWS ESTABLISHED BY THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES".

6) IF CONFLICTS OCCUR IN OR BETWEEN ARCHITECTURAL AND ENGINEERING DOCUMENTS, BETWEEN DOCUMENTS AND FIELD CONDITIONS OR OTHERWISE, IMMEDIATELY CONTACT THE ENGINEER FOR CLARIFICATION AND DIRECTION BEFORE PROCEEDING. COORDINATE ALL DIMENSIONS BETWEEN ARCHITECTURAL AND STRUCTURAL DRAWINGS PRIOR TO PROCEEDING WITH THE WORK.

7) THE CONTRACTOR IS RESPONSIBLE FOR ALL METHODS, PROCEDURES AND SEQUENCES OF CONSTRUCTION. PROVIDE APPROPRIATE SUPERVISION THROUGHOUT THE PROJECT. CONSTRUCTION SITE SAFETY, INCLUDING ALL ADEQUATE TEMPORARY BRACING AND SHORING, IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO EMPLOY THE NECESSARY PROFESSIONAL SERVICES TO DETERMINE THE NECESSARY METHODS AND SUPPORTS REGARDING FORMING AND CONSTRUCTION LOADS. TEMPORARY BRACING AND SHORING SHALL BE DESIGNED TO RESIST ALL CONSTRUCTION LOADS INCLUDING THE WEIGHTS OF ALL SUPPORTED MATERIALS PLUS A LIVE LOAD OF 50 PSF ON HORIZONTAL SURFACES. MAINTAIN TEMPORARY BRACING AND RETAIN IN PLACE UNTIL PERMANENT

WOOD NOTES

1. ROOF DIAPHRAGM SHALL COMPLY WITH THE DESIGN RECOMMENDATIONS OF "A.P.A. DESIGN / CONSTRUCTION GUIDE - DIAPHRAGMS" AND THE F.B.C.
2. PLYWOOD ROOF DECKING SHALL BE 19/32" MINIMUM THICKNESS, AND SHALL BE CONTINUOUS OVER TWO OR MORE SPANS, WITH FACE GRAIN PERPENDICULAR TO THE SUPPORTS. (SEE TYP. PLYWOOD NAILING DETAIL FOR NAILING INFORMATION).
3. INSPECTIONS SHALL COMPLY WITH THE LOCAL BUILDING CODE REQUIREMENTS FOR INSPECTIONS (BY THE MUNICIPALITY, ARCHITECT OR ENGINEER) OF SPECIFIED COMPONENTS OF THE ROOF STRUCTURE REQUIRING INSPECTIONS.
4. ALL STRUCTURAL LUMBER TO BE SOUTHERN PINE No. 2 OR BETTER, MINIMUM BENDING STRESS:

2x4 Fb= 1000 Psi	E= 1,600,000 Psi	Fv= 175 Psi	
2x6 Fb= 1000 Psi	E= 1,600,000 Psi	Fv= 175 Psi	
2x8 Fb= 925 Psi	E= 1,600,000 Psi	Fv= 175 Psi	
2x10 Fb= 850 Psi	E= 1,600,000 Psi	Fv= 175 Psi	
2x12 Fb= 750 Psi	E= 1,600,000 Psi	Fv= 175 Psi	

MASONRY NOTES

1) PROVIDE CLEANOUT OPENING AT THE BOTTOM OF REINFORCED MASONRY CELLS THAT ARE BEING FILLED TO ENSURE THAT THE CELL IS FILLED. PROVIDE WEEP HOLES AT 1/3 AND 2/3 OF SPAN

2) ALL BLOCK MASONRY WALLS SHALL HAVE 9 GAUGE HORIZONTAL REINFORCEMENT " DUR-O-WALL LADDER TYPE SPACED 16" ON CENTER. MASONRY UNITS SHALL BE 2 CELL HOLLOW UNITS CONFORMING TO A.S.T.M. C-90 WITH COMPRESSIVE BE STRENGTH OF f'm=1,500 P.S.I. ON THE NET CROSS-SECTIONAL AREA AND SHALL BE LAID IN RUNNING BOND.

3) MORTAR SHALL COMPLY WITH A.S.T.M. 270 MORTAR TYPE S=1,800 PSI

4) WHERE ANCHOR BOLTS ARE SET IN MASONRY WALL, FILL TWO BLOCK CELLS WITH GROUT @ ANCHOR BOLT LOCATIONS

5) GROUT USED IN THE WORK SHALL CONFORM TO ASTM C476 WITH A SLUMP MIX OF 8" TO 11", PROVIDE CLEANOUT AND INSPECTION HOLES AT FILLED CELLS AT BOTTOM COURSE. CONTRACTOR TO CONSOLIDATE GROUT LIFTS WITH 3/4" VIBRATOR

6) POUR GROUT IN LIFTS NOT TO EXCEED 4'

7) GROUT USED IN THE WORK SHALL OBTAIN A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI IN 28 DAYS. GROUT SHALL COMPLY WITH A.S.T.M. C476.

DESIGN LOADS

WIND DESIGN PER ASCE 7-22 (ALLOWABLE STRESS DESIGN)
MEAN ROOF HEIGHT _____ +29'-0"
WIND DESIGN SPEED _____ 175 MPH
EXPOSURE CATEGORY _____ C
GCpi_____ +/-0.18
Kd _____ 0.85
STRUCTURAL CATEGORY _____ II

ROOF DESIGN LOADS: PRE FAB WOOD TRUSSES
SUPER IMPOSED DEAD LOAD:_____ 25 PSF
LIVE LOAD:_____ 20 PSF

FLOOR DESIGN LOADS:
DEAD LOAD 8" SLAB:_____ 100 PSF
SUPER IMPOSED DEAD LOAD (FLOORS)_____ 30 PSF

LIVE LOAD_____ 40 PSF
STAIR LIVE LOAD:_____ 40 PSF (RESIDENTIAL)

CONCRETE NOTES

1) CONCRETE SHALL CONFORM WITH ACI-318 IT SHALL REACH A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI @ 28 DAYS.
2) MAXIMUM PERMISSIBLE SLUMP IS 5-6" IN STRUCTURAL CONCRETE WITH THE EXCEPTION BEING SAND CEMENT GROUT
3) AGGREGATES SHALL BE CLEAN AND GRADED MAXIMUM SIZES 3/4"
4) NO ADMIXTURES PERMITTED WITHOUT WRITTEN APPROVAL OF ENGINEER
5) CONCRETE TESTING IS REQUIRED AS FOLLOWS: 1 SET OF 5 CYLINDERS FOR EVERY 50 CU. YDS. OF CONCRETE
6) CLEAR COVER FOR REINFORCING BARS SHALL BE:

GRADE BEAMS/PILE CAPS... 3"	UNFORMED FACES ... 3"	BALCONY SLABS ... 1-1/2"	
	FORMED FACES IN CONTACT W/EARTH... 2"	INT. ELEVATED SLABS ... 3/4"	
BEAMS/COLUMNS ... 1 -1/2"			

7) ALL CONCRETE WORK IN ACCORDANCE WITH "SPECIFICATIONS FOR STRUCTURAL CONCRETE" (ACI-301-16)

REINFORCING STEEL NOTES

1) REINFORCING STEEL fy = 60,000 P.S.I. fs = 24,000 P.S.I. Es = 29,000,000 P.S.I. Em = 1350,000 P.S.I. N = Es/Em = 21.48

2) REINFORCING STEEL SHALL CONFORM TO ASTM A-615 GRADE 60. REINFORCING STEEL SHALL BE DETAILED AND FABRICATED ACCORDING TO THE " MANUAL OF STANDARD PRACTICE FOR ETAILING REINFORCED CONCRETE STRUCTURES".

3) TIE BEAMS + FOOTINGS SHALL BE CONTINUOUS REINFORCEMENT, PROVIDED BY LAPPING SPLICES NOT LESS THAN 36". CONTINUITY SHALL BE PROVIDED AT ALL CORNERS BY BENDING 2 BARS FROM EACH DIRECTION AROUND THE CORNER 36" (SAME SIZE AND QUANTITY.)

4) EXTEND BEAM BOTTOM BARS 8" PAST ALL OPENINGS GREATER THAN 3'-0" ON BOTH SIDES.

5) CONTINUITY AT COLUMNS SHALL BE PROVIDED BY CONTINUING HORIZONTAL REBARS THRU COLUMNS OR BY BENDING HORIZONTAL REINF. INTO COLUMNS A DISTANCE OF 36".

6) HOOK TOP OF VERTICAL BARS IN ALL TERMINATING COLUMNS 2" BELOW TOP OF SLAB AND 3" BELOW TOP OF THE TIE BEAM.

STRUCTURAL STEEL

1. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING GRADES:
   A. ALL ANGLE, BASE PLATES, CONN. PLATES (UON)---ASTM A36 (Fy=36 KSI)
   B. STRUCTURAL TUBE----------ASTM A500, GRADE B (Fy=46 KSI)
   C. STEEL PIPE----------ASTM A53, GRADE B (FY=35 KSI)

2. GROUT FOR COLUMN BASE PLATES SHALL BE NON-METALLIC, NON SHRINK GROUT SUCH AS MASTER FLOW 713, AS MANUFACTURED BY MASTER BUILDERS OR APPROVED EQUAL.

3. ALL WELDING SHALL CONFORM TO THE REQUIREMENTS OF THE STANDARD CODE FOR WELDING IN BUILDING CONSTRUCTION" OF THE AMERICAN WELDING SOCIETY. ALL WELDING SHALL BE PERFORMED BY CERTIFIED WELDERS USING E70-XX ELECTRODES.

SHOP DRAWINGS

PRIOR TO ANY FABRICATION, SHOP DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW. SHOP DRAWINGS ARE REQUIRED, BUT NOT LIMITED TO, FOR THE FOLLOWING:

1. WOOD TRUSSES	4. STEEL STAIRS AND RAILS	7. ALUMINUM FENCE
2. STRUCTURAL STEEL	5. RAILINGS/GUARDRAILS	
3. SHORING	6. DECORATIVE TRUSS BRACKETS	

STRUCTURAL DESIGN CRITERIA

1) THE DESIGN COMPLIES WITH THE REQUIREMENTS OF THE FBC AND OTHER REFERENCED CODES AND SPECIFICATIONS. ALL CODES AND SPECIFICATIONS SHALL BE LATEST EDITION AT TIME OF PERMIT.

A: FLORIDA BUILDING CODE 2023 8TH EDITION
B: ACI. 318-19 FOR REINFORCED CONCRETE
C: TMS 402/602-2016 FOR REINFORCED MASONRY
D: ASCE 7-22 FOR WIND ANALYSIS AND DESIGN
E: NDS 2018 + SDPWS 2021 FOR WOOD DESIGN + ANALYSIS
F: AISC 360-16 CONSTRUCTION MANUAL FOR STEEL DESIGN
G: ADM-2020 FOR ALUMINUM DESIGN

SOIL STATEMENT

AS PER GEOTECHNICAL REPORT BY NV5 DATED AUGUST 26, 2024 (PROJECT NUMBER 185593) THIS FOUNDATION HAS BEEN DESIGNED AS A SHALLOW FOUNDATION WITH AN ALLOWABLE SOIL BEARING CAPACITY OF 3000 PSF. CONTRACTOR REFER TO THE GEOTECHNICAL REPORT FOR SITE PREPARATION AND ALL OTHER PERTINENT DATA.

SHORING

N. FORMWORK, SHORING & RESHORING

1. PROVIDE SHORING AND RESHORING DRAWINGS PREPARED BY OR UNDER THE DIRECT SUPERVISION OF A SPECIALTY ENGINEER AND CONFORMING TO THE REQUIREMENTS OF SPECIFICATION SECTION 03100 "CONCRETE FORMWORK".

2. THE DESIGN OF FORMS AND SHORING FOR HORIZONTAL CONCRETE MEMBERS SHALL BE FOR NO LESS THAN DEAD LOAD PLUS 50 PSF CONSTRUCTION LOAD AND FOR THE CUMULATIVE LOADS OF THE SUPPORTED FLOOR. THE DESIGN SHALL INCLUDE A SAFETY FACTOR OF 3 FOR WOOD MEMBERS AND 2 FOR METAL.

3. NO CONSTRUCTION LOAD SHALL BE APPLIED TO ANY MEMBER UNTIL THE CONCRETE HAS ACHIEVED A MINIMUM OF 70% OF THE SPECIFIED 28-DAY STRENGTH.

4. THE MAXIMUM SUPERIMPOSED CONSTRUCTION LOAD TO BE APPLIED ON FLOORS SUPPORTED BY SHORING OR RESHORING SHALL NOT EXCEED 75% OF THE LIVE LOAD SPECIFIED FOR SLABS AND JOISTS. BEAMS SHALL NOT BE LOADED WITH MORE THAN 60% OF THE LIVE LOAD SPECIFIED FOR BEAMS.

5. REMOVAL OF FORMWORK IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR. REMOVE FORMS IN SUCH A MANNER TO ENSURE JOB SAFETY AND TO PREVENT EXCESSIVE DEFLECTION OR DAMAGE TO THE STRUCTURE. THE FOLLOWING ARE MINIMUM GUIDELINES FOR THE REMOVAL OF HORIZONTAL FORMWORK.

6. DO NOT REMOVE POST SHORING UNTIL CONCRETE HAS REACHED 75% OF THE SPECIFIED 28-DAY STRENGTH, BUT NO LESS THAN 3000 PSI. AS A MINIMUM, SHORING AND RESHORING SHALL REMAIN IN PLACE AS FOLLOWS: (TIME MEASURED FROM COMPLETION OF CONCRETE PLACEMENT.):

SLABS, SPANS GREATER	SHORING	RESHORING
THAN 10'-0"	7 DAYS	7 DAYS
JOISTS	7 DAYS	7 DAYS
BEAMS	7 DAYS	14 DAYS
CANTILEVERS:		
SLABS	7 DAYS	14 DAYS
JOISTS	7 DAYS	14 DAYS
BEAMS	7 DAYS	14 DAYS

9. THE SHORING AND RESHORING IS TO BE INSPECTED BY THE SPECIALTY ENGINEER OR HIS AUTHORIZED REPRESENTATIVE, PRIOR TO EACH CONCRETE PLACEMENT HE SHALL SUBMIT A WRITTEN INSPECTION REPORT TO THE SPECIAL INSPECTOR AND TO THE CONTRACTOR STATING THAT THE WORK IS IN GENERAL COMPLIANCE WITH THE SHORING AND RESHORING DRAWINGS. THE FIELD REPORTS SHALL BE SIGNED BY THE INDIVIDUAL CONDUCTING THE INSPECTION. COPIES OF THE FIELD REPORTS SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER, THE SPECIAL INSPECTOR AND THE BUILDING OFFICIAL UNDER A COVER LETTER SIGNED, SEALED AND DATED BY THE SPECIALTY ENGINEER ON A SEMI-MONTHLY BASIS.

ABBREVIATIONS

AB	-	ANCHOR BOLT	L	-	ANGLE
ALT	-	ALTERNATE	LG	-	LONG
APPROX	-	APPROXIMATELY	LP	-	LOW POINT
ARCH	-	ARCHITECTURAL	LLV	-	LONG LEG VERTICAL
B	-	BOTTOM	LLH	-	LONG LEG HORIZONTAL
BLDG	-	BUILDING	MFR	-	MANUFACTURER
BM	-	BEAM	MAS	-	MASONRY
BOTT	-	BOTTOM	MO	-	MASONRY OPENING
BRG	-	BEARING	MATL	-	MATERIAL
BRDG	-	BRIDGING	MAX	-	MAXIMUM
CB	-	CONCRETE BEAM	MB	-	MASONRY BEAM
C/C	-	CENTER TO CENTER	MECH	-	MECHANICAL
CH	-	CHANNEL	MEP	-	MECHANICAL/ELECTRICAL/PLUMBING
CIP	-	CAST IN PLACE	MF	-	MONOLITHIC POUR
CJ	-	CONTROL JOINT	MTL	-	METAL
CL	-	CENTERLINE	MIN	-	MINIMUM
CLR	-	CLEAR	MISC	-	MISCELLANEOUS
CMU	-	CONCRETE MASONRY UNIT	NS	-	NEAR SIDE
COL	-	COLUMN	NIC	-	NOT IN CONTRACT
CONC	-	CONCRETE	NTS	-	NOT TO SCALE
CONFIG	-	CONFIGURATION	O/C	-	ON CENTER
CONT	-	CONTINUOUS	OPNG	-	OPENING
CONTR	-	CONTRACTOR	PART	-	PARTITION
CORR	-	CORRUGATED	PC	-	PILE CAP
CTR	-	CENTER	PL	-	PLATE
DBL	-	DOUBLE	PLF	-	POUNDS PER LINEAR FOOT
DET	-	DETAIL	PSF	-	POUNDS PER SQUARE FOOT
DIA	-	DIAMETER	PSI	-	POUNDS PER INCH
DIM	-	DIMENSION	PT	-	POST TENSION/PRESSURE TREATED
DN	-	DOWN	R	-	RISER
DR	-	DOOR	REINF	-	REINFORCED
DWG	-	DRAWING	REM	-	REMAINDER
EA	-	EACH	REQD	-	REQUIRED
EF	-	EACH FACE	REV	-	REVISED/REVISION
EJ	-	EXPANSION JOINT	RM	-	ROOM
EL	-	ELEVATION	RO	-	ROUGH OPENING
ELECT	-	ELECTRICAL	RQMT	-	REQUIREMENT
ELEV	-	ELEVATOR	SCHED	-	SCHEDULE
EOS	-	EDGE OF SLAB	SECT	-	SECTION
EQ	-	EQUAL	SIM	-	SIMILAR
EW	-	EACH WAY	SJ	-	SAWCUT JOINT
EXIST	-	EXISTING	SL	-	SLOPE
EXP	-	EXPANSION	SQ	-	SQUARE
Fe	-	FORCE	SP	-	SPIRAL
FIN	-	FINISH	SS	-	STAINLESS STEEL
FLR	-	FLOOR	STD	-	STANDARD
FND	-	FOUNDATION	SW	-	SHEARWALL/SHORT WAY
FOM	-	FACE OF MASONRY	STL	-	STEEL
FS	-	FAR SIDE	STRUCT	-	STRUCTURAL
FT	-	FOOT	SYM	-	SYMETRICAL
FTG	-	FOOTING	TB	-	TIE BEAM
GA	-	GAGE	TC	-	TIE COLUMN
GALV	-	GALVANIZED	TE	-	THICKENED EDGE
HC	-	HOLLOW CORE	THK	-	THICK
HDG	-	HOT DIPPED GALVANIZED	T/O	-	THRU OUT
HORIZ	-	HORIZONTAL	TOC	-	TOP OF CONCRETE
HP	-	HIGH POINT	T	-	TOP
IJ	-	ISOLATION JOINT	TEMP	-	TEMPERATURE
INFO	-	INFORMATION	TOS	-	TOP OF STEEL/SLAB
INT	-	INTERIOR	TR	-	TREAD
JST	-	JOIST	TYP	-	TYPICAL
JT	-	JOINT	UNO	-	UNLESS NOTED OTHERWISE
K	-	KIPS	VERT	-	VERTICAL
K/FT	-	KIPS PER FOOT	W/	-	WITH
KJ	-	CONST.JOINT/KEYSTONE JOIST	WD	-	WOOD
			WWF	-	WELDED WIRE FABRIC

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1 SCOPE OF WORK FOR REVISION 3 : OWNER DESIGN CHANGE REMOVING DOUBLE HEIGHT LIVING ROOM.

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Reason: I am the author of this document
Date: 2025.05.30 14:19:09 -04'00'
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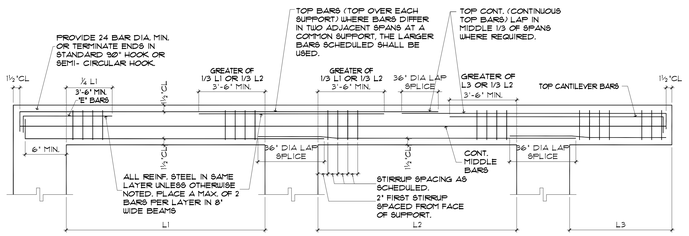


NOTES:

1. "L" INDICATES THE LARGER OF ADJACENT SPANS.
2. WHERE BOTTOM BARS ARE INDICATED AS CONTINUOUS WITH ADJACENT SPANS, SPLICES, WHERE NEEDED, SHALL BE MADE IN THE END 1/3 OF SPAN WITH 24 DIA. LAP.
3. MIDDLE BARS AS SCHEDULED SHALL BE PLACED EACH SIDE OF BEAM. SPLICE BARS AT SUPPORTS W/ 36 BAR DIAM. LAP SPLICES

TYPICAL BEAM REINF. PLACING DIAGRAM

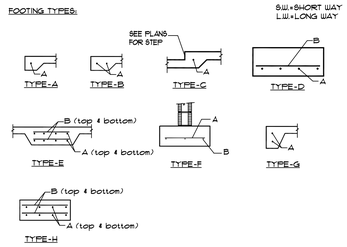
N.T.S.

FOOTING SCHEDULE

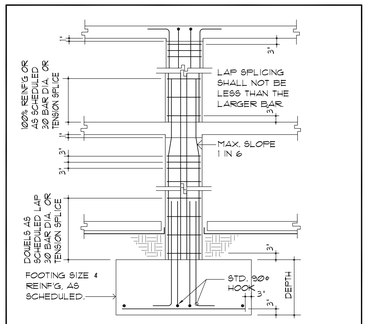
MARK	FOOTING TYPE	WIDTH	LENGTH	DEPTH	REINFORCEMENT A	REINFORCEMENT B	REMARKS
TEB	A	8"	CONT.	8"	(1) #5	---	THICKENED EDGE
TEBC	C	8"	CONT.	8"	(1) #5	---	THICKENED EDGE
TEBG	G	8"	CONT.	12"	(2) #5	---	THICKENED EDGE
TEB6	B	16"	CONT.	12"	(2) #5	---	THICKENED EDGE
MF36	E	36"	CONT.	12"	#5@12"oc BW	#5@12"oc LW	STAIR MONO FOOTING
WF8	F	16"	CONT.	12"	#5@12"oc BW	2 #5 LW	WALL FOOTING
WF24	F	24"	CONT.	12"	#5@12"oc BW	#5 @ 12" oc LW	WALL FOOTING
WF48	D	48"	CONT.	12"	#5@12"oc BW	#5 @ 12" oc LW	WALL FOOTING
F-1	D	36"	36"	12"	(3) #5	(3) #5	PAD FOOTING
F-2	D	48"	48"	12"	(4) #5	(4) #5	PAD FOOTING
F-3	D	60"	60"	12"	(5) #5	(5) #5	PAD FOOTING
F-4	D	72"	72"	12"	(6) #5	(6) #5	PAD FOOTING
F-5	H	60"	84"	12"	(7) #5 LW	(7) #5 BW	PAD FOOTING
F-6	H	48"	60"	12"	(4) #5 LW	(6) #5 BW	PAD FOOTING
F-7	H	84"	84"	12"	(7) #5 LW	(7) #5 BW	PAD FOOTING

FOOTING TYPES:

SW=SHORT WAY
LW=LONG WAY

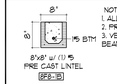


NOTE: PROVIDE 3#5 CORNER BARS FOR WF8 AND 3#5 CORNER BARS FOR WF24 BENT 36" EACH WAY @ ALL CONTINUOUS WALL FOOTING CORNERS.



TYPICAL COLUMN TO FOOTING DETAIL

PRECAST LINTEL SCHEDULE



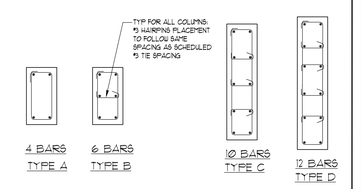
NOTES:
1. ALL BOTTOM REINFORCEMENT SHALL EXTEND 8 INCHES INTO SUPPORT.
2. PRE CAST LINTELS TO BEAR 8".
3. VERIFY DOOR & WINDOW OPENINGS W/MANUFACTURES SPECS PRIOR TO BEAM PLACEMENT

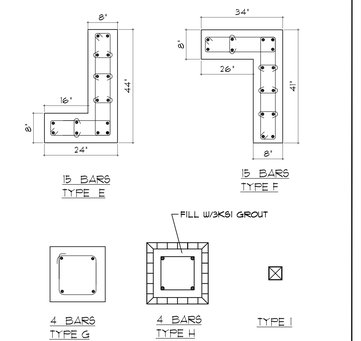
PRODUCT CONTROL NOTICE OF ACCEPTANCE FECP CORPORATION-CAST-CRETE DIVISION ACCEPTANCE # 23-0808.05

COLUMN SCHEDULE

MARK	COLUMN TYPE	DIMENSIONS	REINFORCEMENT VERTICAL	REINFORCEMENT HORIZONTAL (#5 TIES)	REMARKS
C1	A	8" x 12"	4-#5	#5 @ 8"o.c.	CONCRETE COLUMN
C2	B	8" x 24"	6-#6	#5 @ 8"o.c.	CONCRETE COLUMN
C3	B	8" x 26"	6-#6	#5 @ 8"o.c.	CONCRETE COLUMN
C4	B	8" x 23"	6-#5	#5 @ 8"o.c.	CONCRETE COLUMN
C5	A	8" x 16"	4-#6	#5 @ 8"o.c.	CONCRETE COLUMN
C6	A	8" x 11"	4-#7	#5 @ 8"o.c.	CONCRETE COLUMN
C7	D	8" x 41"	12-#6	#5 @ 8"o.c.	CONCRETE COLUMN
C8	F	AS SHOWN	15-#6	#5 @ 8"o.c.	CONCRETE COLUMN
C9	C	8" x 65"	10-#6	#5 @ 8"o.c.	CONCRETE COLUMN
C10	D	8" x 51"	12-#6	#5 @ 8"o.c.	CONCRETE COLUMN
C11	G	12" x 12"	4-#5	#5 @ 8"o.c.	CONCRETE COLUMN
C12	A	8" x 18"	4-#6	#5 @ 8"o.c.	CONCRETE COLUMN
C13	B	8" x 20"	6-#5	#5 @ 8"o.c.	CONCRETE COLUMN
MC	H	12"x12"	4-#5	#3 @ 8"o.c.	MASONRY HOLLOW BLOCK COLUMN
SC1	I	4"x4"x1/2"	---	---	HSS TUBE STEEL COLUMN FOR BASE DETAIL SEE 1 & 2/S-4 FOR CAP DETAIL SEE 1/S-5
SC2	I	5"x5"x3/8"	---	---	

COLUMN TYPES:





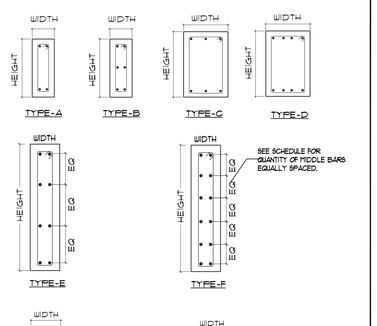
CONCRETE COLUMN NOTES:

1. L-SHAPE COLUMNS TO HAVE TIES IN EACH DIRECTION SPACED PER SCHEDULE.

BEAM SCHEDULE

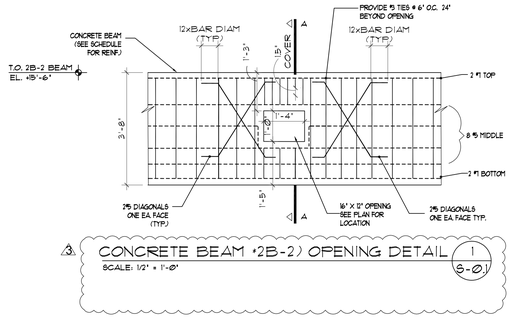
MARK	BEAM TYPE	TOP OF THE BEAM (ARCHITECTURAL)	WIDTH	HEIGHT	REINFORCEMENT TOP	REINFORCEMENT MIDDLE	REINFORCEMENT BOTTOM	# TIES	REMARKS
TB-1	A	+11'-10"	8"	12"	2-#5	-	2-#5	#3 @48" O.C.	TIE BEAM
TB-2	A	+15'-6"	8"	12"	2-#5	-	2-#5	#3 @48" O.C.	TIE BEAM
RKB-1	A	VARIES	8"	12"	2-#5	-	2-#5	#3 @48" O.C.	RAKE BEAM
RKB-2	A	VARIES	12"	8"	2-#5	-	2-#5	#3 @48" O.C.	RAKE BEAM
RTB-1	A	+25'-7"	8"	12"	2-#5	-	2-#5	#3 @48" O.C.	TIE BEAM
1B-1	F	+11'-10"	8"	47.5"	2-#6	8-#5	2-#6	#3 @12" O.C.	CONCRETE BEAM
1B-2	A	+11'-10"	8"	20.5"	2-#5	-	2-#5	#3 @10" O.C.	CONCRETE BEAM
B-3	E	+11'-10"	8"	52"	2-#5	8-#4	2-#6	#3 @12" O.C.	CONCRETE BEAM
1B-4	A	+11'-10"	12"	20.5"	2-#5	-	2-#7	#3 @9" O.C.	CONCRETE BEAM
1B-5	A	+11'-10"	12"	20.5"	2-#5	-	2-#7	#3 @9" O.C.	CONCRETE BEAM
1B-6	D	+11'-10"	12"	20.5"	4-#6	-	4-#8	#3 @9" O.C.	CONCRETE BEAM
1B-7	A	+11'-10"	12"	20.5"	2-#6	-	2-#6	#3 @10" O.C.	CONCRETE BEAM
2B-1	C	+15'-6"	12"	15"	3-#5	-	3-#6	#3 @6" O.C.	CONCRETE BEAM
2B-2	E	+15'-6"	8"	44"	2-#7	8-#5	2-#7	#3 @16" O.C.	CONCRETE BEAM SEE DETAIL 1/S-0.1
2B-3	G	+16'-3"	8"	36"	2-#5	4-#5	4-#7	#3 @12" O.C.	CONCRETE BEAM
2B-4	H	+15'-6"	8"	22"	4-#7	-	4-#7	#3 @8" O.C.	CONCRETE BEAM
2B-5	H	+15'-6"	8"	20"	4-#7	-	4-#7	#3 @7" O.C.	CONCRETE BEAM
2B-6	D	+15'-6"	12"	16"	4-#6	-	4-#6	#3 @5" O.C.	CONCRETE BEAM
2B-7	B	8'-11.5"	8"	24"	2-#5	2-#5	2-#6	#3 @8" O.C.	CONCRETE BEAM # STAIR LANDING
2B-8	H	+16'-2"	8"	24"	4-#7	-	4-#7	#3 @8" O.C.	UPTURNED CONCRETE BEAM
2B-9	D	+15'-6"	12"	17"	4-#7	-	4-#7	#3 @5" O.C.	CONCRETE BEAM
2B-10	B	+15'-6"	8"	24"	2-#7	2-#5	2-#7	#3 @8" O.C.	CONCRETE BEAM
2B-11	A	+15'-6"	8"	18"	2-#5	-	2-#6	#3 @7" O.C.	CONCRETE BEAM
2B-12	F	+15'-6"	8"	64"	2-#6	8-#5	2-#6	#3 @12" O.C.	CONCRETE BEAM
RB-1	A	+25'-7"	8"	23.5"	2-#5	2-#5	2-#5	#3 @ 8" O.C.	CONCRETE BEAM

BEAM TYPES:



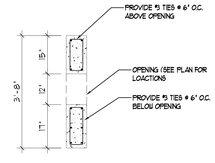
CONCRETE BEAM NOTES:

1. TIE BEAMS SHALL HAVE (4) # 3 TIES AT 12" o.c. AT ALL CORNERS EA WAY AND 48" O.C. REMAINDER.
2. PROVIDE (2) #5 x 6'0" CORNER BARS, BENT 30° EACH WAY @ ALL CORNERS.
3. PROVIDE 1" MIN. CLEAR SPACING BETWEEN BARS.



3/S-0.1 CONCRETE BEAM #2B-2) OPENING DETAIL

SCALE: 1/2" = 1'-0"

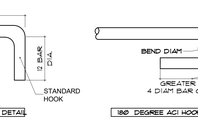


SECTION AA

SCALE: 1/2" = 1'-0"

TENSION EMBEDMENT LENGTH (with std. hook) SCHEDULE

BAR SIZE	f'c = 3000 psi TOP	f'c = 3000 psi BOT	f'c = 4000 psi TOP	f'c = 4000 psi BOT	f'c = 5000 psi TOP	f'c = 5000 psi BOT	f'c = 6000 psi TOP	f'c = 6000 psi BOT	f'c = 7000 psi TOP	f'c = 7000 psi BOT	f'c = 8000 psi TOP	f'c = 8000 psi BOT
# 4	12"	8"	10"	7"	9"	6"	9"	6"	9"	6"	9"	6"
# 5	14"	10"	13"	9"	12"	8"	10"	7"	10"	7"	10"	7"
# 6	17"	12"	14"	10"	13"	9"	13"	9"	12"	8"	12"	8"
# 7	20"	14"	17"	12"	16"	11"	14"	10"	13"	9"	13"	9"
# 8	23"	16"	20"	14"	17"	12"	16"	11"	16"	11"	14"	10"
# 9	26"	18"	21"	15"	20"	14"	19"	13"	17"	12"	16"	11"
# 10	28"	20"	24"	17"	23"	16"	20"	14"	19"	13"	17"	12"
# 11	31"	22"	27"	19"	24"	17"	23"	16	21"	15"	20"	14"



TYPICAL TENSION EMBEDMENT DETAIL

DEFINITIONS OF LAP/SPLICE CATEGORIES

CATEGORY	DEFINITION
1	CLEAR SPACING OF BARS BEING DEVELOPED OR SPLICED NOT LESS THAN db, CLEAR COVER NOT LESS THAN db, AND STIRRUPS OR TIES THROUGHOUT Ld NOT LESS THAN THE CODE MINIMUM OR CLEAR SPACING OF BARS BEING DEVELOPED OR SPLICED NOT LESS THAN 2db, AND CLEAR COVER NOT LESS THAN db.
2	OTHER CASES

NOTE:

1. TOP BARS ARE DEFINED AS HORIZONTAL REINFORCEMENT SO PLACED THAT MORE THAN 12 INCHES OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE DEVELOPMENT LENGTH OR SPLICE.
2. db STANDS FOR NOMINAL BAR DIAMETER.

COMPRESSION LAP SPLICE 'LC'

'LC' = 30 BAR DIAMETER FOR f'c ≥ 3000 psi

MINIMUM COMPRESSION DOWEL EMBEDMENT

22 BAR DIAMETER FOR f'c ≥ 3000 psi

TABLE 5-3 (C) TENSION DEVELOPMENT LENGTHS, ld (INCHES) FOR GRADE 60 UNCOATED BARS f'c=5000 psi, NORMAL- WEIGHT CONCRETE BASED ON ACI 1122

BAR SIZE	ld PER SPACING AND COVER CASE CASE 1 TOP BARS	CASE 1 OTHER BARS	CASE 2 TOP BARS	CASE 2 OTHER BARS
# 3	17	13	25	19
# 4	22	17	33	26
# 5	28	22	42	32
# 6	33	26	50	38
# 7	49	37	73	56
# 8	55	43	83	64
# 9	63	48	94	72
# 10	70	54	105	81
# 11	78	60	117	90
# 14	94	72	140	108
# 18	125	96	187	144

TABLE 5-3 (C) TENSION LAP SPLICE LENGTHS, ld (INCHES) FOR GRADE 60 UNCOATED BARS f'c=5000 psi, NORMAL- WEIGHT CONCRETE BASED ON ACI 1122

BAR SIZE	LAP CLASS	LAP LENGTH PER SPACING AND COVER CASE CASE 1 TOP BARS	CASE 1 OTHER BARS	CASE 2 TOP BARS	CASE 2 OTHER BARS
# 3	A	17	13	25	19
	B	22	17	33	25
# 4	A	22	17	33	26
	B	28	22	43	33
# 5	A	28	22	42	32
	B	36	28	54	42
# 6	A	33	26	50	38
	B	43	33	65	50
# 7	A	49	37	73	56
	B	63	49	94	73
# 8	A	55	43	83	64
	B	72	55	108	83
# 9	A	63	48	94	72
	B	81	63	122	94
# 10	A	70	54	105	81
	B	91	70	137	105
# 11	A	78	60	117	90
	B	101	78	152	117

1 SCOPE OF WORK FOR REVISION 3 : OWNER DESIGN CHANGE REMOVING DOUBLE HEIGHT LIVING ROOM.

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S-0.1

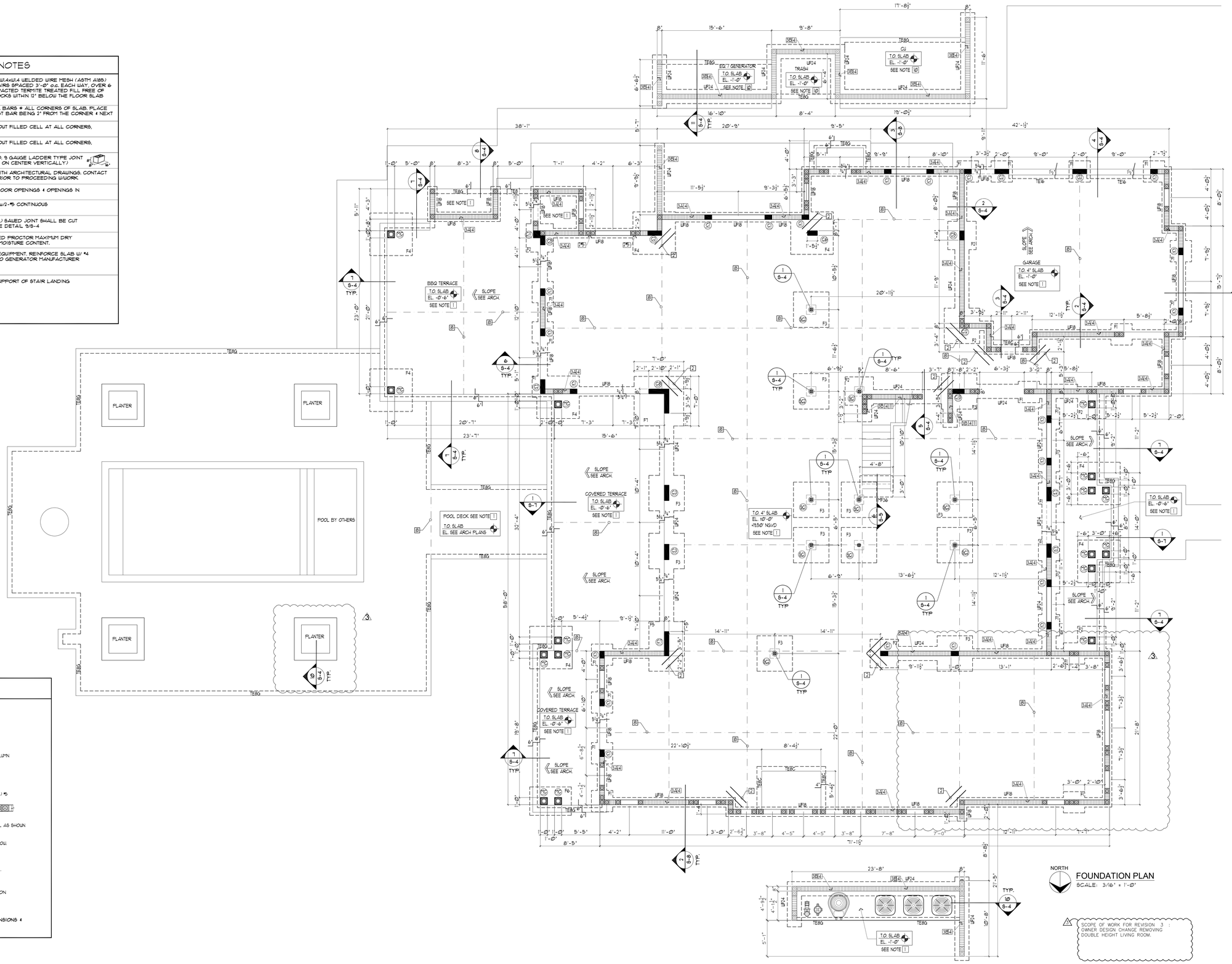
FOUNDATION NOTES

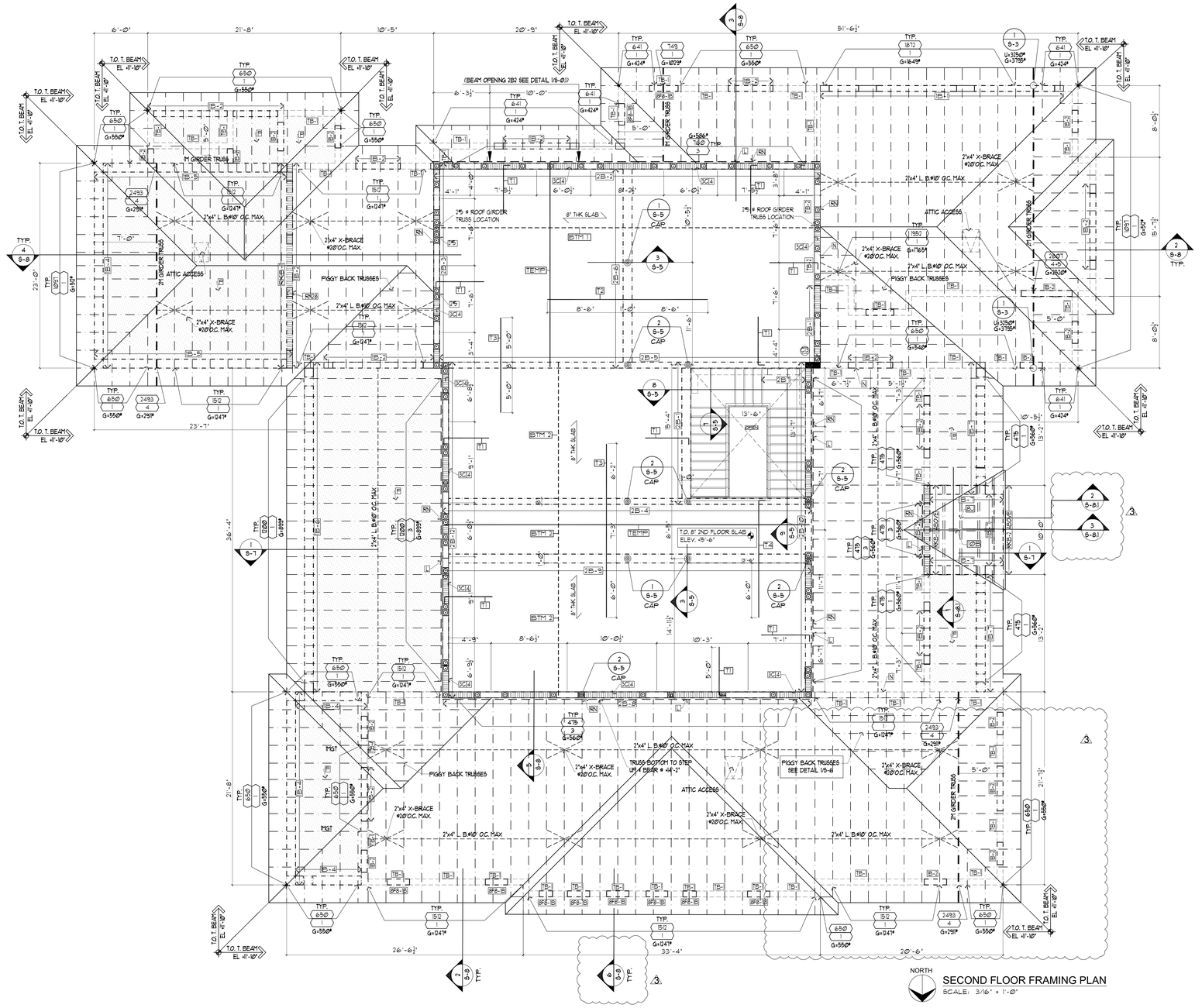
1. 4" REINFORCED CONCRETE SLAB WITH 6X6-W1.4XW1.4 WELDED WIRE MESH (ASTM A185) LOCATED @ MID SLAB, SUPPORTED W/ 2" CHAIRS SPACED 3'-0" O.C. EACH WAY. OVER 6 MIL VAPOR BARRIER ON CLEAN WELL COMPACTED TERMITE TREATED FILL FREE OF ORGANIC MATTER. MAXIMUM SIZE OF ANY ROCKS WITHIN 12" BELOW THE FLOOR SLAB SHALL BE A MAXIMUM OF 3" IN DIAMETER.
2. PROVIDE (2) #4 x 36" LONG CORNER CRACK BARS @ ALL CORNERS OF SLAB. PLACE BARS ON TOP THIRD OF THE SLAB WITH FIRST BAR BEING 2" FROM THE CORNER & NEXT ONE 12" APART.
3A. 1-#5 REINFORCEMENT BAR GRADE 60 IN GROUT FILLED CELL AT ALL CORNERS, NEXT TO ALL OPENINGS, AND 32"O.C.
3B. 1-#5 REINFORCEMENT BAR GRADE 60 IN GROUT FILLED CELL AT ALL CORNERS, NEXT TO ALL OPENINGS, AND 16"O.C.
4. 8"x8"x16" CONCRETE MASONRY UNITS WITH NO. 9 GAUGE LADDER TYPE JOINT REINFORCEMENT EVERY OTHER COURSE (16" ON CENTER VERTICALLY.)
5. CONTRACTOR TO VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS. CONTACT ENGINEER IF DISCREPANCIES ARE FOUND PRIOR TO PROCEEDING W/WORK
6. VERIFY SIZE & LOCATIONS OF ALL WINDOW/DOOR OPENINGS & OPENINGS IN SLAB WITH ARCHITECTURAL DWGS.
7. CONCRETE SILLS TO BE A MINIMUM OF 8"x8" W/2-#5 CONTINUOUS FOR 8"x8"x16" CMU WALLS
8. SAW CUT JOINT, (S.C.J.) @15'-0" O.C. MAX (TYP.) SAWED JOINT SHALL BE CUT WITHIN 24 Hrs. OF CONCRETE PLACEMENT. SEE DETAIL 9/S-4
9. COMPACT INTERIOR FILL TO 95% OF MODIFIED PROCTOR MAXIMUM DRY DENSITY AS PER ASTM D-1557 AT OPTIMUM MOISTURE CONTENT.
10. 8" CONCRETE SLAB PAD FOR GENERATOR EQUIPMENT. REINFORCE SLAB W/ #4 BARS @ 12" OC EACH WAY BOTTOM. REFER TO GENERATOR MANUFACTURER SPECS FOR ANCHORAGE TO THE SLABS.
11. 8"x8"x16" CONCRETE MASONRY LOW WALL, SUPPORT OF STAIR LANDING

LEGEND

- SECTION CALLOUT
- DETAIL CALLOUT
- BEARING CONCRETE COLUMN OR SHEARWALL.
- (*5 SPACED PER PLAN) — REPRESENTS (2) #5 — REPRESENTS (3) #5 — REPRESENTS (4) #5 — 8"X8"X16" C.M.U. WALL REINFORCED W/ #5 VERTICAL AS SHOWN
- STRUCTURAL BEARING ELEMENT BELOW.
- COLUMN STARTS AT THIS LEVEL.
- COLUMN CONTINUES THRU THIS LEVEL.
- COLUMN ENDS AT THIS LEVEL.
- PROPOSED CONCRETE SECTION W/REINFORCEMENT

CONTRACTOR TO VERIFY ALL BUILDING DIMENSIONS & ELEVATIONS WITH ARCHITECTURAL PLAN





2ND FLOOR NOTES

- [X] 1-#5 REINFORCEMENT BAR GRADE 60 IN CONC. FILLED CELL AT ALL CORNERS, NEXT TO ALL OPENINGS AND 40"o.c. TYPICAL FOR ALL SECOND FLOOR WALLS
- [4] 8"x8"x16" CONCRETE MASONRY UNITS WITH NO. 9 GAUGE LADDER TYPE JOINT REINFORCEMENT EVERY OTHER COURSE (16" ON CENTER VERTICALLY)
- [L] (2) 2"x8" PRESSURE TREATED (SYP NO2) WOOD LEDGER ATTACHED TO CONCRETE W/ 3/4" DIAM. HILTI KWIK BOLT 3 EXPANSION ANCHORS @ 12" OC. MIN EMBEDMENT 4.75".
- [RN] 2"x4" PRESSURE TREATED ROOF NAILER ANCHORED TO CONCRETE W/ 1/4 DIAM. x 2 3/4" LONG HEX HEADED TAPCONS (EMBEDDED 1.25") SPACED @ 8" O.C. ATTACH ROOF PLYWOOD SHEATHING TO NAILER W/ 8d RS NAILS SPACED 6" OC.
- [RN2] 2X8 PT NAILER ATTACHED TO EACH SIDE OF CONCRETE RAKE BEAM W/ ½" DIAMETER HILTI KWIK BOLT 3 EXPANSION ANCHORS @ 24" OC. ATTACH SHEATHING TO NAILERR W/ 8d RS NAILS @ 6" OC
- [P] 1/2" PLYWOOD SOFFIT ATTACHED TO BOTTOM CHORD OF TRUSSES W/8d RS NAILS @ 6" OC
- [8J] 3"X8" WOOD JOIST (SYP NO 2) SPACED 24" OC
- [Ø8] HSS 3X8x¼" STEEL TUBE RIDGE BEAM

SLAB REINFORCEMENT LEGEND

- [BTM 1] #5 BOTTOM @ 6" O.C.
- [BTM 2] #5 BOTTOM @ 12" O.C.
- [TEMP] #4 TEMP @ 12" O.C. PLACED OVER BOTTOM BARS (TYP THROUGHOUT)
- [T1] #4x 4'-0" LONG TOP @ 12" o.c. TOP W/ 90° ACI HOOK.
- [T2] #5 TOP @ 6" O.C. (SEE PLANS FOR LENGTH)
- [T3] #5 TOP @ 12" O.C. (SEE PLANS FOR LENGTH)
- [T4] #5 TOP @ 12" o.c. TOP W/ 90° ACI HOOK (SEE PLANS FOR LENGTH)
- [TB] #4 TOP @ 12" o.c. TOP W/ 90° ACI HOOK BOTH ENDS.

LEGEND

- SECTION CALLOUT
- DETAIL CALLOUT
- BEARING CONCRETE COLUMN OR SHEARWALL
- (#5 SPACED PER PLAN) / REPRESENTS (2) #5 / REPRESENTS (3) #5 / REPRESENTS (4) #5 — 8"X8"x16" C.M.U. WALL REINFORCED W/ #5 VERTICAL AS SHOWN
- STRUCTURAL BEARING ELEMENT BELOW.
- COLUMN STARTS AT THIS LEVEL.
- COLUMN CONTINUES THRU THIS LEVEL.
- COLUMN ENDS AT THIS LEVEL.
- PROPOSED CONCRETE SECTION W/REINFORCEMENT

CONTRACTOR TO VERIFY ALL BUILDING DIMENSIONS & ELEVATIONS WITH ARCHITECTURAL PLAN

ROOF LEGEND

HATCH INDICATES, 1/2" PLYWOOD ROOF SOFFIT ATTACHED TO TRUSS BOTTOM CHORD W/8d RS NAILS 6" OC

ROOF TRUSS ANCHOR SCHEDULE

ACTUAL UPLIFT / REQUIRED ANCHOR

B/E = BOTH ENDS OF TRUSS
U/NO = UNLESS NOTED OTHERWISE
U = UPLIFT LOAD
G = GRAVITY LOAD
TYP = TYPICAL

1	NU-VUE NVHTA16 STRAP ANCHOR W/ 18-10dx1.5" NAILS TO TRUSS AND 6-10dx1.5" NAILS TO SEAT. CAPACITY= 2649# (NOA 24-0213.07)
2	(NUVUE) NVHTA-16H (1 PLY) GIRDER STRAP W/ (18) 10d x 1.5" LONG NAILS TO JOIST AND (6) 10d x 1.5" NAILS TO SEAT PLATE ANCHOR CAPACITY: U=3117# (NOA 24-0213.05)
3	MITEK THD28 HANGER ATTACHED TO LEDGER W/18-16d NAILS + TO TRUSS W/12-10dx1.5" NAILS. ALLOWABLE U= 2265#, G= 2645# (FL 17232)
4	(NUVUE) NV358-16 (2 PLY) GIRDER DOUBLE STRAP W/ (18) 10d x 3" LONG NAILS TO JOIST AND (8) 10d x 3" NAILS TO SEAT PLATE ANCHOR CAPACITY: U=3367# (NOA 24-0213.05)
5	(SIMPSON) LGUM28-2 HANGER W/(6) ½" DIAM x 2 ½" LONG SDS WOOD SCREWS TO TRUSS AND (6) ⅜" DIAM x 4" TITEN HD CONCRETE ANCHORS TO WALL. U=2435#, G=1595 (FLORIDA APPROVAL #3504)

SCOPE OF WORK FOR REVISION 3 : OWNER DESIGN CHANGE REMOVING DOUBLE HEIGHT LIVING ROOM.

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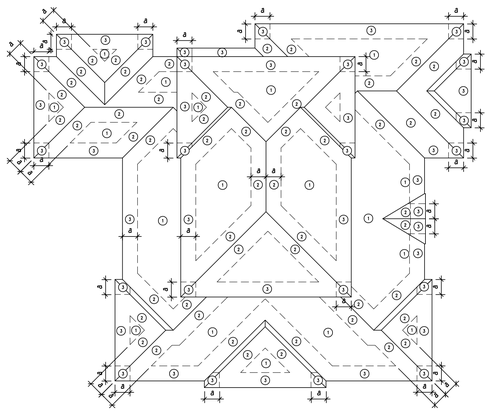
ROOF PRESSURES ZONES

ZONE 1	+34.49 PSF & -77.59 PSF (105F AREA)
ZONE 2	+34.49 PSF & -101.1 PSF (105F AREA)
ZONE 3	+34.49 PSF & -128.55 PSF (105F AREA)

ZONE ('a') = 4.33 FT.
ALL WIND VALUES ARE ASD VALUES



COMPONENT & CLADDING LOADING DIAGRAMS. (NTS)

ROOF TRUSS ANCHOR SCHEDULE

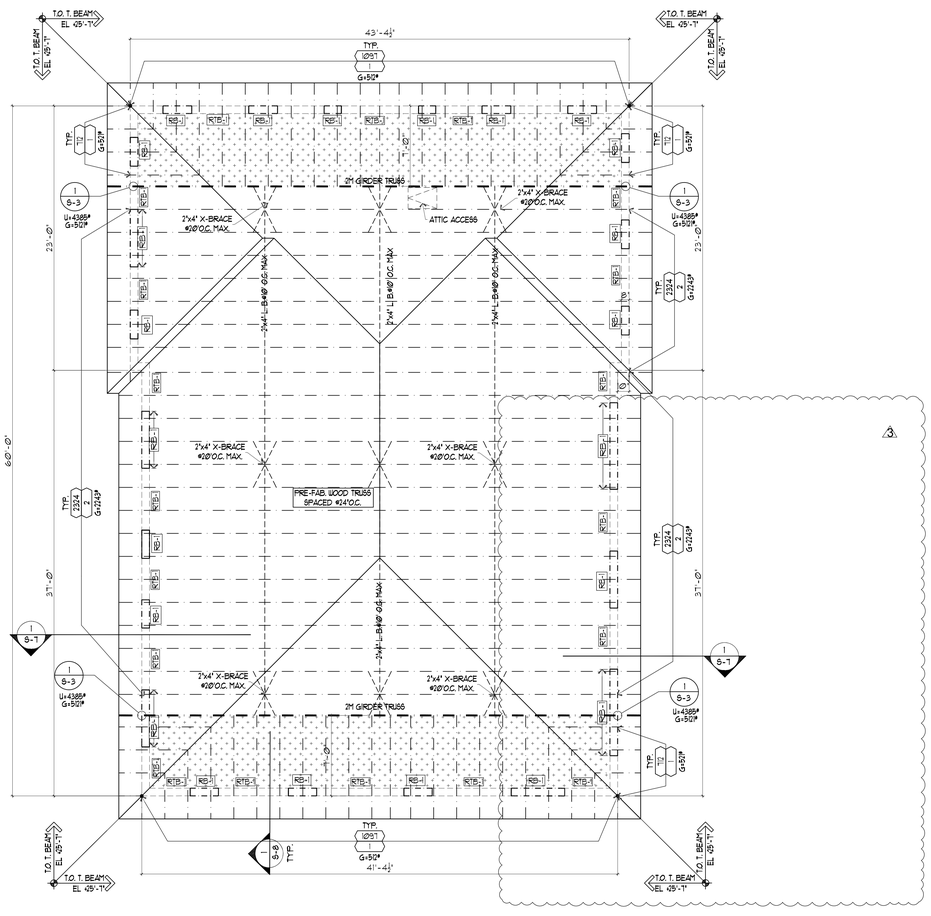
ACTUAL UPLIFT / REQUIRED ANCHOR

B/E = BOTH ENDS OF TRUSS
UNO = UNLESS NOTED OTHERWISE
U = UPLIFT LOAD
G = GRAVITY LOAD
TYP = TYPICAL

1	NU-VUE NVHTA16 STRAP ANCHOR w/ 18-10dx1.5" NAILS TO TRUSS AND 6-10dx1.5" NAILS TO SEAT. CAPACITY= 2649# (NOA 24-0213.07)
2	(NUVUE) NVHTA-16H (1 PLY) GIRDER STRAP w/ (18) 10d x 1.5" LONG NAILS TO JOIST AND (6) 10d x 1.5" NAILS TO SEAT PLATE ANCHOR CAPACITY: U=3117# (NOA 24-0213.05)
3	MITEK THD28 HANGER ATTACHED TO LEDGER W/18-16d NAILS & TO TRUSS W/12-10dx1.5" NAILS. ALLOWABLE U= 2265#, G= 2645# (FL#7232)
4	(NUVUE) NV358-16 (2 PLY) GIRDER DOUBLE STRAP w/ (18) 10d x 3" LONG NAILS TO JOIST AND (8) 10d x 3" NAILS TO SEAT PLATE ANCHOR CAPACITY; U=3367# (NOA 24-0213.05)
5	(SIMPSON) LGUM28-2 HANGER W/(6) 1/4" DIAM x 2 1/2" LONG SDS WOOD SCREWS TO TRUSS AND (6) 3/4" DIAM x 4" TITEN HD CONCRETE ANCHORS TO WALL. U=2435#, G=5595 (FLORIDA APPROVAL #3904)

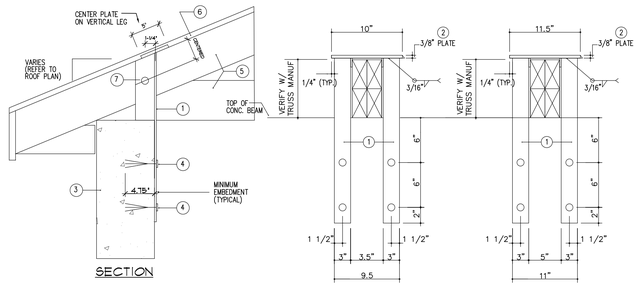
ROOF LEGEND

HATCHING INDICATES OF 2'x4" BLOCKING @ ALL PLYWOOD SEAMS. ATTACH SHEATHING TO BLOCKING W/ 8d RING SHANK NAILS 4" O.C. EACH SIDE OF SEAM



NORTH

ROOF FRAMING PLAN
SCALE: 3/16" = 1'-0"



SCHEDULE	
MARK	DESCRIPTION
1	ANGLE 3" x 3" x 1/4" W/ EXTENDED LEG (Fy=36KSI) SEE DWG.
2	STEEL PLATE 10" x 5" x 3/8" (Fy=36 KSI)
3	CONCRETE BEAM (SEE PLANS & SCHD)
4	(4) 3/4" DIAMETER HILTI KWIK BOLT 3 (4.75" MIN. EMBED.)
5	PREFABRICATED WOOD GIRDER TRUSS
6	19/32" EXTERIOR GRADE PLYWOOD
7	3/4" DIAM THRU BOLT ASTM A307

PER HILTI KWIK BOLT 3
(4) 3/4" DIAMETER ANCHORS IN 3000 PSI CONCRETE (MIN. EMBEDMENT 4.75").
-1/16" SPACING = 0.90 REDUCTION
-1/16" EDGE = 0.87 REDUCTION
SHEAR CAPACITY=6,173# (0.90) (0.87) x 4 BOLTS=14.8K
PER NDS CAPACITY BEARING CAPACITY Fc(perp)=565 PSI.
CAPACITY OF (2) MEMBER
AREA OF PLATE FOR BEARING=(18sq-in 2M) & (22.5 sq-in 3M)
ALLOWABLE LOAD=AREA x 565 PSI=8,475 (2M) & 12,712# (3M)
ALLOWABLE UPLIFT CAPACITY = 8,475 # (2M) & 12,712# (3M)

STEEL SDL STRAP DETAIL 1/S-3
N.T.S.

1 SCOPE OF WORK FOR REVISION 3 :
OWNER DESIGN CHANGE REMOVING DOUBLE HEIGHT LIVING ROOM.

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ALEX KONDRAT LICENSE No. 58086 STATE OF FLORIDA PROFESSIONAL ENGINEER

Digitally signed by Alex Kondrat
DN: CN=Alex Kondrat, E=alexkondrat@gmail.com, O=Alex Shimkovitz, OU=Sr. Structural Engineer
Reason: I am the author of this document
Date: 2025.05.02 14:31:04-04'00'

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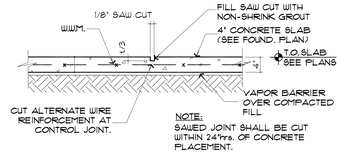
SHEET :

S-3



TYP. SAW CUT JOINT 9 / S-4

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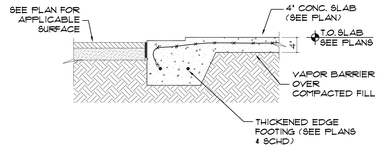

SECTION 4 / S-4

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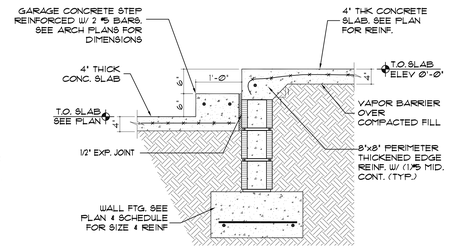

SECTION 3 / S-4

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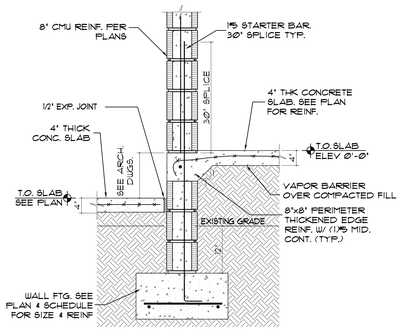

SECTION 2 / S-4

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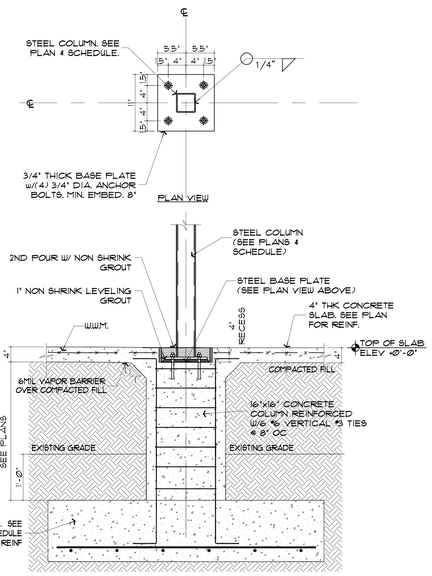

SECTION 1 / S-4

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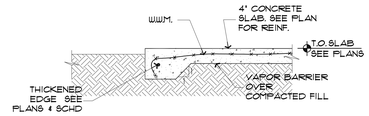

SECTION 8 / S-4

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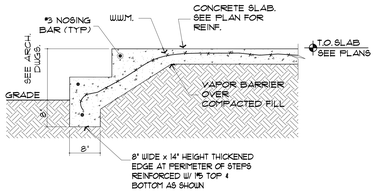

SECTION 7 / S-4

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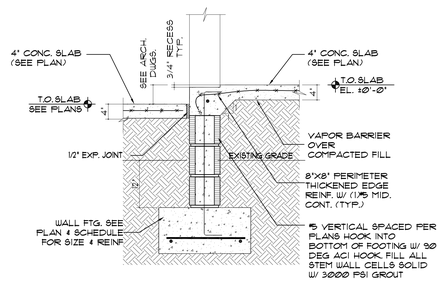

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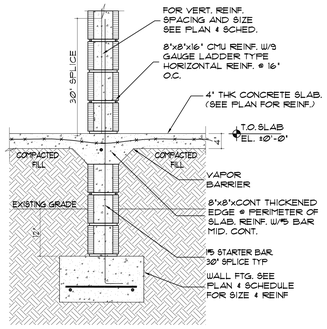

SECTION 5 / S-4

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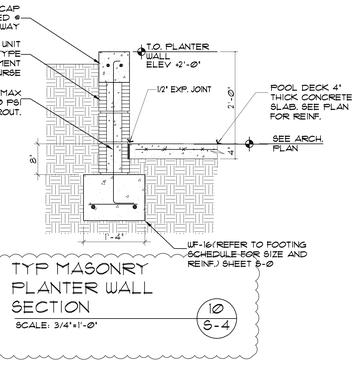
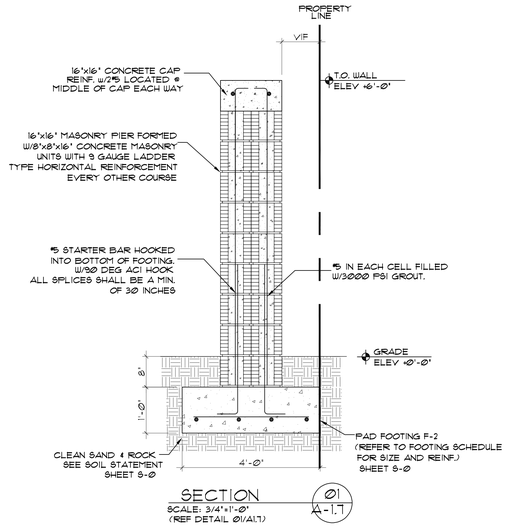


3. TYP MASONRY PLANTER WALL SECTION 10 / S-4

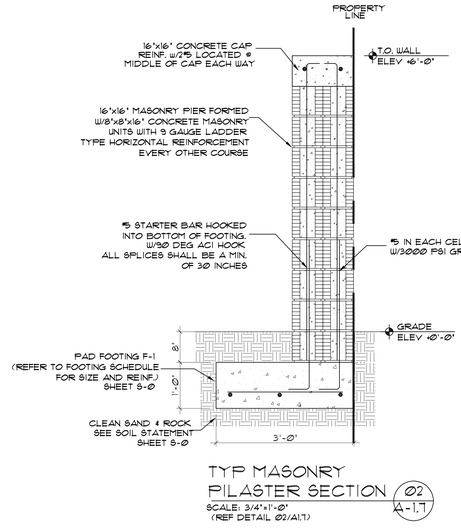
SCALE: 3/4" = 1'-Ø"



SECTION 01 / A-1.7

SCALE: 3/4"=1'-Ø"

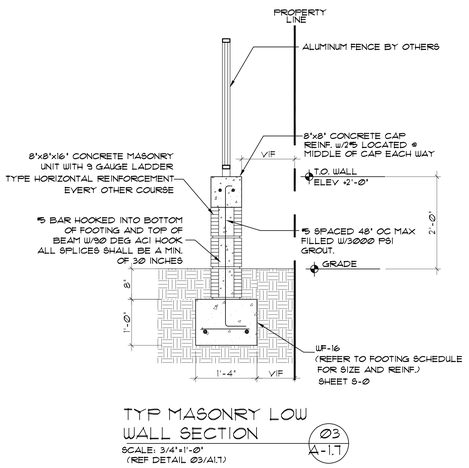
(REF DETAIL Ø1/A1.7)



TYP MASONRY PILASTER SECTION 02 / A-1.7

SCALE: 3/4"=1'-Ø"

(REF DETAIL Ø2/A1.7)



TYP MASONRY LOW WALL SECTION 03 / A-1.7

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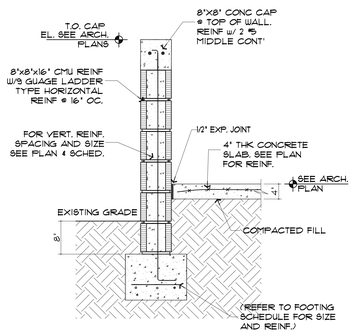
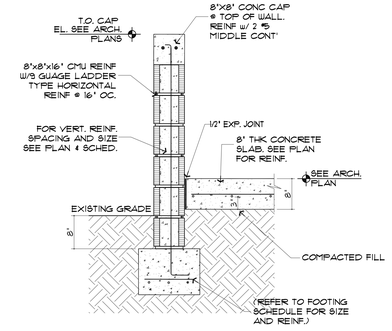
(REF DETAIL Ø3/A1.7)



DETAIL 10 / S-4

SCALE: 3/4" = 1'-Ø"



DETAIL 11 / S-4

SCALE: 3/4" = 1'-Ø"

4 SCOPE OF WORK FOR REVISION 3 : OWNER DESIGN CHANGE REMOVING DOUBLE HEIGHT LIVING ROOM.

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ALEX KONDRAT LICENSE No 58086 STATE OF FLORIDA PROFESSIONAL ENGINEER

Digitally signed by Alex Kondrat DN: cn=Alex Kondrat, o=Alex Kondrat & Associates, Inc., ou=Professional Engineer, email=alexkondrat@bellsouth.net, c=US Date: 2025.05.02 14:16:35-04'00'

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SHEET : S-4

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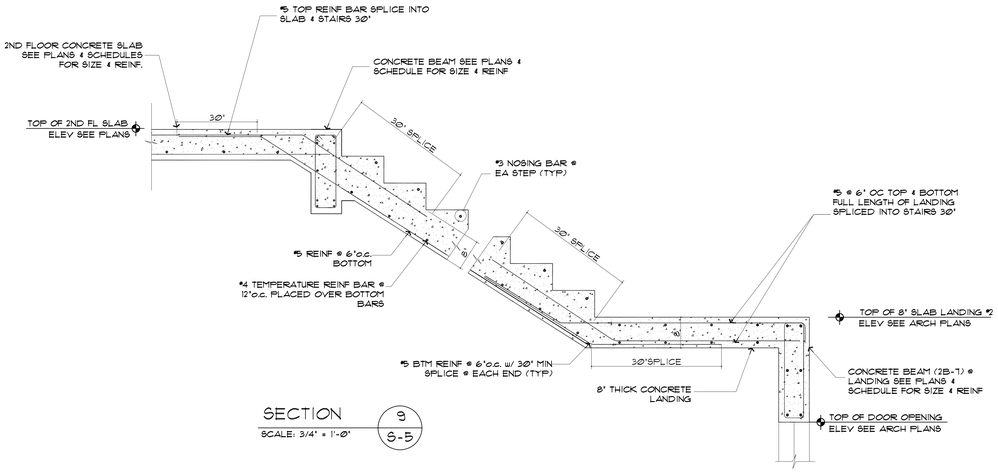

SECTION 9 / S-5
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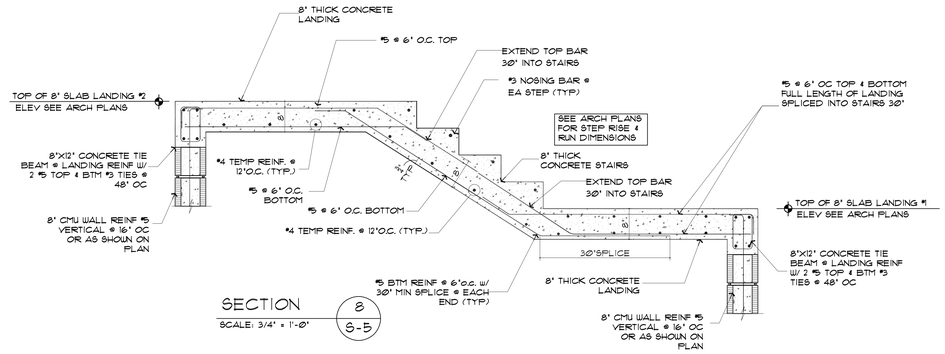

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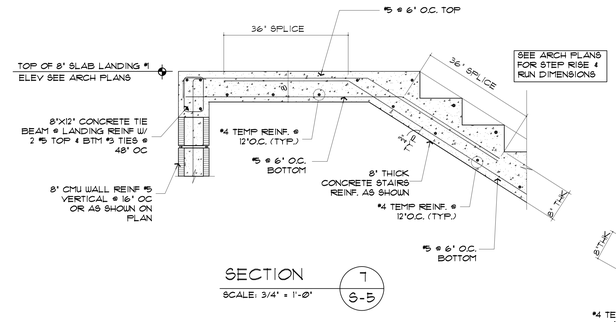

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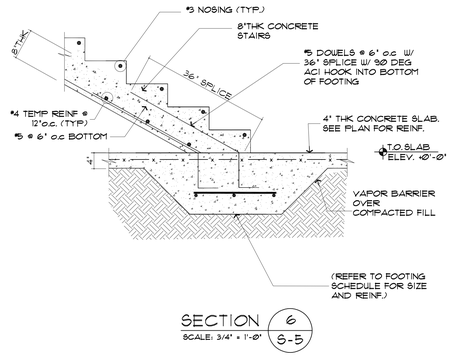

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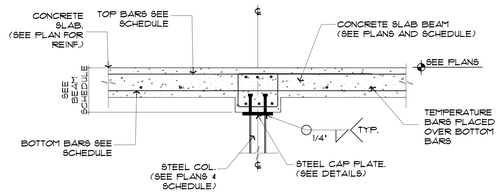

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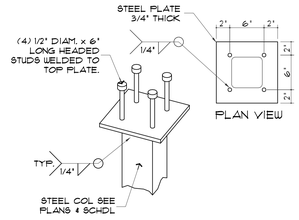
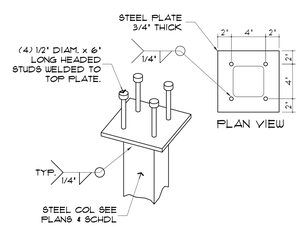


CAP DETAIL 1 / S-5
SCALE: N.T.S



CAP DETAIL 2 / S-5
SCALE: N.T.S

SECTION
4/S-5
5/S-5
10/S-5
REMOVED

3.

SCOPE OF WORK FOR REVISION 3 :
OWNER DESIGN CHANGE REMOVING DOUBLE HEIGHT LIVING ROOM.

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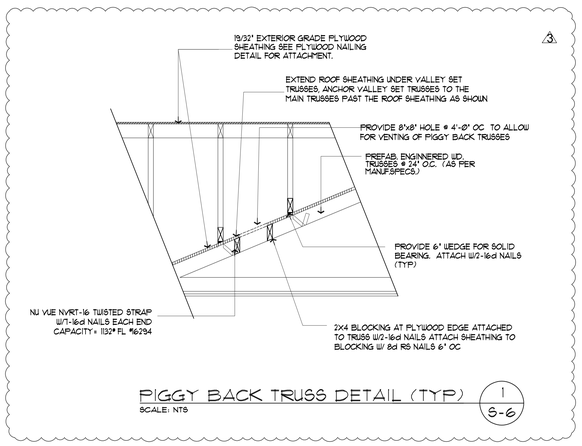
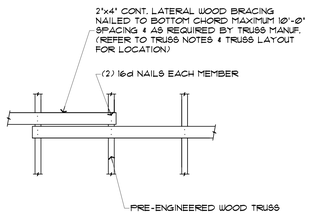


PIGGY BACK TRUSS DETAIL (TYP) 1/S-6

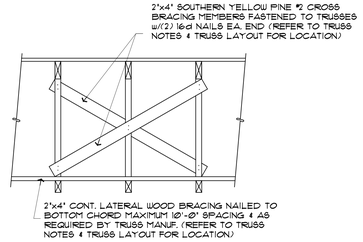
SCALE: NTS



PLAN VIEW

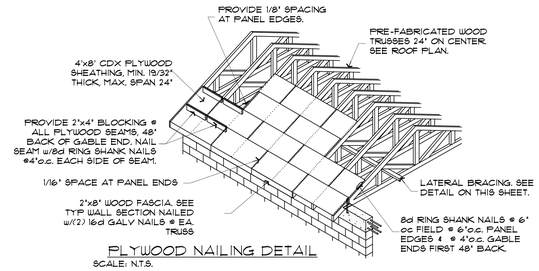
TYP. LATERAL BRACE SPLICE

SCALE: N.T.S.



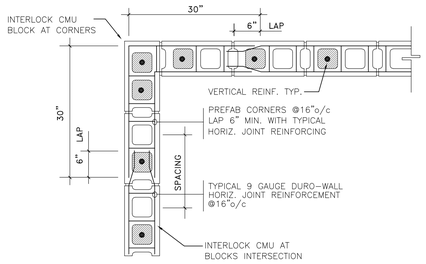
TYP. X-BRACING

SCALE: N.T.S.



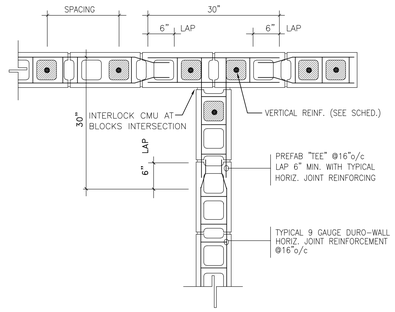
PLYWOOD NAILING DETAIL

SCALE: N.T.S.



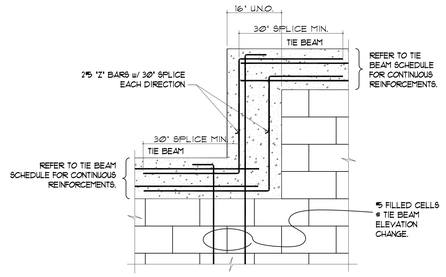
CORNER HORIZONTAL REINFORCING
CORNER VERTICAL REINFORCING

N.T.S.



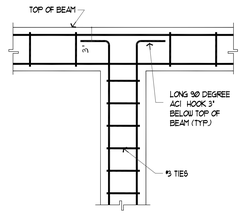
INTERSECTION HORIZONTAL REINFORCING
INTERSECTION & END VERTICAL REINFORCING

N.T.S.



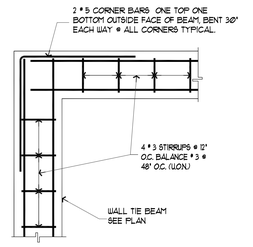
TYP. TIE BEAM ELEVATION CHANGE DETAIL

SCALE: N.T.S.



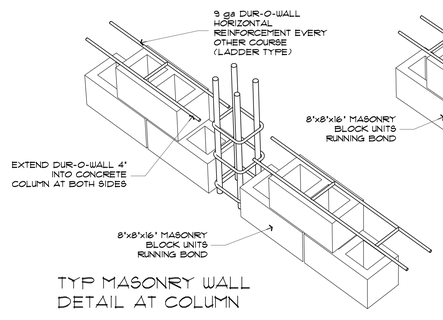
TYP. COLUMN/BEAM REINF. TERMINATION DETAIL

SCALE: N.T.S.

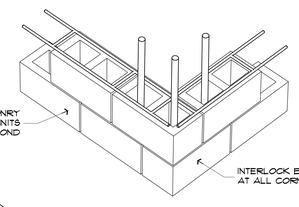


TYP. CORNER REINFORCING DETAIL FOR ALL TIE BEAMS

SCALE: N.T.S.



TYP MASONRY WALL DETAIL AT COLUMN



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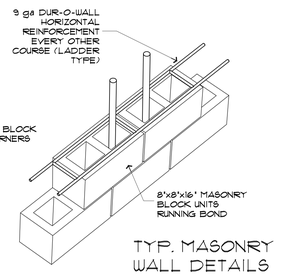

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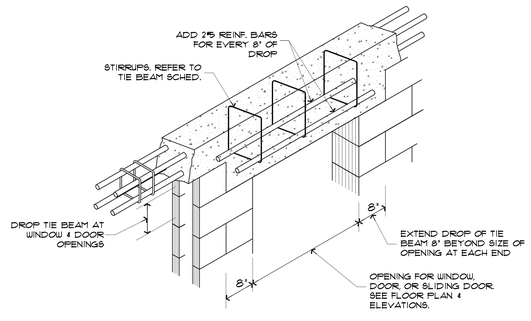

TYP DROP TIE BEAM DETAIL

SCOPE OF WORK FOR REVISION 3 :
OWNER DESIGN CHANGE REMOVING DOUBLE HEIGHT LIVING ROOM.

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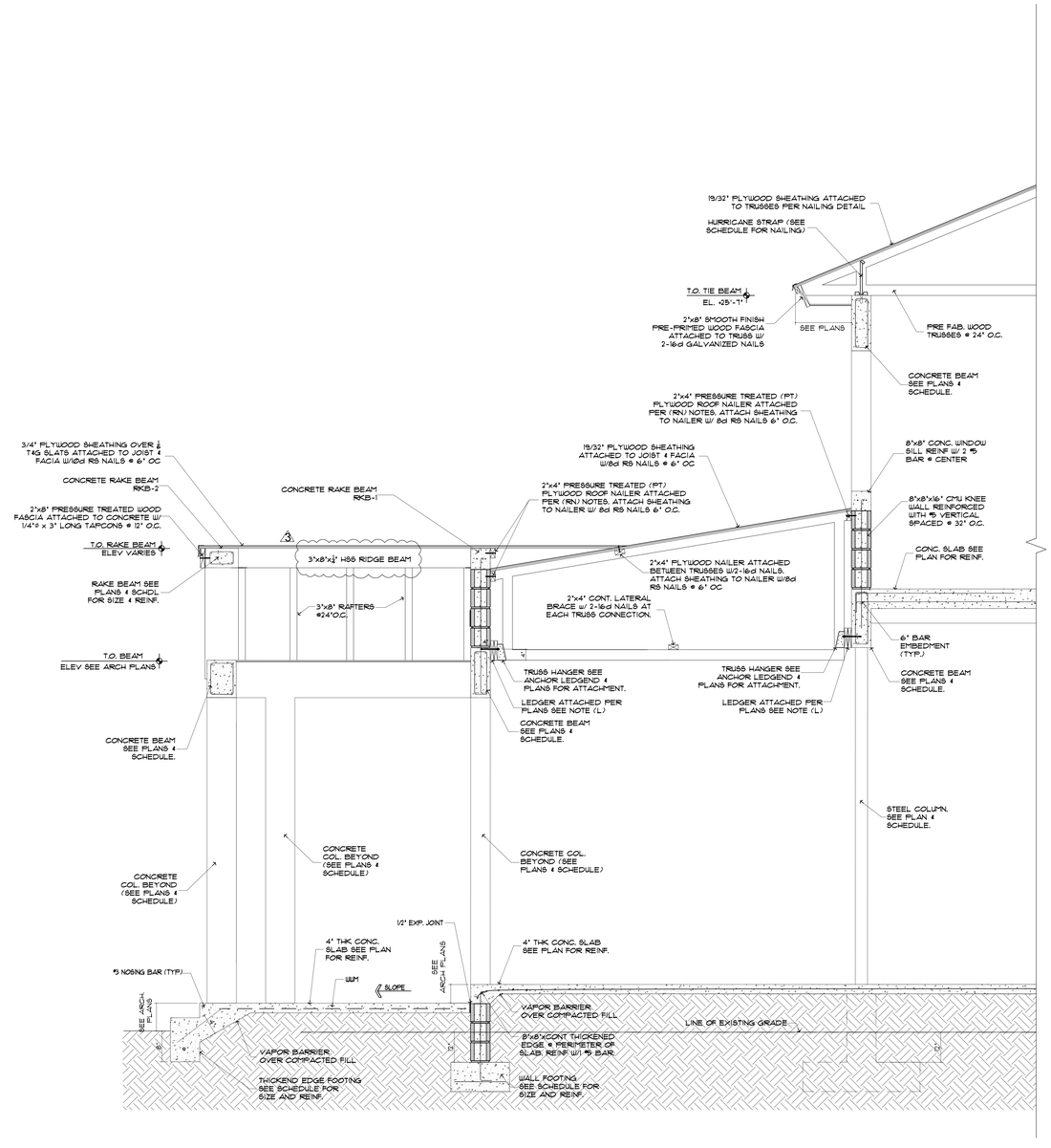
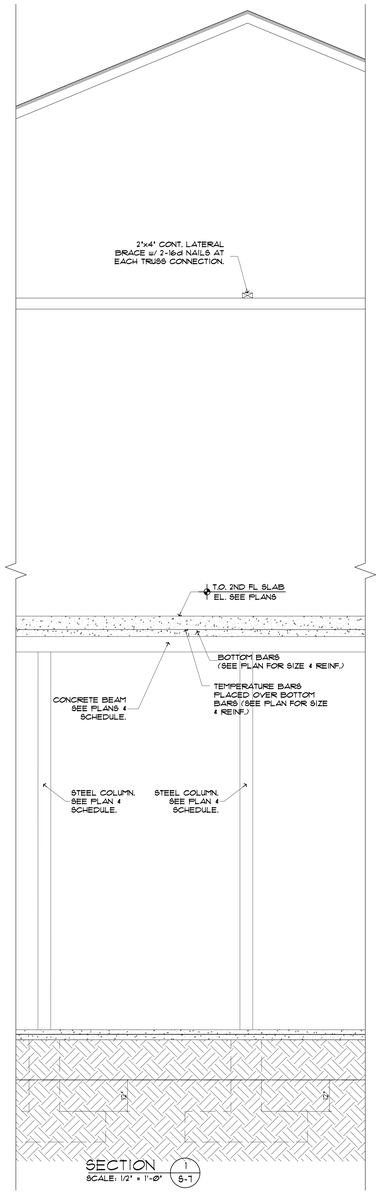
S-6





SECTION 1 / S-1
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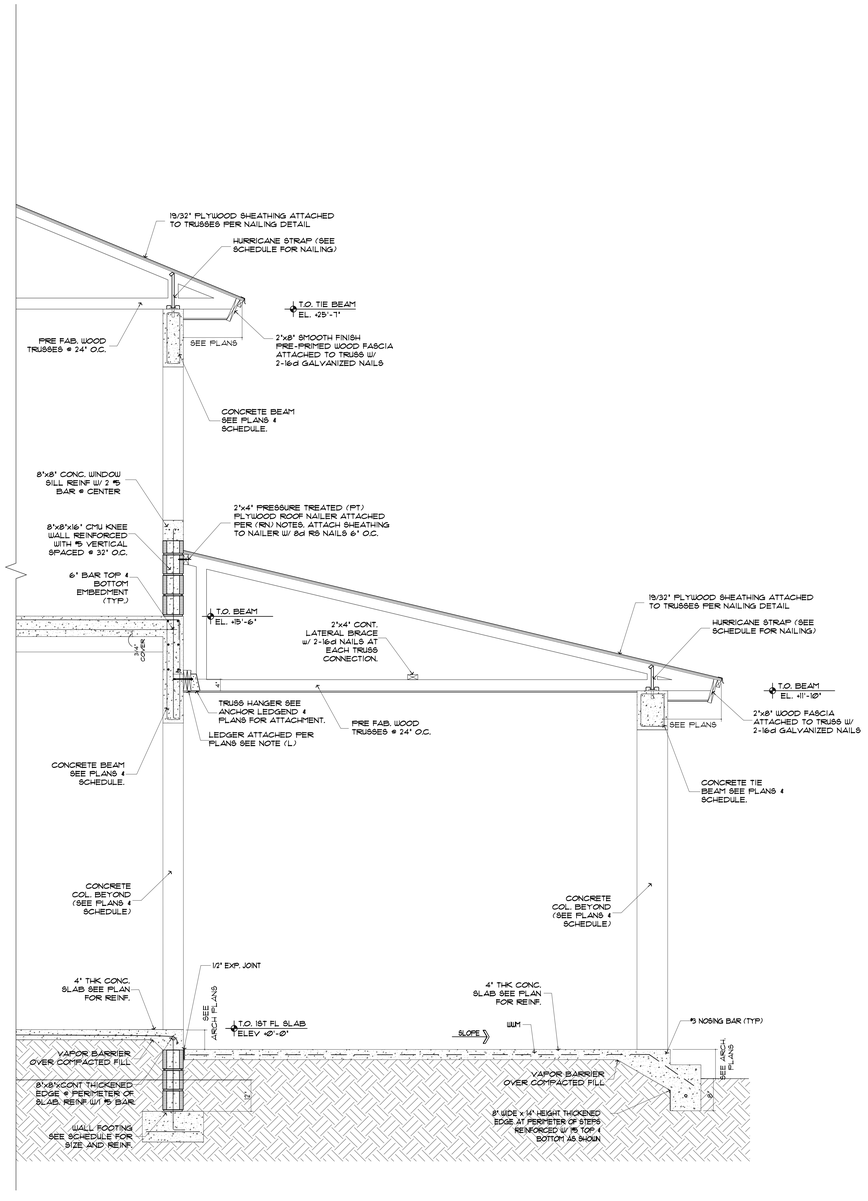

SCOPE OF WORK FOR REVISION 3 : OWNER DESIGN CHANGE REMOVING DOUBLE HEIGHT LIVING ROOM.

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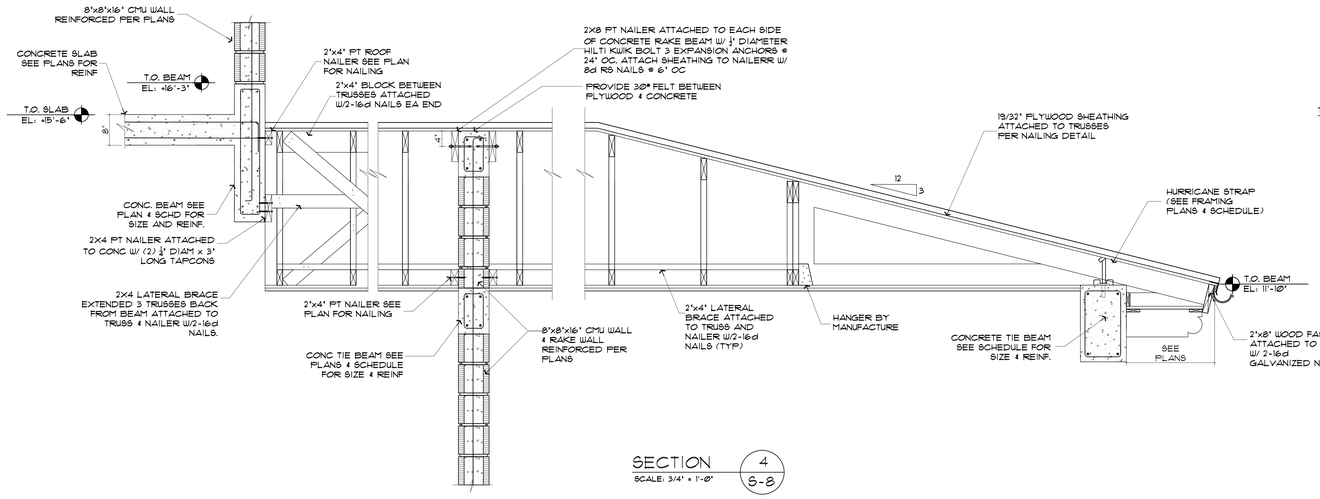
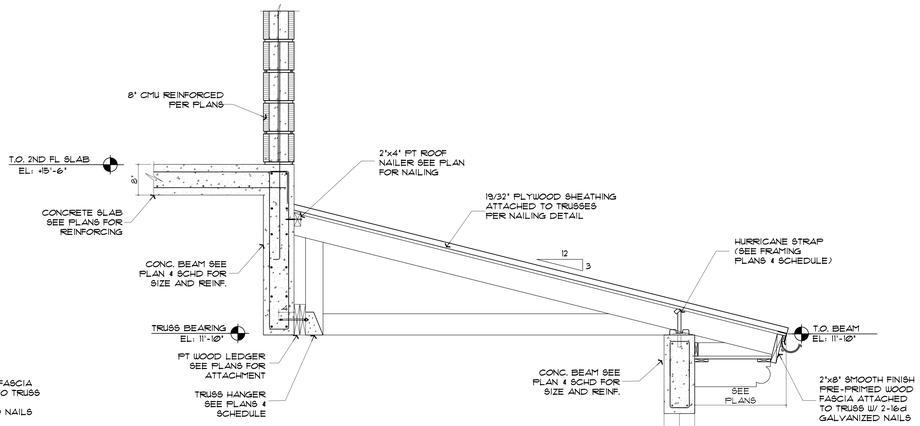
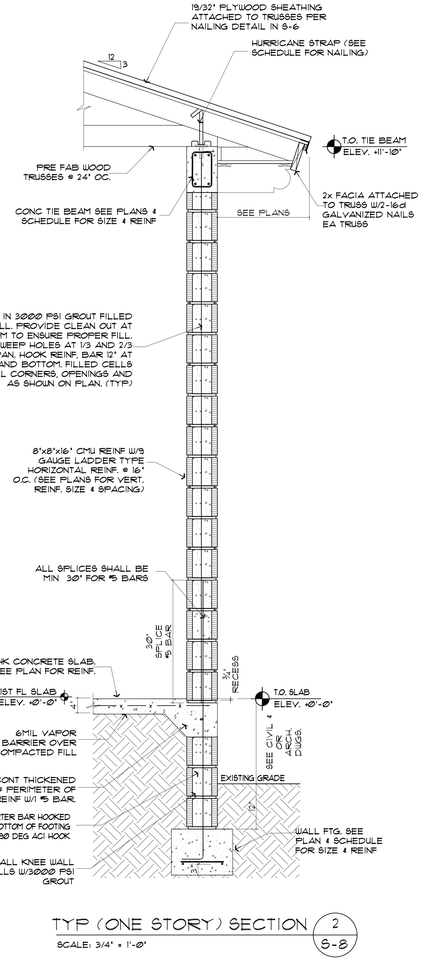
SHEET :

S-7



SECTION 4
SCALE: 3/4" = 1'-0" S-8





TYP (ONE STORY) SECTION 2
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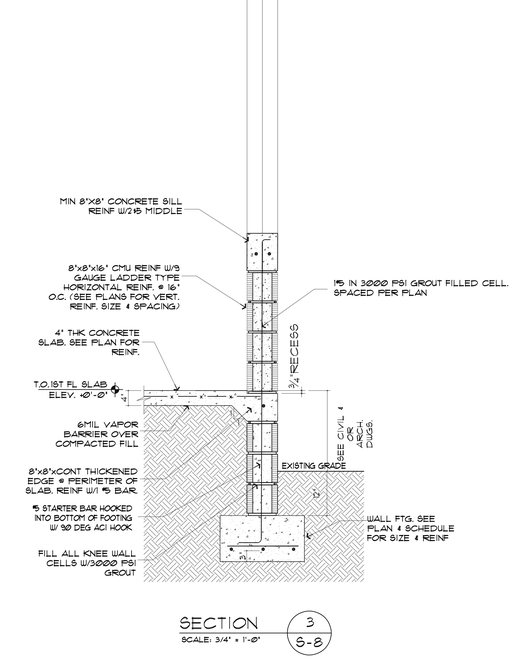

SECTION 3
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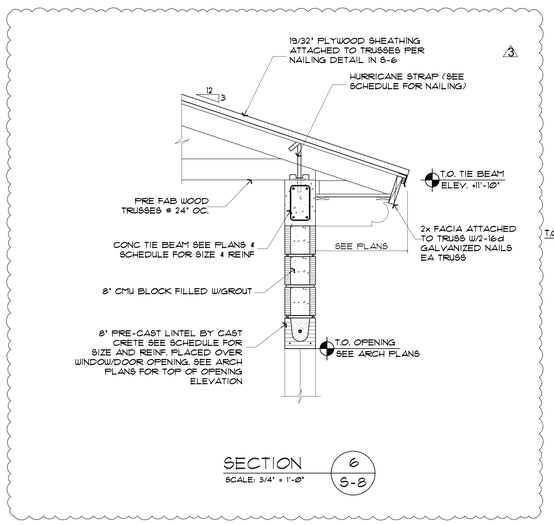

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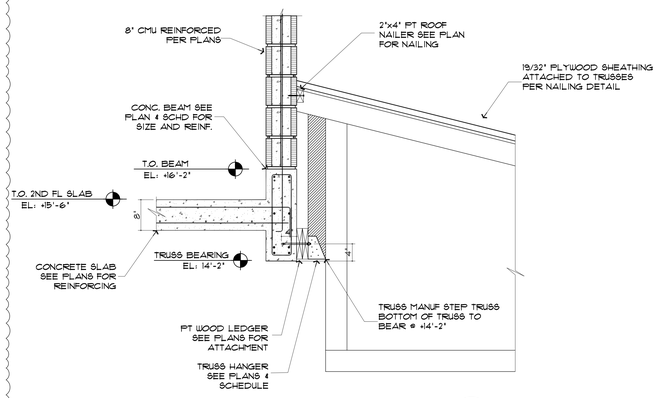
SCALE: 3/4" = 1'-0" S-8



SECTION 5
SCALE: 3/4" = 1'-0" S-8

SECTION 1/S-8 REMOVED

SCOPE OF WORK FOR REVISION 3 :
OWNER DESIGN CHANGE REMOVING DOUBLE HEIGHT LIVING ROOM.

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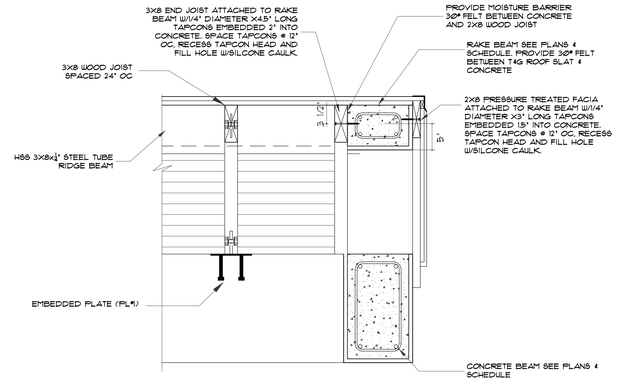
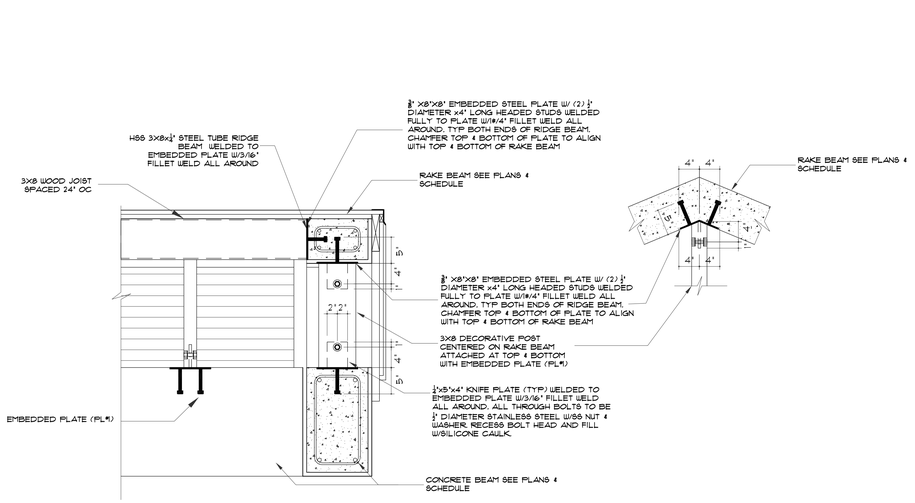


SECTION 3
SCALE: 1" = 1'-0"
S-8.1



SECTION 2
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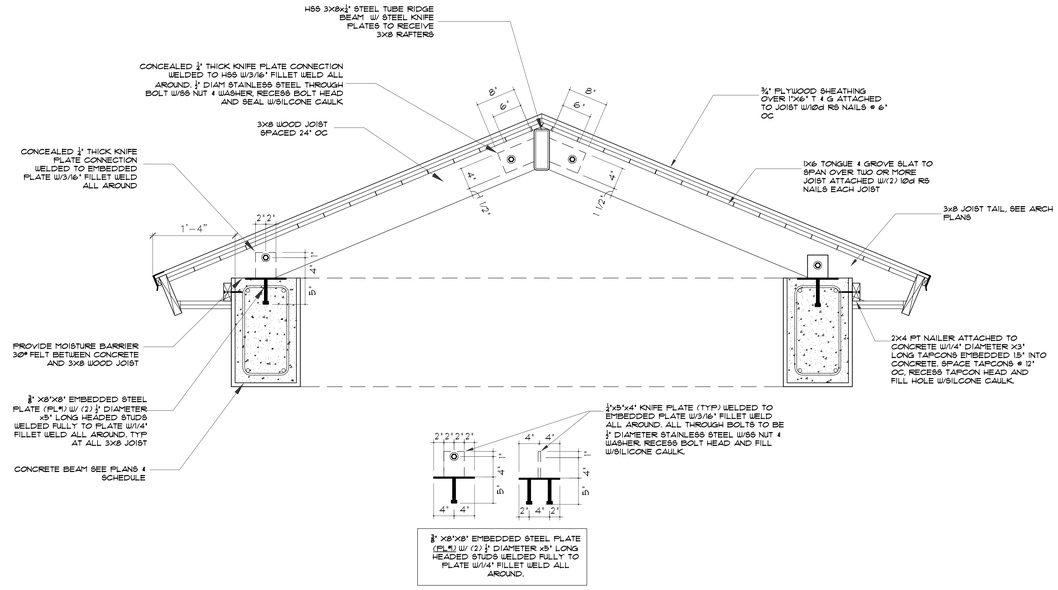
S-8.1



¾" X8"X8" EMBEDDED STEEL PLATE (PL*) W/ (2) ¾" DIAMETER X5" LONG HEADED STUDS WELDED FULLY TO PLATE W/1/4" FILLET WELD ALL AROUND.

SECTION 1
SCALE: 1" = 1'-0"
S-8.1

△3. SCOPE OF WORK FOR REVISION 3 : OWNER DESIGN CHANGE REMOVING DOUBLE HEIGHT LIVING ROOM.

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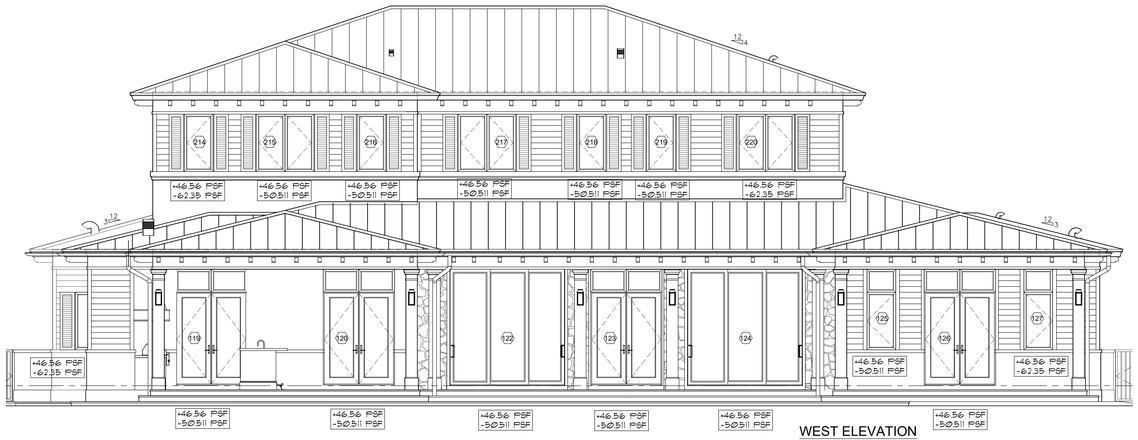
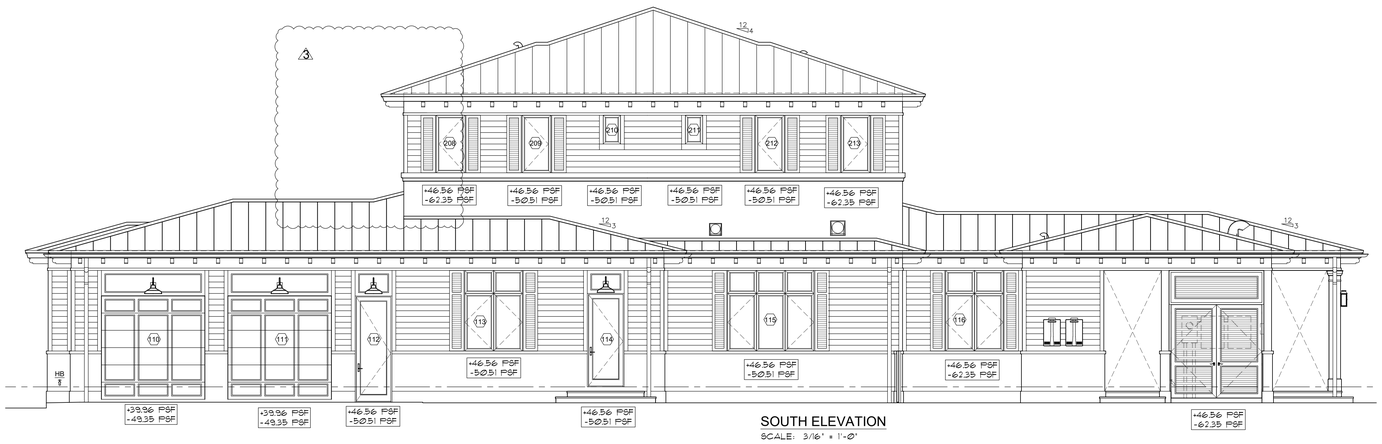
EAST ELEVATION

SCALE: 3/16" = 1'-0"



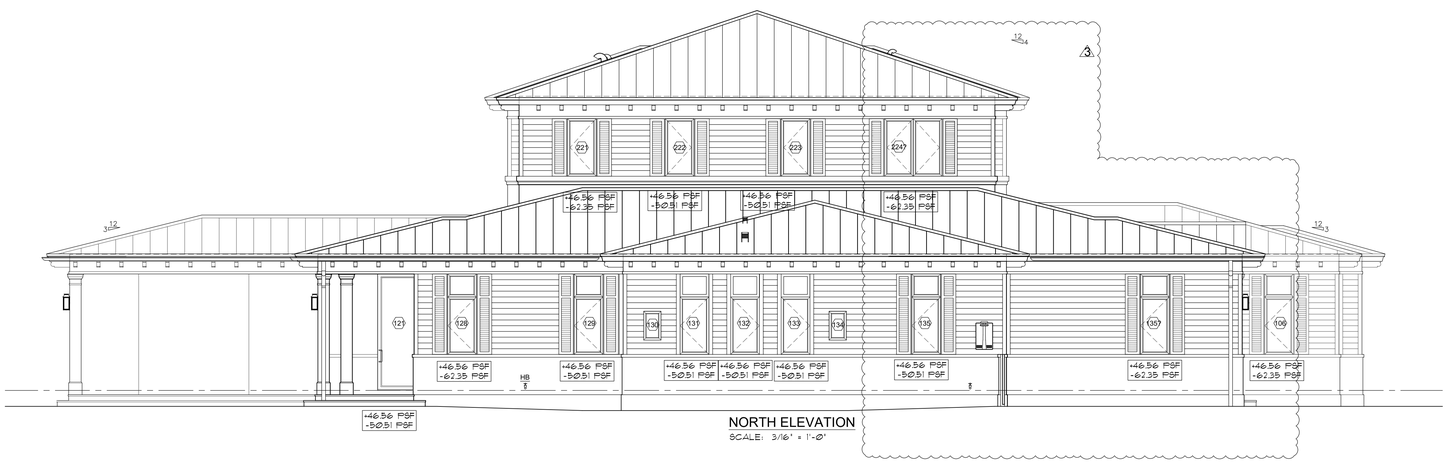
WEST ELEVATION

SCALE: 3/16" = 1'-0"



SOUTH ELEVATION

SCALE: 3/16" = 1'-0"



NORTH ELEVATION

SCALE: 3/16" = 1'-0"

3 SCOPE OF WORK FOR REVISION 3 : OWNER DESIGN CHANGE REMOVING DOUBLE HEIGHT LIVING ROOM.

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