

GENERAL NOTES

1) A SPECIAL INSPECTION BY A FLORIDA CERTIFIED ENGINEER OR SPECIAL INSPECTOR WILL BE REQUIRED FOR REINFORCED MASONRY CONSTRUCTION, SOIL COMPACTION, ROOF TRUSSES OVER 35' LONG OR 8'-0" HIGH & STRUCTURAL STEEL CONNECTIONS & FLE INSTALLATION.

2) THE USE OF A SCALE TO OBTAIN DIMENSIONS NOT SHOWN ON THE PLANS IS STRICTLY FORBIDDEN. THE ENGINEER WILL NOT BE RESPONSIBLE FOR ERRORS RESULTING FROM SUCH ACTIONS.

3) CONTRACTOR TO LOCATE ALL EXISTING UTILITIES PRIOR TO EXCAVATION.

4) ALL SPECIFIED MATERIALS AND CONNECTORS CAN BE SUBSTITUTED WITH EQUAL OR BETTER WITH THE APPROVAL OF ENGINEER OF RECORD.

5) PRIOR TO CONSTRUCTION PROVIDE TERMITES PROTECTION TREATMENT TO SOIL IN ACCORDANCE WITH NATIONAL PEST CONTROL ASSOCIATION' STANDARDS

POISON SOIL UNDER NEW STRUCTURE FOR WEEDS AND TERMITES AND IN ACCORDANCE WITH LOCAL CODES.

UPON COMPLETION OF THE APPLICATION OF THE TERMITE PROTECTIVE TREATMENT, A CERTIFICATE OF COMPLIANCE SHALL BE ISSUED TO THE BUILDING DEPARTMENT BY THE LICENSED PEST CONTROL COMPANY THAT CONTAINS THE FOLLOWING STATEMENT: "THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES."

TREATMENT IS IN ACCORDANCE WITH RULES AND LAWS ESTABLISHED BY THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES'.

6) IF CONFLICTS OCCUR IN OR BETWEEN ARCHITECTURAL AND ENGINEERING DOCUMENTS, BETWEEN DOCUMENTS AND FIELD CONDITIONS OR OTHERWISE, IMMEDIATELY CONTACT THE ENGINEER FOR CLARIFICATION AND DIRECTION BEFORE PROCEEDING. COORDINATE ALL DIMENSIONS BETWEEN ARCHITECTURAL AND STRUCTURAL DRAWINGS PRIOR TO PROCEEDING WITH THE WORK.

7) THE CONTRACTOR IS RESPONSIBLE FOR ALL METHODS, PROCEDURES AND SEQUENCES OF CONSTRUCTION PROVIDE APPROPRIATE SUPERVISION THROUGHOUT THE PROJECT. CONSTRUCTION SITE SAFETY, INCLUDING ALL ADEQUATE TEMPORARY BRACING AND SHORING, IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO EMPLOY THE NECESSARY PROFESSIONAL SERVICES TO DETERMINE THE NECESSARY METHODS AND SUPPORTS REGARDING FORMING AND CONSTRUCTION LOADS. TEMPORARY BRACING AND SHORING SHALL BE DESIGNED TO RESIST ALL CONSTRUCTION LOADS INCLUDING THE WEIGHTS OF ALL SUPPORTED MATERIALS PLUS A LIVE LOAD OF 50 PSF ON HORIZONTAL SURFACES. MAINTAIN TEMPORARY BRACING AND RETAIN N PLACE UNTIL PERMANENT.

WOOD NOTES

1. ROOF DIAPHRAGM SHALL COMPLY WITH THE DESIGN RECOMMENDATIONS OF "APA DESIGN / CONSTRUCTION GUIDE - DIAPHRAGMS" AND THE F.B.C.

2. FLYWOOD ROOF DECKING SHALL BE 19/32" MINIMUM THICKNESS AND SHALL BE CONTINUOUS OVER TWO OR MORE SPANS, WITH FACE GRAIN PERPENDICULAR TO THE SUPPORTS. (SEE TYP. FLYWOOD NAILING DETAIL FOR NAILING INFORMATION).

3. INSPECTIONS SHALL COMPLY WITH THE LOCAL BUILDING CODE REQUIREMENTS FOR INSPECTIONS (BY THE MUNICIPALITY, ARCHITECT OR ENGINEER) OF SPECIFIED COMPONENTS OF THE ROOF STRUCTURE REQUIRING INSPECTIONS.

4. ALL STRUCTURAL MEMBER TO BE SOUTHERN PINE No. 2 OR BETTER, MINIMUM BENDING STRESS:

2x4 Fb= 1800 Psi	E= 14000000 Psi	Fv= 175 Psi
2x6 Fb= 1800 Psi	E= 14000000 Psi	Fv= 175 Psi
2x8 Fb= 925 Psi	E= 14000000 Psi	Fv= 175 Psi
2x10 Fb= 800 Psi	E= 14000000 Psi	Fv= 175 Psi
2x12 Fb= 750 Psi	E= 14000000 Psi	Fv= 175 Psi

MASONRY NOTES

1) PROVIDE CLEANOUT OPENING AT THE BOTTOM OF REINFORCED MASONRY CELLS THAT ARE BEING FILLED TO ENSURE THAT THE CELL IS FILLED. PROVIDE WEEP HOLES AT 1/3 AND 2/3 OF SPAN.

2) ALL BLOCK MASONRY WALLS SHALL HAVE 5 GAUGE HORIZONTAL REINFORCEMENT " DUR-O-WALL LADDER TYPE SPACED 8" ON CENTER. MASONRY UNITS SHALL BE 7 CELL HOLLOW UNITS CONFORMING TO ASTM C-90 WITH COMPRESSIVE BE STRENGTH OF 1913000 PSI ON THE NET CROSS-SECTIONAL AREA AND SHALL BE LAID IN RUNNING BOND.

3) MORTAR SHALL COMPLY WITH ASTM 710 MORTAR TYPE S-13000 PSI

4) WHERE ANCHOR BOLTS ARE SET IN MASONRY WALL, FILL TWO BLOCK CELLS WITH GROUT & ANCHOR BOLT LOCATIONS

5) GROUT USED IN THE WORK SHALL CONFORM TO ASTM C416 WITH A SLUMP MIX OF 8" TO 11". PROVIDE CLEANOUT AND INSPECTION HOLES AT FILLED CELLS AT BOTTOM COURSE. CONTRACTOR TO CONSOLIDATE GROUT LIFTS WITH 3/4" VIBRATOR

6) POUR GROUT IN LIFTS NOT TO EXCEED 4'

7) GROUT USED IN THE WORK SHALL OBTAIN A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI IN 28 DAYS. GROUT SHALL COMPLY WITH ASTM C416.

DESIGN LOADS

WIND DESIGN PER ASCE 7-12 (ALLOWABLE STRESS DESIGN)

MEAN ROOF HEIGHT _____ 35'-0"

WIND DESIGN SPEED _____ 175 MPH

EXPOSURE CATEGORY _____ C

GZ= _____ 1'-0" B

Kd _____ 0.85

STRUCTURAL CATEGORY _____ II

ROOF DESIGN LOADS- PRE EXIST WOOD TRUSSES

SUPER IMPOSED DEAD LOAD _____ 25 PSF

LIVE LOAD _____ 20 PSF

FLOOR DESIGN LOADS:

DEAD LOAD BY SLABS _____ 100 PSF

SUPER IMPOSED DEAD LOAD (FLOORS) _____ 30 PSF

LIVE LOAD _____ 40 PSF

STAIR LIVE LOAD: _____ 40 PSF (RESIDENTIAL)

CONCRETE NOTES

1) CONCRETE SHALL CONFORM WITH ACI-318 IT SHALL REACH A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI # 28 DAYS.

2) MAXIMUM PERMISSIBLE SLUMP IS 5'-6" IN STRUCTURAL CONCRETE WITH THE EXCEPTION BEING SAND/CEMENT GROUT.

3) AGGREGATES SHALL BE CLEAN AND GRADED MAXIMUM SIZES 3/4"

4) NO ADMIXTURES PERMITTED WITHOUT WRITTEN APPROVAL OF ENGINEER

5) CONCRETE TESTING IS REQUIRED AS FOLLOWS: 1 SET OF 5 CYLINDERS FOR EVERY 50 CU YDS. OF CONCRETE

6) CLEAR COVER FOR REINFORCING BARS SHALL BE:

GRADE BEAMS/PILE CAPS... 3"	UNFORMED FACES ... 3"	BALCONY SLABS ... 1-1/2"
BEAMS/COLUMNS ... 1-1/2"	FORMED FACES ... 2"	INT. ELEVATED SLABS ... 3/4"

CONTACT WEATHER... 2"

REINFORCING STEEL NOTES

1) REINFORCING STEEL (fy = 60,000 P.S.I.; fs = 24,000 P.S.I.; Es = 29,000,000 P.S.I.; Ew = 1350,000 P.S.I.; N = EX-18 # 248)

2) REINFORCING STEEL SHALL CONFORM TO ASTM A-618 GRADE 60. REINFORCING STEEL SHALL BE DETAILED AND FABRICATED ACCORDING TO THE "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES".

3) THE BEAMS & FOOTINGS SHALL BE CONTINUOUS REINFORCEMENT PROVIDED BY LAPPING 9PLACES NOT LESS THAN 30" CONTINUITY SHALL BE PROVIDED AT ALL CORNERS BY BENDING 2 BARS FROM EACH DIRECTION AROUND THE CORNER 30" (SAME SIZE AND QUANTITY).

4) EXTEND BEAM BOTTOM BARS 8" PAST ALL OPENINGS GREATER THAN 3'-0" ON BOTH SIDES.

5) CONTINUITY AT COLUMNS SHALL BE PROVIDED BY CONTINUING HORIZONTAL REBARS THRU COLUMNS OR BY BENDING HORIZONTAL REIN. INTO COLUMNS A DISTANCE OF 30".

6) HOOK TOP OF VERTICAL BARS IN ALL TERMINATING COLUMNS 3" BELOW TOP OF SLAB AND 3" BELOW TOP OF THE TIE BEAM.

STRUCTURAL STEEL

1. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING GRADES:

A. ALL ANGLE BASE PLATES CONN. PLATES (AISC)---ASTM A36 (Fy=36 KSI)

B. STRUCTURAL TUBE-----ASTM A500 GRADE B (Fy=48 KSI)

C. STEEL PIPE-----ASTM A53, GRADE B (Fy=35 KSI)

2. GROUT FOR COLUMN BASE PLATES SHALL BE NON-METALLIC, NON SHRINK GROUT SUCH AS MASTER FLOU 70, AS MANUFACTURED BY MASTER BUILDERS OR APPROVED EQUAL.

3. ALL WELDING SHALL CONFORM TO THE REQUIREMENTS OF THE STANDARD CODE FOR WELDING IN BUILDING CONSTRUCTION OF THE AMERICAN WELDING SOCIETY. ALL WELDING SHALL BE PERFORMED BY CERTIFIED WELDERS USING E70-X8 ELECTRODES.

SHOP DRAWINGS

PRIOR TO ANY FABRICATION, SHOP DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW. SHOP DRAWINGS ARE REQUIRED, BUT NOT LIMITED TO FOR THE FOLLOWING:

1. WOOD TRUSSES	4. STEEL STAIRS/HAND RAILS
2. STRUCTURAL STEEL	5. RAILINGS/GUARDRAILS
3. SHORING	

STRUCTURAL DESIGN CRITERIA

1) THE DESIGN COMPLIES WITH THE REQUIREMENTS OF THE F.B.C. AND OTHER REFERENCED CODES AND SPECIFICATIONS. ALL CODES AND SPECIFICATIONS SHALL BE LATEST EDITION AT TIME OF PERMIT.

A. FLORIDA BUILDING CODE 2023 8TH EDITION

E. NDS 109 & 509/518 2017 FOR WOOD DESIGN & ANALYSIS

B. ACI 318-19 FOR REINFORCED CONCRETE

F. AISC 360-16 CONSTRUCTION MANUAL FOR STEEL DESIGN

C. TMS 402/602-2006 FOR REINFORCED MASONRY

G. ADM-2010 FOR ALUMINUM DESIGN

D. ASCE 7-12 FOR WIND ANALYSIS AND DESIGN

SOIL STATEMENT

AS PER GEOTECHNICAL REPORT BY N/S DATED AUGUST 26, 2024 (PROJECT NUMBER 05993) THIS FOUNDATION HAS BEEN DESIGNED AS A SHALLOW FOUNDATION WITH AN ALLOWABLE SOIL BEARING CAPACITY OF 3000 PSF. CONTRACTOR REFER TO THE GEOTECHNICAL REPORT FOR SITE PREPARATION AND ALL OTHER PERTINENT DATA.

SHORING

N. FORMWORK, SHORING & RESHORING

1. PROVIDE SHORING AND RESHORING DRAWINGS PREPARED BY OR UNDER THE DIRECT SUPERVISION OF A SPECIALTY ENGINEER AND CONFORMING TO THE REQUIREMENTS OF SPECIFICATION SECTION 05100 "CONCRETE FORMWORK".

2. THE DESIGN OF FORMS AND SHORING FOR HORIZONTAL CONCRETE MEMBERS SHALL BE FOR NO LESS THAN DEAD LOAD PLUS 50 PSF CONSTRUCTION LOAD AND FOR THE CUMULATIVE LOADS OF THE SUPPORTED FLOOR. THE DESIGN SHALL INCLUDE A SAFETY FACTOR OF 3 FOR WOOD MEMBERS AND 2 FOR METAL.

3. NO CONSTRUCTION LOAD SHALL BE APPLIED TO ANY MEMBER UNTIL THE CONCRETE HAS ACHIEVED A MINIMUM OF 75% OF THE SPECIFIED 28-DAY STRENGTH.

4. THE MAXIMUM SUPERIMPOSED CONSTRUCTION LOAD TO BE APPLIED ON FLOORS SUPPORTED BY SHORING OR RESHORING SHALL NOT EXCEED 75% OF THE LIVE LOAD SPECIFIED FOR SLABS AND JOISTS. BEAMS SHALL NOT BE LOADED WITH MORE THAN 60% OF THE LIVE LOAD SPECIFIED FOR BEAMS.

5. REMOVAL OF FORMWORK IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR. REMOVE FORMS IN SUCH A MANNER TO ENSURE JOB SAFETY AND TO PREVENT EXCESSIVE DEFLECTION OR DAMAGE TO THE STRUCTURE. THE FOLLOWING ARE MINIMUM GUIDELINES FOR THE REMOVAL OF HORIZONTAL FORMWORK:

6. DO NOT REMOVE POST SHORING UNTIL CONCRETE HAS REACHED 75% OF THE SPECIFIED 28-DAY STRENGTH, BUT NO LESS THAN 3000 PSI. AS A MINIMUM, SHORING AND RESHORING SHALL REMAIN IN PLACE AS FOLLOWS (TIME MEASURED FROM COMPLETION OF CONCRETE PLACEMENT).

SLABS, SPANS GREATER THAN 10'-0"	SHORING	RESHORING
JOISTS	1 DAYS	1 DAYS
BEAMS	1 DAYS	14 DAYS
CANTILEVERS		
SLABS	1 DAYS	14 DAYS
JOISTS	1 DAYS	14 DAYS
BEAMS	1 DAYS	14 DAYS

9. THE SHORING AND RESHORING IS TO BE INSPECTED BY THE SPECIALTY ENGINEER OR HIS AUTHORIZED REPRESENTATIVE. PRIOR TO EACH CONCRETE PLACEMENT HE SHALL SUBMIT A WRITTEN INSPECTION REPORT TO THE SPECIAL INSPECTOR AND TO THE CONTRACTOR STATING THAT THE WORK IS IN GENERAL COMPLIANCE WITH THE SHORING AND RESHORING DRAWINGS. THE FIELD REPORTS SHALL BE SIGNED BY THE INDIVIDUAL CONDUCTING THE INSPECTION. COPIES OF THE FIELD REPORTS SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER, THE SPECIAL INSPECTOR AND THE BUILDING OFFICIAL UNDER A COVER LETTER SIGNED, SEALED AND DATED BY THE SPECIALTY ENGINEER ON A SEMI-MONTHLY BASIS.

ABBREVIATIONS

AB - ANCHOR BOLT	L - ANGLE
ALT - ALTERNATE	LG - LONG
APPROX - APPROXIMATELY	LP - LOW POINT
ARCH - ARCHITECTURAL	LLV - LONG LEG VERTICAL
B - BOTTOM	LLH - LONG LEG HORIZONTAL
BLDG - BUILDING	MFR - MANUFACTURER
BH - BEAM	MAS - MASONRY
BOIT - BOTTOM	MO - MASONRY OPENING
BRG - BEARING	MATL - MATERIAL
BRDG - BRIDGING	MAX - MAXIMUM
CB - CONCRETE BEAM	MB - MASONRY BEAM
C/C - CENTER TO CENTER	MECH - MECHANICAL
CH - CHANNEL	MEP - MECHANICAL/ELECTRICAL/PLUMBING
CIP - CAST IN PLACE	MF - MONOLITHIC FOUR
CJ - CONTROL JOINT	MTL - METAL
CL - CENTERLINE	MIN - MINIMUM
CLR - CLEAR	MISC - MISCELLANEOUS
CMU - CONCRETE MASONRY UNIT	NS - NEAR SIDE
COL - COLUMN	NIC - NOT IN CONTRACT
CONC - CONCRETE	NTS - NOT TO SCALE
CONF - CONFIGURATION	O/C - ON CENTER
CONT - CONTINUOUS	OPNG - OPENING
CONTR - CONTRACTOR	PART - PARTITION
CORR - CORRUGATED	PC - FILE CAP
CTR - CENTER	PL - PLATE
DBL - DOUBLE	PLF - POUNDS PER LINEAR FOOT
DET - DETAIL	PSF - POUNDS PER SQUARE FOOT
DIA - DIAMETER	PPI - POUNDS PER INCH
DIM - DIMENSION	PT - POST TENSION/PRESSURE TREATED
DOIN - DOIN	R - RISER
DR - DOOR	REIN - REINFORCED
DWG - DRAWING	REIM - REHANDER
EA - EACH	REQD - REQUIRED
EF - EACH FACE	REV - REVISED/REVISION
EJ - EXPANSION JOINT	RM - ROOM
ELEV - ELEVATION	RO - ROUGH OPENING
ELECT - ELECTRICAL	REQT - REQUIREMENT
ELEV - ELEVATOR	SCHED - SCHEDULE
EOB - EDGE OF SLAB	SECT - SECTION
EQ - EQUAL	SIM - SIMILAR
EW - EACH WAY	SJ - SAUCUT JOINT
EXIST - EXISTING	SL - SLOPE
EXP - EXPANSION	SQ - SQUARE
FE - FORCE	SP - SPIRAL
Fin - FINISH	SS - STAINLESS STEEL
FLR - FLOOR	STD - STANDARD
FND - FOUNDATION	SW - SHEARWALL/SHORT WAY
FCM - FACE OF MASONRY	STL - STEEL
FS - FAR SIDE	STRUCT- STRUCTURAL
FT - FOOT	SYMT - SYMMETRICAL
FTG - FOOTING	TB - TIE BEAM
GA - GAGE	TC - TIE COLUMN
GALV - GALVANIZED	TE - THICKENED EDGE
HC - HOLLOW CORE	THK - THICK
HDG - HOT DIPPERD GALVANIZED	T/O - THRU OUT
HORIZ - HORIZONTAL	TOC - TOP OF CONCRETE
HP - HIGH POINT	T - TOP
IJ - ISOLATION JOINT	TEMP - TEMPERATURE
INFO - INFORMATION	TOS - TOP OF STEEL/SLAB
INT - INTERIOR	TR - TREAD
JOIST - JOIST	TYP - TYPICAL
J - JOINT	UNO - UNLESS NOTED OTHERWISE
K - KIPS	VERT - VERTICAL
K/FT - KIPS PER FOOT	W - WITH
K - CONST JOINT/KEYSTONE JOIST	WD - WOOD
	WUF - WELDED WIRE FABRIC

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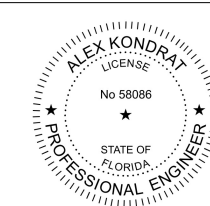
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DRAWING INFORMATION :

PERMIT SET 4.25.25

SHEET :

S-0

FOOTING SCHEDULE

MARK	FOOTING TYPE	WIDTH	LENGTH	DEPTH	REINFORCEMENT				REMARKS
					A	B			
TE8	A	8'	CONT.	8'	(1) #5	---			THICKENED EDGE
TE8C	C	8'	CONT.	8'	(1) #5	---			THICKENED EDGE
TE8G	G	8'	CONT.	12'	(2) #5	---			THICKENED EDGE
TE16	B	16'	CONT.	12'	(2) #5	---			THICKENED EDGE
WF36	E	36'	CONT.	12'	5 #12'cc SW 5 #12'cc LW				STAIR MONO FOOTING
WF3	F	16'	CONT.	12'	5 #12'cc SW 2 #5 LW				WALL FOOTING
WF4	F	24'	CONT.	12'	5 #12'cc SW 5 #12'cc LW				WALL FOOTING
WF48	D	48'	CONT.	12'	5 #12'cc SW 5 #12'cc LW				WALL FOOTING
F-1	D	36'	36'	12'	(3) #5	(3) #5			PAAD FOOTING
F-2	D	48'	48'	12'	(4) #5	(4) #5			PAAD FOOTING
F-3	D	60'	60'	12'	(5) #5	(5) #5			PAAD FOOTING
F-4	D	12'	12'	12'	(6) #5	(6) #5			PAAD FOOTING
F-5	H	60'	64'	12'	(6) #5 LW (7) #5 SW				PAAD FOOTING
F-6	H	48'	60'	12'	(4) #5 LW (6) #5 SW				PAAD FOOTING

FOOTING TYPES:

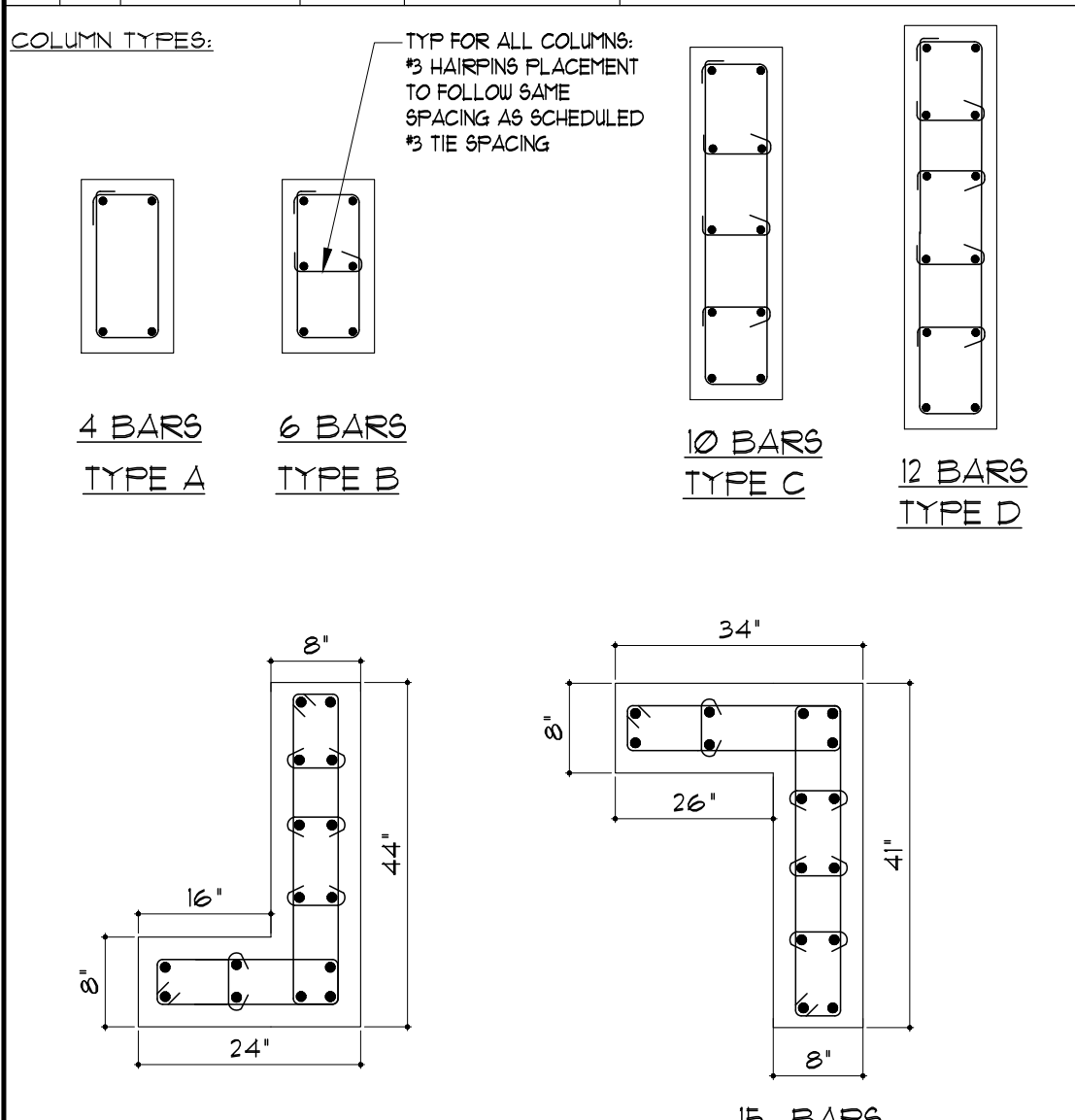
SW=SHORT WAY
LW=LONG WAY

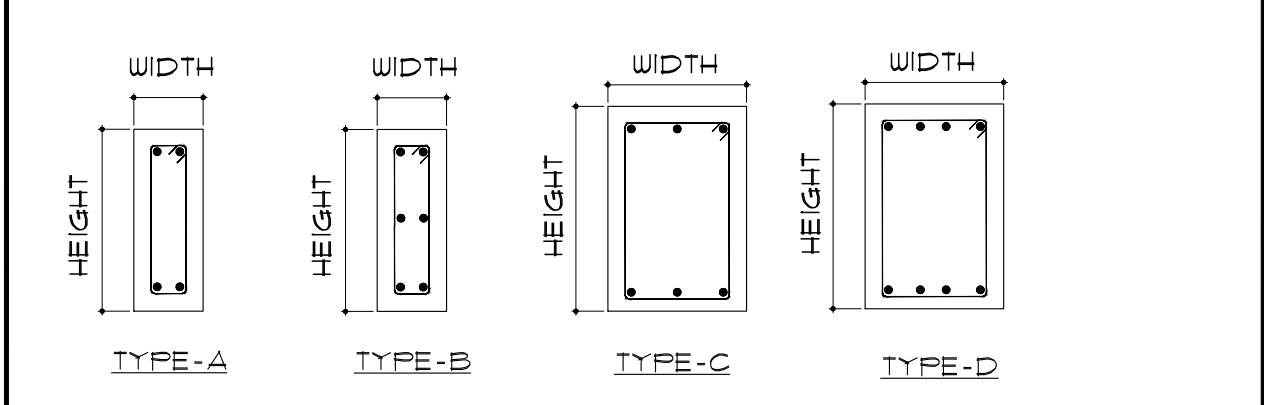
SEE PLANS
FOR STEP

The diagrams illustrate the cross-sections and reinforcement for different footing types:

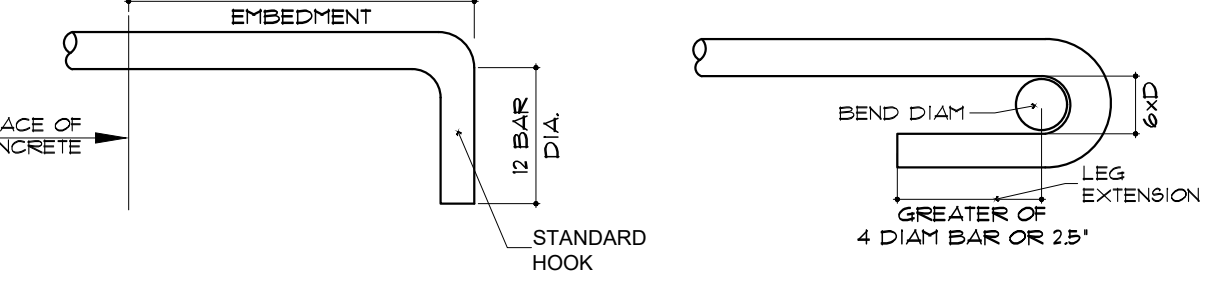
- TYPE-A:** A simple rectangular footing with reinforcement bars A and B.
- TYPE-B:** A footing with a central step, showing reinforcement bars A and B.
- TYPE-C:** A footing with a central step, showing reinforcement bars A and B.
- TYPE-D:** A footing with a central step, showing reinforcement bars A and B.
- TYPE-E:** A footing with a central step, showing reinforcement bars A and B.
- TYPE-F:** A footing with a central step, showing reinforcement bars A and B.
- TYPE-G:** A footing with a central step, showing reinforcement bars A and B.

NOTE: PROVIDE 1# CORNER BARS FOR WF3 AND 3# CORNER BARS FOR WF4 BENT 30° EACH WAY. # ALL CONTINUOUS WALL FOOTING CORNERS.

COLUMN SCHEDULE									
MARK	COLUMN TYPE	DIMENSIONS		REINFORCEMENT					REMARKS
				VERTICAL	HORIZONTAL (1# TIES)				
C1	A	8' x 12'		4-#5	3 # 8' O.C.				CONCRETE COLUMN
C2	B	8' x 24'		6-#5	3 # 8' O.C.				CONCRETE COLUMN
C3	B	8' x 21'		6-#5	3 # 8' O.C.				CONCRETE COLUMN
C4	B	8' x 23'		6-#5	3 # 8' O.C.				CONCRETE COLUMN
C5	A	8' x 16'		4-#5	3 # 8' O.C.				CONCRETE COLUMN
C6	A	8' x 11'		4-#1	3 # 8' O.C.				CONCRETE COLUMN
C7	E	AS SHOWN		15-#5	3 # 8' O.C.				CONCRETE COLUMN
C8	F	AS SHOWN		15-#5	3 # 8' O.C.				CONCRETE COLUMN
C9	C	8' x 48'		10-#5	3 # 8' O.C.				CONCRETE COLUMN
C10	D	8' x 51'		12-#5	3 # 8' O.C.				CONCRETE COLUMN
C11	G	12' x 12'		4-#5	3 # 8' O.C.				CONCRETE COLUMN
C12	A	8' x 18'		4-#5	3 # 8' O.C.				CONCRETE COLUMN
C13	B	8' x 20'		6-#5	3 # 8' O.C.				CONCRETE COLUMN
MC	H	12'x12'		4-#5	3 # 8' O.C.				MASONRY HOLLOW BLOCK COLUMN
SC1	I	4'x4'x3'8"		---	---				1/4" TUBE STEEL COLUMN FOR BASE DETAIL SEE 1 & 2/8-4
SC2	I	4'x4'x12"		---	---				FOR CAP DETAIL SEE 1/8-5
COLUMN TYPES:									
									
TYP FOR ALL COLUMNS: 15 WARPING PLACEMENT TO FOLLOW SAME SPACING AS SCHEDULED 15 TIE SPACING									
CONCRETE COLUMN NOTES: 1. L-SHAPE COLUMNS TO HAVE TIES IN EACH DIRECTION SPACED PER SCHEDULE.									

BEAM SCHEDULE									
MARK	BEAM TYPE	TOP OF BEAM ELEVATION (AS SHOWN)	WIDTH	HEIGHT	REINFORCEMENT			# 3 TIES	REMARKS
					TOP	MIDDLE	BOTTOM		
TB-1	A	11'-10"	8'	12'	2-#5	-	2-#5	3 # 48" O.C.	TIE BEAM
TB-2	A	14'-6"	8'	12'	2-#5	-	2-#5	3 # 48" O.C.	TIE BEAM
RKB-1	A	VARIES	8'	12'	2-#5	-	2-#5	3 # 48" O.C.	RAKE BEAM
RKB-2	A	VARIES	12'	8'	2-#5	-	2-#5	3 # 48" O.C.	RAKE BEAM
IB-1	F	11'-10"	8'	41.5"	2-#5	8-#5	2-#5	3 # 12" O.C.	CONCRETE BEAM
IB-2	A	11'-10"	8'	20.5"	2-#5	-	2-#5	3 # 10" O.C.	CONCRETE BEAM
IB-3	E	11'-10"	8'	51"	2-#5	8-#5	2-#5	3 # 12" O.C.	CONCRETE BEAM
IB-4	A	11'-10"	12'	20.5"	2-#5	-	2-#1	3 # 8" O.C.	CONCRETE BEAM
IB-5	A	11'-10"	12'	20.5"	2-#5	-	2-#1	3 # 8" O.C.	CONCRETE BEAM
IB-6	D	11'-10"	12'	20.5"	4-#5	-	4-#5	3 # 8" O.C.	CONCRETE BEAM
IB-7	A	11'-10"	12'	20.5"	2-#5	-	2-#5	3 # 10" O.C.	CONCRETE BEAM
ZB-1	F	14'-6"	8'	52.5"	2-#5	8-#5	2-#5	3 # 12" O.C.	CONCRETE BEAM
ZB-2	D	14'-6"	8'	32"	2-#1	4-#5	2-#1	3 # 10" O.C.	CONCRETE BEAM
ZB-3	E	14'-6"	8'	36"	2-#5	4-#5	4-#1	3 # 12" O.C.	CONCRETE BEAM
ZB-4	D	14'-6"	16'	16"	4-#1	-	4-#1	3 # 6" O.C.	CONCRETE BEAM
ZB-5	A	14'-6"	8'	16"	2-#5	-	2-#5	3 # 11" O.C.	CONCRETE BEAM
ZB-6	D	14'-6"	12'	14"	4-#5	-	4-#5	3 # 8" O.C.	CONCRETE BEAM
ZB-7	D	14'-6"	12'	14"	4-#5	-	4-#5	3 # 8" O.C.	CONCRETE BEAM
ZB-8	A	14'-6"	8'	16"	2-#1	-	2-#1	3 # 11" O.C.	CONCRETE BEAM
ZB-9	D	14'-6"	12'	14"	4-#5	-	4-#5	3 # 8" O.C.	CONCRETE BEAM
ZB-10	A	14'-6"	8'	14"	2-#5	-	2-#5	3 # 6" O.C.	CONCRETE BEAM
ZB-11	A	14'-6"	8'	18"	2-#5	-	2-#5	3 # 8" O.C.	CONCRETE BEAM
ZB-12	F	14'-6"	8'	52.5"	2-#5	8-#5	2-#5	3 # 12" O.C.	CONCRETE BEAM
ZB-13	G	14'-6"	8'	32"	2-#5	2-#5	4-#1	3 # 12" O.C.	CONCRETE BEAM
RTB-1	A	24'-1"	8'	12'	2-#5	-	2-#5	3 # 48" O.C.	CONCRETE BEAM
RB-1	A	23'-6"	8'	22.5"	2-#5	2-#5	2-#5	3 # 12" O.C.	CONCRETE BEAM
BEAM TYPES:									
									
CONCRETE BEAM NOTES: 1. TIE BEAMS SHALL HAVE (4) # 3 TIES AT 12" O.C. AT ALL CORNERS EA. WAY AND 48" O.C. REMAINING. 2. PROVIDE (2) # 5 x 60" CORNER BARS, BENT 30° EACH WAY. # ALL CORNERS. 3. PROVIDE 1" MIN. CLEAR SPACING BETWEEN BARS.									

TENSION EMBEDMENT LENGTH (with std. hook) SCHEDULE													
BAR SIZE	f'c = 3000 psi		f'c = 4000 psi		f'c = 5000 psi		f'c = 6000 psi		f'c = 10000 psi		f'c = 8000 psi		
	TOP	BOT	TOP	BOT	TOP	BOT	TOP	BOT	TOP	BOT	TOP	BOT	
# 4	12"	8"	10"	7"	9"	6"	9"	6"	9"	6"	9"	6"	
# 5	14"	10"	13"	9"	11"	8"	10"	7"	10"	7"	10"	7"	
# 6	17"	12"	14"	10"	13"	9"	13"	9"	12"	8"	12"	8"	
# 7	20"	14"	17"	12"	16"	11"	14"	10"	13"	9"	13"	9"	
# 8	23"	16"	20"	14"	17"	12"	16"	11"	16"	11"	14"	10"	
# 9	26"	18"	21"	15"	20"	14"	19"	13"	17"	12"	16"	11"	
# 10	28"	20"	24"	17"	23"	16"	20"	14"	19"	13"	17"	12"	
# 11	31"	22"	27"	19"	24"	17"	23"	16"	21"	15"	20"	14"	



TYPICAL TENSION EMBEDMENT DETAIL

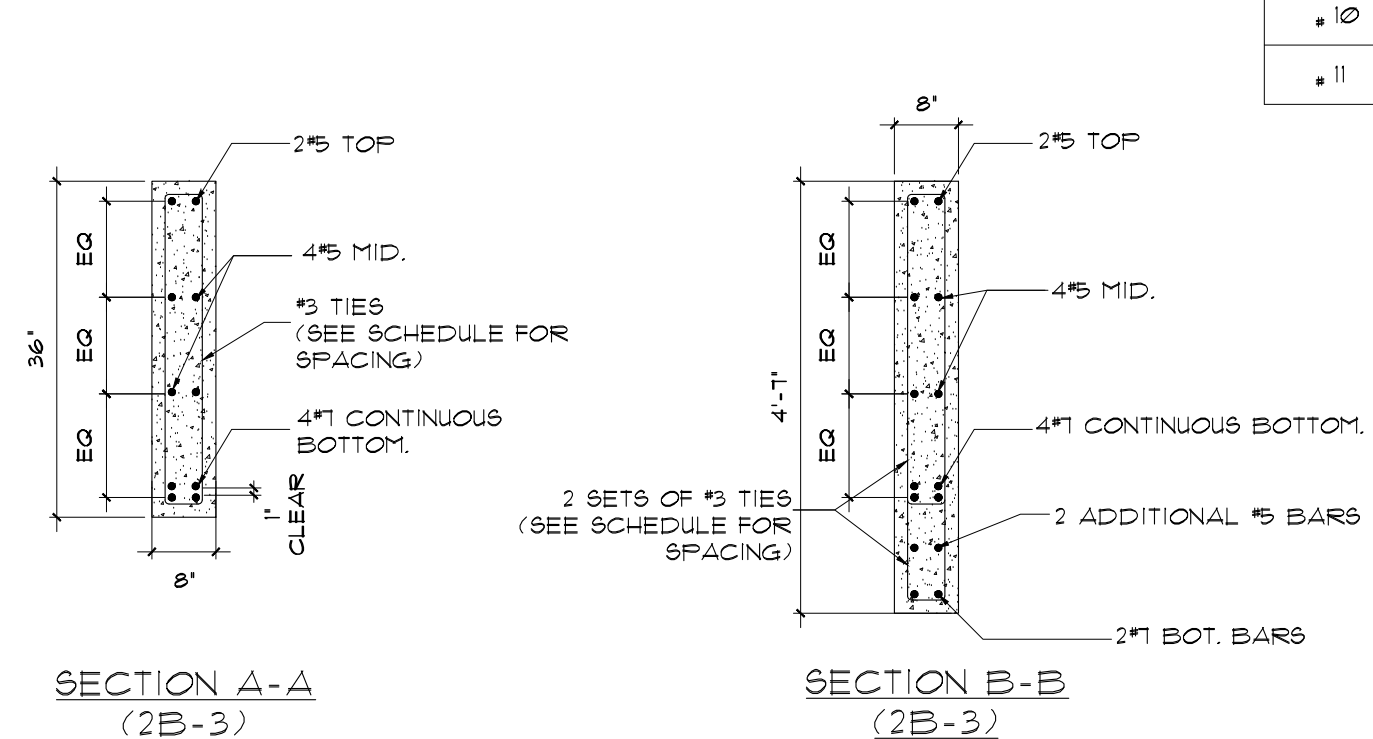
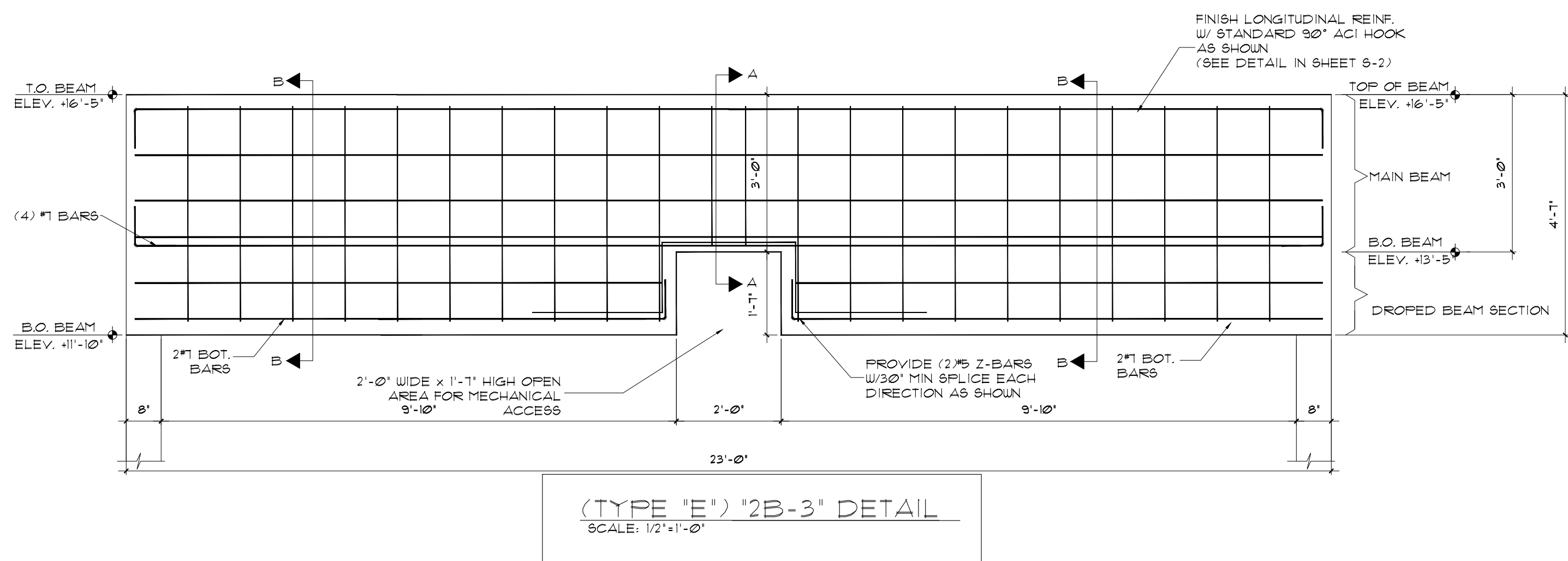
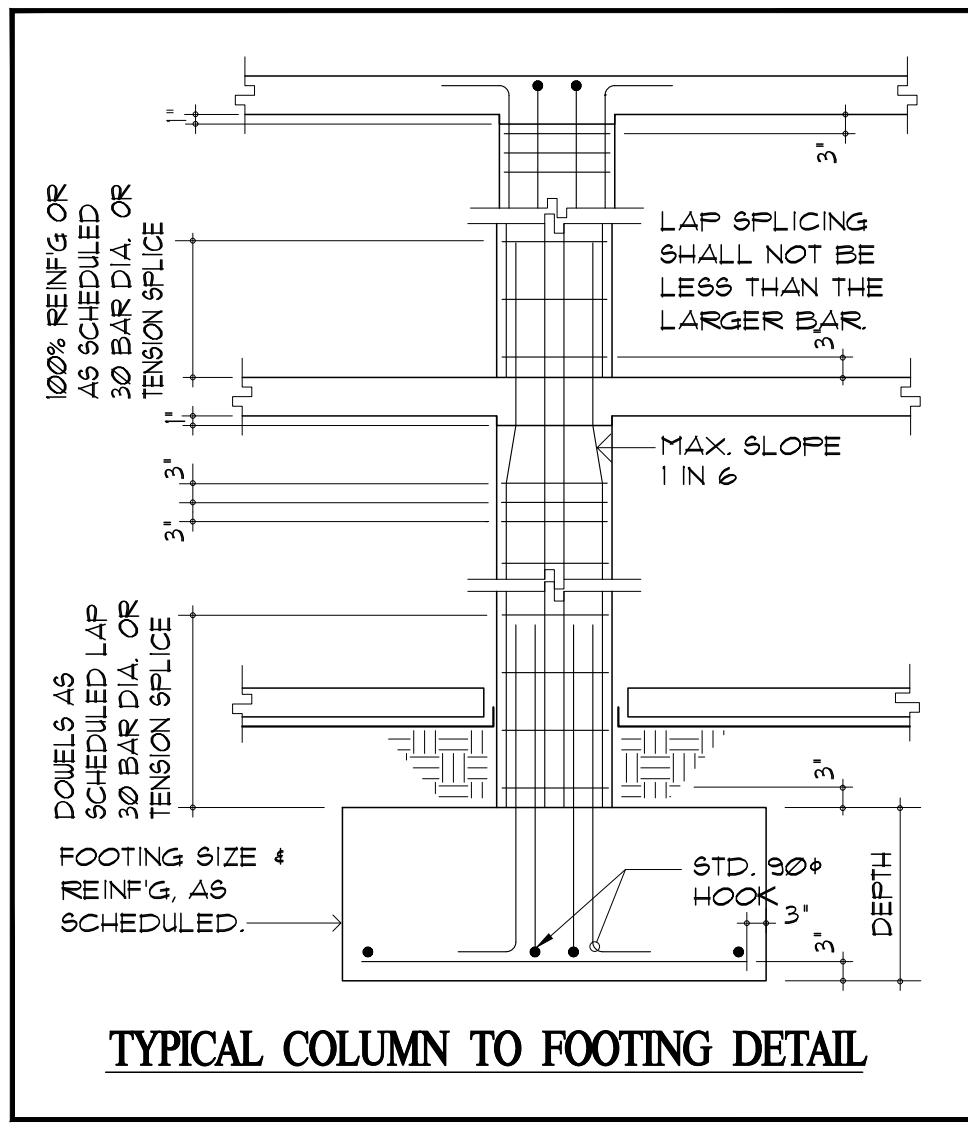
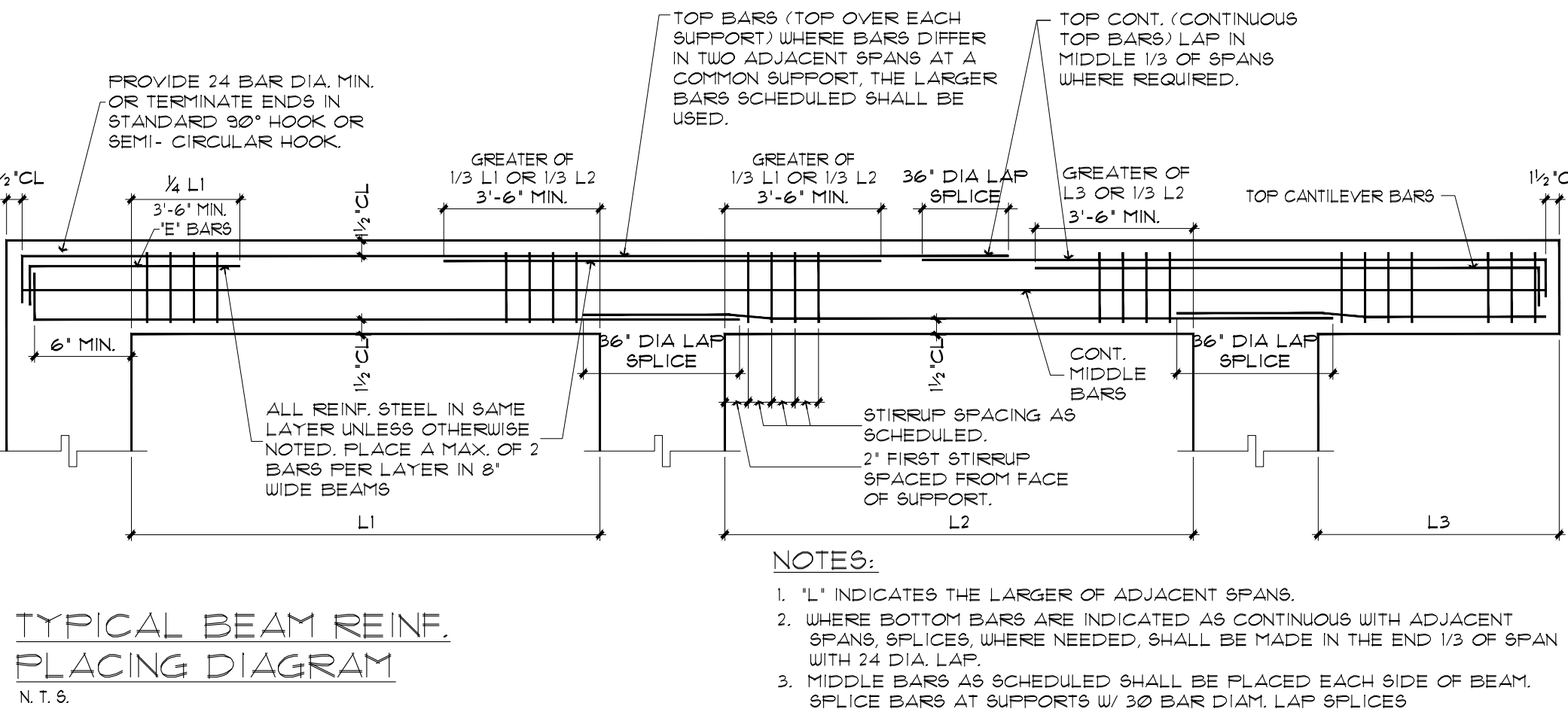
DEFINITIONS OF LAP/SPLICE CATEGORIES	
CATEGORY	DEFINITION
1	CLEAR SPACING OF BARS BEING DEVELOPED OR SPLICED, NOT LESS THAN 48" CLEAR COVER, NOT LESS THAN 48", AND STIRRUPS OR TIES THROUGHOUT L2 NOT LESS THAN THE CODE MINIMUM.
2	OTHER CASES

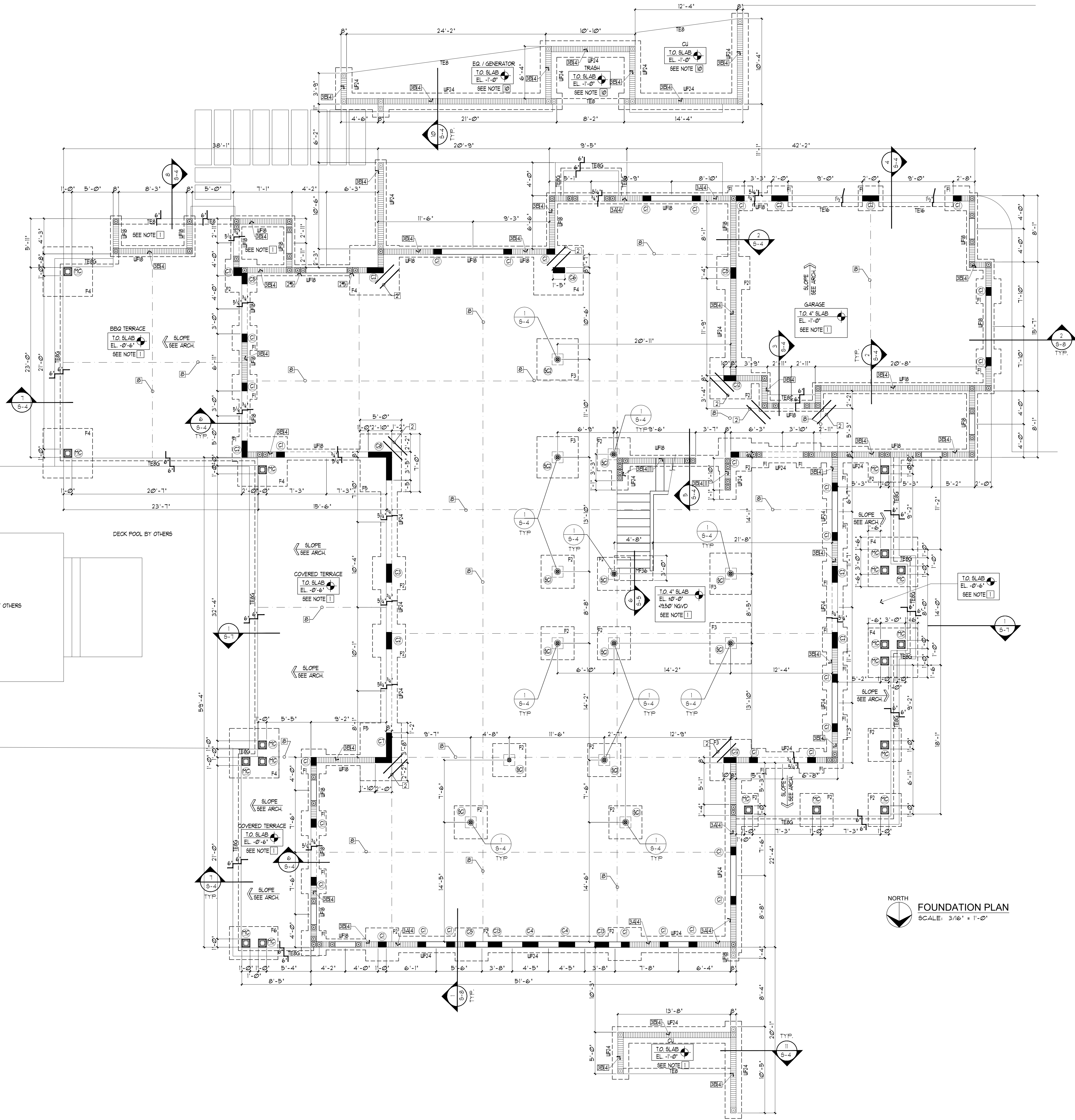
NOTE:
1. TOP BARS ARE DEFINED AS HORIZONTAL REINFORCEMENT SO PLACED THAT MORE THAN 12 INCHES OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE DEVELOPMENT LENGTH OR SPLICE.
2. 48" STANDS FOR NOMINAL BAR DIAMETER.

COMPRESSION LAP SPLICE 'LC'	
'LC' = 30 BAR DIAMETER FOR f'c ≤ 3000 psi	
MINIMUM COMPRESSION DOWEL EMBEDMENT	
22 BAR DIAMETER FOR f'c ≤ 3000 psi	

BAR SIZE		TENSION DEVELOPMENT LENGTHS (Ld) IN INCHES FOR GRADE 60 UNCOATED BARS (f'c=5000 psi) NORMAL-WEIGHT CONCRETE (ACI 12.2)			
		CASE 1		CASE 2	
		TOP BARS	OTHER BARS	TOP BARS	OTHER BARS
# 3	A	17	13	25	19
# 4	A	22	17	33	26
# 5	A	28	22	42	32
# 6	A	33	26	50	38
# 7	A	49	37	73	56
# 8	A	55	43	83	64
# 9	A	63	48	94	72
# 10	A	70	54	105	81
# 11	A	78	60	117	90
# 14	A	94	72	140	108
# 18	A	125	96	181	144

BAR SIZE	LAP CLASS	LAP LENGTH PER SPACING AND COVER CASE			
		CASE 1		CASE 2	
		TOP BARS	OTHER BARS	TOP BARS	OTHER BARS
# 3	A	17	13	25	19
# 4	A	22	17	33	26
# 5	A	28	22	42	32
# 6	A	33	26	50	38
# 7	A	49	37	73	56
# 8	A	55	43	83	64
# 9	A	63	48	94	72
# 10	A	70	54	105	81
# 11	A	78	60	117	90





FOUNDATION NOTES	
1	4" REINFORCED CONCRETE SLAB WITH 6X6-W4XU4 WELDED WIRE MESH (ASTM A185) LOCATED @ MID SLAB, SUPPORTED W/7" CHAIRS SPACED 3'-0" O.C. EACH WAY. OVER 6" MIL VAPOR BARRIER ON CLEAN WELL COMPACTED TYPHISTE TREATED FILL FREE OF ORGANIC MATTER. MAXIMUM SIZE OF ANY ROCKS WITHIN 12" BELOW THE FLOOR SLAB SHALL BE A MAXIMUM OF 3" IN DIAMETER.
2	PROVIDE (2) #4 X 36" LONG CORNER CRACK BARS @ ALL CORNERS OF SLAB. PLACE BARS ON TOP THIRD OF THE SLAB WITH FIRST BAR BEING 2" FROM THE CORNER & NEXT ONE 12" APART.
3	1-#6 REINFORCEMENT BAR GRADE 60 IN CONC. FILLED CELL AT ALL CORNERS, NEXT TO ALL OPENINGS, AND 16" O.C.
4	1-#6 REINFORCEMENT BAR GRADE 60 IN CONC. FILLED CELL AT ALL CORNERS, NEXT TO ALL OPENINGS, AND 24" O.C.
5	8"X8"X16" CONCRETE MASONRY UNITS WITH NO. 3 GAUGE LADDER TYPE JOINT REINFORCEMENT EVERY OTHER COURSE (16" IN CENTER VERTICALLY).
6	CONTRACTOR TO VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS. CONTACT ENGINEER IF DISCREPANCIES ARE FOUND PRIOR TO PROCEEDING W/WORK.
7	VERIFY SIZE & LOCATIONS OF ALL WINDOW/DOOR OPENINGS & OPENINGS IN SLAB WITH ARCHITECTURAL DIAGS.
8	CONCRETE SILLS TO BE A MINIMUM OF 8"X8" W/2-#5 CONTINUOUS FOR 8"X8"X16" CMU WALLS.
9	SAW CUT JOINT: (5 C.I.) #5-#8 O.C. MAX (TYP.) SAWN JOINT SHALL BE CUT WITHIN 24 HRS. OF CONCRETE PLACEMENT. SEE DETAIL 9.9-4.
10	COMPACT INTERIOR FILL TO 95% OF MODIFIED PROCTOR MAXIMUM DRY DENSITY AS PER ASTM D-1557 AT OPTIMUM MOISTURE CONTENT.
11	8" CONCRETE SLAB PAD FOR GENERATOR EQUIPMENT. REINFORCE SLAB W/ #4 BARS @ 12" O.C. EACH WAY BOTTOM. REFER TO GENERATOR MANUFACTURER SPECS FOR ANCHORAGE TO THE SLAB.
12	8"X8"X16" CONCRETE MASONRY LOW WALL, SUPPORT OF STAIR LANDING

LEGEND	
	SECTION CALLOUT
	TYP
	DETAIL CALLOUT
	BEARING CONCRETE COLUMN OR SHEARWALL
	(#5 SPACED PER PLAN) REPRESENTS (2) #5 REPRESENTS (3) #5 REPRESENTS (4) #5 8"X8"X16" CMU WALL REINFORCED W/ #5 VERTICAL AS SHOWN
	STRUCTURAL BEARING ELEMENT BELOW
	COLUMN STARTS AT THIS LEVEL COLUMN CONTAINS THRU THIS LEVEL COLUMN ENDS AT THIS LEVEL
	PROPOSED CONCRETE SECTION WIRE REINFORCEMENT
CONTRACTOR TO VERIFY ALL BUILDING DIMENSIONS & ELEVATIONS WITH ARCHITECTURAL PLAN	

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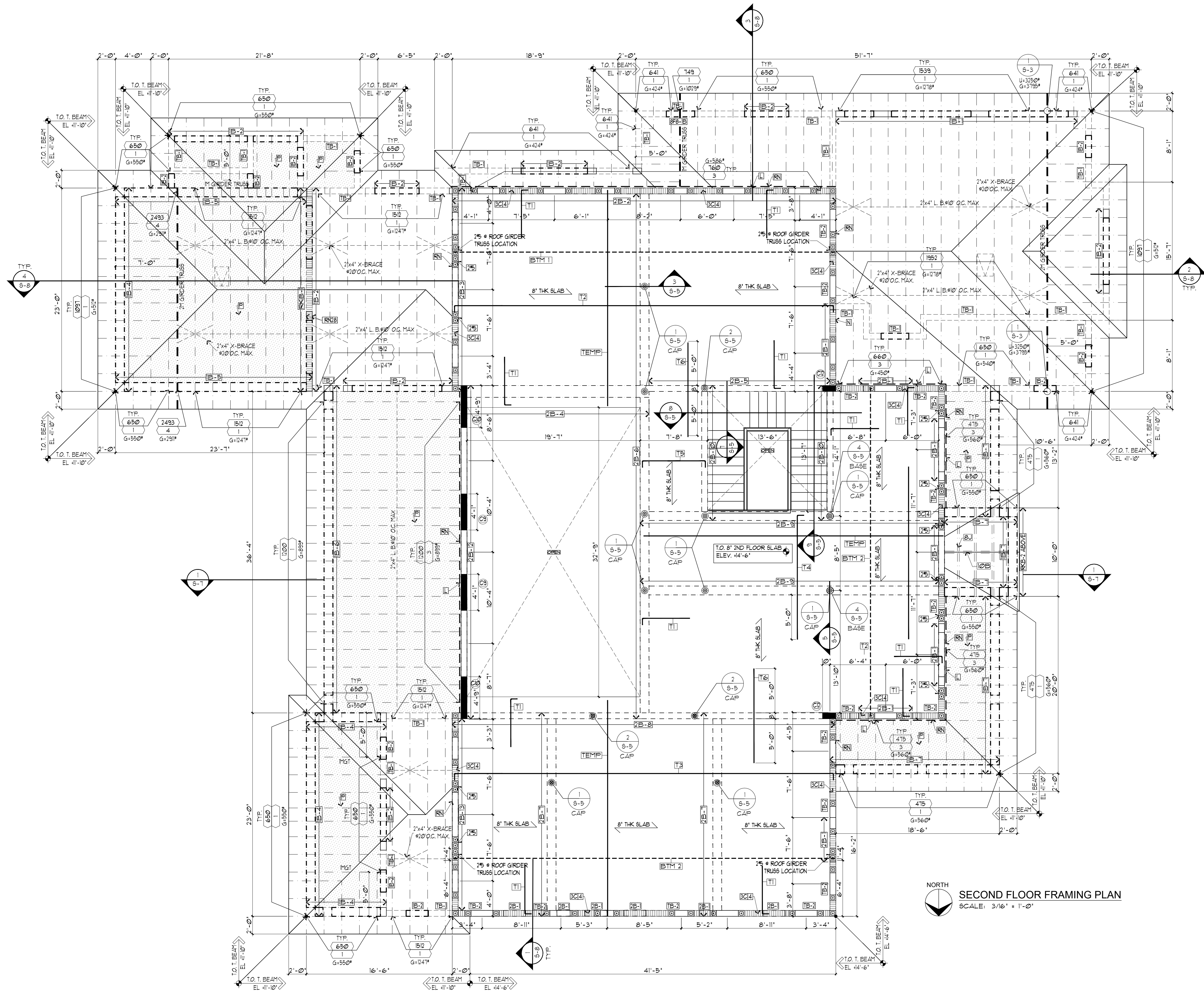
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SEAL :

DRAWING INFORMATION :
PERMIT SET 4.25.25

SHEET :
S-1

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2ND FLOOR NOTES	
1. 5	1-5 REINFORCEMENT BAR GRADE 60 IN CONC. FILLED CELL AT ALL CORNERS NEXT TO ALL OPENINGS AND 40\"/>
2	2 8\"/>
3	3 (2) 2\"/>
4	4 2\"/>
5	5 2\"/>
6	6 1\"/>
7	7 3\"/>
8	8 1\"/>
9	9 3\"/>
10	10 3\"/>

SLAB REINFORCEMENT LEGEND	
BTM 1	1 5 BOTTOM # 6\"/>
BTM 2	2 5 BOTTOM # 12\"/>
TEMP	3 4 TEMP # 12\"/>
1	1 4x 4\"/>
2	2 5 TOP # 6\"/>
3	3 5 TOP # 12\"/>
4	4 4 TOP # 12\"/>
5	5 4 TOP # 12\"/>
6	6 4 TOP # 12\"/>

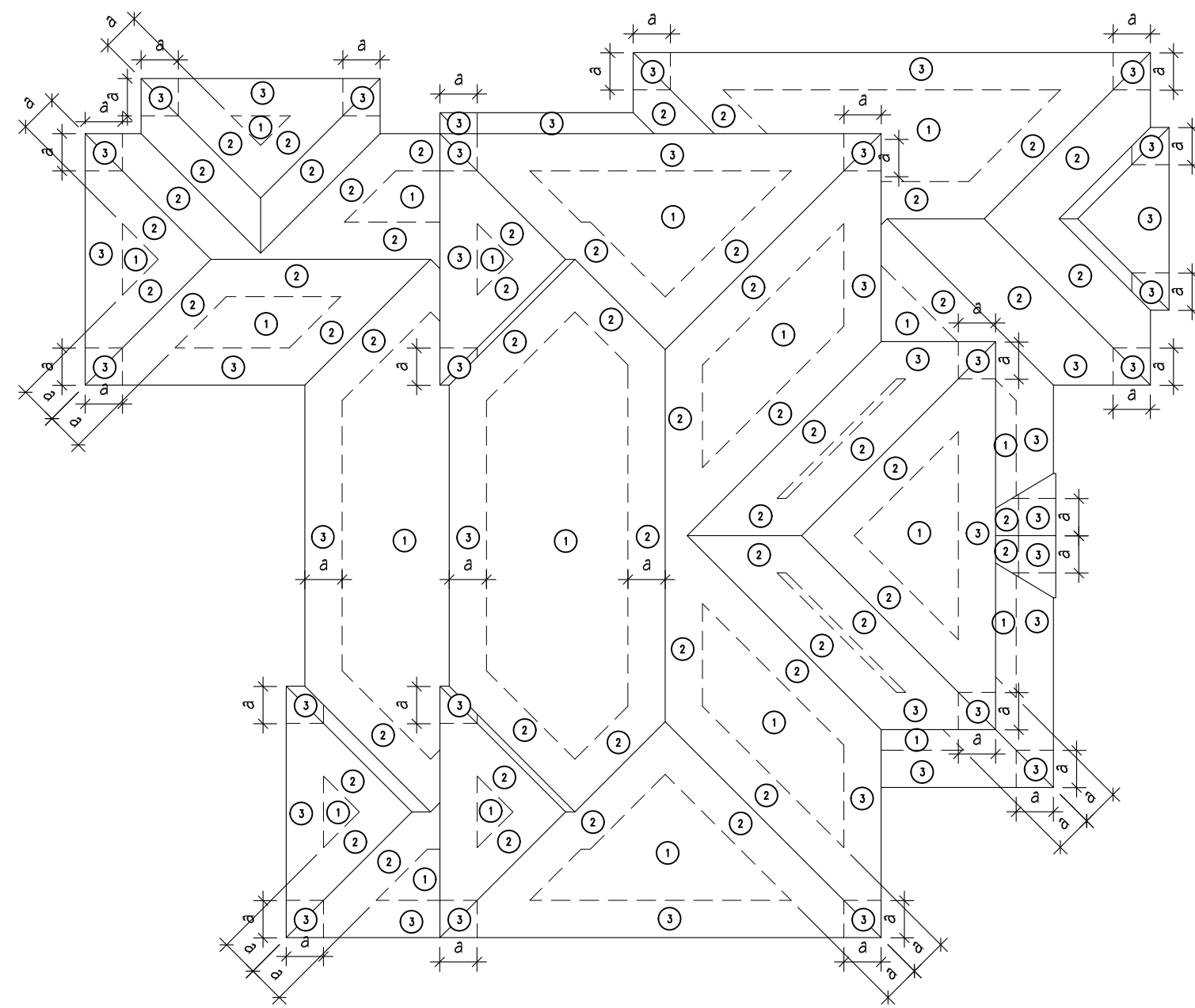
LEGEND	
	SECTION CALLOUT
	DETAIL CALLOUT
	BEARING CONCRETE COLUMN OR SHEARWALL
	(5 SPACED PER PLAN) REPRESENTS (2) 5 REPRESENTS (3) 5 REPRESENTS (4) 5 8\"/>
	STRUCTURAL BEARING ELEMENT BELOW
	COLUMN STARTS AT THIS LEVEL COLUMN CONTINUES THRU THIS LEVEL COLUMN ENDS AT THIS LEVEL
	PROPOSED CONCRETE SECTION W/ REINFORCEMENT
CONTRACTOR TO VERIFY ALL BUILDING DIMENSIONS & ELEVATIONS WITH ARCHITECTURAL PLAN	

ROOF LEGEND	
HATCH INDICATES 1/2\"/>	

ROOF TRUSS ANCHOR SCHEDULE	
ACTUAL UPLIFT	1 NU-VUE NVHTA-16H (1 PLY) GIRDERS STRAP W/ (18) 18d x 18\"/>
REQUIRED ANCHOR	2 (NUVUE) NVHTA-16H (1 PLY) GIRDERS STRAP W/ (18) 18d x 18\"/>
	3 MITEK THD28 HANGER ATTACHED TO LEDGER W/ 18-16d NAILS 4 TO TRUSS W/ 12-10d x 15\"/>
	4 (NUVUE) NV358-16 (2 PLY) GIRDERS DOUBLE STRAP W/ (18) 18d x 3\"/>
	5 (SIMPSON) LGHP28-2 HANGER W/ (6) 1\"/>

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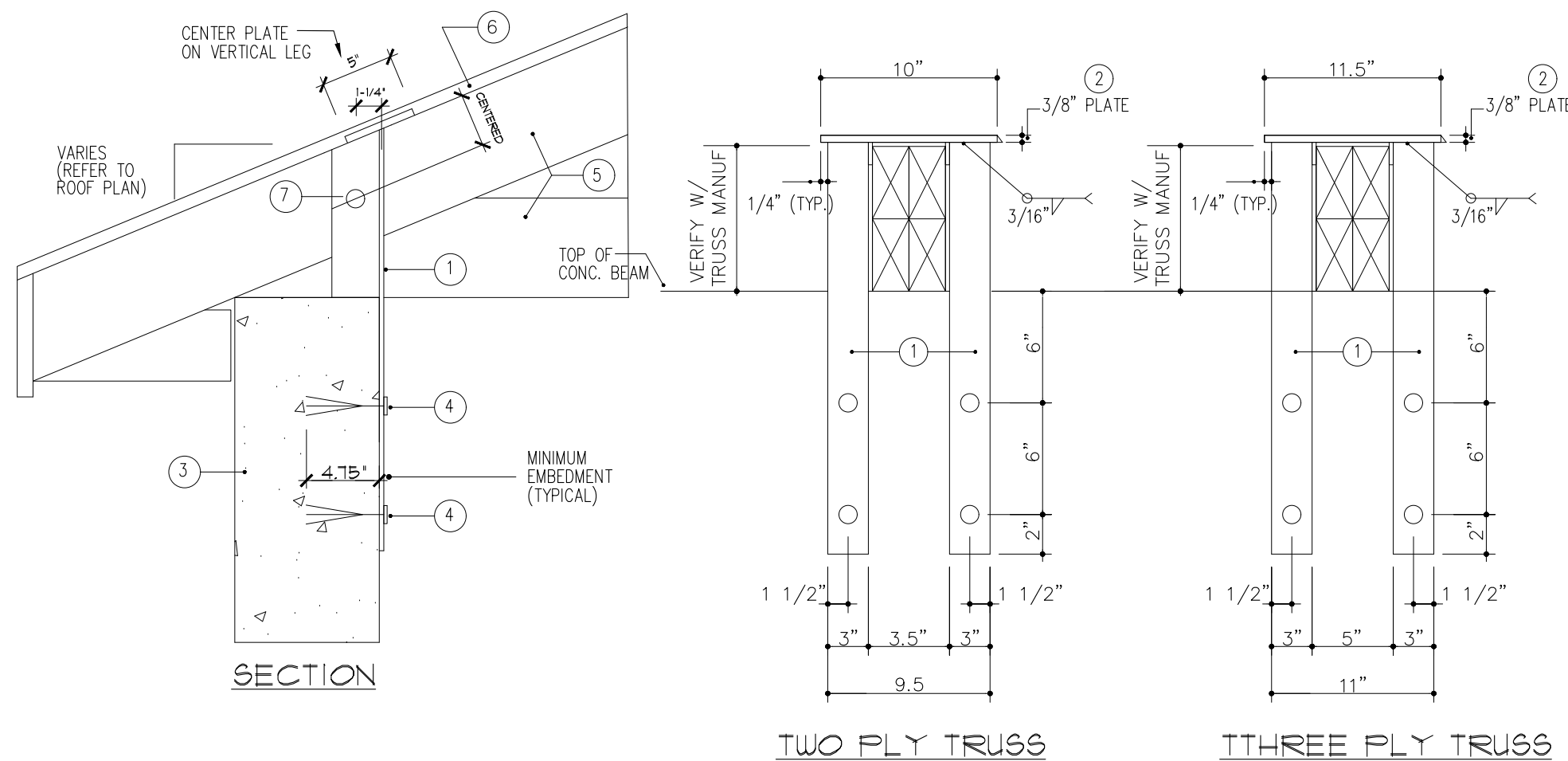
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ZONE 2 _____ +34.72 P&F 4 - 101.81 P&F
ZONE 3 _____ +34.72 P&F 4 - 109.70 P&F



COMPONENT & CLADDING
LOADING DIAGRAMS. (NTS)

ACTUAL UPLIFT	1. NU-VIEW NV14T16 STRAP ANCHOR W/ (8) 10dX13" NAILS TO TRUSS AND 6 10dX13" NAILS TO SEAT. CAPACITY = 26.0K (21015P1)
	2. (NUVIEW) NV14T16-1 (FLY) GIRDER DOUBLE STRAP W/ (18) 10d x 13" NAILS TO JOIST AND (6) 10d x 13" NAILS TO SEAT FLAT ANCHOR CAPACITY: U=311K (2024-21015P1)
REQUIRED ANCHOR	MITER THD12S HANGER ATTACHED TO LEDGER W/18-16d NAILS & TO TRUSS W/16-15d NAILS. ALLOWABLE U: 226K; G: 264K (FM732)
B/E = BOTH ENDS OF TRUSS AND W/ LESS NOTED OTHERWISE U = UPLIFT LOAD G = GRAVITY LOAD TYP = TYPICAL	3. (NUVIEW) NV358-16 (2 FLY) GIRDER DOUBLE STRAP W/ (18) 10dX13" NAILS TO JOIST AND (6) 10d x 13" NAILS TO SEAT ANCHOR CAPACITY: U=336K (2024-21015P5)
	4. (SMPSM) LGH72-8 HANGER W/ (6) 1" DIAM x 2" LONG SCS WOOD SCREWS TO TRUSS AND (6) 8" DIAM x 4" MITER HD CONCRETE ANCHOR TO WALL. U=233K; G=1595K (FLORIDA APPROVAL #9304)

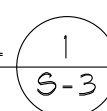
HATCHING INDICATES OF 2'x4' BLOCKING @ ALL PLYWOOD SEAMS. ATTACH SHEATHING TO BLOCKING W/ 8d RING SHANK NAILS 4' O.C. EACH SIDE OF SEAM



MARK	DESCRIPTION
1	ANGLE 3" x 3" x 1/4" W/ EXTENDED LEG (FY=36KSI) SEE DWG.
2	STEEL PLATE 10" x 5" x 3/8" (FY=36 KSI)
3	CONCRETE BEAM (SEE PLANS & SCHED.)
4	(4) 3/4" DIAMETER HIGHT KWIK BOLT 3 (4.75" MIN. EMBED.)
5	PREFABRICATED WOOD GIRDER TRUSS
6	19/32" EXTERIOR GRADE PLYWOOD
7	3/4" DIAM THRU BOLT ASTM A307

FER HULL KUKK BOLT 3
 (4) 3/4" DIAMETER ANCHORS IN 3000 PSI CONCRETE (MIN.
 EMBEDMENT 415")
 -W/6" SPACING = Ø20 REDUCTION
 -W/6" EDGE = Ø21 REDUCTION
 9-BAR CAPACITY = 614.2 (9.90) (Ø67) x 4 BOLTS = 4.4K
 PER NODAL POINT BEARING CAPACITY $F_c(\text{per pt}) = 565 \text{ PSI}$
 CAPACITY OF (2) MEMBER
 AREA OF PLATE FOR BEARINGS = $(565 \times 12 \text{ IN}) + (225 \times 4 \text{ IN}) = 8.1 \text{ K}$
 ALLOWABLE LOAD = AREA x 565 PSI = 4.575 (21) + 4.125 (31)
 ALLOWABLE UPLIFT CAPACITY = 8.475 (21) + 12.175 (31)

NTS



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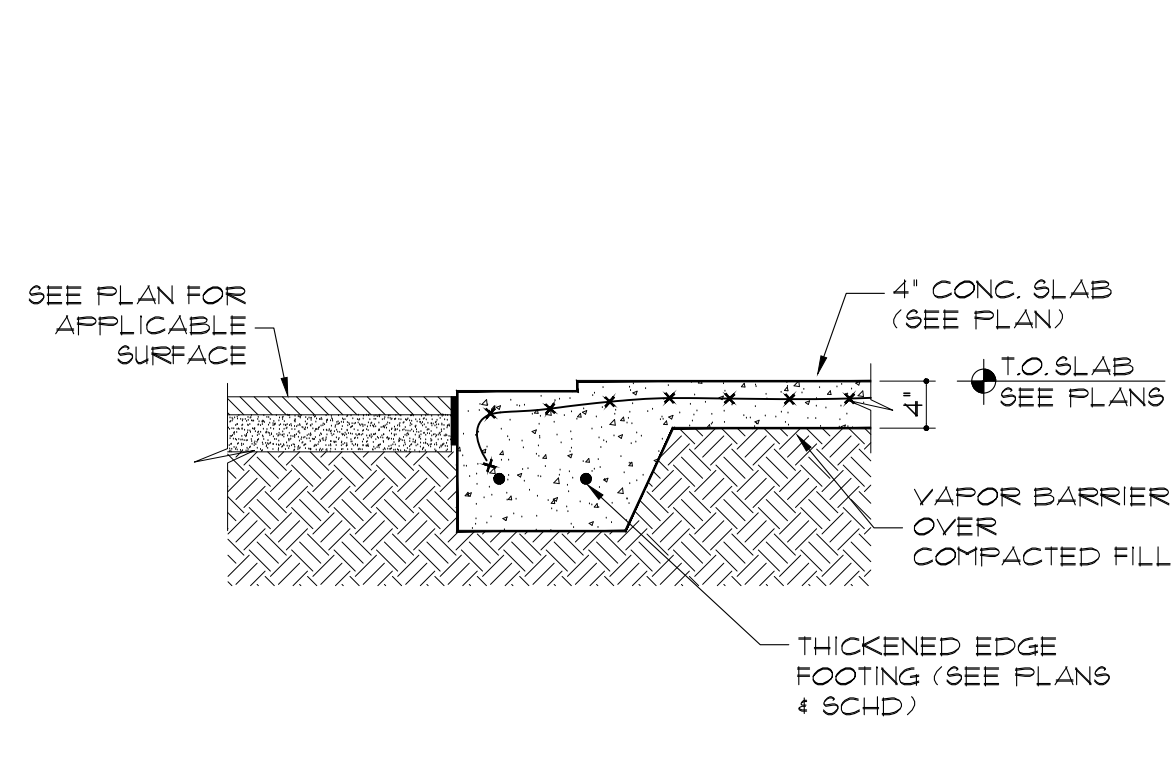
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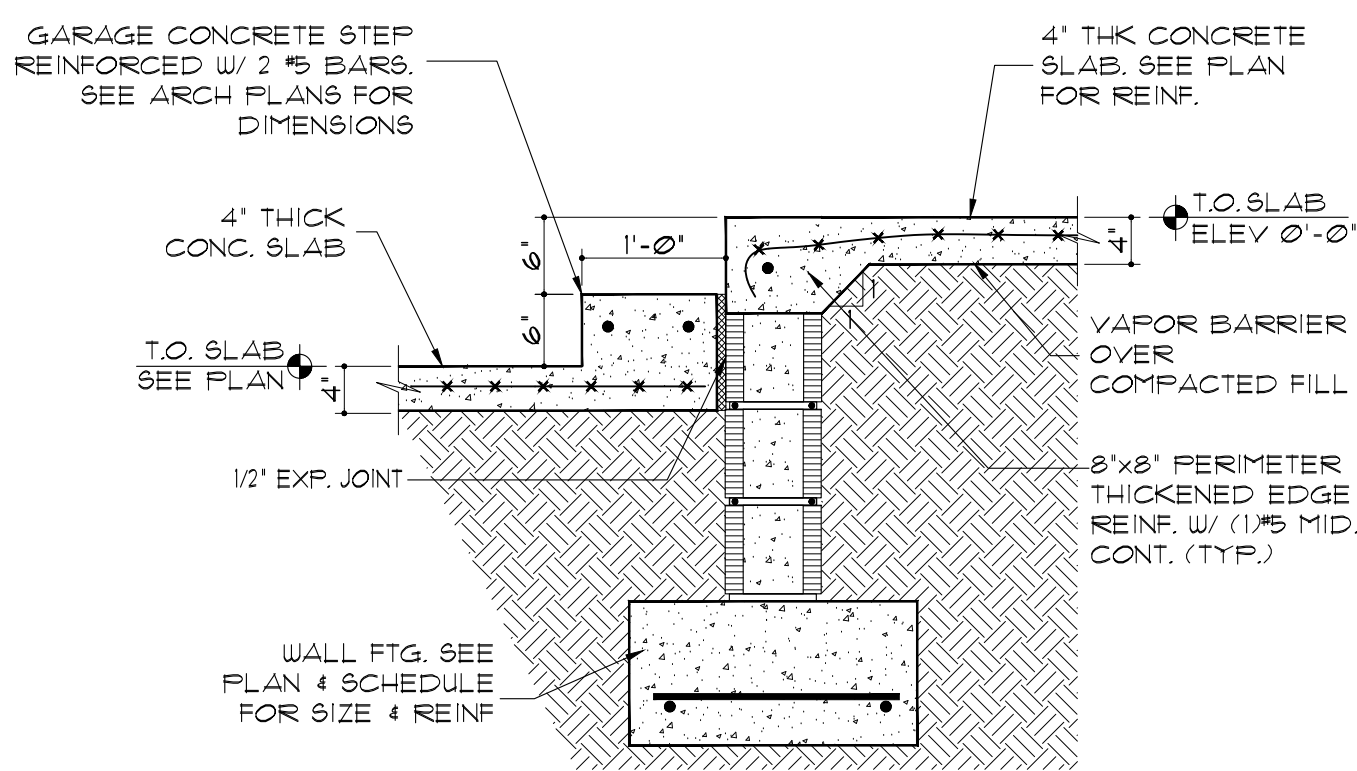
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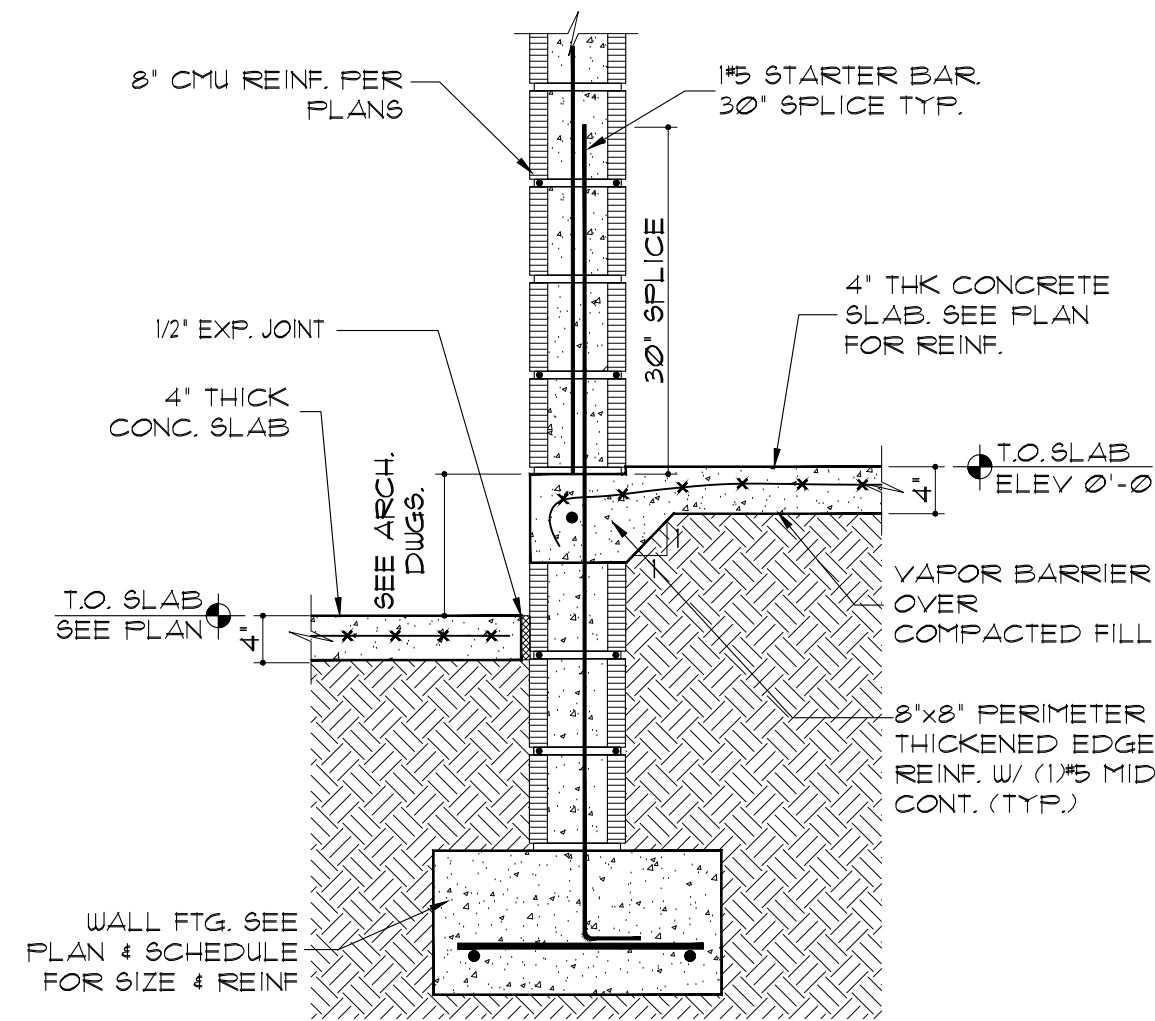
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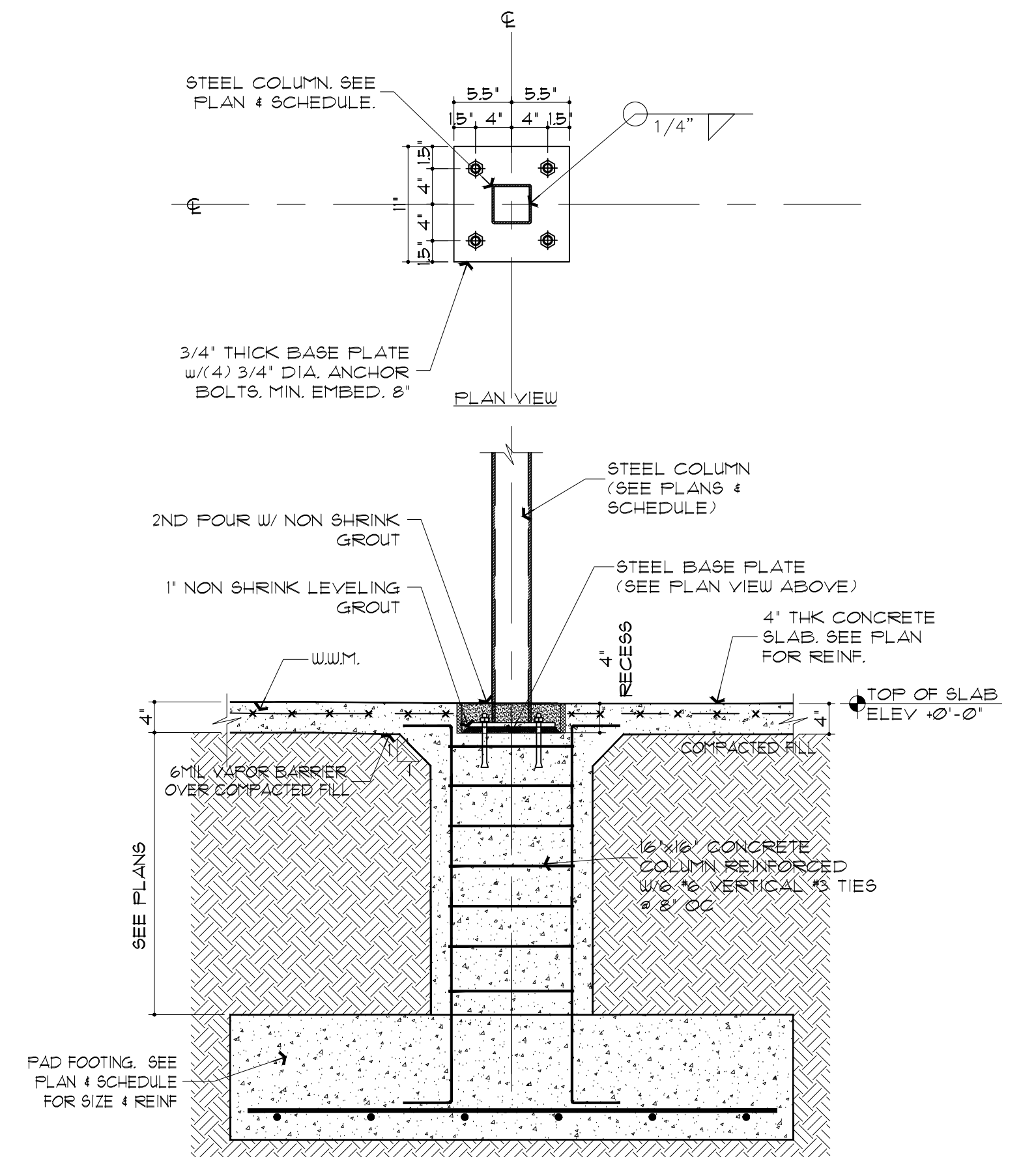
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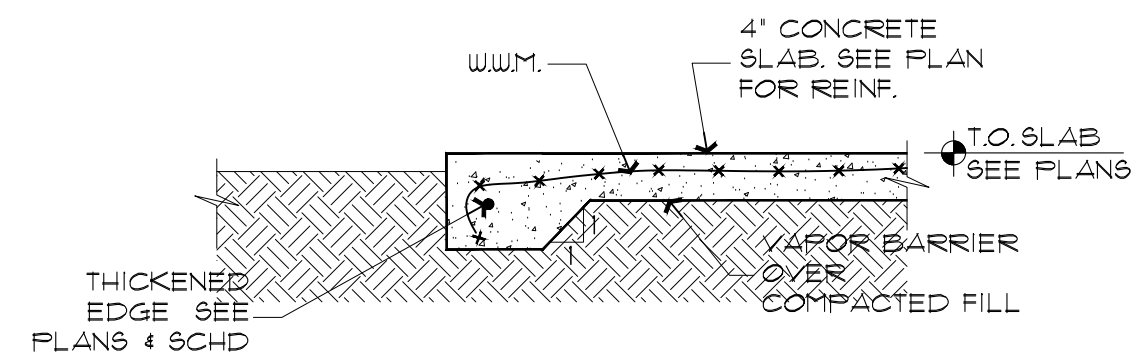
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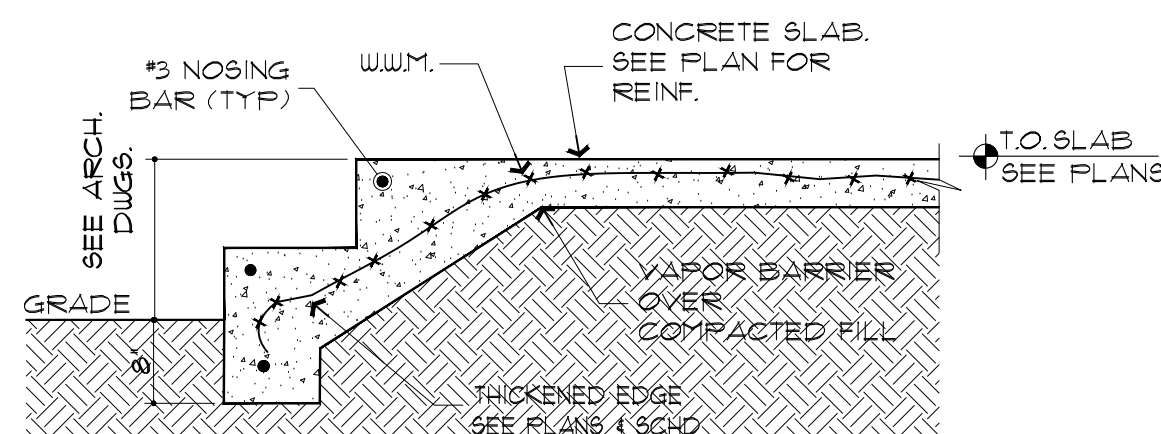
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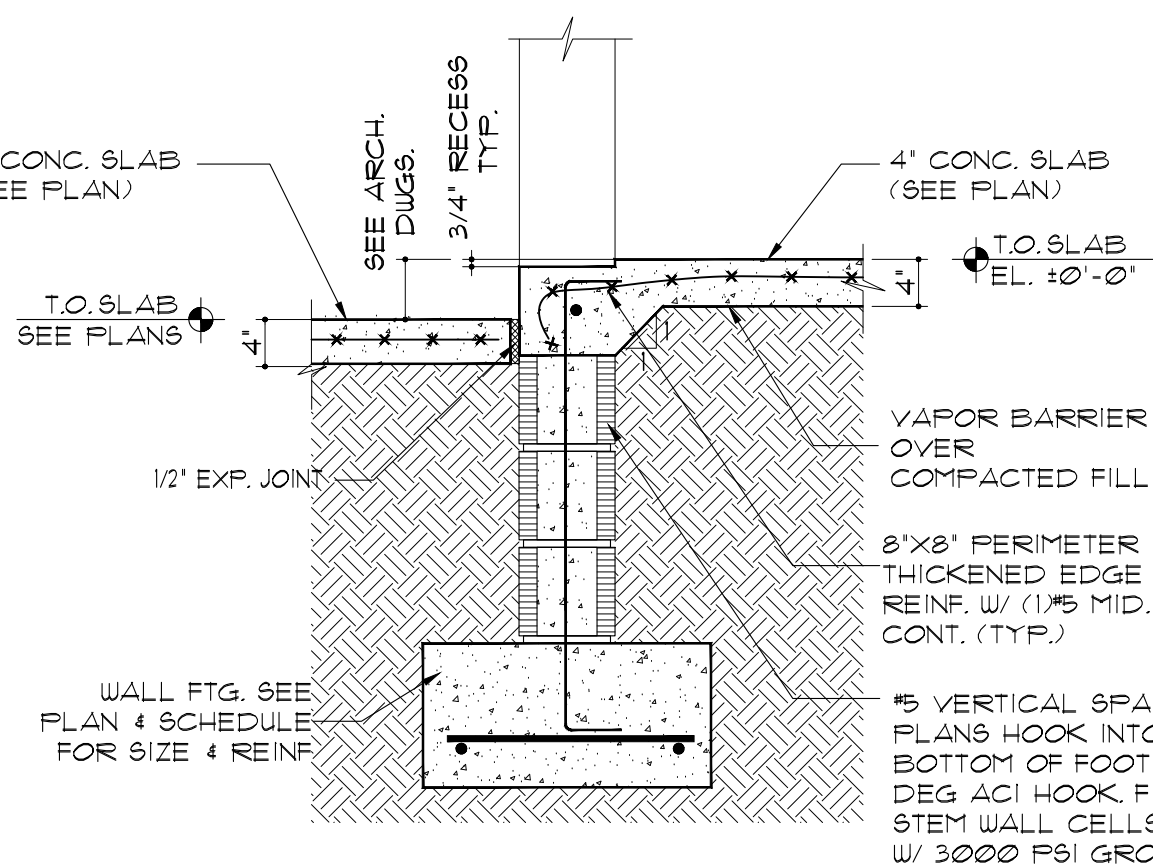
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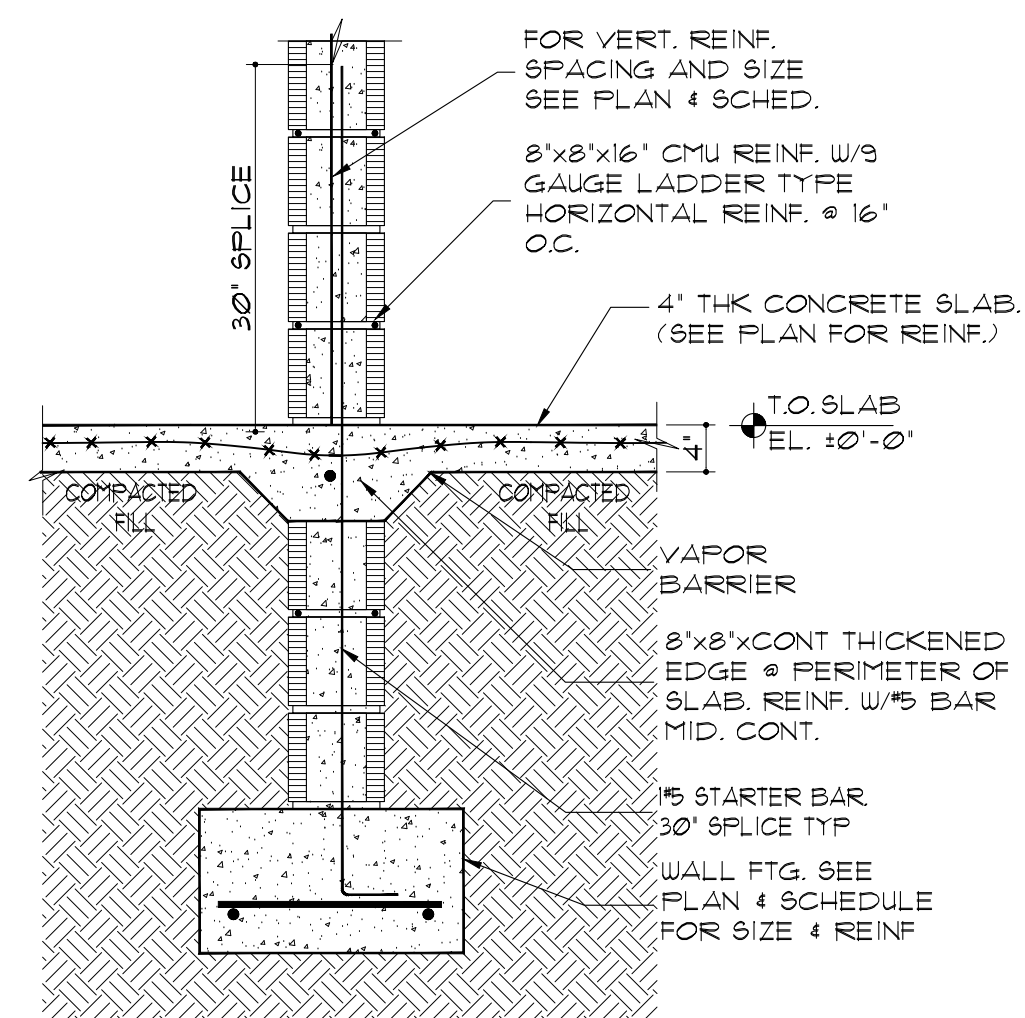
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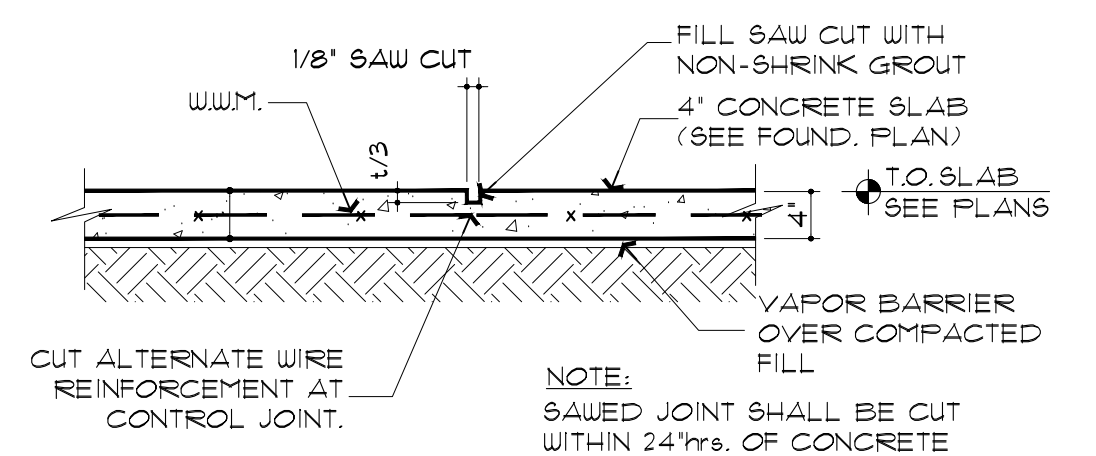
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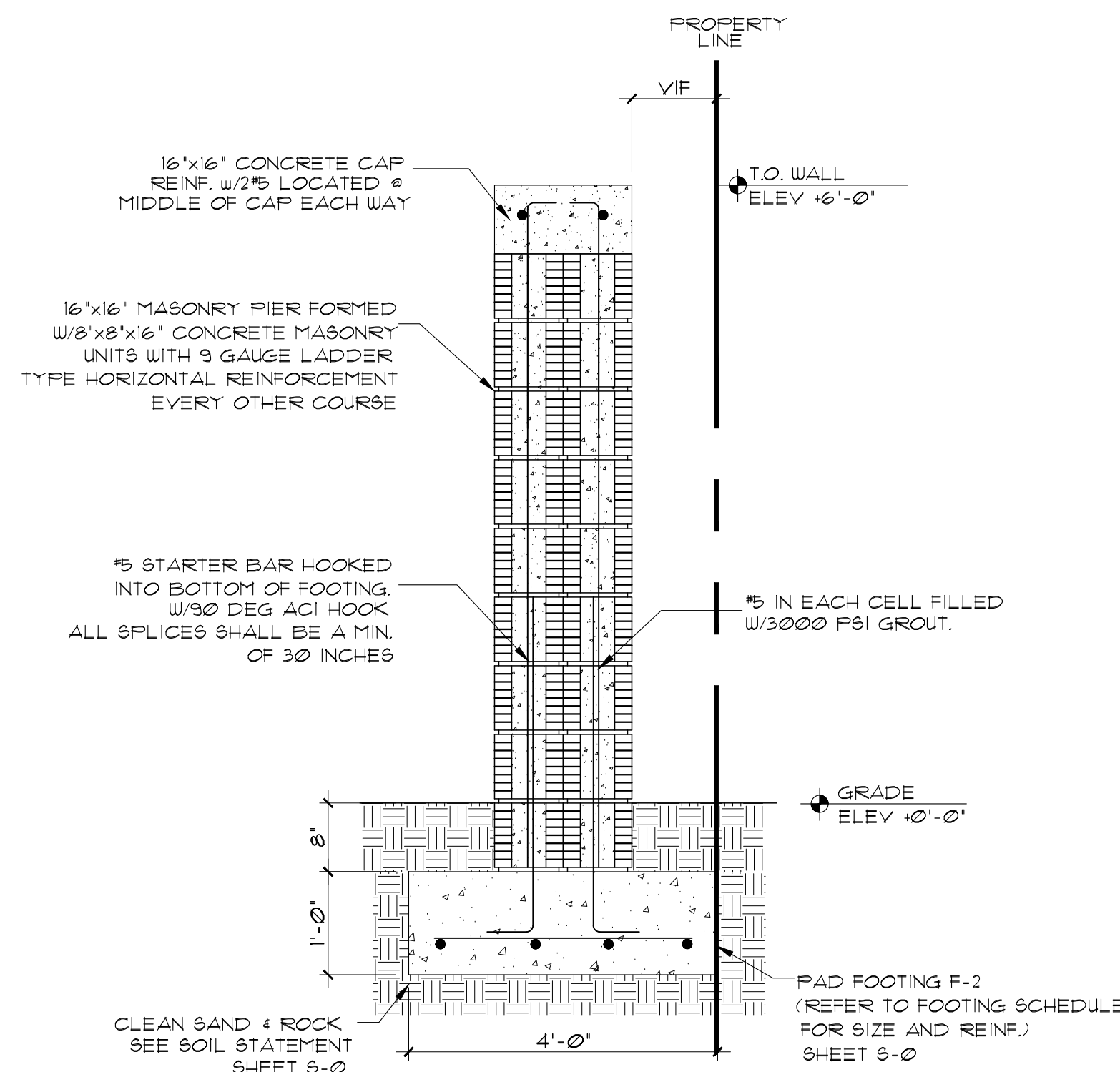
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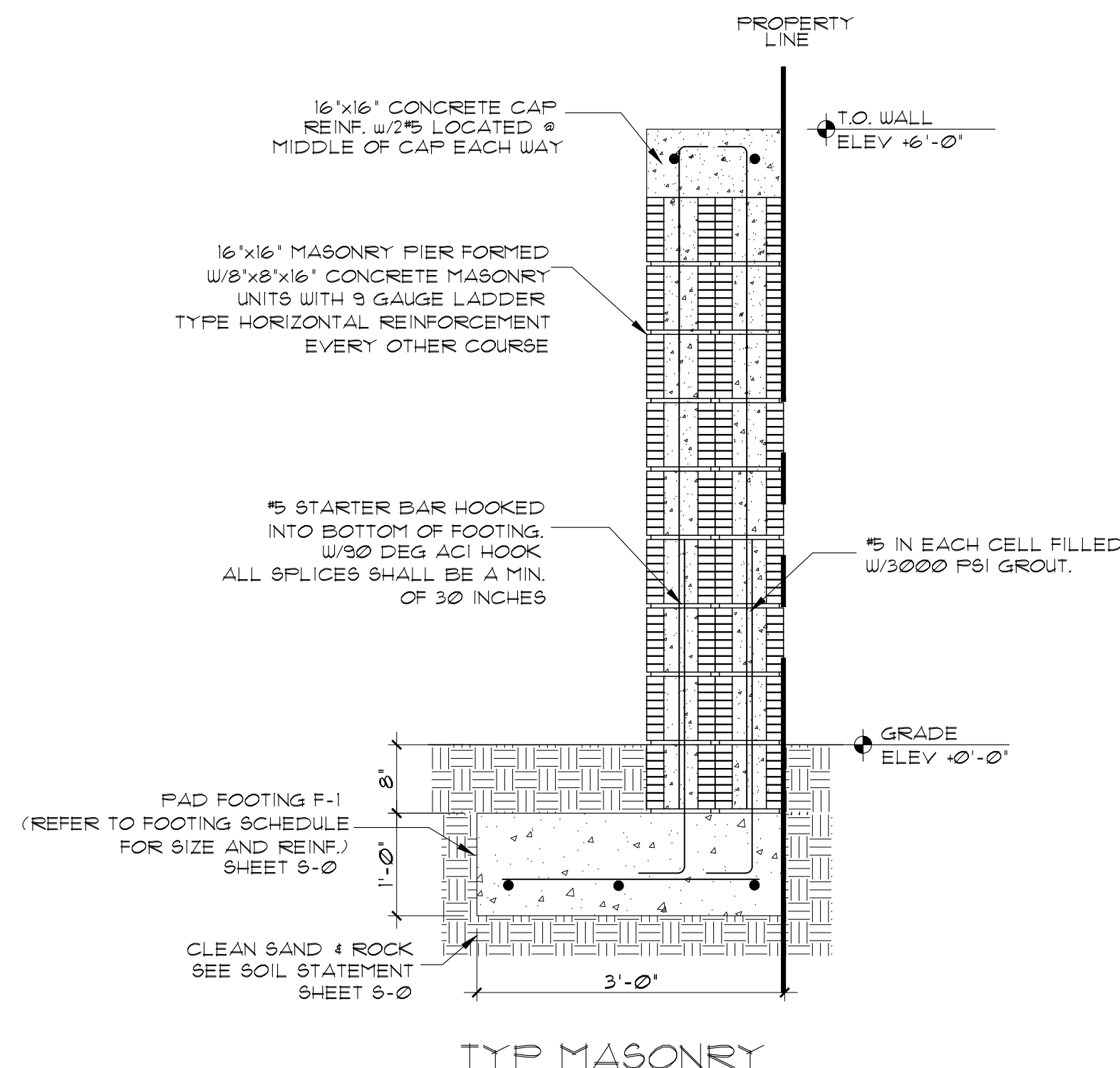
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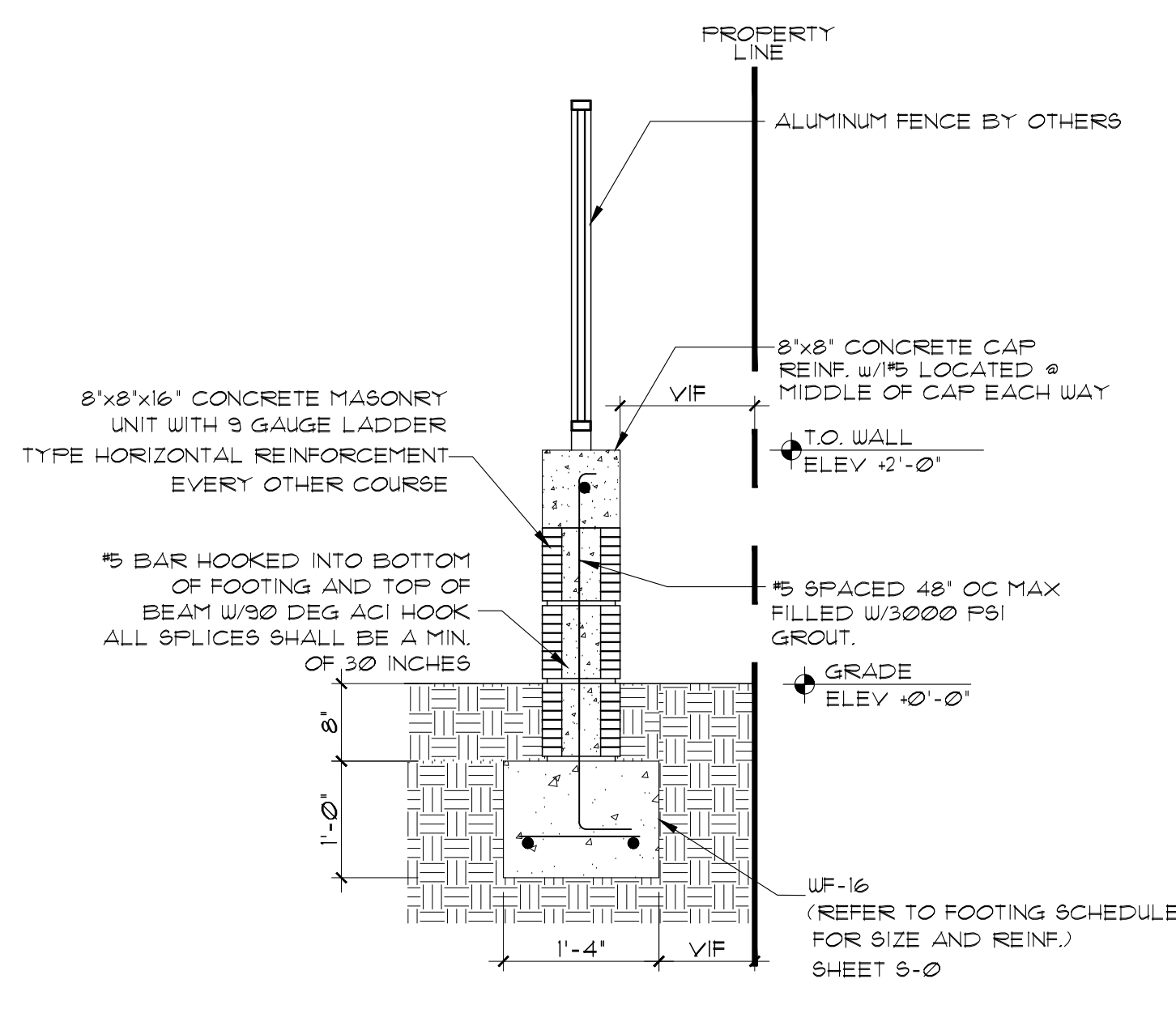
TYP. SAW CUT JOINT 9
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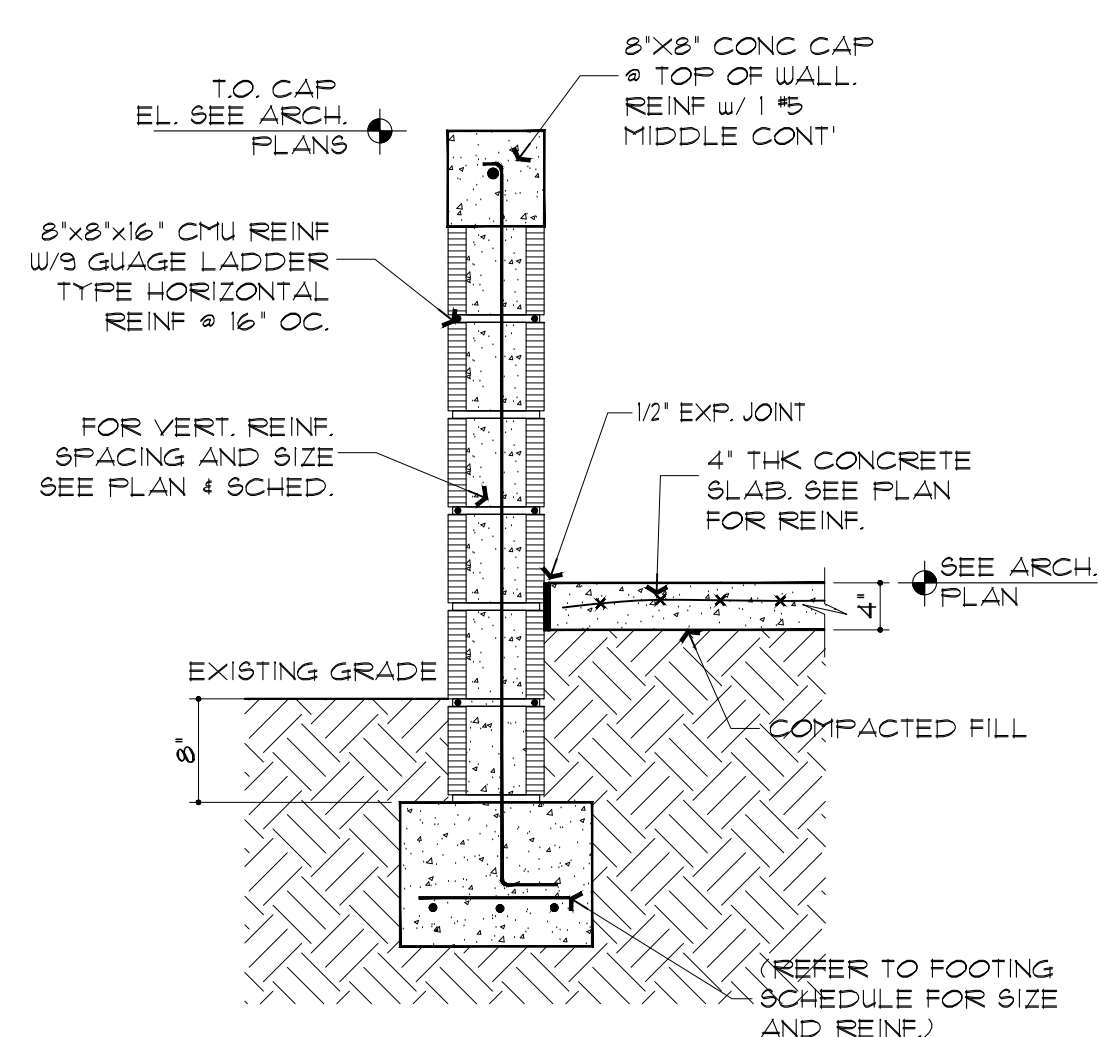
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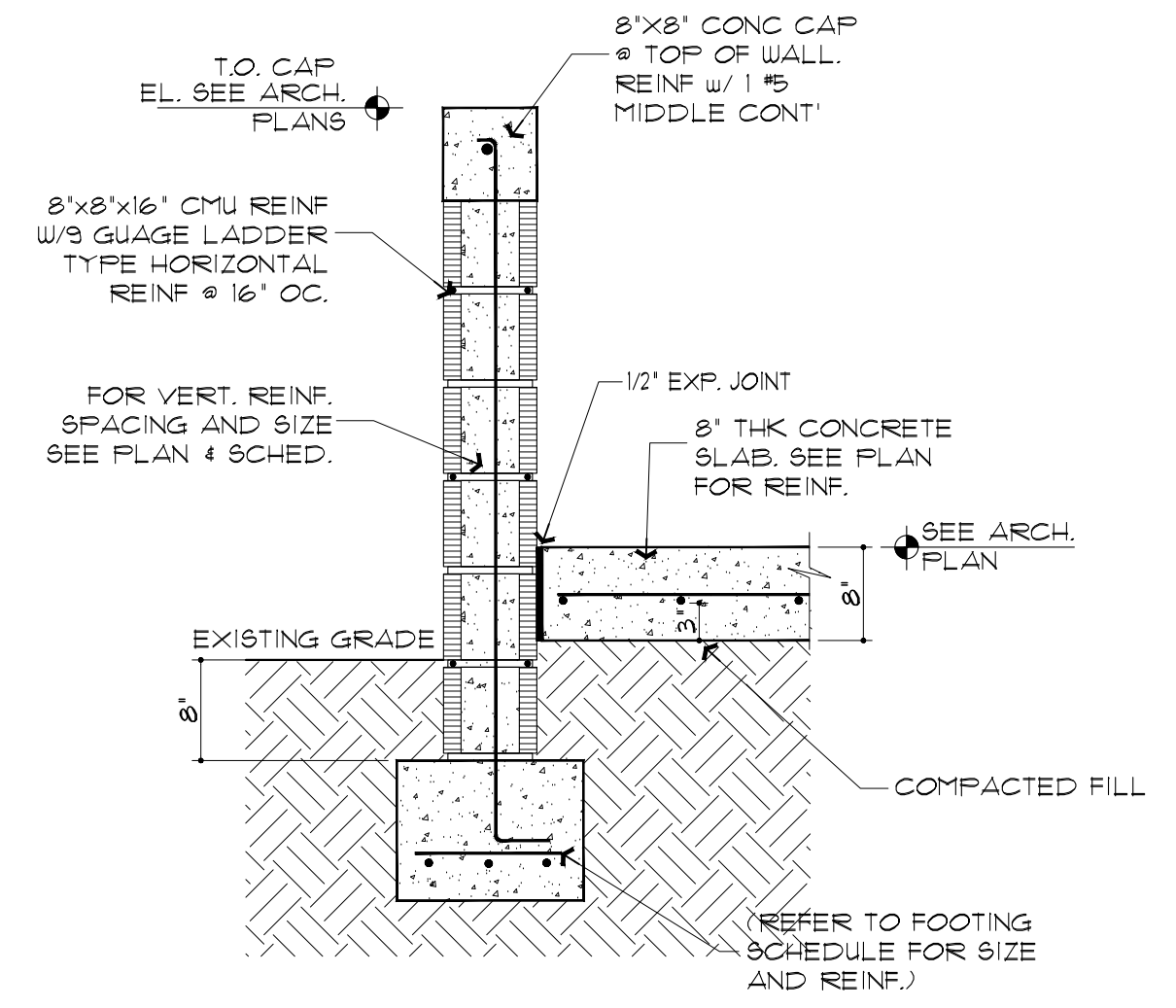
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TYP MASONRY LOW WALL SECTION 03
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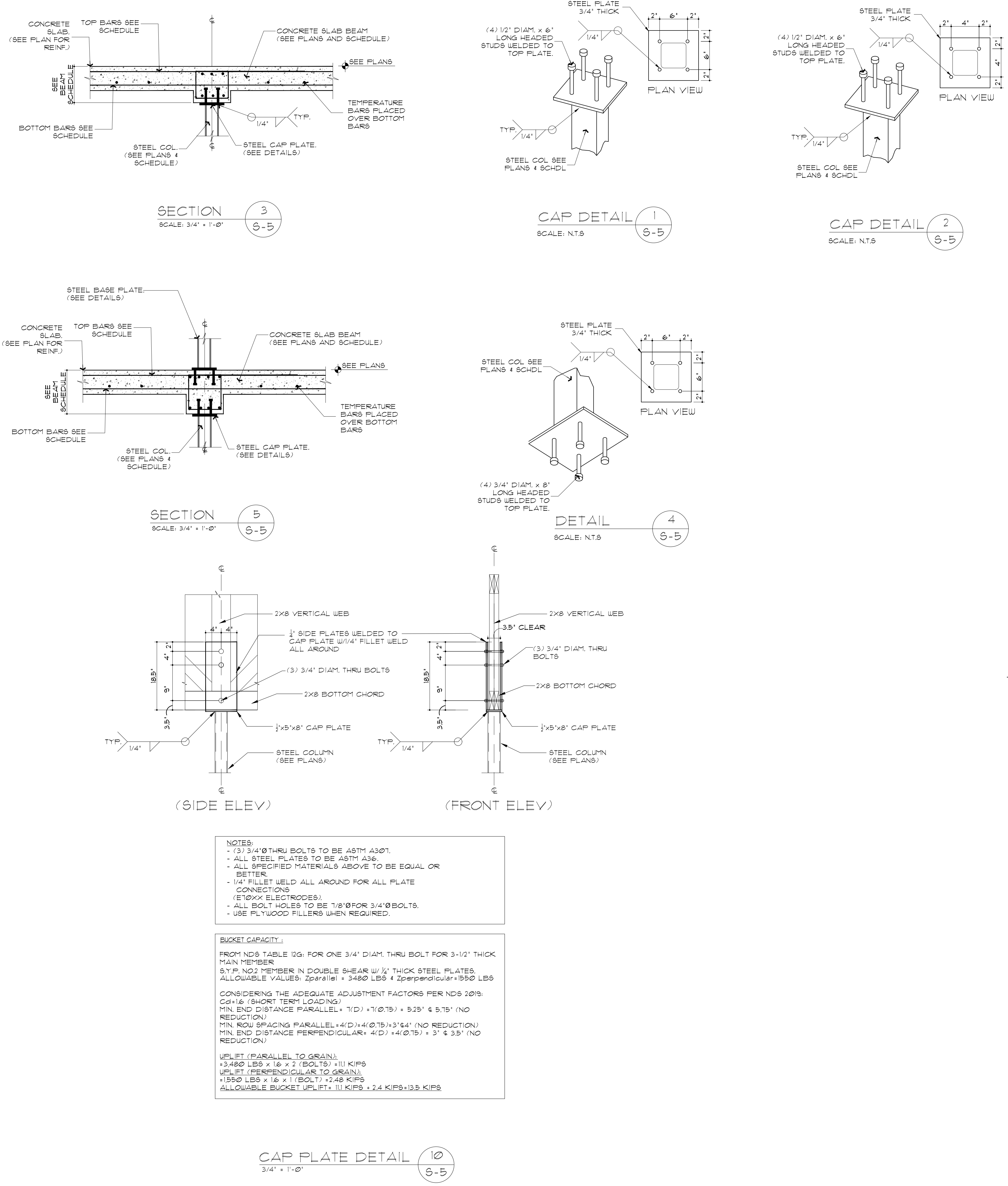


DETAIL 10
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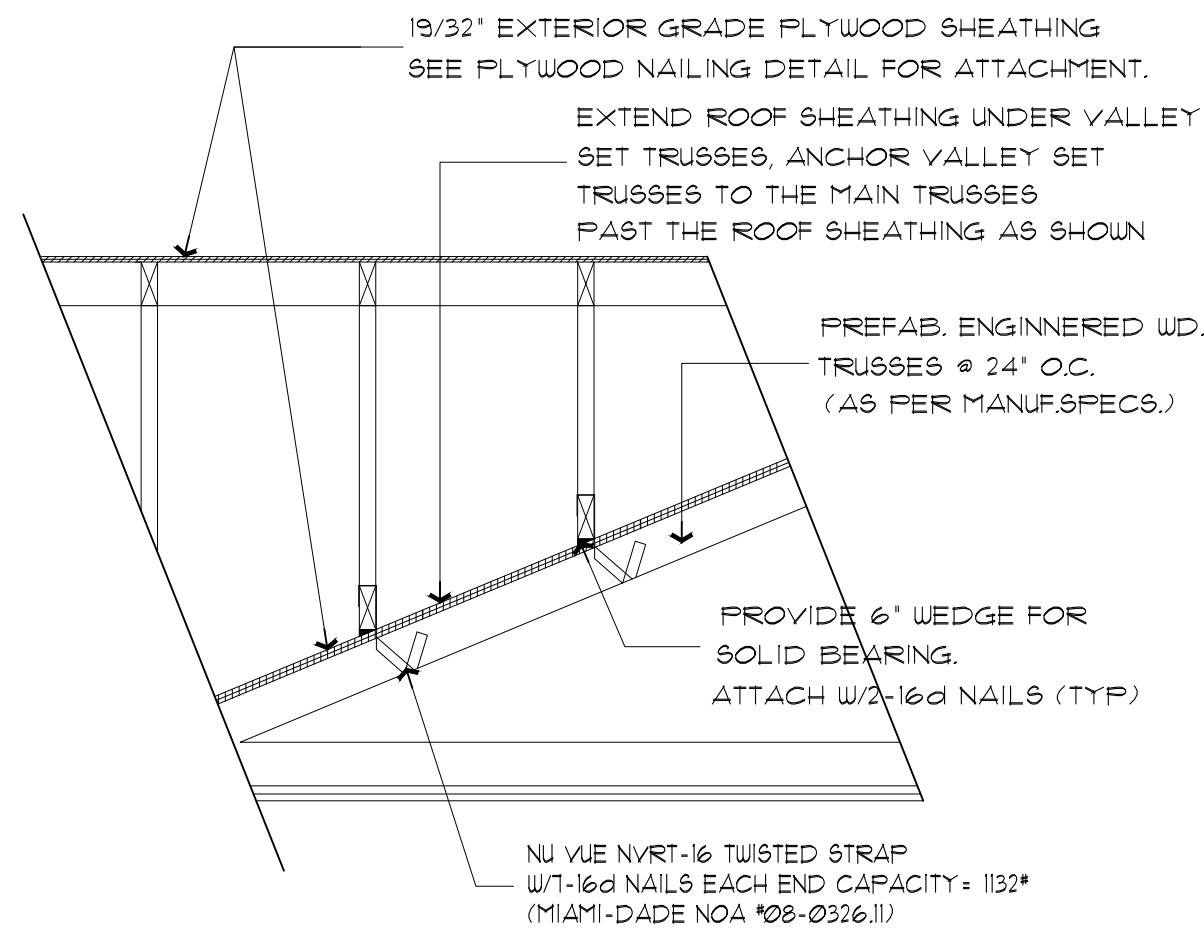


DETAIL 10
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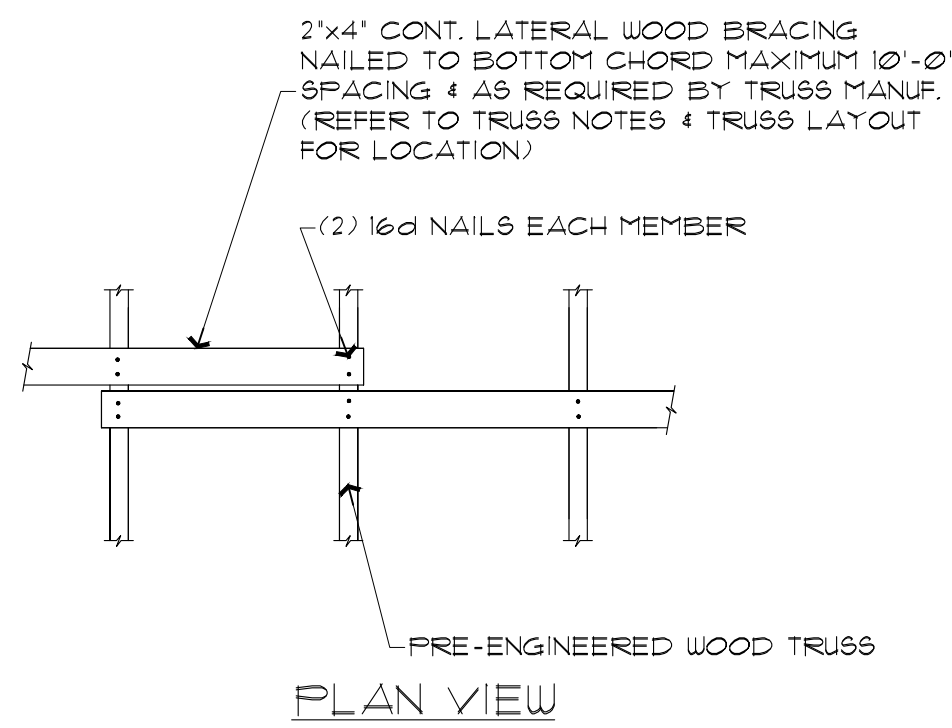
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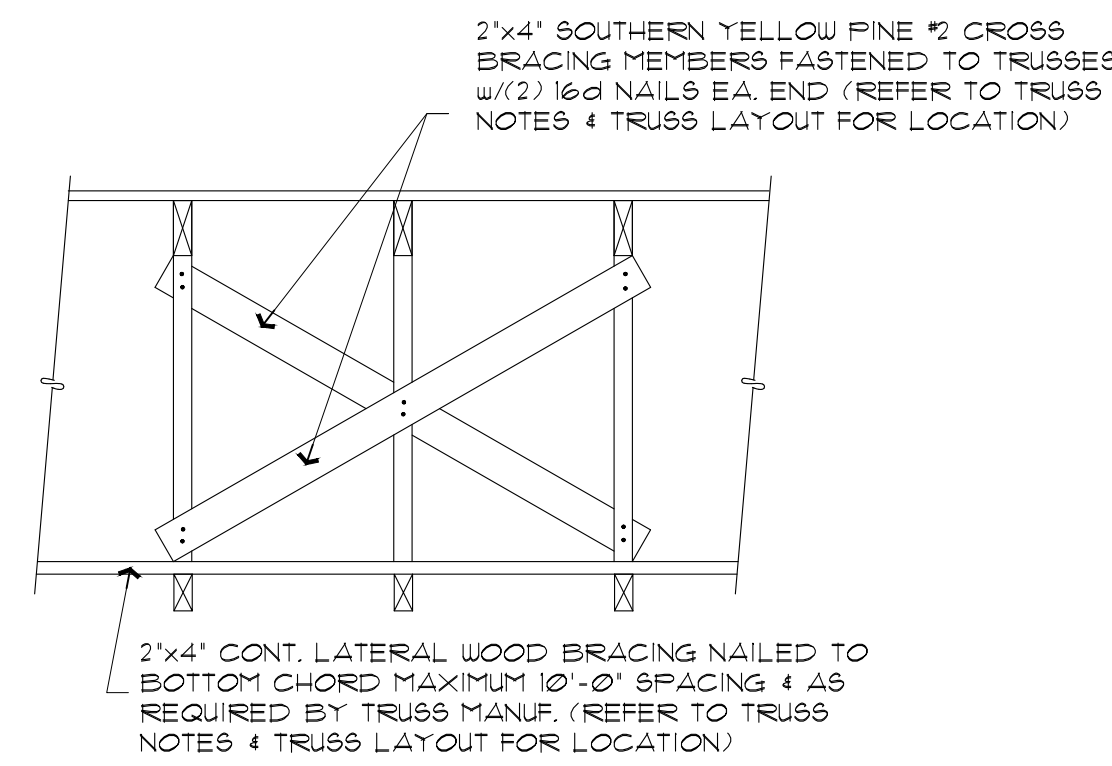
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S-5



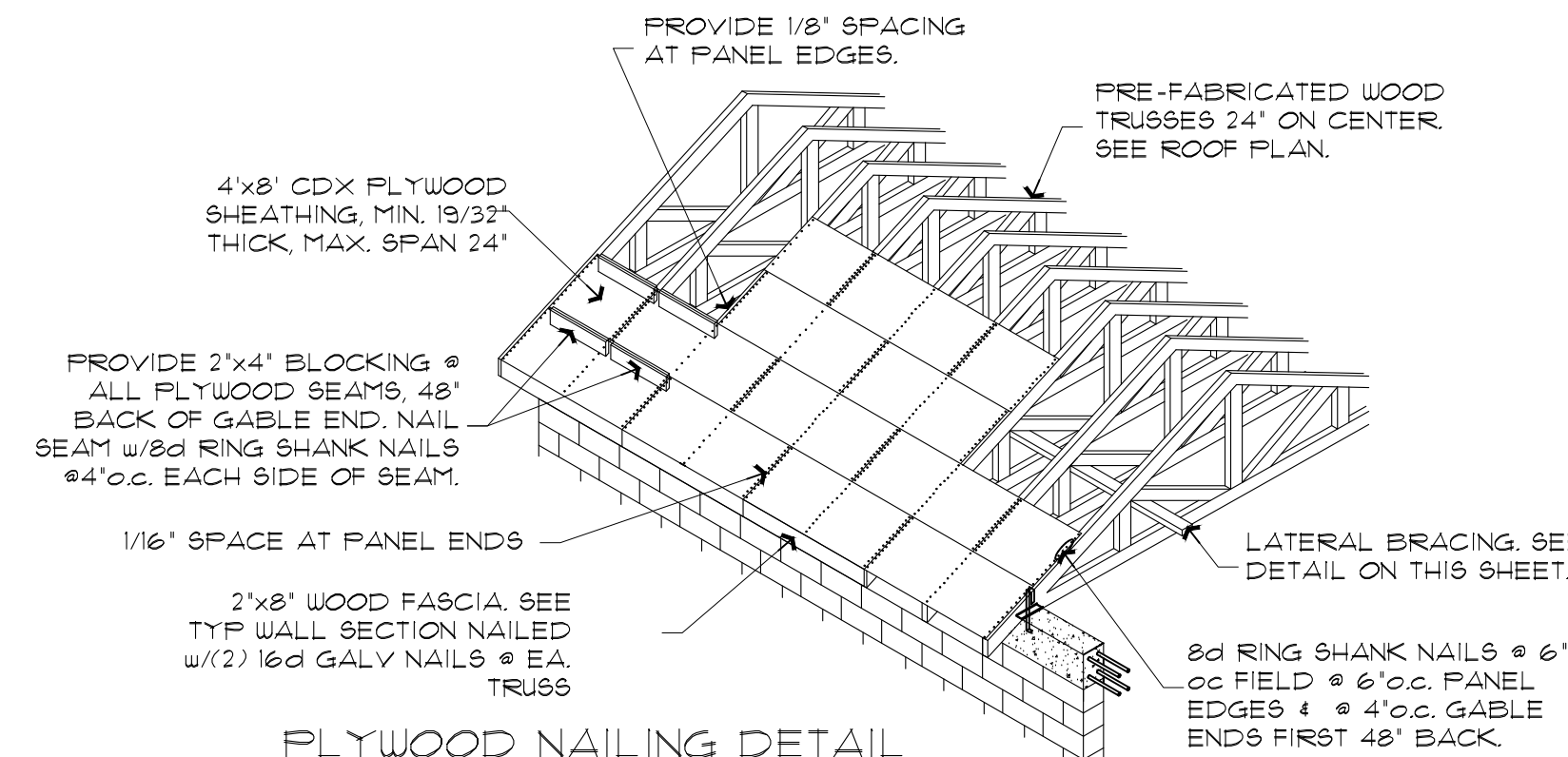
TRUSS CONNECTION
FOR PIGGY BACKS
N.T.S.



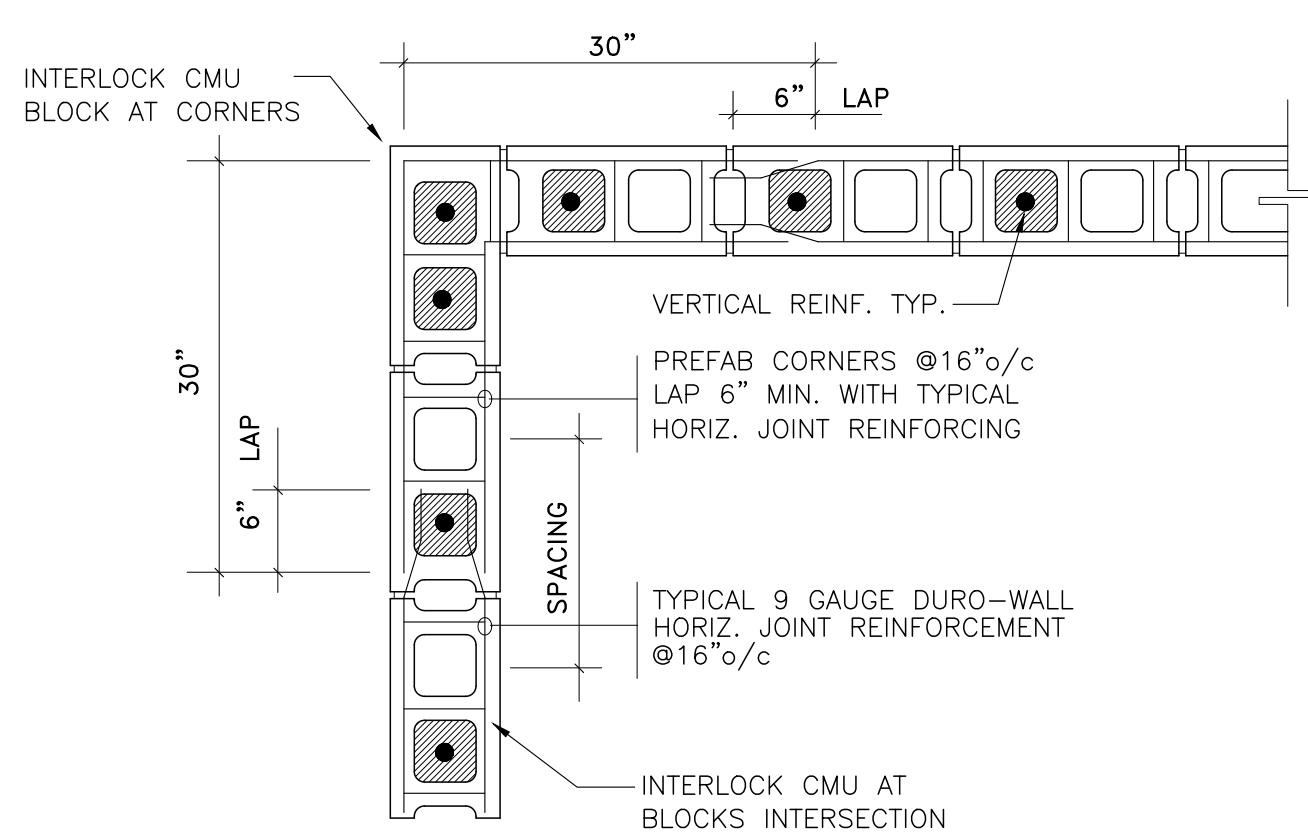
TYP. LATERAL BRACE SPLICE
SCALE: N.T.S.



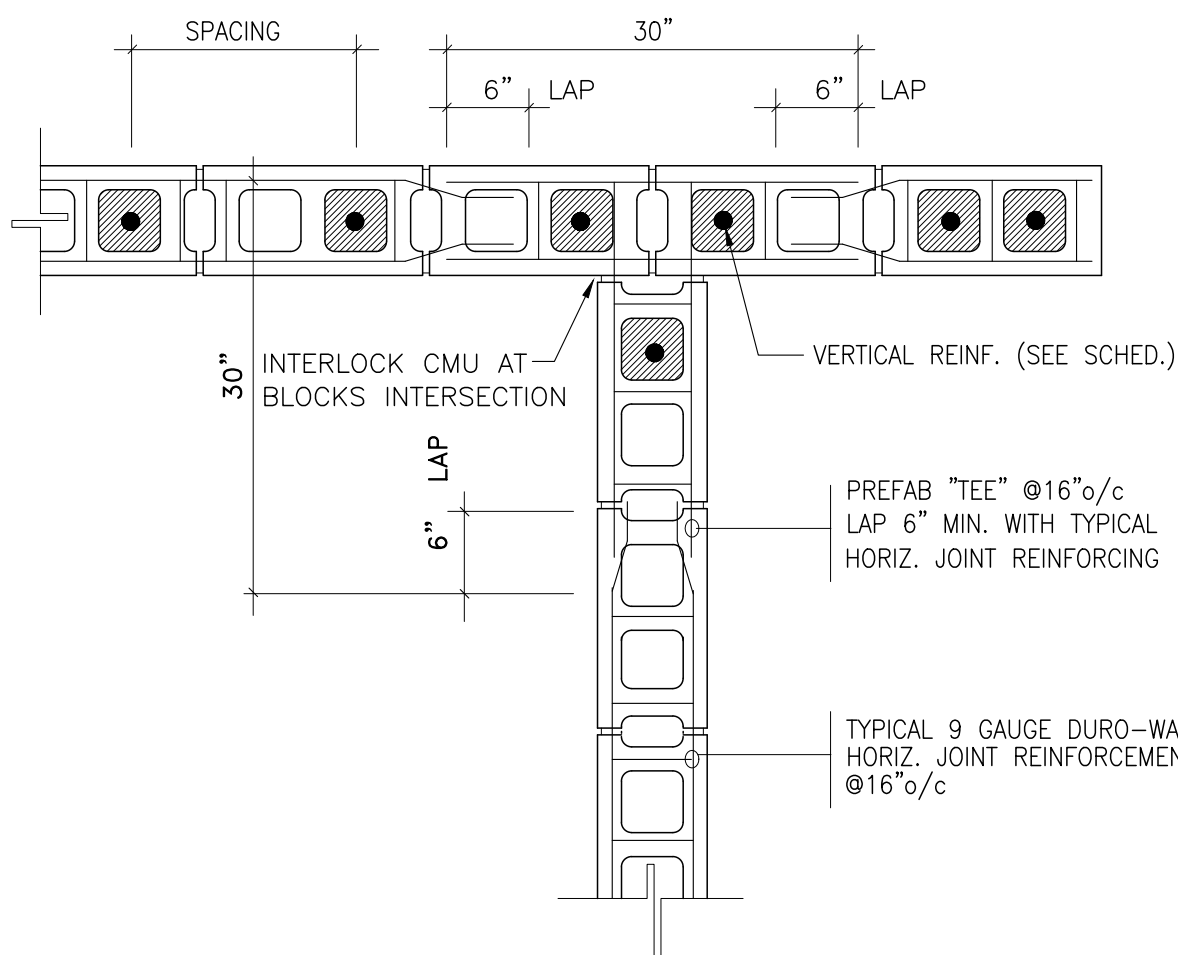
TYP. X-BRACING
SCALE: N.T.S.



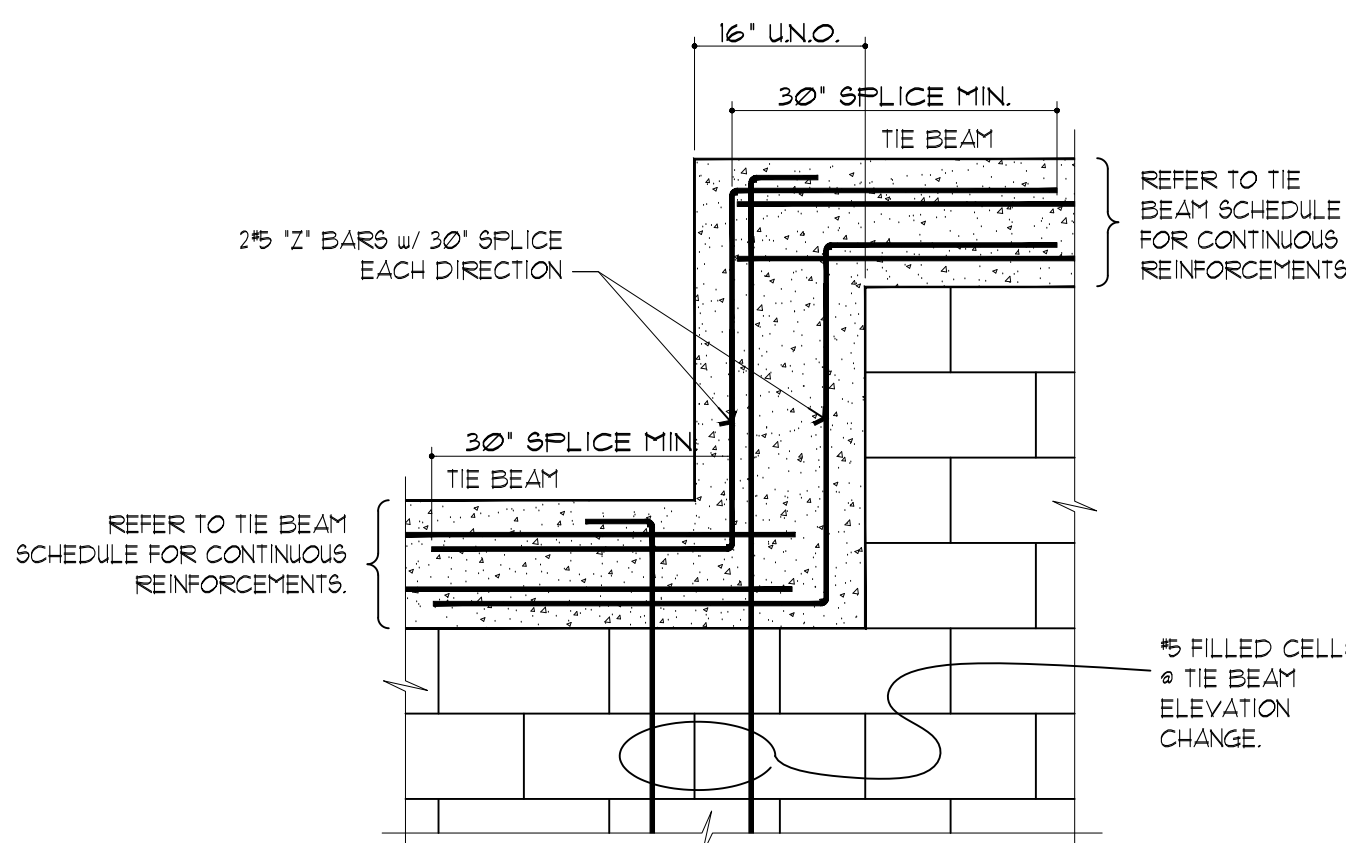
PLYWOOD NAILING DETAIL
SCALE: N.T.S.



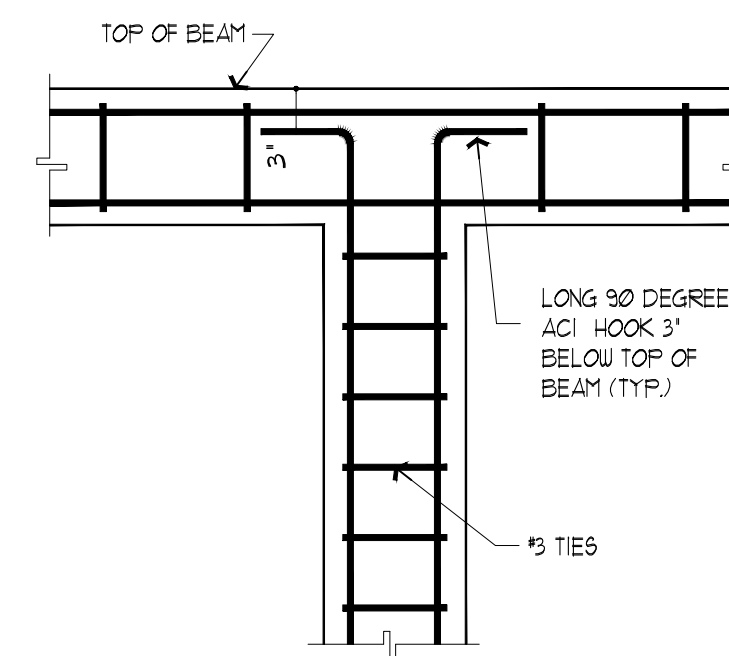
CORNER HORIZONTAL REINFORCING
CORNER VERTICAL REINFORCING
N.T.S.



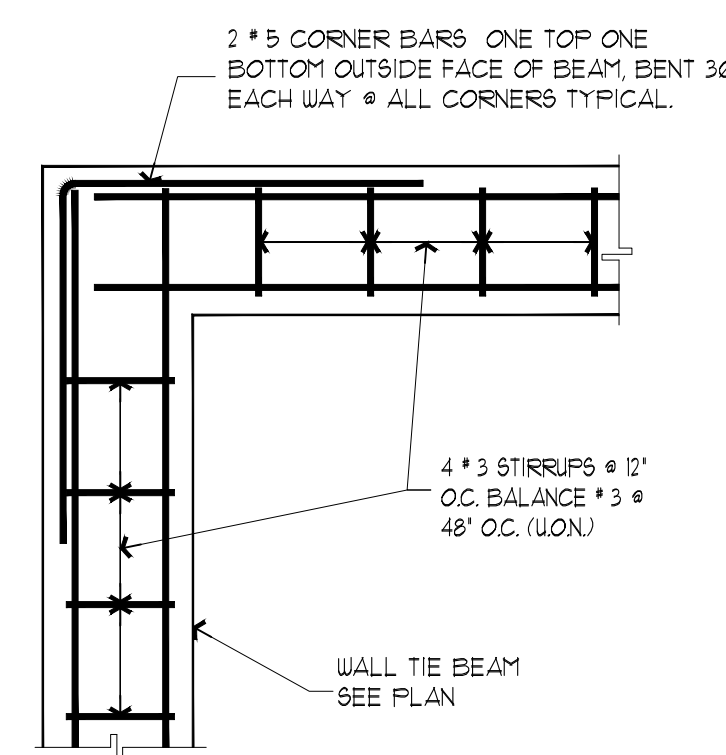
INTERSECTION HORIZONTAL REINFORCING
INTERSECTION & END VERTICAL REINFORCING
N.T.S.



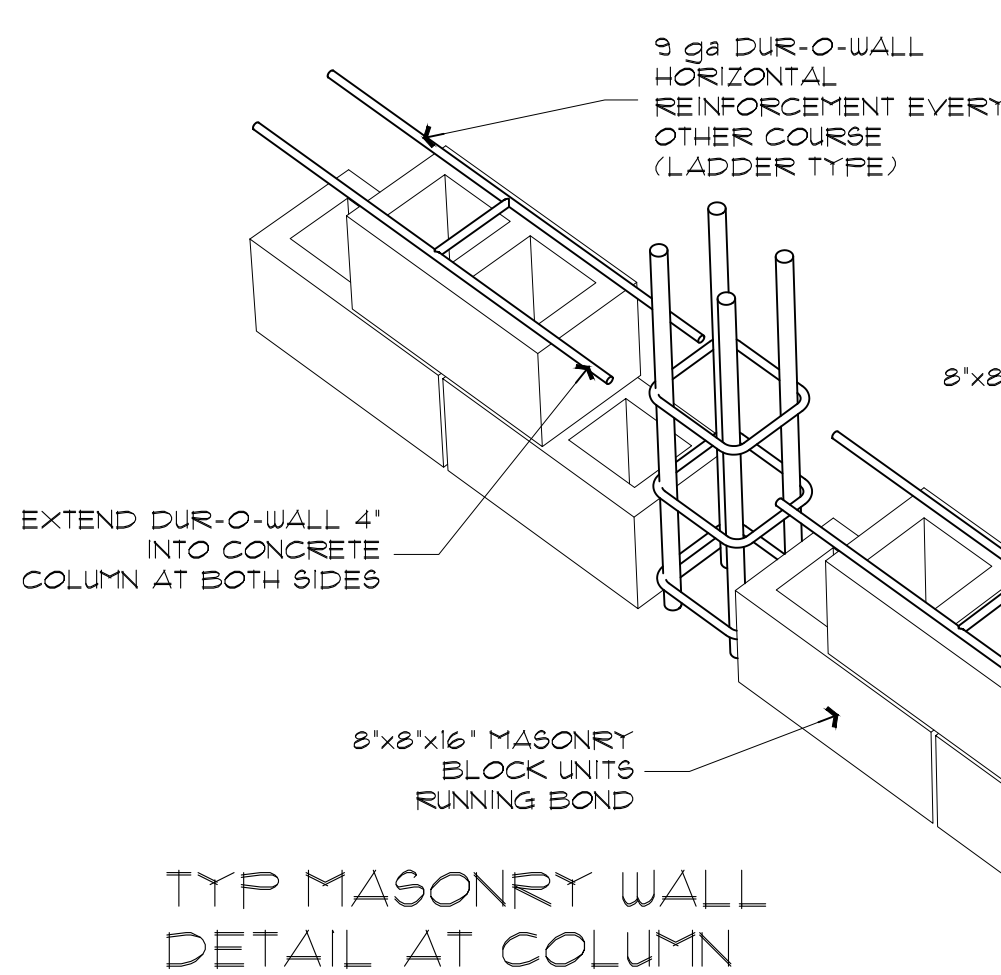
TYP. TIE BEAM ELEVATION
CHANGE DETAIL
SCALE: N.T.S.



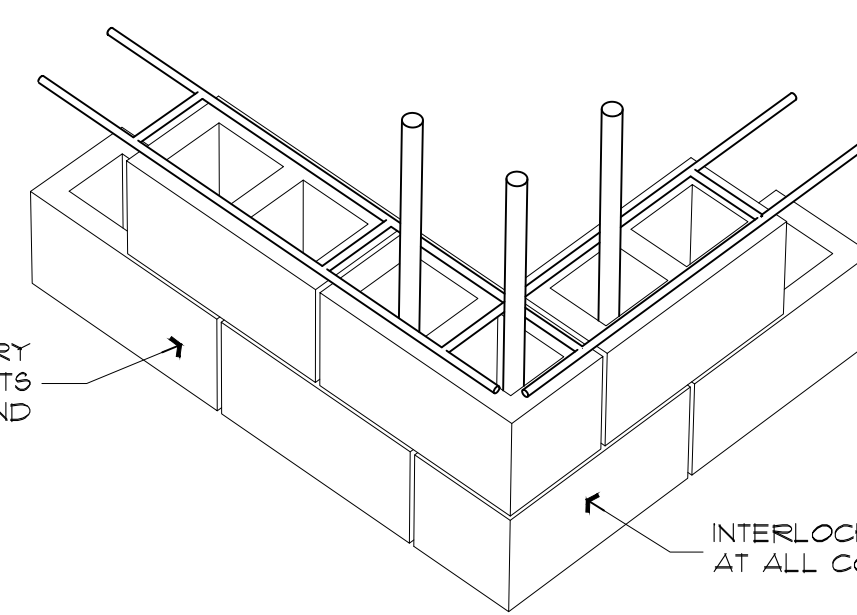
TYP. COLUMN/BEAM REINF.
TERMINATION DETAIL
SCALE: N.T.S.



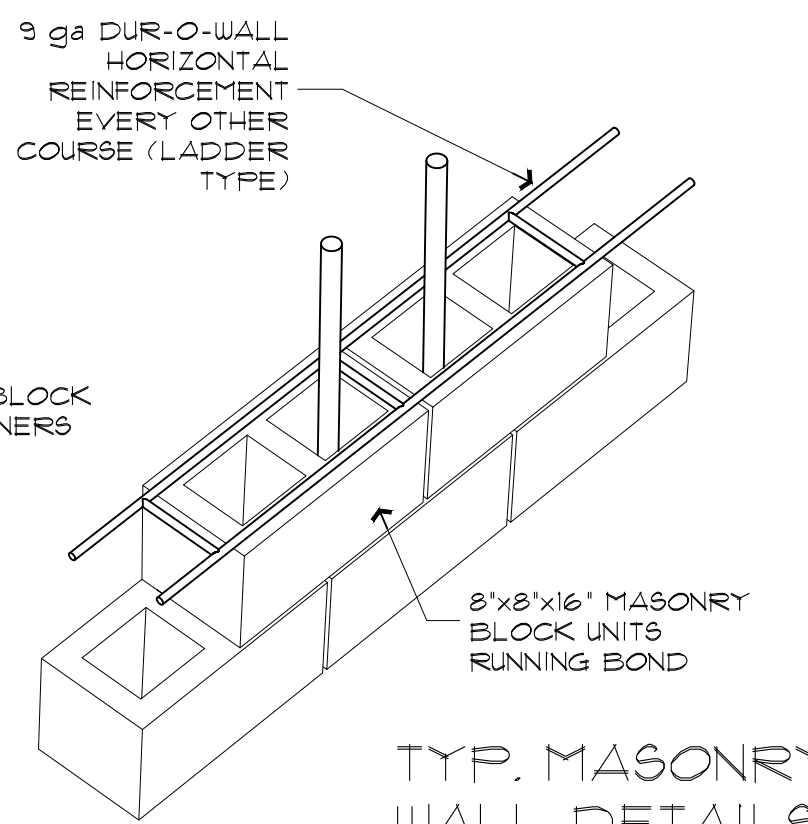
TYP. CORNER
REINFORCING DETAIL FOR
ALL TIE BEAMS
SCALE: N.T.S.



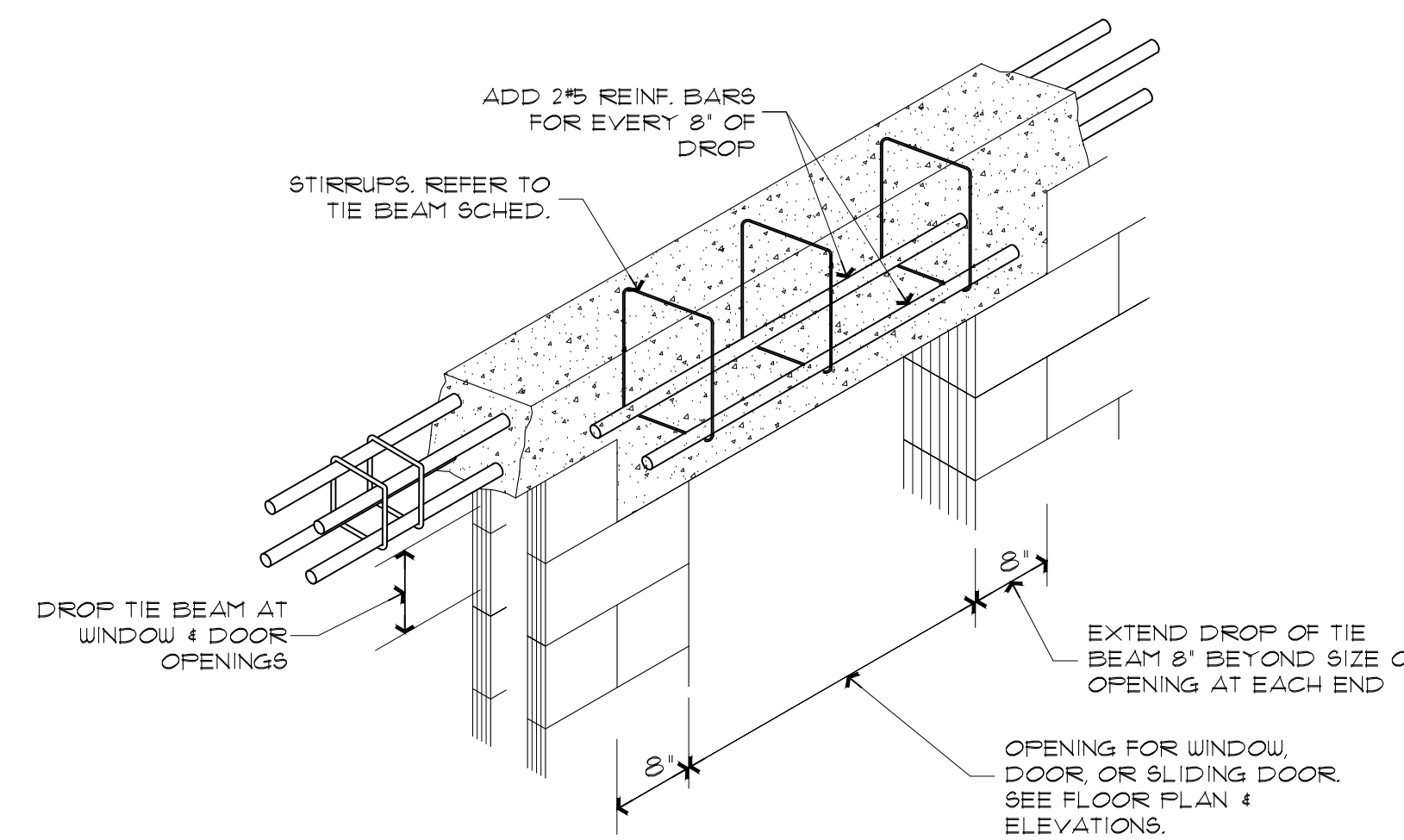
TYP MASONRY WALL
DETAIL AT COLUMN



TYP. CORNERS
WALL DETAILS



TYP. MASONRY
WALL DETAILS



TYP DROP TIE BEAM DETAIL

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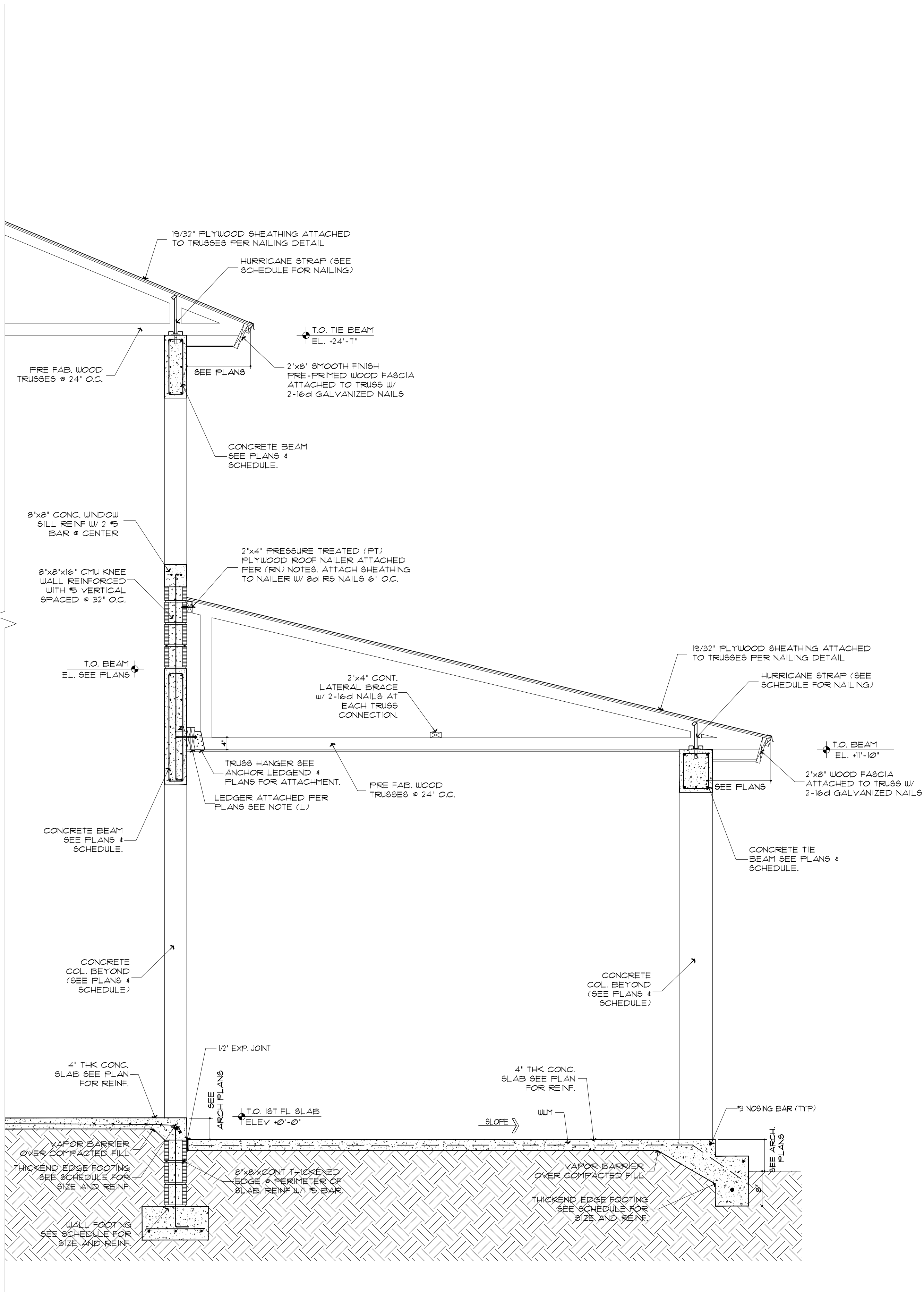
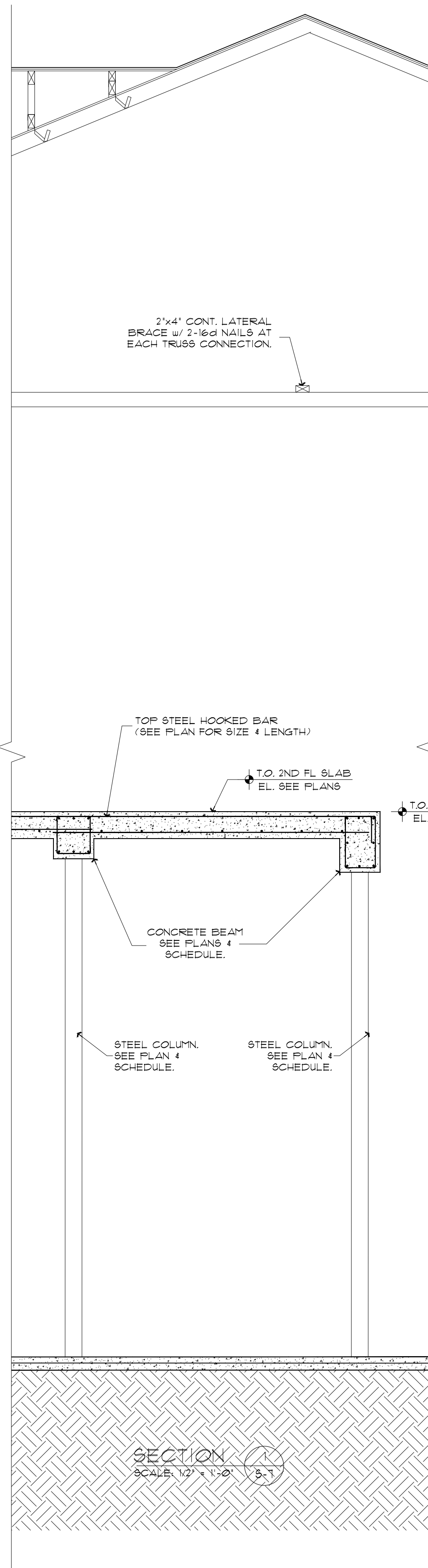
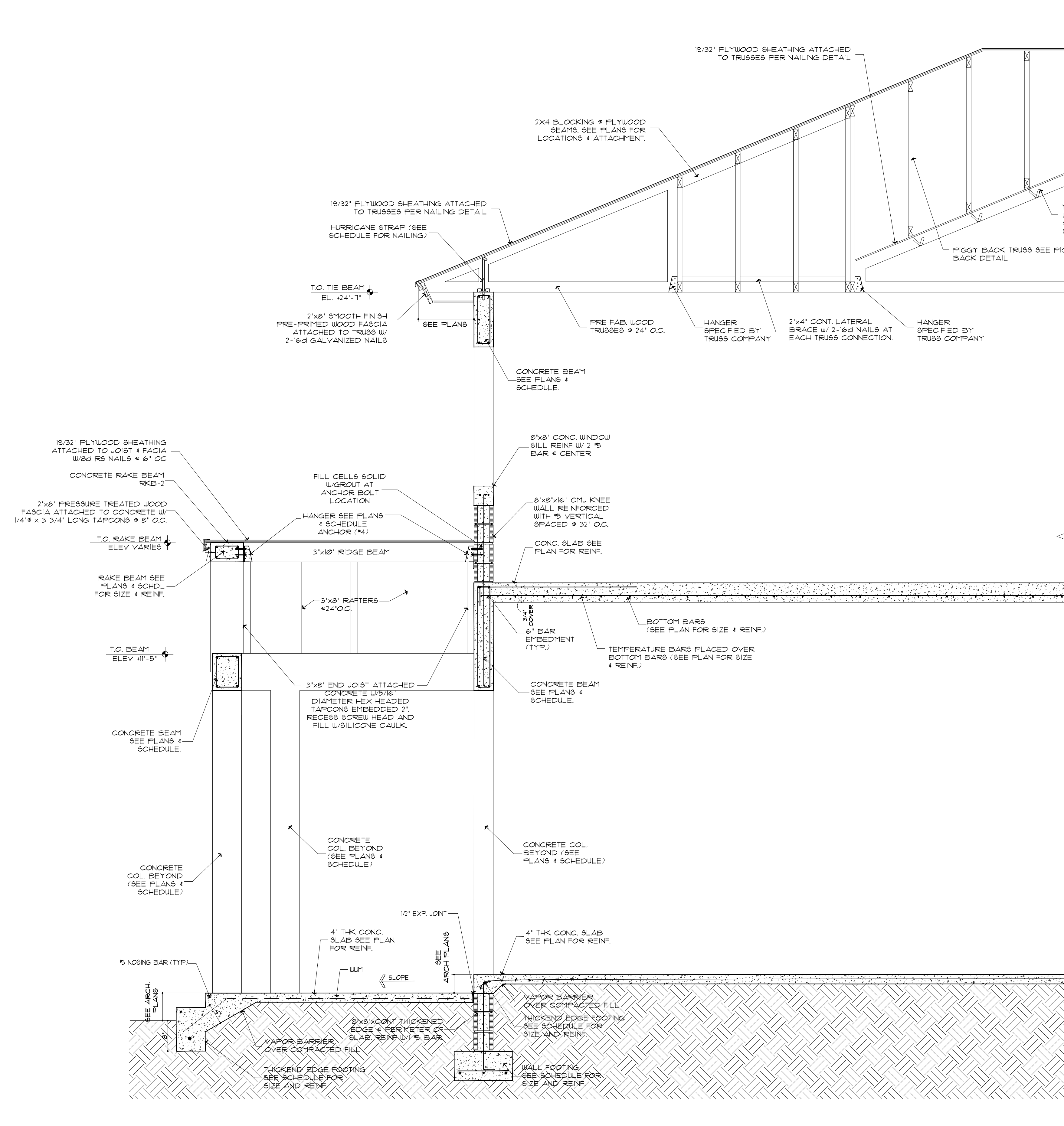
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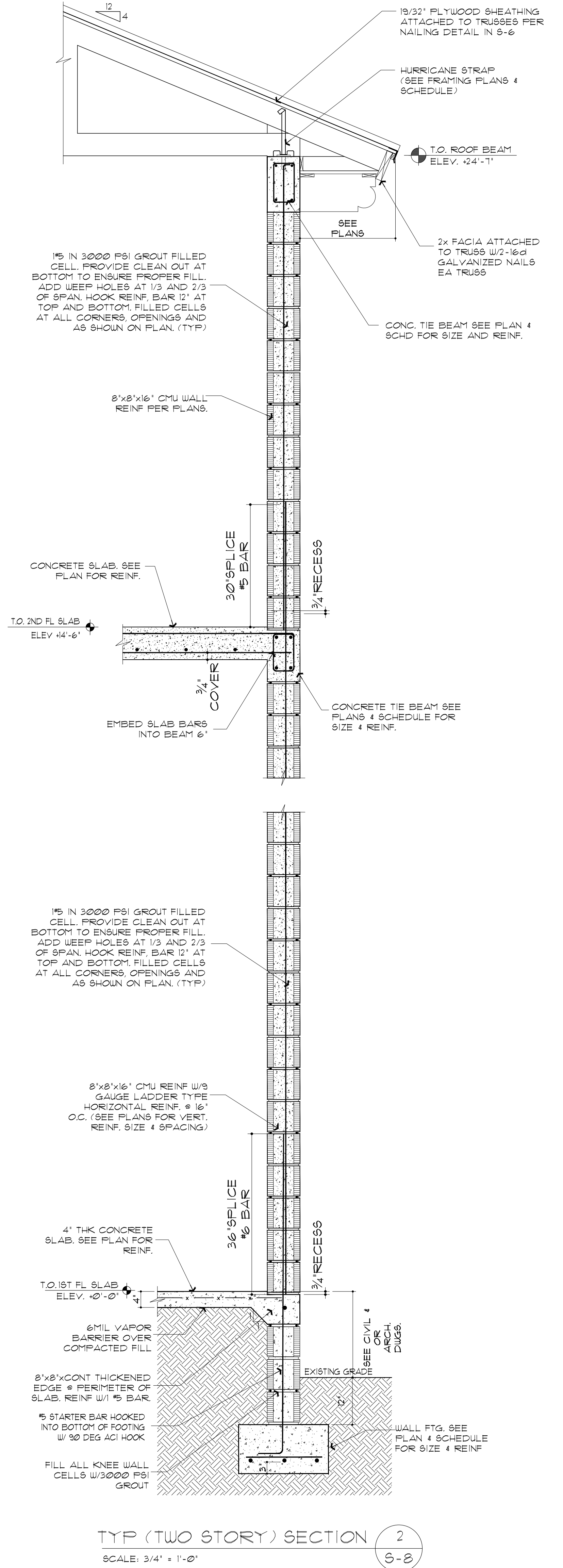
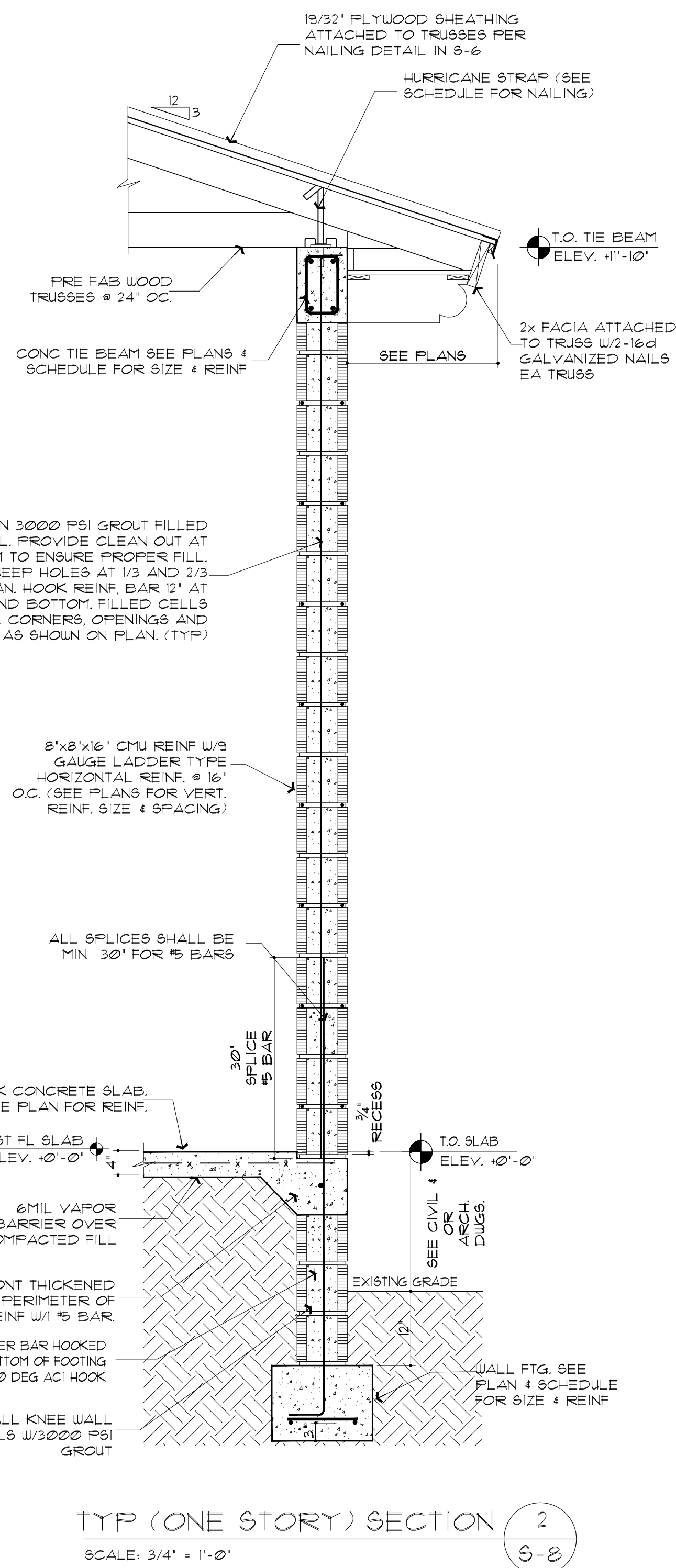
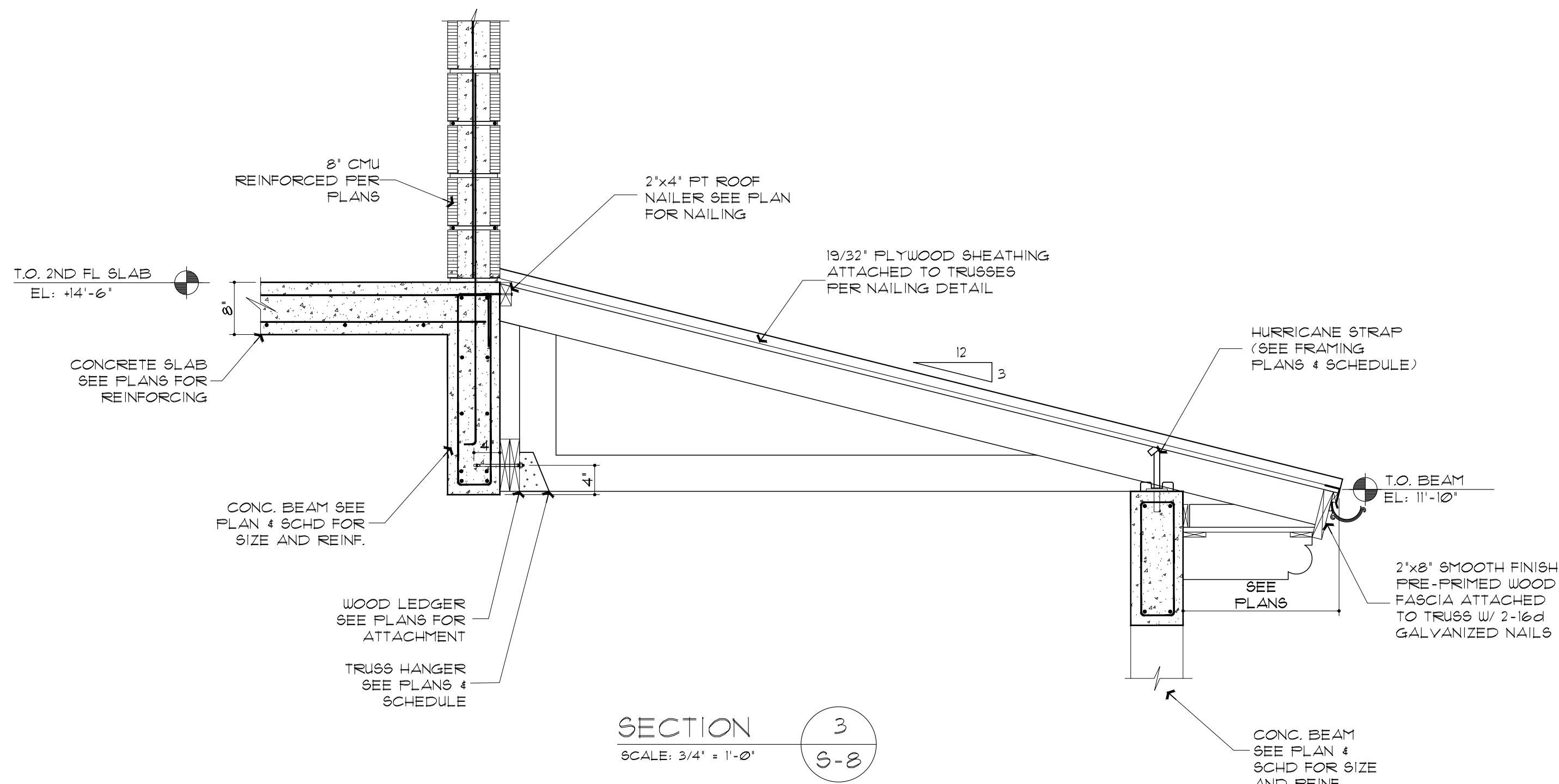
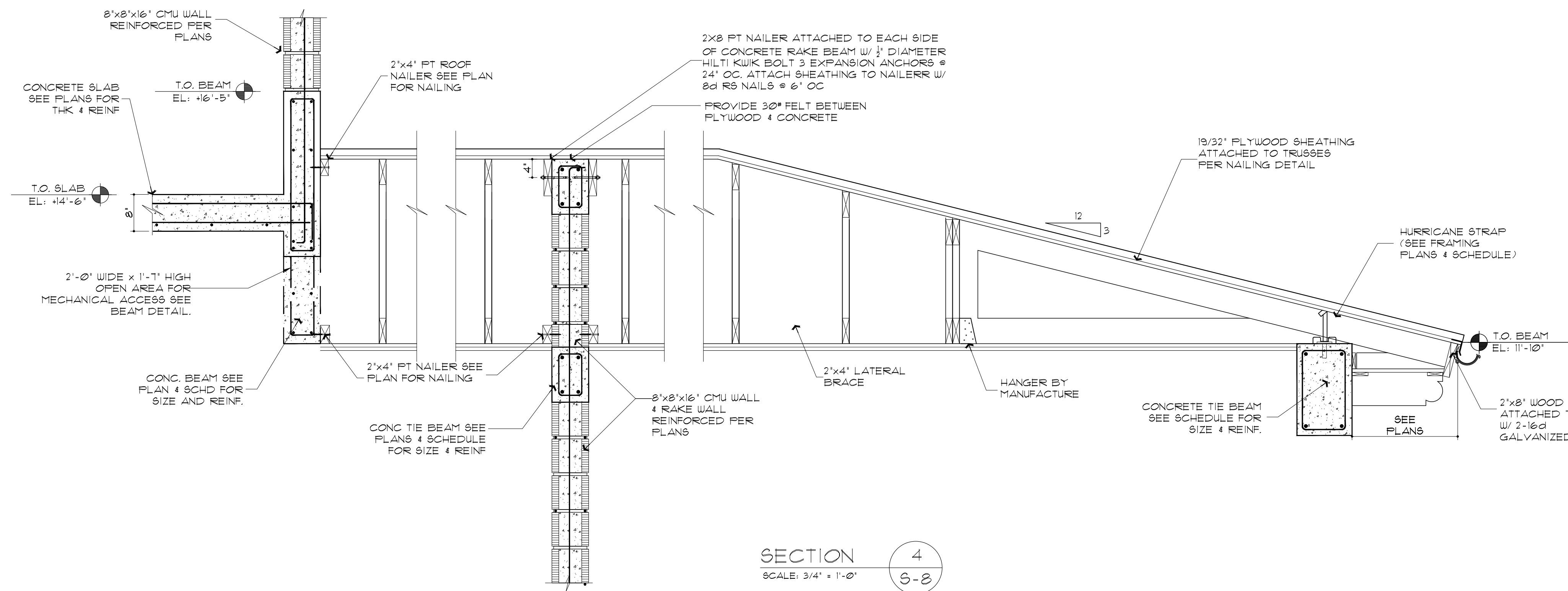
PERMIT SET 4.25.25

SHEET :

S-6



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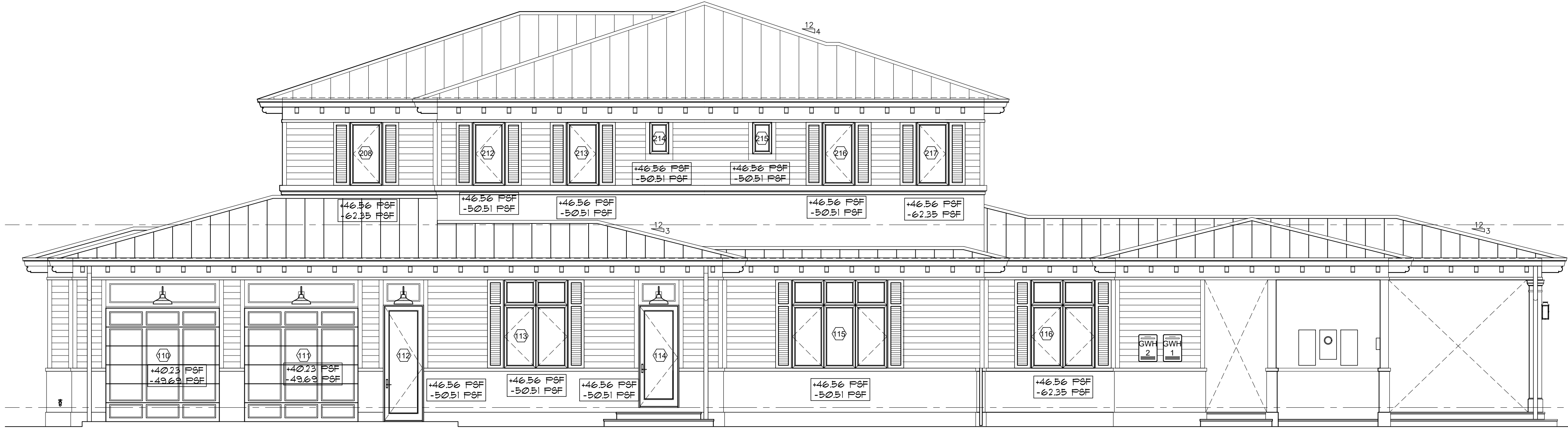
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EAST ELEVATION
SCALE: 3/16" = 1'-0"



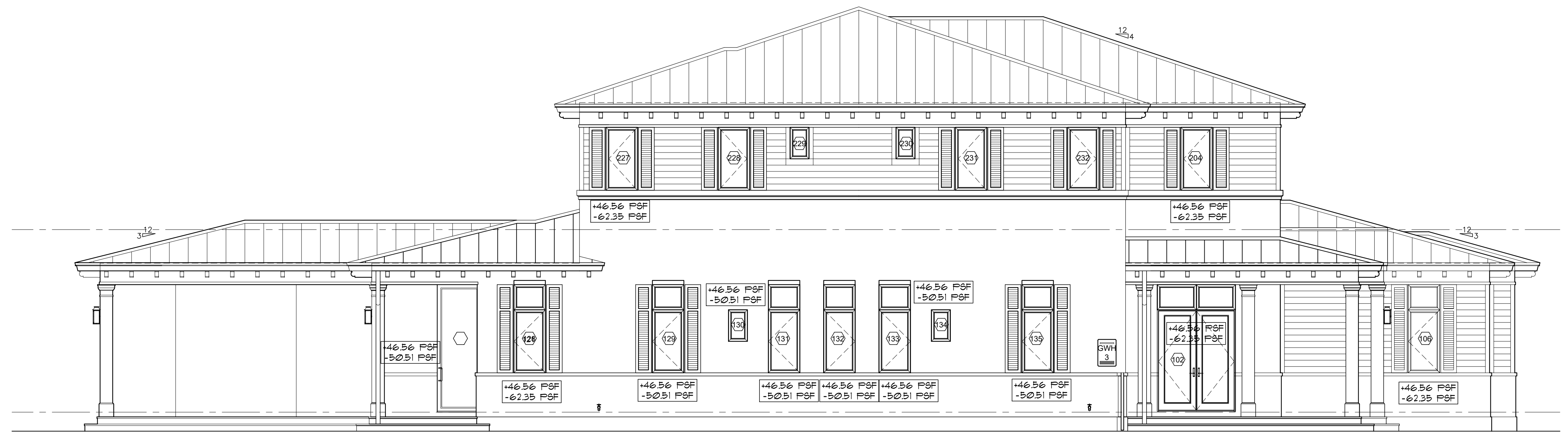
WEST ELEVATION
SCALE: 3/16" = 1'-0"



SOUTH ELEVATION
SCALE: 3/16" = 1'-0"

ALL WIND PRESSURE
VALUES SHOWN ARE ASD
WIND VALUES

FOR WINDOW & DOOR
PRESSURES NOT
SHOWN ON THE
ELEVATION USE THE
FOLLOWING PRESSURES
+46.56 PSF -50.51 PSF



NORTH ELEVATION
SCALE: 3/16" = 1'-0"

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