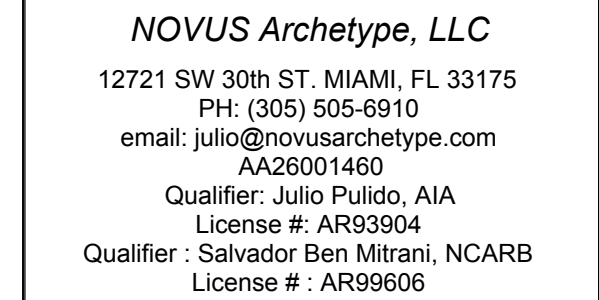




FOUNDATION SYSTEM CONSISTS OF SHALLOW SPREAD
FOOTINGS BEARING ON SAND & ROCK.
ALLOWABLE SOIL BEARING PRESSURE USED IN THE
DESIGN OF FOOTINGS IS 2000 PSF.
ENGINEER OF RECORD WILL SUBMIT A SIGNED AND
SEALED LETTER TO THE BUILDING OFFICIAL ATTESTING
THAT THE SITE HAS BEEN OBSERVED AND THE FOUND
CONDITIONS ARE SIMILAR TO THOSE UPON WHICH THE
DESIGN IS BASED BEFORE CONSTRUCTION STARTED.

ALL CONCRETE MASONRY UNITS(CMU) SHALL CONFORM TO ASTM C 90 "STANDARD SPECIFICATIONS FOR HOLLOW LOAD BEARING CONCRETE MASONRY UNITS, WITH A NET AREA COMPRESSIVE STRENGTH OF MASONRY OF 1900 PSI.

ALL MASONRY WALLS SHALL BE LOAD BEARING IN THIS LEVEL AND REINFORCED WITH # 48' U.N.O.

1. TERMITE TREATMENT OF SOILS UNDER SLAB ON GRADE SHALL BE PERFORMED BEFORE POURING SLAB. TERMITE PROTECTION SHALL BE PROVIDED BY REGISTERED TERMITICIDES OR OTHER APPROVED METHODS OF TERMITE PROTECTION LABELED AS A PREVENTIVE TREATMENT TO NEW CONSTRUCTION.

"D" BARS: 24 CORNER BARS x 36 INCHES LONG PLACED DIAGONALLY TO THE CORNER 12" APART WITH THE FIRST BAR PLACED 2 INCHES FROM THE CORNER. ALL REINFORCEMENT SHALL BE APPROPRIATELY POSITIONED IN THE UPPER THIRD OF THE SLAB.

	NEW 8' CMU WALL
	NEW CONC. COLUMN OR TIE COLUMN
	CONC. COLUMN FINISHED
	NEW TIE BM
WF	NEW WALL FOOTING
EWf	EXIST. WALL FOOTING
ETB	EXIST. TIE BEAM

WRITTEN DIMENSIONS ON THESE DRAWINGS SHALL HAVE PRECEDENCE OVER SCALE DIMENSIONS. CONTRACTORS SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSION AND CONDITIONS ON THE JOB AND THIS OFFICE MUST BE NOTIFIED OF ANY VARIATION FROM THE DIMENSIONS AND CONDITIONS SHOWN BY THESE DRAWINGS

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Interior
Remodeling &
Addition

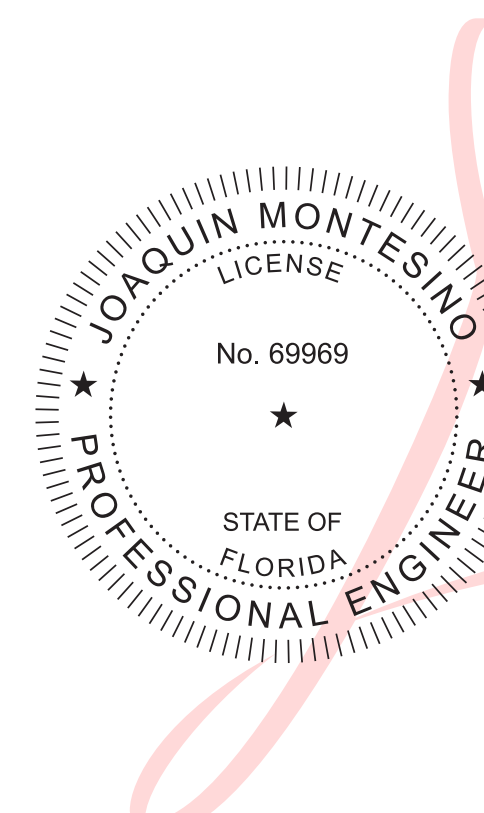
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33156

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Project number	24-011
Date	07-10-24
Drawn by	JP
Checked by	JP

SHEET No.

S-1



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signed by
JOAQUIN
MONTESINO
Date:
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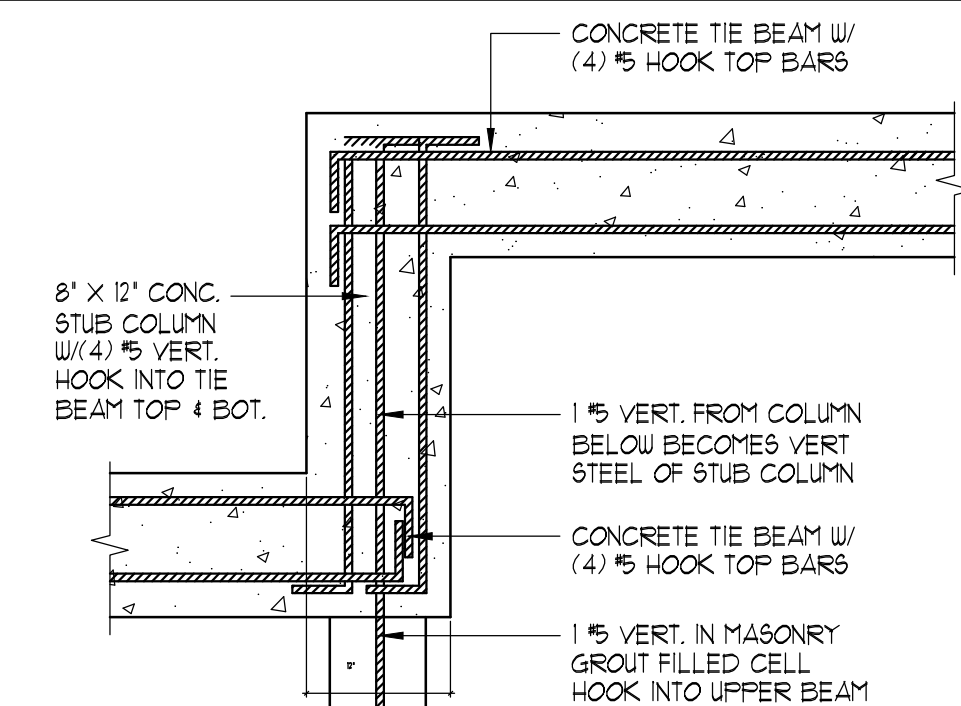
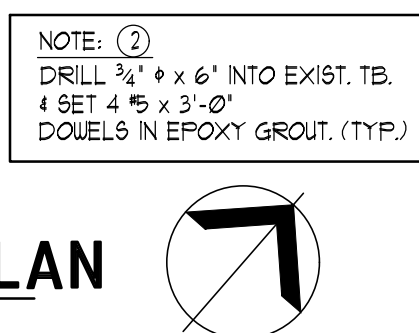
Project number	24-011
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SHEET TITLE	

SHEET No. _____

S-2



SCALE: 1/4"=1'-0"

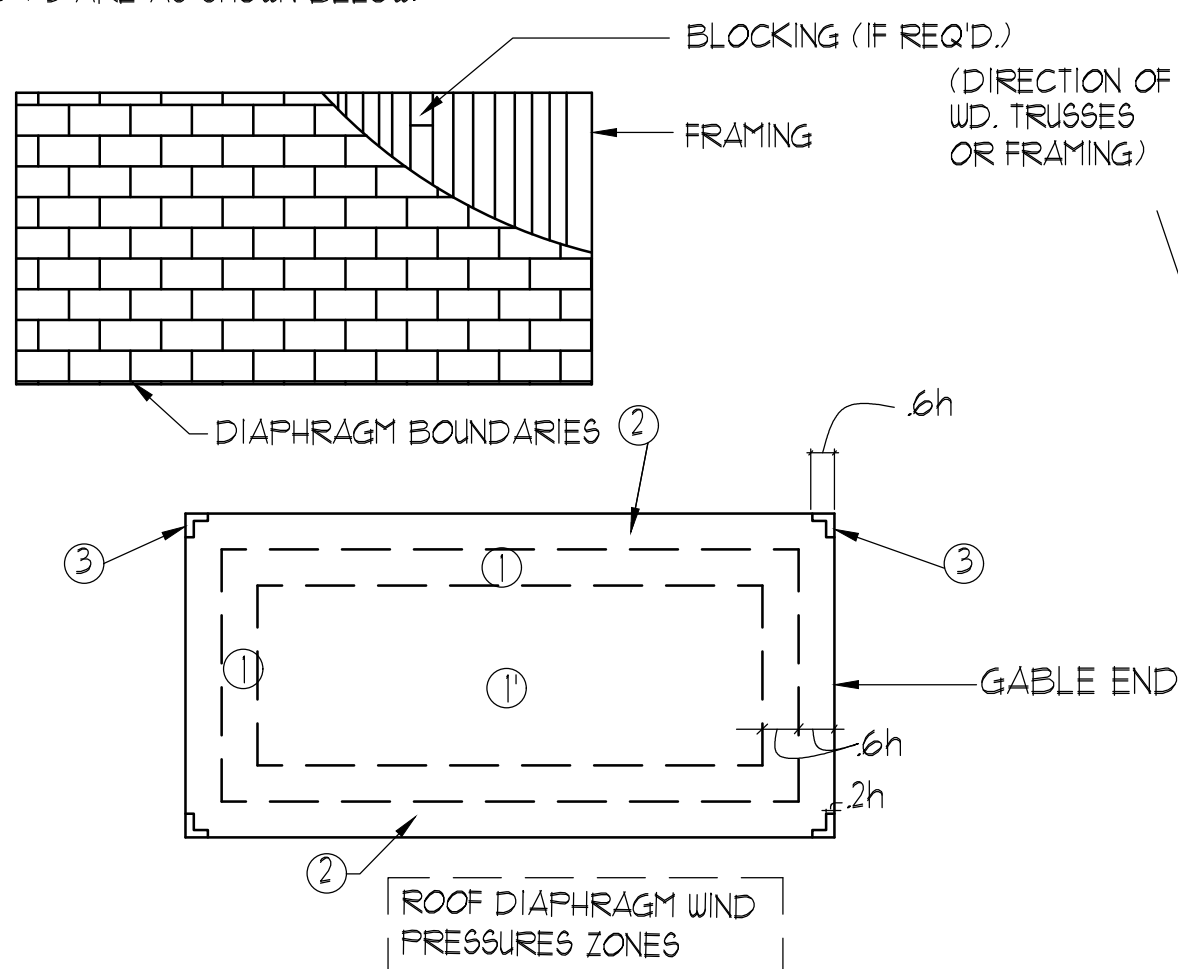


**CONC. BEAM LEVEL
CHANGE DETAIL**

N.T.S.

PLYWOOD ROOF SHEAR DIAPHRAGM SCHEDULE							
ROOF	APA RATED SHEATHING EXP. 1, OR EQUAL.	2'-0" MAX.	5/8" NOM.	4'x ZONE 2 43	4'x ZONE 2 43	100 RING SHANK NAILS	UNBLOCKED DIAPHRAGM UNO- SEE DTLS.
				6' x ZONE 1	6'x ZONE 1		

NOTES: 1. INSTALL PLYWOOD PANELS AS SHOWN ON DETAIL BELOW:
2. SPACE NAILS @ 6" O.C. ALONG INTERMEDIATE FRAMING MEMBERS FOR ROOF.
3. ZONES 12 & 3 ARE AS SHOWN BELOW:

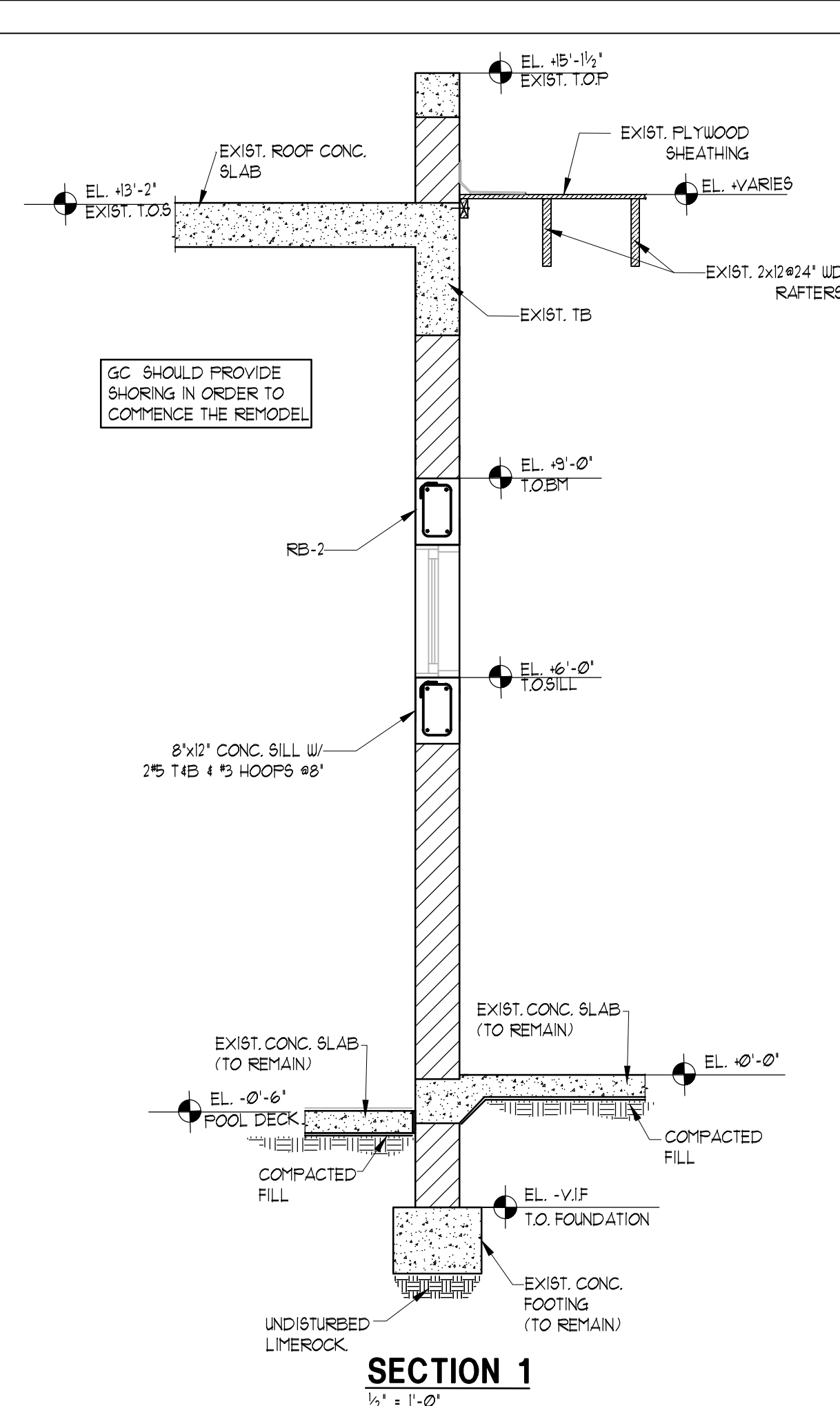


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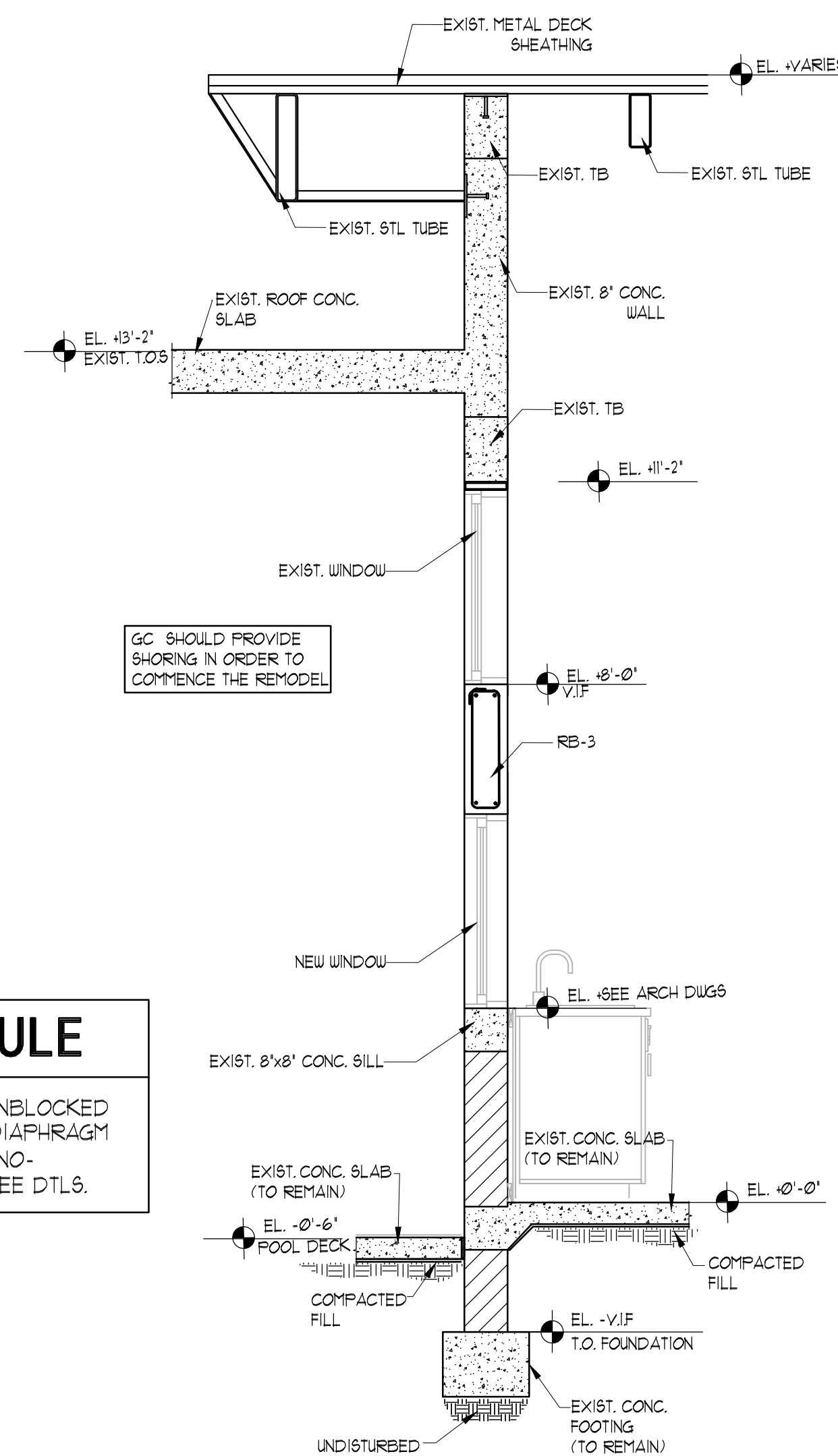
RIVETED TRUSS ANCHOR SCHEDULE CONCRETE TIE-BEAM TO TRUSS

A-X MARK	NUMBER OF STRAPS	SIZE	ASSEMBLY PRODUCT CODE (Nu-Vue)	PRODUCT CODE (Nu-Vue)	GAUGE	DIMENSIONS		NUMBER OF 10d COMM NAILS	* OF PLY	ALLOWABLE LOADS				
						OUT OF CONCRETE	OVERHALL LENGTH			LATERAL	UPLIFT	133% 10d COMM		
A-1	1	16	NVSTA 16		14	16	20	9	1	1430 #	950 #	1426 #	23-02100 #	NOA #

- USE ALL FASTENERS IN SCHEDULE TO ACHIEVE VALUES INDICATED
- MINIMUM EDGE DISTANCE IS 2"
- ALLOW 4" EMBEDMENT IN CONCRETE
- DESIGN LOADS BASED ON USE OF SOUTHERN YELLOW PINE, SG=055
- PROVIDE 6-10d COMM. NALS @ STEEL SEAT OF STRAPS FOR 1 FLY TRUSS
- PROVIDE 8-10d COMM. NALS @ STEEL SEAT OF STRAPS FOR 2 & 3 FLY TRUSSES



SECTION 1



SECTION 2

 $\frac{1}{2} = 1 - 0$

GC MUST COORDINATED THIS DRAWINGS IN CONJUNCTION WITH ARCHITECTURE, MEP AND TRUSS MANUFACTURE. GC MUST COORDINATE THE FABRICATION OF WOOD TRUSSES WITH THE MEP DRAWINGS BEFORE COMMENCE THE CONSTRUCTION PHASE

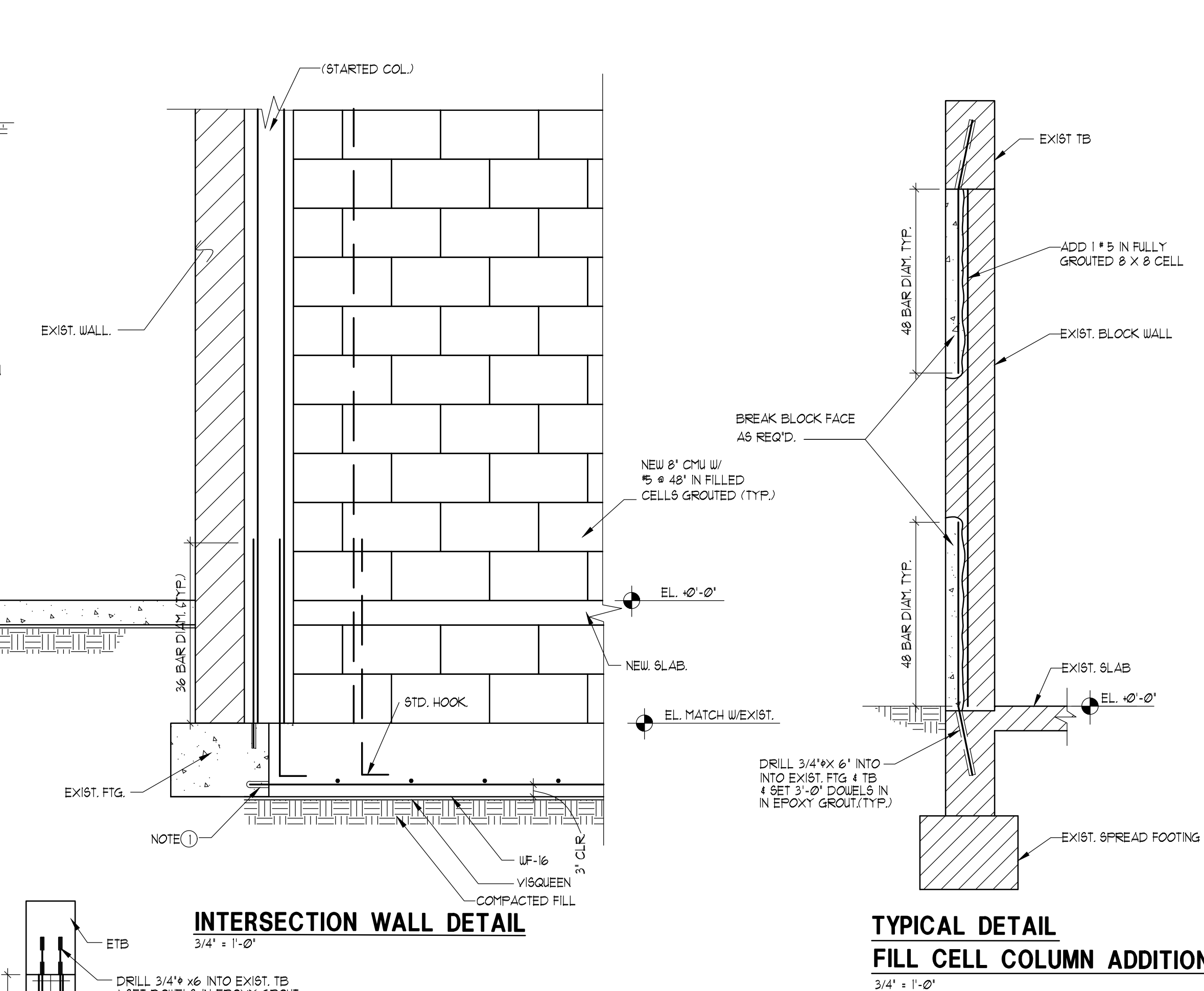
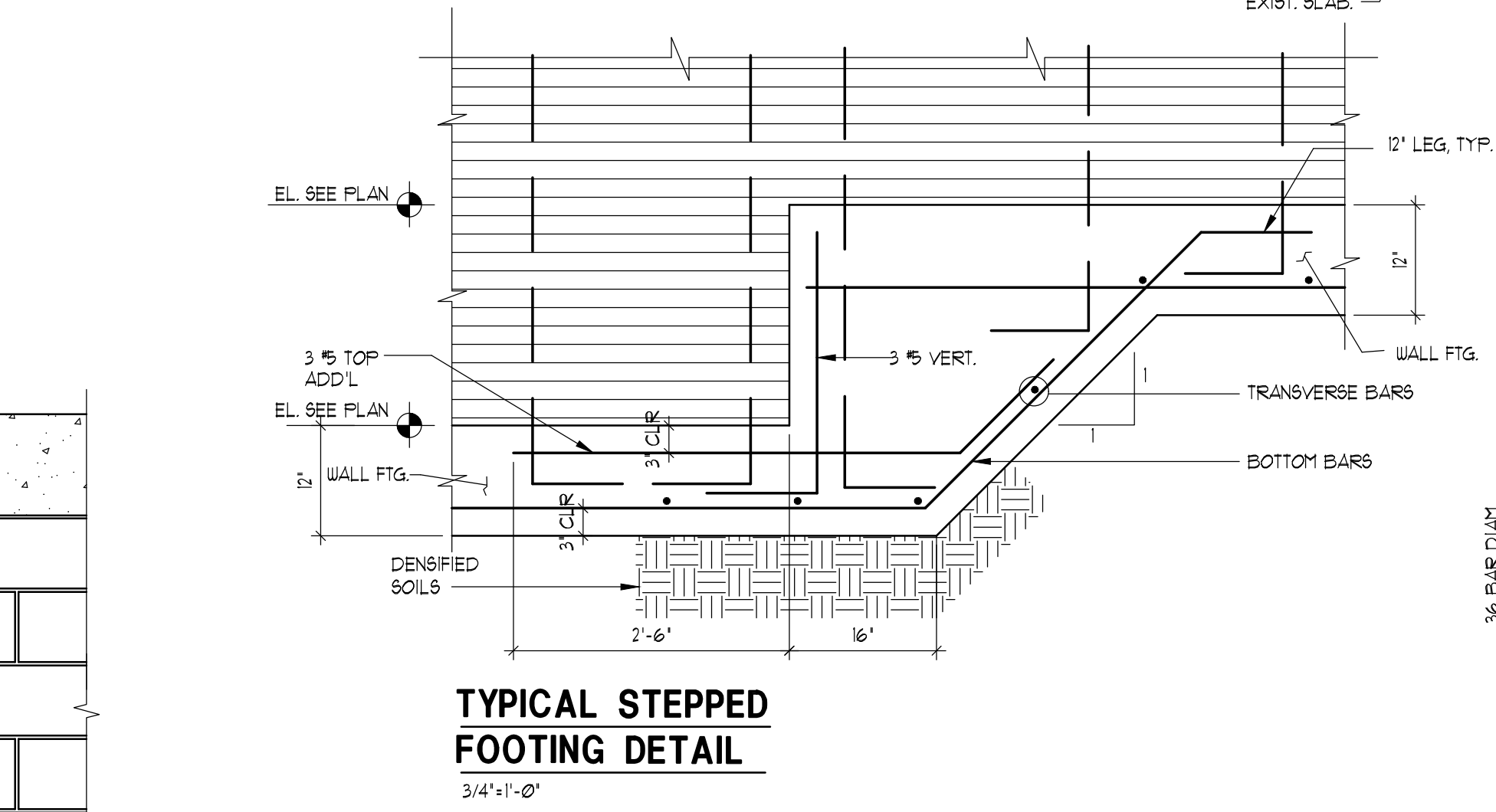
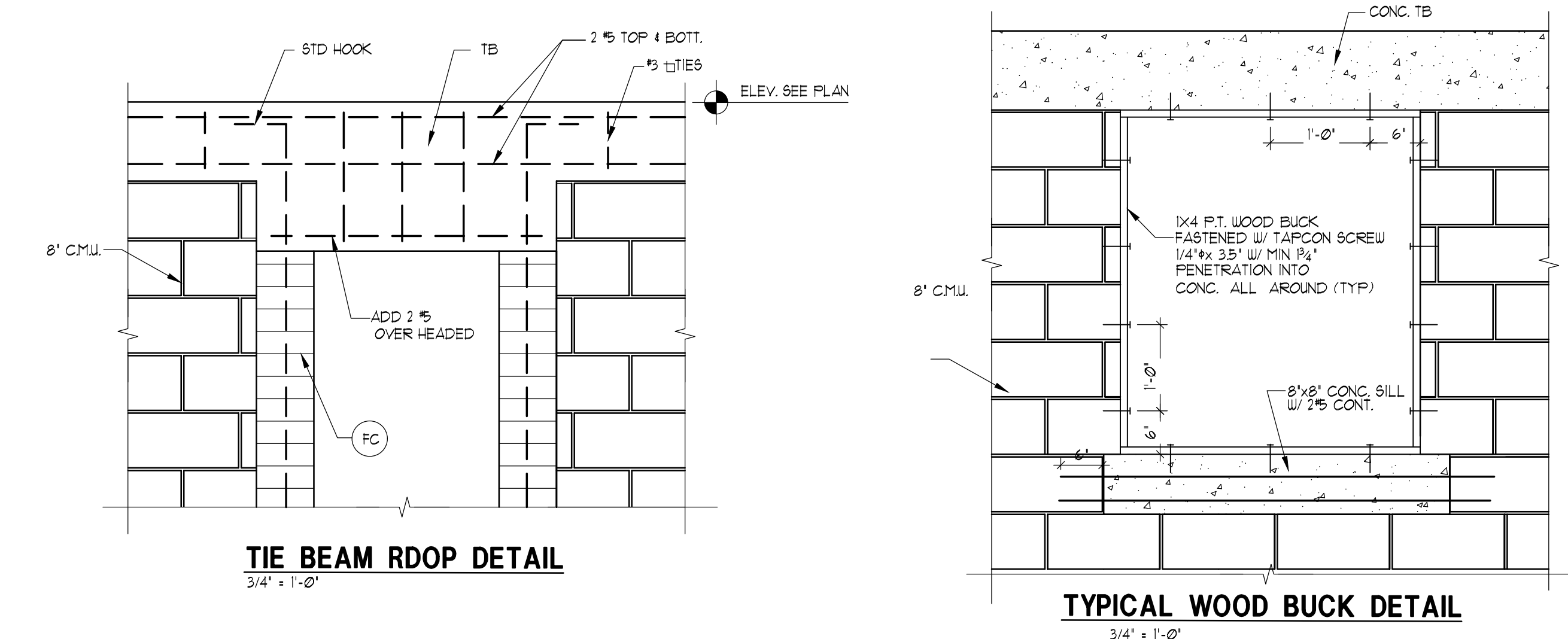
ROOF WOOD RAFTERS SUPERIMPOSED LOADS	
DEAD	30 P.S.F.
LIVE	30 P.S.F.
BASIC WIND PRESSURE, q_h	67.62 P.S.F.
NET WIND UPLIFT, ZONE 1	-62.5 P.S.F.
NET WIND UPLIFT, ZONE 2 10'-0" WIDE EDGE STRIP	-63.20 P.S.F.
NET WIND UPLIFT, ZONE 3 10'-0" WIDE EDGE STRIP	-81.6 P.S.F.

LEGEND:

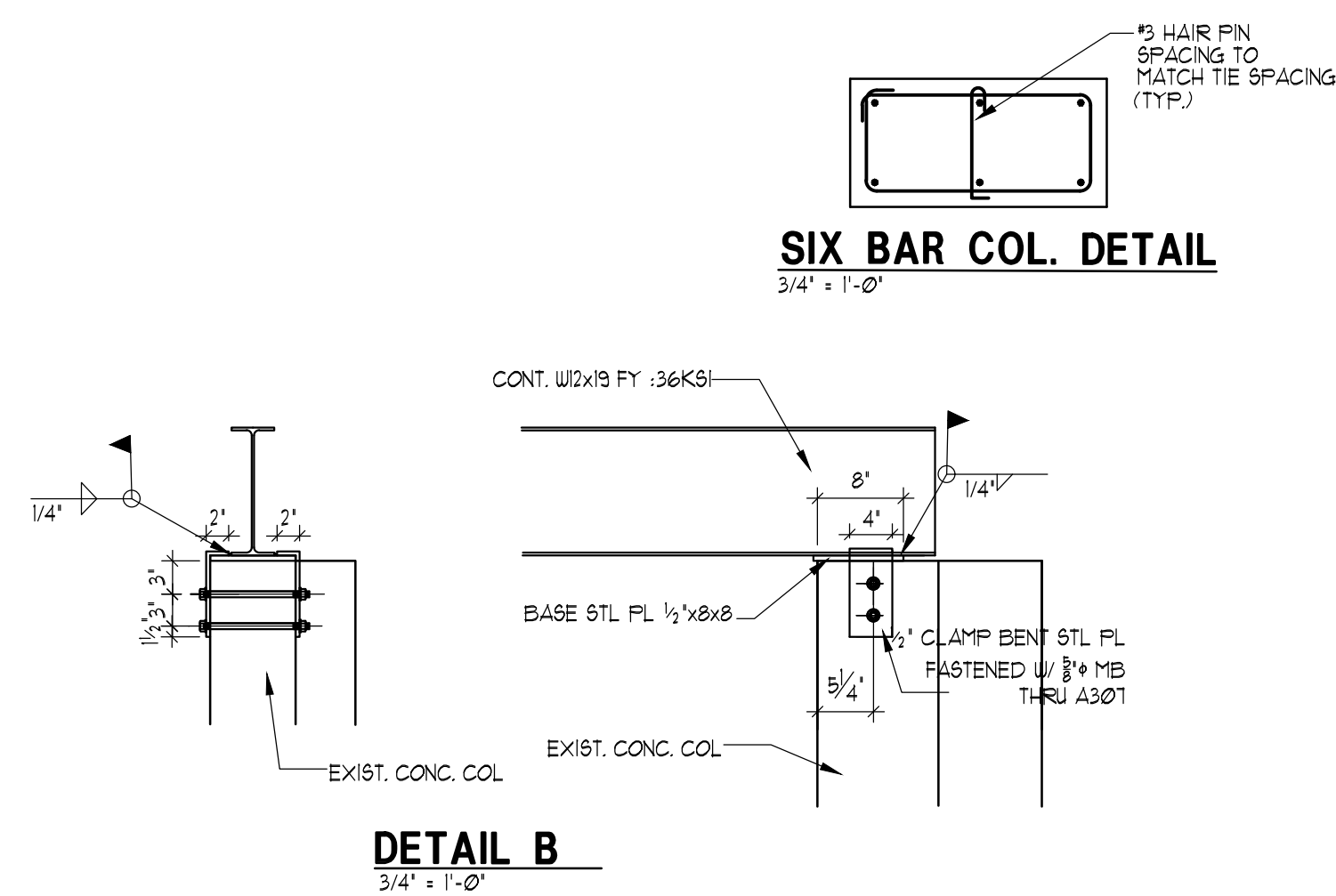
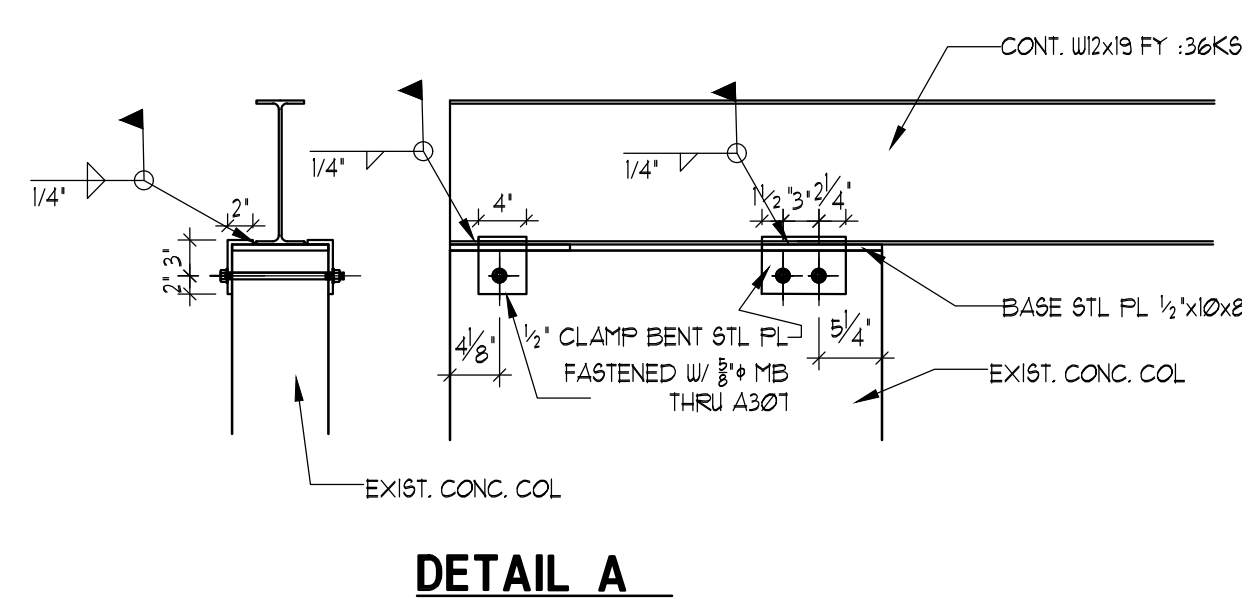
	NEW 8' CMU WALL
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FOOTING SCHEDULE						
MARK	SIZE LxWxTHICKNESS	REINFORCEMENT				REMARKS
		BOTTOM		TOP		
		SHORT BARS	LONG BARS	SHORT BARS	LONG BARS	
UF-16	CONT (16"x12")		2 #5	—	—	PROVIDE 2#5 X 2'-6"x2'-6" CORNER BARS
F40	4'-0"x 4'-0"x 12"	6 #4	6 #4			
TE	CONT 8' X 8'	—	1 #5	—	—	THICK EDGE



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Digitally signed
by JOAQUIN
MONTESINO
Date:
2025.01.17
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Project number	24-011
Date	07-10-24
Drawn by	JP
Checked by	JP
SHEET TITLE	

1. FOUNDATION:

- F. FOUNDATION SYSTEMS CONSIST OF SHALLOW SUPPORT FOOTINGS DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 12000 P.S.F. FOOTINGS SHALL BEAR ON UNDISTURBED SOIL.
- F. PROVIDE ANY BRACING OR SHORING AS REQUIRED IN ORDER TO PREVENT SETTLEMENT OR DISPLACEMENT OF ADJACENT EXISTING FOUNDATIONS AND/OR STRUCTURES.
- F. FOUNDATION EXCAVATIONS SHALL BE KEPT RYD. FOUNDATION CONCRETE SHALL BE POURED ONLY INTO EXCAVATIONS ARE COMPLETELY RYD. TAKE EVERY PRECAUTION DURING DEWATERING OPERATIONS SO THAT EXISTING FOUNDATIONS ARE NOT DAMAGED.
- F. CENTERS OF COLUMNS SHALL COINCIDE WITH CENTERS OF FOOTINGS UNLESS OTHERWISE NOTED IN FOUNDATION PLANS AND FOOTING DETAILS.
- F. THE BEST AVAILABLE INFORMATION GATHERED FROM SOIL BORING LOGS, SOIL CONDITION ENCOREDURING FOUNDATION EXCAVATIONS MUST DICTATE TYPE OF FOUNDATION. FOUNDATION EXCAVATIONS SHALL BE MINIMUM 6 INCHES INTO THE EXISTING LAYER OF BEARING STRATA.
- F. FOUNDATIONS MAY BE EARTH-PORTED IF SOIL CONDITIONS PERMIT. EXCAVATE TO EXISTING FOUNDATION.
- F. FOUNDATION EXCAVATIONS SHALL BE INSPECTED BY ENGINEER OF RECORD AND GEOTECHNICAL ENGINEER AND NO FOUNDATIONS SHALL BE POURED UNTIL THE ENGINEER CERTIFIES THAT SOIL CONDITIONS ARE AS PER GEOTECHNICAL REPORT.

2. GENERAL:

- A. ALL MATERIALS AND CONSTRUCTION SHALL COMPLY WITH THE FLORIDA BUILDING CODE, 2010 EDITION, AND ALL APPLICABLE FEDERAL, STATE AND LOCAL ORDINANCES.
- B. ALL MATERIALS AND SPECIFICATIONS COMPLY TO THE BEST OF MY KNOWLEDGE WITH THE FLORIDA BUILDING CODE, 2010 THE EDITION HVZ.
- C. THE CONTRACTOR SHALL VERIFY ALL CONDITIONS OF EXISTING STRUCTURES AFFECTING THE PROPOSED CONSTRUCTION. ANY DISCREPANCIES OR VARIATIONS IN ACTUAL FIELD CONDITIONS/DIMENSIONS FROM THOSE SHOWN IN THE CONTRACT DOCUMENTS SHALL BE REPORTED TO THE ARCHITECT/ENGINEER FOR REVIEW AND APPROVAL PRIOR TO PROCEEDING WITH THE WORK. WORK STOPPING REQUIRED FOR REVIEW.
- D. THE CONTRACTOR SHALL BE RESPONSIBLE TOGETHER WITH ARCHITECTURAL, AIR CONDITIONING, HEATING, AND ELECTRICAL READINGS TO LOCATE DERESSED SLABS, FLOORS, RADIANT OUTLETS, REPAIRS, OPENINGS, REGISTS, BOLT LOCATIONS, ETC. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE ATTENTION OF ARCHITECT/ENGINEER BEFORE PROCEEDING WITH THE WORK. ANY DISCREPANCIES, OMISSIONS OR VARIATIONS FOUND ON THE READINGS OR IN THE SPECIFICATIONS SHALL BE REPORTED TO THE ARCHITECT/ENGINEER AND BE IMMEDIATELY COMMUNICATED TO ARCHITECT/ENGINEER.
- E. WHEN PERFORMING WORK BELOW GRADE, CARE SHALL BE TAKEN TO AVOID DAMAGING EXISTING UTILITIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DURING CONSTRUCTION SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER. ANY DAMAGE TO THE EXISTING UTILITIES SHALL BE REPORTED IMMEDIATELY TO THE ARCHITECT/ENGINEER FOR REVIEW AND APPROVAL.
- F. GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR UPDATING HIS CONSTRUCTION DOCUMENTS WITH ANY REVISIONS AND/OR COMMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING ALL NECESSARY SKETCHES ISSUED DURING THE COURSE OF CONSTRUCTION.
- G. ALL INFORMATION OBTAINED ON THESE READINGS SHALL APPLY UNLESS SPECIFICALLY NOTED OTHERWISE. CONSTRUCTION DETAILS AND SECTIONS NOT COMPLETELY SHOWN OR NOTED SHALL BE SIMILAR TO DETAILS AND SECTIONS SHOWN IN THE NOTES AND DETAILS.
- H. THE GENERAL CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING, AND PROTECTION OF ADJACENT PROPERTY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ADJACENT PROPERTY.
- I. EXCAVATION OF THE EXTERIOR PERIMETER OF WALLS SHALL NOT BE PLACED UNTIL AFTER THE WALLS ARE SUPPORTED BY THE COMPLETION OF INTERIOR FLOOR SYSTEMS. DO NOT PROCEED WITH BACKFILL UNTIL 7 DAYS AS A MINIMUM AFTER THE INTERIOR FLOOR SYSTEMS ARE COMPLETED.
- J. BACKFILL SHALL NOT BE PLACED UNTIL AFTER THE COMPLETION AND INSPECTION OF WATERPROOFING WHERE WATERPROOFING OCCUR.
- K. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE REMOVAL AND DISPOSAL OF ALL ACCUMULATED WATER FROM EXCAVATIONS AND DEWATERING OPERATIONS IN SUCH A WAY AS NOT TO CAUSE INCONVENIENCE TO THE WORK AND DAMAGE TO THE STRUCTURE.
- L. STRUCTURAL NOTES SHALL BE USED IN CONJUNCTION WITH THE SPECIFICATIONS. IF A CONFLICT EXISTS, THE MORE STRINGENT GOVERNS.
- M. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL UTILITIES STRUCTURES AND UTILITIES LINES AND CONDUITS FROM DAMAGE. GENERAL CONTRACTOR IS SOLELY RESPONSIBLE FOR DAMAGE OR INJURY DUE TO HIS ACT OR NEGLIGENCE.
- N. GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION AND CONSTRUCTION PROCEDURES.
- O. DO NOT MAKE ANY CHANGES OR MODIFICATIONS NOTED. IF DIMENSIONS ARE MISSING CONSULT ARCHITECT/ENGINEER FOR CORRECTION.

3. SHOP DRAWINGS SUBMITTALS:

- A. SUBMITTALS TO STRUCTURAL ENGINEER OF RECORD:
- I. CONCRETE TEST REPORTS FOR CAST-IN-PLACE CONCRETE AS PER ACI 301-11.
 - II. REINFORCING STEEL SHOP DRAWINGS.
 - III. PREFABRICATED WOOD AND/OR METAL ROOF TRUSSES, ERECTION DRAWINGS, AND DESIGN DATA.
 - IV. STRUCTURAL STEEL SHOP DRAWINGS AND ERECTION DRAWINGS.
 - V. SHORING AND RESHORING SHOP DRAWINGS AND CALCULATIONS.

B. SHOP DRAWINGS REQUIRING ENGINEERING BY A SPECIALTY ENGINEER SHALL COMPLY WITH THE FOLLOWING:

- A. THE SPECIALTY ENGINEER SHALL BE A FLORIDA REGISTERED PROFESSIONAL ENGINEER.
- B. THE SPECIALTY ENGINEER SHALL HAVE AT LEAST FIVE YEARS OF SOLID PROFESSIONAL DESIGN EXPERIENCE IN THE DESIGN AND DETAILING OF THE STRUCTURAL COMPONENT AND/OR STRUCTURAL SYSTEM BEING SUBMITTED FOR APPROVAL.
- C. THE SPECIALTY ENGINEER SHALL BE AN EMPLOYEE OR OFFICER OF A FABRICATOR, AN EMPLOYEE OR OFFICER OF ANOTHER COMPANY SUPPLYING COMPONENTS TO THE FABRICATOR.
- D. ALL SHOP DRAWINGS ARE AN AID FOR FIELD PLACEMENT AND ARE SUPERSEDED BY THE STRUCTURAL DRAWINGS. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO BE CERTAIN THAT ALL CONSTRUCTION IS IN FULL AGREEMENT WITH THE LATEST STRUCTURAL DRAWINGS.
- E. THE CONTRACTOR SHALL SUPPLY THE ENGINEER THREE COPIES OF SHOP DRAWING A MINIMUM OF ONE WEEK PRIOR TO PLACEMENT. THE REVIEW OF SHOP DRAWINGS SHALL BE LIMITED ONLY TO GENERAL CORRELATION WITH THE STRUCTURAL DRAWINGS AND SPECIFICATIONS. THE REVIEW DOES NOT GUARANTEE IN ANY WAY THAT THE SHOP DRAWINGS ARE CORRECT NOR DOES IT IMPLY THAT THEY SUPERSEDE THE STRUCTURAL DRAWINGS REVIEW. IT DOES NOT INCLUDE CHECK OF DIMENSIONS OR MATERIALS.
- F. THESE STRUCTURAL DRAWINGS SHALL NOT BE USED TO PRODUCE SHOP DRAWINGS WITHOUT PRIOR EXPRESSED WRITTEN APPROVAL FROM THE STRUCTURE ENGINEER OF RECORD.
- G. SHOP DRAWINGS NOT SUBMITTED FOR APPROVAL SHALL BE CLEARLY CLOUDED AND NOTED AS NOT FOR REVIEW SHALL BE LIMITED TO THOSE ITEMS WHICH CAUSED THE RE-SUBMITTAL.

4. CONCRETE:

- A. ALL CONCRETE WORK SHALL CONFORM TO ALL REQUIREMENTS OF ACI 308-11 "SPECIFICATIONS FOR STRUCTURAL CONCRETE BUILDINGS".
- B. CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE AS FOLLOWS:
- | | |
|-------------------------|-----------|
| FOOTINGS | 3,000 PSI |
| SLABS ON GRADE | 3,000 PSI |
| COLUMNS AND TIE COLUMNS | 3,000 PSI |
| ALL OTHER CONCRETE | 3,000 PSI |
- C. FORMWORK SHALL COMPLY WITH ACI 347-04, "RECOMMENDED PRACTICE FOR CONCRETE FORMWORK".
- D. MIX DESIGNS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO CONSTRUCTION OF ANY CONCRETE WORK. SUFFICIENT STATISTICAL DATA FOR EACH CLASS OF CONCRETE.
- E. NO WATER SHALL BE ADDED TO THE CONCRETE AT THE JOB SITE.
- F. THE QUALITY SHALL CONTRACT AN INDEPENDENT TESTING LABORATORY TO PERFORM CONCRETE CYLINDER TESTS AS FOLLOWS: SIX CYLINDER TESTS FOR 300 CUBIC YARDS OF CONCRETE POURED, OR FRACTION THEREOF FOR EACH CLASS OF CONCRETE POURED EACH DAY. ONE CYLINDER SHALL BE TESTED AT 3 DAYS AND 7 DAYS AFTER 18 DAYS, AND ONE REVERSEMENT TO BE TESTED AT 56 DAYS IF REQUIRED. FOLLOW ASMT ASTM STANDARDS FOR SAMPLING AND TESTING. ONE SLUMP TEST SHALL BE TAKEN FOR EACH SET OF TEST CYLINDERS CAST. SLUMP SHALL BE TAKEN WITH 10% OF CYLINDER 8" SHALL BE ACCEPTED IF CONCRETE IS TAMPED WITH IN ANY WAY. AFTER SAID TEST IS PERFORMED, REPEAT TEST IF WATER IS ADDED AFTER INITIAL SAMPLING.
- G. TRANSPORTING, PLACING, CURING AND DEPOSITING OF CONCRETE SHALL COMPLY WITH ACI 308-11 "SPECIFICATIONS FOR STRUCTURAL CONCRETE".
- H. CONSTRUCTION JOINTS IN STRUCTURAL MEMBERS AND BEAMS SHALL BE LOCATED AT 1/3 OF THE SPAN WITH REINFORCING CONTINUOUS ACROSS THE JOINT. PROVIDE A CONTINUOUS 2" X 4" SHEAR KEY AT SLABS, AT BEAMS PROVIDE 12" DEEP BEAM KEY OR KEY WITH CORDIN 8" SHALL BE ACCEPTED IF CONCRETE IS TAMPED WITH IN ANY WAY. AFTER SAID TEST IS PERFORMED, REPEAT TEST IF WATER IS ADDED AFTER INITIAL SAMPLING.
- I. DEPTHS 8" SMALLER THAN THE BEAM DEPT.
- J. DEPTHS SHALL BE APPROVED BY STRUCTURAL ENGINEER OF RECORD BEFORE POUR.
1. MAXIMUM WATER/CEMENT RATIO FOR CONCRETE CONTAINING A SUPERPLASTICIZER ADJUSTMENT SHALL BE 0.40. SLUMP AFTER ADDITION OF SUPERPLASTICIZER SHALL BE 6" +/- 1".
2. MINIMUM CEMENT COVER FOR REINFORCEMENT:
- | | |
|---|--------|
| 1. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH | 3" |
| CONCRETE EXPOSED TO EARTH OR WEATHER | 2" |
| 6" BARS AND LARGER | 3" |
| 6" BARS AND SMALLER | 1 1/2" |
| CONCRETE EXPOSED TO LEATHER OR IN CONTACT WITH EARTH | 2" |
| SLABS AND WALLS | 3/4" |
| BEAMS AND COLUMNS | 1" |
| SHIELD WALLS | 1 1/2" |

5. REINFORCING STEEL:

- A. REINFORCING STEEL SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-14.
- B. REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A 630-09e (S1) GRADE 60.
- C. REINFORCED USE FABRIC SHALL CONFORM TO ASTM A 185-06.
- D. REINFORCING SHALL BE HELD SECURELY IN POSITION WITH STANDARD ACCESSORIES DURING PLACING OF CONCRETE IN ACCORDANCE WITH ACI 318-14 MANUAL OF PRACTICE.
- E. ALL TOP REINFORCING SHALL TERMINATE WITH STANDARD HOOKS AT DISCONTINUOUS EDGES OR ENDS.
- F. ALL BOTTOM BARS SHALL BE BEAR 6" MINIMUM OVER SUPPORTS UNLESS:
- G. ALL REINFORCING BARS HAVING CONTINUOUS SHALL BE LAPPED WITH CLASS 90° TENSION LAP SPICES AT SUPPORTS. LAP LENGTH SHALL BE 18" AND LAP SPICES SHALL BE LOCATED AT CENTER BETWEEN SUPPORTS AS REQUIRED WITH CLASS 90° TENSION LAP SPICES.
- H. ALL TOP BARS SHALL BE BEAR 6" MINIMUM OVER SUPPORTS WITH STANDARD HOOKS.
- I. SLAB TOP BARS ARE SHOWN IN PLAN AS SOLID LINE.
- J. SLAB BOTTOM BARS ARE SHOWN IN PLAN AS DASHED LINE.
- K. IN BEAMS WITH MULTIPLE LAYERS OF REINFORCING AT TOP OR BOTTOM SEPARATE ALL TOP AND BOTTOM LAYERS FOR EACH BEAM BY 18" AND 6" AND SPACES BARS AS BAR SIZE FOR 18" BARS AND LARGER.
- L. IN BEAMS 8" CHIEFS WIDE PROVIDE A MAXIMUM OF 2 BARS PER LAYER OR REINFORCING.
- M. PROVIDE 4 #6 TOP REINFORCING WITH CLASS 90° TENSION LAP SPICES.
- N. PROVIDE 4 #6 TOP REINFORCING WITH CLASS 90° TENSION LAP SPICES.
- O. INTERMEDIATE REINFORCING SHALL BE PROVIDED AT 18" AND 6" AND SPACES BARS AS BAR SIZE FOR 18" BARS AND LARGER.
- P. NOT USED DURING CONSTRUCTION SHALL BE CREDITED TO THE OWNER AT THE SAME COST PER TON AS ORIGINALLY CHARGED.
- Q. ALL TOP AND BOTTOM AND INTERMEDIATE REINFORCING USED IN BALCONIES AND TERRACES SHALL BE GALVANIZED.
- R. FOR CLASS 90° TENSION LAP SPICES FOLLOW CIRI DESIGN HANDBOOK 2008.

6. CONCRETE MASONRY WORK:

- B. CONCRETE MASONRY WALLS NOTED AS LOAD BEARING WALLS, SHALL BE IN PLACE BEFORE THE SLABS AND BEARS SUPPORTING BY THEM ARE POURED AS NOTED ON THE DRAWINGS. THE WALLS SHALL BE CONCRETE MASONRY.
- C. CONCRETE MASONRY WALLS WITH NON-LOAD BEARING WALLS SHALL BE PLACED AFTER CONCRETE FRAME SUPPORTING THEM IS 28 DAYS OLD AND ALL JOINTS AND CORNERS SHALL BE REINFORCED WITH 4 #4 BARS. THE WALLS SHALL HOLD CLEAR OF CONCRETE ABOVE, UNTIL AN OCCUPIED DEAD LOAD DEFLECTION OF CONCRETE SLAB OR BEAM ABOVE HAS ACCURATELY FILLED, UNTIL WITH MORTAR AND GROUT. THE WALLS SHALL BE REINFORCED WITH 4 #4 BARS PER JOINT.
- C. ALL CONCRETE MASONRY UNITS (CMU) SHALL CONFORM TO ASTM C 90. STANDARD SPECIFICATIONS FOR HOLLOW LOAD BEARING CONCRETE MASONRY UNITS, WITH A NET AREA COMPRESSIVE STRENGTH OF CONCRETE MASONRY UNITS OF 1800 PSI.
- D. MORTAR SHALL BE 1 PART OF CEMENT TO 4 PARTS OF TYPE I MORTAR AVERAGE STRENGTH OF 1800 PSI.
- E. CONCRETE MASONRY STRENGTH, f_m , SHALL BE A MINIMUM OF 800 PSI.
- F. VERTICAL REINFORCING BARS SHALL BE PLACED IN THE WALLS AT 48" ON CENTER. DIAPHRAGM SPICES, PROVIDE CLEAR OUT HOLES AT PLACE OF FILLED CELLS FOR LAP INSPECTION AND VERIFYING THAT THE CELLS HAVE BEEN FILLED. SOLID WITH GROUT.
- G. FILLED CELLS SHALL BE FILLED WITH 3000 PSI GROUT AS PER THIS 402-60-60. FILLING OF CELLS SHALL BE DONE IN FOUR FOOT TIPS WITH A MAXIMUM OF FOUR FEET OF TIPS PER HOUR. THE WALLS SHALL BE FILLED WITH SOLID CELLS. GROUT SHALL CONFORM TO ASTM C 74. SLUMP SHALL BE BETWEEN 8" AND 12".
- H. CONCRETE MASONRY SHALL BE HORIZONTALLY REINFORCED WITH STANDARD NO. 9 LADDER-TYPE GALVANIZED STEEL REINFORCING EVERY SECOND COURSE. EXTEND REINFORCING A MINIMUM 4 FEET INTO THE COLUMNS. REINFORCING SHALL BE PLACED ABOVE EACH WALLING STUD. PROVIDE AN OVERLAP OF EACH COURSE CONNECTING NON-LOAD BEARING WALLS TO CONCRETE COLUMNS AND OTHER WALLS TO THE MORTAR BED OF SOLID-GROUTED HOLLOW UNITS.
- I. REINFORCING BARS SHALL BE PLACED IN THE WALLS. REINFORCING SHALL BE SECURED IN PLACE AT PLACE OF BAR AND ABOVE BEFORE GROUTING OF CELL WITH GALVANIZED BAR POSITIONERS.
- K. REINFORCING BARS BE SHALL CENTERED IN THE BLOCK CELL, DOES NOT LIE ON TOP WITH THE BLOCK CELL BE NOT SLOPED MORE THAN 1 IN 12. HORIZONTAL GROUT BETWEEN DEEDS OF REINFORCING WALL BE UP TO 4" BY THE BLOCK CELL APART.)
- L. ANCHOR BOLTS SHALL BE EMBEDDED IN WALLS IN GROUTED CELLS.
- M. ANCHOR BOLTS SHALL BE EMBEDDED IN WALLS IN GROUTED CELLS SHALL HAVE THE BLOCK SHALL REMOVED SO THAT WEDGE ANCHOR IS EMBEDDED IN SOLID CONCRETE GROUT, FILL ONE COURSE BELOW AND ABOVE ANCHOR LOCATION.
- N. ALL CONCRETE MASONRY WORK HAS BEEN DESIGNED AND DETAILED. REPAIRING CONCRETE WORK SHALL BE COMPLETELY INSPECTED BY SPECIAL OR THRESHOLD INSPECTORS.
- O. REINFORCING BARS SHALL BE LOCATED AS INDICATED IN PLAN OR CALLED OUT BY NOTES. IF ANY DISCREPANCIES AND NOTES DISAGREE CONSULT ENGINEER OF RECORD FOR CLARIFICATION.
- P. LAY MASONRY UNITS IN RUNNING BOND.
- Q. REINFORCE JOINTS OF WALLS WITH 4 #4 BARS AND NOTES DISAGREE CONSULT ENGINEER OF RECORD FOR CLARIFICATION.
- R. PRESSURE TREATED WOOD FOR LEDGERS IN CONTACT WITH MASONRY WALL.

7. STRUCTURAL STEEL

- I. STRUCTURAL STEEL SHALL COMPLY WITH AISC SPECIFICATIONS FOR DESIGN FABRICATION AND ERECTION FOR STRUCTURAL STEEL BUILDINGS THIRTEENTH EDITION, WITH ADDITIONAL REQUIREMENTS FOR WELDING, SEVENTH EDITION, AND AISC 360 F 350 C3, EXCEPT OTHER SHAPES WHICH SHALL CONFORM TO AISC 360 F 36, 36 C3.
- J. STRUCTURAL STEEL TUBES SHALL CONFORM TO ASTM A500 GRADE B, F_y 46 ksi, 48 K.
- K. STRUCTURAL STEEL PIPES SHALL CONFORM TO ASTM A53, TYPE S, GRADE B, F_y 35 KSI.
- L. ANCHOR BOLTS SHALL CONFORM TO EITHER ASTM A 307 OR ASTM F 1554, GRADE 36, FRAYING BOLT END SHALL CONFORM TO ASTM 335, WITH HARDENED WASHERS AND HEX NUTS.
- M. ALL EXTERIOR STEEL SHAPES, PLATES NUTS, BOLTS, WASHERS SHALL BE GALVANIZED GALVALUME.
- N. SPACING OF STEEL MEMBERS IS NOT ALLOWED UNLESS SPECIFIED IN STRUCTURAL READINGS OR APPROVED BY ENGINEER OF RECORD.
- O. ALL BOLTS NUTS AND WASHERS SHALL BE NEW RUST-FREE, CLEAN AND WELL LUBRICATED.
- P. BOLT HEADS THROUGH STEEL MEMBERS SHALL BE SHUT-ROILLED, CUT OR PUNCHED. DO NOT USE TORCH OR FLAME TO CUT OR ENLARGE HOLES.
- Q. STEEL STRUCTURE STEEL STRUCTURE STEEL STRUCTURE SHALL BE FILLED WITH 3000 PSI CONCRETE GROUT. PROVIDE 1/4" DIA. FLANGE DEEP HOLES EACH SIDE 1/2" FROM BOTTOM OF COLUMN.
- R. DO NOT PAINT PARTS OF STEEL MEMBERS TO BE EMBEDDED IN CONCRETE AND SURFACES TO BE IN CONTACT WITH CONCRETE.
- S. FOR FIREPROOFING OF STRUCTURAL STEEL MEMBERS SEE ARCHITECTURAL READINGS.
- T. SEE ARCHITECTURAL, MECHANICAL, AND PLUMBING READINGS FOR ADDITIONAL MISCELLANEOUS STRUCTURAL STEEL NOT SHOWN IN STRUCTURAL READINGS.
- U. FOR PAINTING OF NON-GALVANIZED STRUCTURAL STEEL SEE STRUCTURAL STEEL SPECIFICATIONS.
- V. WELDING SHALL BE A5.1 D1.1 TO ELECTRODES UNLESS OTHERWISE NOTED. CONFORMING TO A5.1 D1.1 (WELDING OF GRADE 60 REINFORCEMENT IS NOT PERMITTED).
- W. ALL WELD AND FIELD WELDING SHALL BE IN ACCORDANCE WITH A5.1 D1.1 STRUCTURAL STEEL WELDING CODE. LATERAL BRACING SHALL BE AS AGUED. SPECIAL WELDER CERTIFICATES TO ARCHITECT/ENGINEER FOR APPROVAL BEFORE ANY CUT OR FIELD WELDING IS STARTED.
- X. HEADED SHEAR STUDS SHALL BE NELSON ANCHORS WITH FLUXED NUTS OR APPROVED EQUIVALENT BARR ANCHORS (A5.1 SHALL BE NELSON TYPE D1.1 OR APPROVED STUDS AND D1.1 SHALL BE BARR ANCHORS). ALL WELDING SHALL BE WITH THE MANUFACTURER'S STANDARD EQUIPMENT IN ACCORDANCE WITH THEIR RECOMMENDATIONS.
- Y. EXPANSION BOLTS SHALL BE HILTI Kwik Bolt 3 OR APPROVED WITH EQUIVALENT QLC ALLOWABLE TENSION AND THEIR VALUES. EXPANSION BOLTS SHALL BE INSTALLED IN CONCRETE OR CONCRETE ON EXISTING CONCRETE. DO NOT CUT OR DRILL INTO EXISTING OR NEW EXISTING CONCRETE DURING INSTALLATION.
- Z. THE STEEL STRUCTURE IS DESIGNED AS A WHOLE WITH THE FLOOR AND ROOF DIAPHRAGM ACTION BRACING THE FRAMES AGAINST GRAVITY AND LATERAL AND WIND LOADS. THE STEEL STRUCTURE SHALL BE DESIGNED TO RESIST ALL SUCH LOADS TO MAINTAIN STEEL STRUCTURE STABLE UNTIL THE STRUCTURE IS COMPLETE.

8. SHORING AND RESHORING:

- A. SHORING AND RESHORING DRAWINGS SHALL BE PREPARED BY A STATE OF FLORIDA REGISTERED SPECIALTY ENGINEER WITH A MINIMUM OF TEN YEARS OF EXPERIENCE IN SHORING AND RESHORING DESIGN AND DETAILING.
- B. SHORING AND RESHORING DRAWINGS SHALL INCLUDE AT LEAST THE FOLLOWING ITEMS:
1. LOCATION, SIZE, TYPE AND CAPACITY OF ALL SHORING.
 2. LOCATION, SIZE, TYPE AND CAPACITY OF ALL BRACING.
 3. LOCATION, SIZE AND TYPE OF ALL BLOCKING, NUD BILLS, TEMPORARY LATERAL BRACING AND OTHER ACCESSORIES REQUIRED TO ADEQUATELY AND SAFELY SUPPORT AND BRACE THE STRUCTURE DURING CONSTRUCTION.
- C. AN ANALYSIS PROCEDURE, SEQUENCE OF INSTALLATION LOAD RELIEF AND REMOVAL OF ALL SHORING AND RESHORING.
- C. SHORING AND RESHORING SUBMITTAL FOR APPROVAL SHALL INCLUDE AT LEAST TWO COPIES FOR AND ONE FOR THE ARCHITECT.
- D. DESIGN DETAIL AND ERRECT FORMS SHORING AND RESHORING IN COMPLIANCE WITH ACI 347-04.0 PROJECT SPECIFICATIONS, AND THESE NOTES FORMS, SHORING AND RESHORING SHALL BE DESIGNED FOR THE WEIGHT OF THE FLOOR OR ROOF, ROOF A CONSTRUCTION LOAD OF 50 PSF, AND FOR THE CULMINATING LOADS OF ALL OTHER STRUCTURAL MEMBERS, USE A DESIGN FACTOR OF SAFETY OF 3 FOR WOOD SHORES AND 2 FOR METAL SHORES.
- E. NO CONSTRUCTION LOAD SHALL BE APPLIED TO ANY MEMBER UNTIL THE CONCRETE STRENGTH IS 10% OF THE SPECIFIED 28 DAY STRENGTH.
- F. FORMS MAY BE REMOVED 7 HOURS AFTER CONCRETE POURS PROVIDED THAT CONCRETE STRENGTH IS 10% OF THE SPECIFIED 28 DAY STRENGTH AND NOT LESS THAN 3500 PSI. REMOVAL SHALL BE IMMEDIATELY AFTER FORMS ARE STRIPPED AND REMOVED, REMOVAL OF FORMS IS THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR. REMOVAL OF FORMS SHALL BE CARRIED OUT IN SUCH A MANNER AS TO DAMAGE THE STRUCTURE, INSURE SAFETY AND PREVENT CREEP DEFLECTION ON STRUCTURAL MEMBERS.

9. WOOD FRAMING

- A. WOOD FRAMING MEMBERS OTHER THAN TRUSSES SHALL BE #2 SOUTHERN PINE WITH A FIBER BENDING STRESS AS PER NDS108.
- B. SIZES SHOWN ARE NOMINAL.
- C. MEMBERS SHALL BE FREE OF CRACKS AND KNOTS.
- D. MOISTURE CONTENT SHALL BE 19% OR LESS.
- E. PRESSURE-TREATED WOOD SHALL BE USED ONLY WHERE SPECIFICALLY NOTED. THE DRAWINGS NO FRAMING MEMBER SHALL BE OF PRESSURE-TREATED WOOD UNLESS OTHERWISE NOTED.

D. WELDING

- A. WELDING SHALL BE DONE WITH E-70 ELECTRODES.
B. WELDERS SHALL BE AWS-CERTIFIED.

II. WIND ANALYSIS DESIGN PARAMETERS:

- A. WIND DESIGN HAS BEEN DONE IN ACCORDANCE WITH ASCE 7-16.
- B. EXPOSURE 'C'
- C. RISK CATEGORY II
- D. INTERNAL PRESSURE COEFFICIENT, $GCP_i = \pm 0.18$.
- E. WIND VELOCITY, $V_u = 175$ MPH.

12. LETTER OF COMPLIANCE:

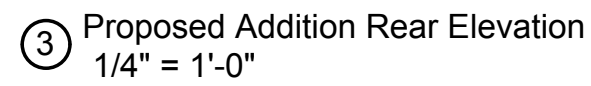
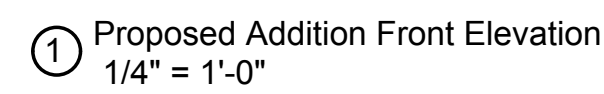
- I, AS THE SPECIAL INSPECTOR OF THIS BUILDING, UPON COMPLETION OF THE BUILDING AND PRIOR TO THE ISSUANCE OF THE CERTIFICATE OF OCCUPANCY, WILL FILE A SIGNED AND SEALED STATEMENT WITH THE ENFORCEMENT AGENCY THAT STATES THAT TO THE BEST OF MY KNOWLEDGE AND BELIEF, THE ABOVE DESCRIBED CONSTRUCTION OF ALL LOAD-BEARING STRUCTURAL COMPONENTS COMPLIES WITH THE PERMITTED DOCUMENTS, AND THE SHORING AND RESHORING TO THE SHORING AND RESHORING PLANS SUBMITTED TO THE ENFORCING AGENCY".

ELECTRONICAL SIGNATURE STATEMENT:
THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY
JOAQUIN MONTESSINO, PE ON 1-3-24. PRINTED COPIES OF THIS DOCUMENT
ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED
ON ANY ELECTRONIC COPIES.

Digitally
signed by
JOAQUIN
MONTESINO
Date:
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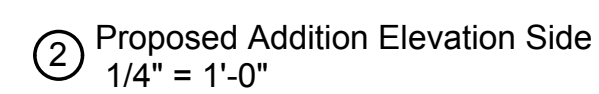
WRITTEN DIMENSIONS ON THESE DRAWINGS SHALL HAVE PRECEDENCE OVER SCALE DIMENSIONS. CONTRACTORS SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS ON THE JOB AND THIS OFFICE MUST BE NOTIFIED OF ANY VARIATION FROM THE DIMENSIONS AND CONDITIONS SHOWN BY THESE DRAWINGS

G2 CONSULTING GROUP, INC.
JOAQUIN MONTESINO, PE
CONSULTING ENGINEERS
9725 SW. 4 TERR. MIAMI FL. 33174
PHONE & FAX (786) 294-0032
LIC. #69969 CA 25882



DOOR PRESSURES ASD		
MARK	NEGATIVE (-)	POSITIVE (+)
① ZONE4	42.2PSF	38.7 PSF
② ZONE5	50.6 PSF	38.7 PSF

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signed by
JOAQUIN
MONTESINO
Date:
2025.01.17
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Project number	24-011
Date	07-10-24
Drawn by	JP
Checked by	JP

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