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THE GENERAL CONTRACTOR AND ALL TRADES PERFORMING WORK ON THIS PROJECT SHALL BE EXCLUSIVELY RESPONSIBLE FOR VERIFYING ALL DIMENSIONS AND CONDITIONS WITH REPORTING ANY DISCREPANCIES TO THE DE LA LLAMA + ASSOCIATES, INC, BEFORE COMMENCING ANY WORK.

GENERAL STRUCTURAL NOTES

COORDINATION:

1. COORDINATE ALL DIMENSIONS, ELEVATIONS & OPENINGS WITH ARCHITECTURAL DRAWINGS. REPORT ANY DISCREPANCIES TO OUR OFFICE.
2. SEE ARCHITECTURAL DRAWINGS FOR SLAB FINISHES, DETAILS. AND EXACT LOCATION OF DEPRESSED SLAB AREAS AND FLOOR DRAINS.

OWNER, ARCHITECT, AND CONTRACTOR NOTES:

1. IF OUR OFFICE WERE TO BE HELD RESPONSIBLE FOR THE STRUCTURAL ADEQUACY OF THE CONSTRUCTED BUILDING, WE WOULD NEED TO PERFORM THE REVIEW OF ALL REQUIRED SHOP DRAWINGS AND PERFORM THE STRUCTURAL INSPECTIONS FOR THE BUILDING ENVELOPE AND ITS STRUCTURAL COMPONENTS.
2. CONTRACTOR, BUILDER AND SUBCONTRACTORS INVOLVED IN ANY FORM OF CONSTRUCTION USING THESE DOCUMENTS SHALL INFORM THE OWNER AND DEVELOPER IN WRITING PRIOR TO CONSTRUCTION OF THE FOLLOWING RESPONSIBILITIES, PERFORMANCE CRITERIA AND LIMITATIONS AND RISKS ASSOCIATED WITH CONSTRUCTION. IF THE OWNER, DEVELOPER OR CONTRACTOR IS NOT ABLE TO ACCEPT RESPONSIBILITIES OR PERFORMANCE CRITERIA AND LIMITATIONS, NOTIFY OUR OFFICE PRIOR TO START OF CONSTRUCTION. IT SHALL BE EXPRESSLY UNDERSTOOD THAT THE ENGINEER IS NOT RESPONSIBLE OR LIABLE FOR THE LACK OF PERFORMANCE OF MATERIALS, SYSTEMS OR DESIGNS NOT BEING LIMITED TO ITEMS OUTLINED BELOW. CONTRACTORS AND SUBCONTRACTORS SHALL THOROUGHLY REVIEW ALL CONDITIONS AND RESPONSIBILITIES STATED IN THESE NOTES, GENERAL STRUCTURAL NOTES, PLANS SECTIONS AND DETAILS AND SHALL NOTIFY THE ENGINEER AND OWNER IN WRITING PRIOR TO CONSTRUCTION OF ANY CONDITIONS OR RESPONSIBILITIES WHICH ARE NOT ACCEPTABLE OR NOT UNDERSTOOD.
3. THE ENGINEER SHALL NOT BE RESPONSIBLE FOR VARIATIONS TO PLANS BETWEEN BID PROCESS AND FINALIZED APPROVED DOCUMENTS RELEASED FOR CONSTRUCTION. ADDITIONS AND ALTERATIONS MAY BE MADE BETWEEN RELEASE OF BID DOCUMENTS AND FINALIZED CONSTRUCTION DOCUMENTS.
4. THESE DRAWINGS HAVE BEEN PREPARED USING STANDARDS OF PROFESSIONAL CARE AND COMPLETENESS NORMALLY EXERCISED UNDER SIMILAR CIRCUMSTANCES BY REPUTABLE STRUCTURAL ENGINEERS IN THIS OR SIMILAR LOCALITIES. THEY NECESSARILY ASSUME THAT THE WORK DEPICTED WILL BE PERFORMED BY AN EXPERIENCED CONTRACTOR, SUBCONTRACTOR AND/OR WORKPERSONS WHO HAVE A WORKING KNOWLEDGE OF THE APPLICABLE CODE STANDARDS AND REQUIREMENTS AND OF INDUSTRY ACCEPTED STANDARD GOOD PRACTICE. AS NOT EVERY CONDITION OR ELEMENT IS (OR CAN BE) EXPLICITLY SHOWN ON THESE DRAWINGS, IT IS UNDERSTOOD THAT THE CONTRACTOR WILL USE INDUSTRY ACCEPTED STANDARD GOOD PRACTICE FOR ALL WORK NOT EXPLICITLY SHOWN.
5. THESE DRAWINGS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO DESIGN AND PROVIDE ADEQUATE SHORING, BRACING, FORMWORK, ETC. AS REQUIRED FOR THE PROTECTION OF LIFE AND PROPERTY DURING CONSTRUCTION. CONSTRUCTION MATERIALS SHALL BE UNIFORMLY SPREAD OUT SUCH THAT DESIGN LIVE LOAD PER SQUARE FOOT AS STATED HEREIN IS NOT EXCEEDED. VISITS TO THE SITE BY THE STRUCTURAL ENGINEER SHALL NOT INCLUDE INSPECTION OF THE ABOVE ITEMS.
6. THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFICATION OF ALL DIMENSIONS, CONDITIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO START OF CONSTRUCTION. THE CONTRACTOR SHALL INFORM THE ARCHITECT IN WRITING OF ANY DISCREPANCIES OR OMISSIONS NOTED ON THE DRAWINGS. ANY SUCH DISCREPANCY, OMISSION, OR VARIATION NOT REPORTED BEFORE START OF CONSTRUCTION SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
7. TYPICAL DETAILS AND NOTES SHALL APPLY, THOUGH NOT NECESSARILY INDICATED AT A SPECIFIC LOCATION ON PLANS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT. DETAILS MAY SHOW ONLY ONE SIDE OF CONNECTION OR MAY OMIT INFORMATION FOR CLARITY. WHERE DISCREPANCIES OCCUR IN THESE DRAWINGS, NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS.
8. DUE TO MOISTURE VARIATIONS AS A RESULT OF AMBIENT CONDITIONS AND MATERIAL CONDITIONS AT TIME OF CONSTRUCTION, WOOD MEMBERS ARE SUSCEPTIBLE TO VOLUME AND SHAPE CHANGES AND DIMENSIONAL VARIATIONS. SPANNING MEMBERS MAY ALSO DEFLECT VERTICALLY OR LATERALLY. SHEATHING, DRYWALL OR ARCHITECTURAL SURFACES SUPPORTED BY WOOD MEMBERS MAY DISTORT, CRACK AND BULGE DUE TO THESE FACTORS.
9. PLAIN CONCRETE, REINFORCED CONCRETE, POST-TENSIONED CONCRETE, OR CONCRETE MASONRY DEVELOP CRACKS. THE CRACKS ARE DUE TO INHERENT SHRINKAGE, CREEP AND RESTRAINING EFFECTS. CRACKS ARE NORMALLY COSMETIC AND THE SYSTEM MAINTAINS SERVICEABILITY AND STRENGTH REQUIREMENTS. JOINTS MAY BE INDICATED TO CONTROL CRACKING, BUT ARE NOT MEANT TO ELIMINATE ALL CRACKING, AS THIS IS NOT PRACTICAL. THE CONTRACTOR SHALL USE ALL STANDARD MEANS TO INSURE PROPER PROTECTION AND CURING OF CEMENTIOUS MATERIALS TO REDUCE CRACKING. SURFACE SPALLING OR EXTREME CRACKING MAY BE CAUSED BY POOR MATERIAL OR PLACEMENT.

APPLICABLE CODES:

1. ALTHOUGH THE ENGINEER OF RECORD HAS STRIVED TO MEET ALL APPLICABLE CODES AND LOCAL ORDINANCE REQUIREMENTS, THE CONTRACTOR IS STILL RESPONSIBLE FOR COMPLYING WITH ALL APPLICABLE CODES AND ORDINANCES OF THE EVENT OF CONFLICT OR OVERSIGHT IN THE DRAWINGS THE CONTRACTOR MUST IMMEDIATELY NOTIFY THE ARCHITECT OR ENGINEER OF ANY CONFLICT OF DISCREPANCY ENCOUNTERED SO THAT APPROPRIATE REMEDIES MAY BE UNDERTAKEN.

DESIGN LOAD & CRITERIA					
DESIGN CODES	BUILDING CODES	FLORIDA BUILDING CODE (2023 EDITION) ASCE 7-22			
	STRUCTURAL STEEL	ANSI/AISC 360-16 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS (16th EDITION)			
	CONCRETE	ACI 318-19 BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE.			
WIND	RISK CATEGORY	II			
	BASIC WIND SPEED (3 SECOND GUST)	V_{ULT} = 175 M.P.H. V_{ASD} = 136 M.P.H.			
	EXPOSURE CATEGORY	"C"			
	DIRECTIONALITY FACTOR	Kd = 0.85			
	TOPOGRAPHIC FACTOR	Kzt = 1.0			
GRAVITY	LOCATION	LIVE LOAD		SUPERIMPOSED DEAD LOAD	
		UNIFORM	CONCENTRATED	UNIFORM	CONCENTRATED
	ROOF	30 PSF	-----	35 PSF	-----
	APARTMENT	40 PSF	-----	35 PSF	-----
	CORRIDORS	40 PSF	-----	35 PSF	-----
	STAIRS	40 PSF	-----	20 PSF	-----
	TERRACE	100 PSF	-----	35 PSF	-----
	NO LIVE LOAD REDUCTION HAS BEEN USED.				

2. THIS BUILDING IS LOCATED IN A HVHZ (HIGH VELOCITY HURRICANE ZONE) WIND BORN DEBRIS REGION AND HAS BEEN DESIGNED AS ENCLOSED. ALL PARTS OF THE BUILDING ENVELOPE, INCLUDING BUT NOT LIMITED TO, CLADDING SYSTEMS, EXTERIOR DOORS AND WINDOWS, SKYLIGHTS, AND GLASS BLOCK MEMBERS SHALL MEET IMPACT TEST CRITERIA OR BE PROTECTED WITH AN EXTERNAL PROTECTION DEVISE THAT HAS BEEN TESTED AND MEETS THE IMPACT TEST CRITERIA IN ACCORDANCE WITH SECTION 1626 OF THE FLORIDA BUILDING CODE AND SHALL HAVE A CURRENT NOTICE OF ACCEPTANCE (NOA).

TERMITE PROTECTION:

1. AS PER F.B.C. 105.11 BUILDING COMPONENTS AND BUILDING SURROUNDINGS REQUIRED TO BE PROTECTED FROM TERMITE DAMAGE IN ACCORDANCE WITH 1503.6, 2304.11 , 2603.9, OR REQUIRED TO HAVE CHEMICAL SOIL TREATMENT IN ACCORDANCE WITH 1816 SHALL NOT BE COVERED OR CONCEALED UNTIL THE RELEASE FROM THE BUILDING OFFICIAL HAS BEEN RECEIVED.
2. ALL BUILDINGS SHALL HAVE PRE-CONSTRUCTION TREATMENT PROTECTION AGAINST SUBTERRANEAN TERMITES AS PER F.B.C. 1816. A CERTIFICATE OF COMPLIANCE SHALL BE ISSUED TO THE BUILDING DEPARTMENT BY THE LICENSED PEST CONTROL COMPANY THAT CONTAINS THE FOLLOWING STATEMENT: "THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES. TREATMENT IS IN ACCORDANCE WITH THE RULES AND LAWS ESTABLISHED BY THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES".

GEOTHECNICAL CRITERIA:

1. FOUNDATIONS HAVE BEEN DESIGNED PER THE RECOMMENDATIONS DONE BY ALL STATE ENGINEERING & TESTING CONSULTANTS, INC. PROJECT DATED FEBRUARY 28 th 2024. FOR THE PROPOSED PROJECT LOCATED AT 13100 SW 80th AVE, MIAMI, FLORIDA 33156

2. FOUNDATIONS HAVE BEEN DESIGNED AS A SHALLOW FOUNDATION SYSTEM WITH A SOIL CAPACITY OF 25000 lb/F2

CONTRACTOR SHALL FAMILIARIZE WITH TERMS SPECIFIED ON THE SOILS REPORT BEFORE SOIL PREPARATION OPERATIONS BEGIN. AN ORIGINAL COPY OF THE REPORT SHALL BE KEPT AT THE JOB SITE AT ALL TIMES DURING BUILDING PAD PREPARATION.

CONCRETE:

1. ALL STRUCTURAL CONCRETE HAS BEEN DESIGNED IN ACCORDANCE WITH THE ACI 318 LATEST EDITION AND THE BUILDING CODE.
2. THE CONCRETE REQUIREMENTS ARE:
   A. CEMENT SHALL BE TYPE I OR II CONFORMING TO ASTM C150.
   B. FINE AND COURSE AGGREGATES SHALL CONFORM TO ASTM C33.
   C. NOMINAL MAXIMUM SIZE OF COARSE AGGREGATE SHALL BE NOT LARGER THAN ¾"
   D. SLUMP SHALL BE 3" TO 6" (UNLESS UTILIZING WATER REDUCING ADMIXTURES)
3. COMPRESSIVE STRENGTH AT 28 DAYS:
   ALL EXTERIOR CONCRETE (EXPOSED TO EARTH OR WEATHER) SHALL COMPLY WITH ACI 318-14, TABLES 19.3.1.1 & 19.3.2.1, AND SHALL HAVE A MAXIMUM WATER-SOLUBLE ION CONTENT IN CONCRETE PER TABLE BELOW. SEE TABLE BELOW FOR MAXIMUM WATER-CEMENT RATIO.

DISTANCE TO MOISTURE AND CHLORIDES (*)	EXPOSURE CLASS	MAXIMUM WATER-SOLUBLE CHLORIDE ION (CI-) CONTENT IN CONCRETE BY WEIGHT OF CEMENT
> 4 MILES	C0 (N/A)	N/A
2-4 MILES	C1 (MODERATE)	0.30
< 2 MILES	C2 (SEVERE)	0.15

(*) INCLUDES DEICING CHEMICALS, SALT, SALT WATER, BRACKISH WATER, SEAWATER, OR SPRAY FROM THESE SOURCES.

f'c [PSI]	WATER/CEMENT RATIO NON-AIR ENTRAINED (NO EXPOSURE TO FREEZING AND THAWING)
2000	0.82
3000	0.68
4000	0.57
5000	0.48
6000	0.41

f'c [PSI]	MINIMUM MODULUS OF ELASTICITY (KSI)
3000	3156
4000	3644
5000	4074
6000	4463
7000	4821

f'c [PSI]	MINIMUM MODULUS OF ELASTICITY (KSI)
8000	4884
9000	4947
10000	5215
11000	5469
12000	5712

FOOTINGS: .... 3000 PSI
SLAB ON FILL: .... 3000 PSI
COLUMNS: .... 4000 PSI
BEAMS & SLAB .... 4000 PSI
ALL OTHER POURED-IN-PLACE CONCRETE: .... 4000 PSI

4. TEST: AS PER ACI 5.6.2 THE MINIMUM SAMPLING FREQUENCY PER CLASS OF CONCRETE SHALL BE ONE SET PER POUR PER DAY FOR VOLUMES BETWEEN 5 AND 25 CU YD, AND ADDITIONAL SETS FOR EVERY ADDITIONAL 50 CU YD THEREAFTER. A SET CONSISTS OF A MIN. OF 6 CONCRETE SPECIMENS THAT SHALL BE TAKEN FROM EACH CLASS OF CONCRETE. SPECIMENS SHALL BE TESTED ACCORDING TO A.S.T.M. C-39, ONE AT 3, ONE AT 7, 3 AT 28 DAYS AND 1 AT 56 DAYS.
5. COVER:
   CONCRETE DEPOSITED AGAINST THE GROUND: .... 3"
   FORMED CONCRETE IN CONTACT WITH THE GROUND: .... 2"
   INTERIOR BEAMS AND COLUMNS: .... 1 ½"
   EXTERIOR BEAMS AND COLUMNS .... 2"
   INTERIOR SLABS: .... ¾"
   EXTERIOR SLABS: .... 1 ½"
   EXTERIOR POST-TENSIONED SLABS: .... 1"
   INTERIOR POST-TENSIONED SLABS: .... ¾"
6. LIGHTWEIGHT INSULATING CONCRETE (LWIC) SHALL HAVE A MAXIMUM SPECIFIC WEIGHT OF 38 PCF.
7. LIGHTWEIGHT CONCRETE SHALL HAVE A MAXIMUM SPECIFIC WEIGHT OF 115 PCF, MINIMUM F'C OF 3000 PSI AND A MAXIMUM SLUMP OF 6" (+/- 1").

REINFORCING STEEL:

1. REINFORCING BARS CONFORMING TO A.S.T.M. A-615 GRADE 60, INCLUDING COLUMN AND BEAM TIES.
2. WELDED FIRE FABRIC CONFORMING TO A.S.T.M. A-185 AND SUPPORTED ON SLAB BOLSTERS SPACED AT 3'-0" O/C.
3. REINFORCING BARS REQUIRED TO BE WELDED SHALL CONFORM TO THE REQUIREMENTS OF ASTM A706 GRADE 60. WELDING OF REINFORCING OTHER THAN SPECIFIED IS PROHIBITED.
4. MECHANICAL CONNECTORS SHALL BE IN ACCORD WITH ACI 439-3R-83.
5. FABRICATION AND DETAILING ACCORDING TO A.C.I.-315.
6. ALL ACCESSORIES TO HAVE UPTURNED LEGS AND BE PLASTIC DIPPED AFTER FABRICATION. THE CONTRACTOR SHALL INCLUDE IN HIS BASE BID THE COST
7. ALL REINFORCING STEEL AT BALCONIES, EXPOSED CORRIDORS, PLANTERS SHALL BE HOT DIP GALVANIZED.
8. EXTRA STOCK: OF 5000 LBS. OF ADDITIONAL REINFORCING STEEL, INCLUDING FABRICATION, BENDING, FURNISHING AND PLACING. THIS EXTRA STOCK SHALL BE FURNISHED AND USED FOR SPECIAL CONDITIONS AS DIRECTED BY THE ARCHITECT, THE ARCHITECTS AGENT, OR THE OWNERS CONSTRUCTION SUPERVISOR. THE PRICE OF ALL UN-USED EXTRA STOCK SHALL BE CREDITED TO THE OWNERS ACCOUNT.

SHORING, RE-SHORING, AND TEMPORARY BRACING:

1. ALL SHORING, RE-SHORING AND TEMPORARY BRACING REQUIRED IN THIS PROJECT IS TO BE DESIGNED BY FLORIDA PROFESSIONAL ENGINEER WHO SPECIALIZES IN FORM WORK/TEMPORARY BRACING DESIGN. AND WHO HAS BEEN HIRED BY THE GENERAL CONTRACTOR. THE GENERAL CONTRACTOR IS THE ULTIMATE RESPONSIBLE PARTY FOR SHORING, RE-SHORING, AND TEMPORARY BRACING REQUIRED ON THIS PROJECT AND MUST SATISFY HIMSELF WITH THE ADEQUACY OF THE INSTALLATION OF THESE ITEMS AT ALL TIMES. IN THE SPECIFIC CASE OF POURED SLABS IT IS HIGHLY RECOMMENDED THAT FROM THE SHORING DESIGNER OR HIS AGENT PRIOR TO THE PLACEMENT OF CONCRETE THE GENERAL CONTRACTOR SECURE A CERTIFICATION OF THE INSTALLED SHORING.

SLAB ON FILL:(PLACED ACCORDING TO ACI-302):

1. THE FLOOR SLAB ON GRADE SHALL BE SUPPORTED ON A 4" COMPACTED LAYER OF FREE DRAINING, GRANULAR SUBBASE MATERIAL. IF OWNER IS CONCERNED ABOUT VAPOR TRANSMISSION THOUGHT THE CONCRETE FLOOR SLAB, A VAPOR BARRIER SHALL BE USED. IF USED, SEE NOTE 6. VAPOR BARRIER IS REQUIRED AT ALL SLABS ON GRADE COVERED WITH WOOD, TILE, CARPET OR OTHER MOISTURE SENSITIVE OR IMPERVIOUS COVERINGS, OR WHEN SLAB SUPPORTS EQUIPMENTS SENSITIVE TO MOISTURE.
2. JOINTS:

   ISOLATION JOINTS MUST BE USED AT JUNCTIONS WITH WALLS AND COLUMNS, USE 1/2" THICK PREMOLDED JOINTS FULL DEPTH OF SLAB. CONTROL JOINTS PLACED AT CENTERLINE OF COLUMN LINES PROVIDE INTERMEDIATE JOINTS IF COLUMN SPACING IS GREATER THAN 30'. IN SIDEWALKS PROVIDE TOOLED JOINTS SPACED AT INTERVALS EQUAL TO THE WIDTH OF THE SLAB.

   4" & 5" SLABS: .... 1" DEEP TOOLED
3. JOINTS MUST BE SAWED BEFORE 24 HOURS AFTER CONCRETING. CONSTRUCTION JOINTS MUST BE PLACED IN THE WHERE BUILDING EXPANSION JOINTS ARE SHOWN AND WHERE CONTROL JOINTS ARE SHOWN OR AS PER FOLLOWING:

   SLAB 4" THICK .... 12 FT
   5" THICK .... 13 FT
   6" THICK .... 15 FT
   7" THICK .... 18 FT
   8" THICK .... 20 FT
   9" THICK .... 23 FT
   10"THICK .... 25 FT
4. WHEN CONCRETING AND OPERATING ARE CONCLUDED FOR THE DAY, CONSTRUCTION JOINTS SHALL BE FORMED WITH BURKE KEYED KOLD METAL JOINT FORM OR APPROVED EQUAL.
5. VAPOR BARRIERS:
   WATERPROOF MEMBRANES NOT LESS THAN 6-MIL POLYVINYL CHLORIDE (OVERLAPPED 6" AT JOINTS) WITH A PERMEANCE OF LESS THAN 0.3% PERMS IN ACCORDANCE WITH A.S.T.M. E-98 SHALL BE PROVIDED UNDER INTERIOR SLAB. WHERE NO VAPOR BARRIER IS USED THE SUB-GRADE MUST BE DAMPENED WITH WATER IN ADVANCE OF CONCRETING NO FREE WATER STANDING ON THE SUB-GRADE NOR ANY MUDDY OR SOFT SPOT IS PERMITTED.
6. ANY STRUCTURAL MEMBER PENETRATING SLAB ON FILL IS TO BE 1/2" PRE-MOLDED JOINT FILLER COMPLYING WITH A.S.T.M. D-1752, TYPE I.
7. FINISHING:
   NO PREMATURE FINISHING SHALL BE ALLOWED. IMMEDIATE FOLLOWING FLOATING TROWELING WITH STEEL TROWELS SHOULD BE COMMENCED IF REQUIRED BROOMING SHALL BE AFTER THE STEEL TROWELING OPERATION.
8. SLAB FINISHES: (UNLESS OTHERWISE NOTED BY THE ARCHITECT)
   BUILDING: .... STEEL TROWELED
   OUTSIDE SLAB: .... BROOMED

MASONRY WALLS:

1. CONCRETE BLOCK MASONRY WALLS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH f'm AS SCHEDULED AND COMPLY WITH "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES": TMS 402/602.
2. THE CONCRETE BLOCK UNITS, TO BE TYPE II-NONMOISTURE CONTROLLED, CONFORMING TO ASTM C90, WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH AS SCHEDULED (AVERAGE OF THREE).
3. THE MORTAR SHALL COMPLY WITH ASTM C270, AND SHALL BE TYPE "M" WITH MINIMUM COMPRESSIVE STRENGTH AS SCHEDULED AT 28 DAYS.
4. GROUT FOR GROUTING CELLS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AS SCHEDULED AT 28 DAYS AND SHALL BE IN ACCORDANCE WITH WITH ASTM C476. GROUT TO HAVE A SLUMP BETWEEN 8" TO 11".

WALL TYPE:	UNIT STRENGTH (PSI)	MORTAR STRENGTH (PSI)	GROUT STRENGTH (PSI)	f'm DESIGN STRENGTH (PSI)
CMU-1	2000	2000	2000	2000
CMU-2	3250	3250	2500	2500
CMU-3	4500	4500	3000	3000

ALL CMU WALLS SHALL BE TYPE CMU-1 U.N.O.

5. MASONRY WALLS SHALL BE REINFORCED HORIZONTALLY EVERY OTHER COURSE, WITH 9 GAUGE GALVANIZED, PREFABRICATED STEEL, DUR-O-WAL, "LADDER TYPE", OR EQUAL. EXTEND 4" INTO ADJACENT COLUMNS.
6. FOR CORNER BARS SEE NOTE 13 AT TYPICAL BEAM DIAGRAM ON SCHEDULES SHEET.
7. PROVIDE CLEAN OUTS WHEN GROUTING BLOCK CELLS AND CLEAN OUT BLOCK CELLS OF ALL MORTAR DROPPINGS, MAXIMUM VERTICAL DROP FOR GROUTING IS 4'-0".
8. WHENEVER ANCHORS BOLTS ARE TO BE SET IN MASONRY. TWO CELLS AT THE SETTING LOCATION AND TWO COURSES ABOVE AND BELOW ANCHORS SHALL BE FILLED WITH GROUT.

STRUCTURAL STEEL:

1. STRUCTURAL STEEL HAS BEEN DESIGNED IN ACCORDANCE WITH THE BUILDING CODE AND AISC SPECIFICATIONS NINTH EDITION. STRUCTURAL STEEL SHALL MEET THE FOLLOWING REQUIREMENTS UNLESS NOTED OTHERWISE ON THE DRAWINGS:

TYPE	ASTM	GRADE	Fy
ALL STANDARD STEEL SHAPES	A36	-----	36 KSI
ANCHOR BOLTS	A307	-----	
STRUCTURAL BOLTS	A325	-----	
HOLLOW STRUCTURAL STEEL	A500	B	46 KSI

2. ALL WELDING SHALL BE IN ACCORDANCE WITH LATEST AWS CODE. ALL WELDS SHALL USE E70XX ELECTRODES SERIES LOW HYDROGEN ELECTRODES.
3. WELDERS SHALL BE AWS CERTIFIED.
4. PROVIDE ONE SHOP COAT OF RUST INHIBITING PAINT MIN. 3 ML. DRY FILM THICKNESS).
5. ALL STEEL EXPOSED TO THE WEATHER SHALL BE HOT DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A123 FOR MEMBERS AND ASTM A153 FOR CONNECTION ELEMENTS. THIS INCLUDES ALL STRUCTURAL STEEL USED TO SUPPORT MECHANICAL EQUIPMENT AND ALL ROOF BRACING AND OTHER STRUCTURAL MEMBERS.
6. PLATES SHALL BE TOUCHED AFTER WELDING HAS TAKEN PLACE.
7. ALL ANCHOR BOLTS/NUT SHALL BE A307 U.N.O. HEAD PLATE SHALL CONFORM TO ASTM-A36.
8. NUTS SHALL BE HEAVY HEX
9. DRILL OR PUNCH BOLT HOLES 1/16" LARGER THAN BOLT DIAMETER.
10. STRUCTURAL BOLTS CONNECTION TYPE SHALL BE SLIP CRITICAL CLASS A
11. FINISH COLOR OF PAINT TO BE SELECTED BY THE OWNER AND APPLIED IN THE FIELD.

SAFETY OSHA AND LABOR LAWS:

1. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS FULLY COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE AND INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, TEMPORARY BRACING, ETC. THAT MAY BE NECESSARY. SUCH MATERIAL SHALL REMAIN THE CONTRACTOR'S PROPERTY AFTER COMPLETION OF THE PROJECT.
2. THE STRUCTURAL ENGINEER OF RECORD DOES NOT POSSES, NOR PRESUMES TO POSSES ANY KNOWLEDGE OR EXPERTISE IN MATTERS TO JOB SITE EMPLOYEE SAFETY, OSHA OR LABOR LAE REQUIREMENTS FOR A CONSTRUCTION PROJECT. SAFETY AND COMPLIANCE WITH OSHA AND LABOR LAWS ARE THE ABSOLUTE RESPONSIBILITY OF THE GENERAL CONTRACTOR AND THOSE CONSULTANTS HE HIRES TO ADDRESS THESE MATTERS. THE STRUCTURAL ENGINEER OF RECORD SPECIALIZES IN STRUCTURAL DESIGN ONLY, AND THE BOARD OF PROFESSIONAL REGULATION FORBIDS HIM FROM ASSUMING RESPONSIBILITY OUTSIDE HIS AREA OF EXPERTISE.

SHOP DRAWING SUBMITTALS:

1. SUBMIT MINIMUM 3 COPIES OF ALL SHOP DRAWINGS LISTED BELOW. IF SIGNED AND SEALED SHOP DRAWINGS ARE REQUIRED, THEN SUBMIT TWO ADDITIONAL SIGNED AND SEALED PRINTS FOR APPROVAL.
2. SHOP DRAWINGS:

   REINFORCED STEEL
   CONCRETE MIX DESIGN. (SIGNED & SEALED)
   RAILINGS (SIGNED & SEALED)
   WINDOWS & DOORS (SIGNED & SEALED)
   SHORING AND RE-SHORING (SIGNED & SEALED)

NOTE:
ENGINEERING CALCULATIONS AND SHOP DRAWINGS SIGNED AND SEALED BY A P.E. REGISTERED IN FLORIDA MUST BE SUBMITTED TO EOR FOR REVIEW AND APPROVAL.

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S-1.2	ROOF FLOOR FRAMING PLAN
S-2.0	SCHEDULES
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S-3.2	SECTION
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S-5.1	WALL WIND PRESSURES
S-5.2	ROOF WIND PRESSURES

TO THE BEST OF THE ENGINEERS' KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES FOR STRUCTURAL REQUIREMENTS AT THE TIME OF THEIR PREPARATION AS DETERMINED BY THE LOCAL AUTHORITIES IN ACCORDANCE WITH THE PROVISIONS OF THE LATEST EDITION OF THE FLORIDA BUILDING CODE.

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TO THE BEST OF THE ENGINEERS' KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES FOR STRUCTURAL REQUIREMENTS AT THE TIME OF THEIR PREPARATION AS DETERMINED BY THE LOCAL AUTHORITIES IN ACCORDANCE WITH THE PROVISIONS OF THE LATEST EDITION OF THE FLORIDA BUILDING CODE.

DE LA LLAMA
ARCHITECTURE
PLANNING
INTERIOR DESIGN

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PROJECT NAME

P8
PINECREST
EIGHT

13100 SW 80TH AVE
PINECREST, FLORIDA 33156

FILE NAME
24-0609

DATE
NOVEMBER 7TH, 2024

REVISION	DATE	BY

DRAWING NAME
GENERAL NOTES

DESING BY:
RAISSA LOPEZ

DRAWN BY:
N. C / A.G

SHEET N0.
S-0.0