

GENERAL NOTES

THE FLOOR SLAB SHALL BE PLACED WITH A VIBRATORY SCREED USING A LASER LEVEL TO ACCOMPLISH A FINISH WITH CLASS A TOLERANCE; HAVING TRUE PLANES WITHIN 1/4 INCH IN 10 FEET, AS DETERMINED BY A 10 FOOT STRAIGHTEDGE PLACED ANYWHERE ON THE SLAB IN ANY DIRECTION. BEFORE INSTALLING THE FLOOR COVERING, (MINIMUM OF 28 DAYS AFTER FLOOR SLAB IS PLACED) ALL JOINTS SHALL BE INSPECTED / MEASURED FOR SLAB CURL, AND JOINTS NOT MEETING SPECIFICATIONS SHALL BE GROUND FLAT, PRIOR TO ACCEPTANCE OF SLAB BY THE ARCHITECT. (NOTE: THE COST FOR GRINDING SHALL BE CARRIED AS AN ALLOWANCE BY THE GENERAL CONTRACTOR WITH THE GRINDING BEING PERFORMED UNDER THE DIRECTION OF THE SUB – CONTRACTOR).

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03610 ADHESION & EXPANSION ANCHORS

ANCHORING ADHESIVE SHALL BE A TWO COMPONENT 100 % SOLIDS EPOXY BASED SYSTEM SUPPLIED IN MANUFACTURER'S STANDARD SIDE-BY-SIDE CARTRIDGE AND DISPENSED THROUGH A STATIC-MIXING NOZZLE SUPPLIED BY THE MANUFACTURER. EPOXY SHALL MEET THE MINIMUM REQUIREMENTS OF ASTM C-881 SPECIFICATIONS FOR TYPE I, II, IV, AND V, GRADE 3, CLASS B AND C AND MUST DEVELOP A MINIMUM 12650 PSI COMPRESSIVE YIELD STRENGTH AFTER 7 DAYS CURE. ADHESIVE MUST HAVE DADE COUNTY NOTICE OF ACCEPTANCE. UNLESS OTHERWISE INDICATED ON DRAWING ANCHORS MUST BE INSTALLED PER MANUFACTURER'S SPECIFICATIONS. UNLESS OTHERWISE INDICATED ON DRAWINGS ANCHORS SPACING, AND EMBEDMENT EQUAL TO 9 TIMES THE ANCHOR DIAMETER, AND EDGE DISTANCE OF 1.5 TIMES THE EMBEDMENT MUST BE PROVIDED.

DIAMETER OF HOLE SHALL BE 1/16 INCH LARGER THAN DIAMETER OF ANCHOR BOLT AND 1/8 INCH LARGER THAN DIAMETER OF REINFORCING BAR.

EXPANSION ANCHORS MUST BE A THREADED STUD WITH AN INTEGRAL CONE EXPANDER AND A SINGLE PIECE EXPANSION CLIP. THE STUD SHALL BE CARBON STEEL WITH A MINIMUM 70,000 PSI TENSILE STRENGTH, OR TYPE 303, 304 OR 316 STAINLESS STEEL, AS CALLED FOR ON THE DRAWINGS. ANCHORS SHALL MEET FEDERAL SPECIFICATION A-A-1923A, TYPE 4. UNLESS NOTED OTHERWISE ON DRAWINGS, EXPANSION BOLTS MUST BE KWIK BOLT FROM HILTI WITH DADE COUNTY NOTICE OF ACCEPTANCE #06-0810.13. FOLLOW ALL MANUFACTURER'S SPECIFICATIONS FOR INSTALLATION. UNLESS NOTED ON DRAWINGS THE EMBEDMENT MUST BE 9 TIMES THE ANCHOR DIAMETER, THE MINIMUM EDGE DISTANCE TO BE 12 ANCHOR DIAMETER AND THE CENTER TO CENTER SPACING OF ANCHORS MUST BE TWO TIMES THE EDGE DISTANCE.

IN ALL CASE, ANCHORS MUST HAVE LENGTH IDENTIFICATION HEAD MARKS. CONTRACTOR MUST PROVIDE WITH LOAD PROOF INDICATOR KIT FOR VERIFICATION OF BOLTS CAPACITY. LOCATION AND QUANTITY OF BOLTS TO BE TESTED WILL BE AT THE INSPECTOR'S OPTION. IN NO CASE LESS THAN ONE BOLT PER SIZE, PER APPLICATION.

04220 CONCRETE MASONRY UNIT:

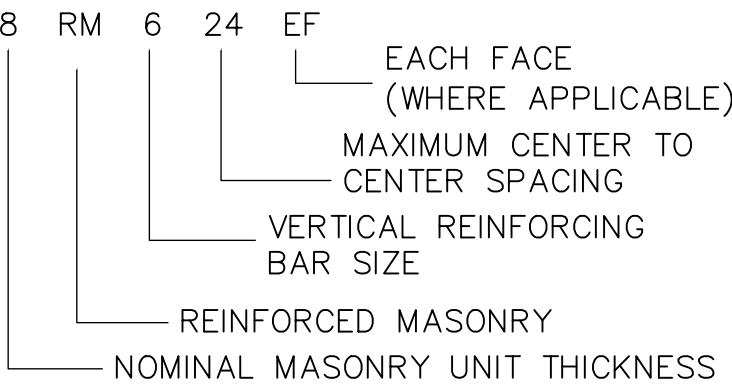
ALL MASONRY CONSTRUCTION TO BE IN ACCORDANCE WITH "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" ACI 530-16 AND STRUCTURES", ACI 530.1 AND ALL APPLICABLE LOCAL BUILDING CODE PROVISIONS. ALL MASONRY WALLS TO BE CONSTRUCTED ENTIRELY OF UNITS CONFORMING TO ASTM C 90, AND REINFORCED WITH #9 GAGE LADDER TRUSS TYPE HORIZONTAL MASONRY REINFORCING LOCATED AT 16" O.C.. FOR TIE-BEAM/TIE COLUMNS OR UNREINFORCED MASONRY, AND LADDER TYPE FOR REINFORCED MASONRY.

ALL MASONRY TO BE LAID IN TYPE "M" MORTAR (AVERAGE COMPRESSIVE STRENGTH AT 28 DAYS OF 2500 PSI ON THE JOB) WITH FULL HEAD AND BED JOINTS. ALL MASONRY CONSTRUCTION TO BE EITHER BOUND BY TIE BEAM, TIE COLUMN MEMBERS AND TIED TO FRAME BY EXTENDING HORIZONTAL JOINT REINFORCING A MINIMUM OF 4" INTO CONCRETE TIE-COLUMN. PROVIDE 12 GAGE GALVANIZED DOVETAIL TRIANGULAR TIES (BY DUR-O-WALL OR EQUIVALENT) AT 16" O.C. WHERE CONCRETE FRAME IS PLACED PRIOR TO MASONRY, UNLESS NOTED OTHERWISE IN DRAWINGS.

WHERE ANCHOR BOLTS ARE SET IN MASONRY WALL, FILL BLOCK CELLS WITH GROUT FOR BOLTED COURSE, ONE COURSE ABOVE AND TWO BELOW ANCHOR ELEVATION.

MASONRY COMPRESSIVE STRENGTH, F'm SHALL BE 2000 PSI. NET AREA COMPRESSIVE STRENGTH OF CONCRETE UNITS SHALL BE 2800 PSI. VERIFICATION THROUGH TEST OF MASONRY COMPRESSIVE STRENGTH MUST BE PERFORMED PRIOR TO CONSTRUCTION AND EVERY 5000 SQUARE FEET DURING CONSTRUCTION.

REINFORCED MASONRY LEGEND



04230 REINFORCED UNIT MASONRY:

ALL REINFORCED MASONRY CONSTRUCTION SHALL BE IN ACCORD WITH APPLICABLE PROVISIONS OF CONCRETE REINFORCEMENT, CAST-IN-PLACE CONCRETE, AND CONCRETE MASONRY. VERTICAL REINFORCING SHALL ANCHOR INTO SUPPORTING CONCRETE MEMBERS A TENSION LAP PER SECTION 3200 PLUS 3" OR FULL DEPTH PLUS A STANDARD HOOK. LAPS WITHIN REINFORCED MASONRY SHALL BE 48 BAR DIAMETERS. CONTRACTOR SHALL COORDINATE PLACING OF DOWELS TO ACCOMMODATE MODULE OF MASONRY UNITS. ALL VERTICAL CELLS AND BEAMS WITH REINFORCING SHALL BE FILLED WITH COARSE GROUT CONSISTING OF 3000 PSI CONCRETE WITH #8 COARSE AGGREGATE. GROUT SLUMP TO BE BETWEEN 8" AND 11". USE HIGH-SLUMP (SUPERPLASTICIZED) WHERE HEIGHT OF LIFT EXCEEDS 4'. WHERE HEIGHT OF OPEN CELL EXCEEDS 4', USE HIGH-LIFT GROUTING TECHNIQUE WHICH REQUIRES A CLEAN-OUT OPENING AT THE BOTTOM OF ALL REINFORCED CELLS AND PLACING THE GROUT IN MAXIMUM 4' LIFTS WITH A 30 TO 60 MINUTE DELAY BETWEEN LIFTS. ALL WALL CORNERS SHALL BE MASONRY BONDED. HORIZONTAL JOINT REINFORCEMENT FOR REINFORCED MASONRY SHALL BE LADDER TYPE #9 GAGE AT 16" O/C VERTICALLY. EXTENDED AT LEAST 4" INTO CONCRETE COLUMNS.

SEE DETAILS AND NOTES ON DRAWINGS FOR SIZE AND SPACING OF REINFORCING BARS (IN NO CASE SHALL WALLS HAVE LESS THAN #5 VERTICAL BARS AT 48 INCHES O.C. IN 8" CMU WALL,) LAP SPLICES OF REINFORCING IN MASONRY AS NOTED BELOW, UNLESS NOTED OTHERWISE.

TIE ALL VERTICAL BARS AT 5'-0" MAXIMUM VERTICALLY WITH SINGLE WIRE LOOP TIE BY A.A. PRODUCTS COMPANY. PROVIDE VERTICAL DOWELS FROM FOOTINGS CONTINUOUS THROUGH STEM WALLS INTO MASONRY ABOVE. DOWELS SHALL MATCH SIZE AND SPACING OF ALL VERTICAL REINFORCING.

BLACK DOTS ON MASONRY WALLS SHOWN ON PLAN DO NOT REPRESENT THE EXACT LOCATION AND/OR QUANTITY OF BARS, BUT RATHER A LEGEND FOR REINFORCED MASONRY. FOR EXACT VERTICAL BARS SPACING AND LOCATIONS SEE NOTES ON PLAN AND DETAILS.

VERTICAL WALL REINFORCEMENT AND COLUMN REINFORCEMENT SHALL PENETRATE INTO BOND BEAMS AND TERMINATE ONTO A STANDARD HOOK AT THE 2" FROM TOP OF WALLS. EXTEND ALL HORIZONTAL BOND BEAM REINFORCING IN MASONRY CONTINUOUS AROUND CORNERS AND INTERSECTIONS OR PROVIDE BENT CORNER BARS TO MATCH AND LAP HORIZONTAL BOND BEAM REINFORCING AT CORNERS AND INTERSECTIONS.

ALL REINFORCING IN MASONRY SHALL BE ACCURATELY LOCATED PRIOR TO GROUTING AND THE POSITION MAINTAINED DURING GROUTING. CONTRACTOR TO PROVIDE A 4" CLEAN OUT ON TOP OF FOOTING AND ON TOP OF BEAMS (AT SPLICES). IF WALL IS TALLER THAN 10' TO NEXT FLOOR OR BOND BEAM, THEN A 2" INSPECTION HOLE MUST BE PROVIDED AT MID-HEIGHT. MINIMUM LAP SPLICES IN MASONRY SHALL BE AS FOLLOWS:

GRADE 60 BARS

1 BAR/CELL 48 BAR DIAMETER

2 BAR/CELL 63 BAR DIAMETER AT 8" CMU WALL

2 BAR/CELL 48 BAR DIAMETER AT 12" CMU WALL

VERTICAL BAR POSITIONER MUST BE PROVIDED TO ENSURE PROPER LOCATION OF THE REBAR AS INDICATED ON DRAWINGS, AS REQUIRED BY ACI 530-13. VERTICAL CELLS TO BE GROUTED SHALL PROVIDE VERTICAL ALIGNMENT, SUFFICIENT TO MAINTAIN CLEAR UNOBSTRUCTED CONTINUOUS VERTICAL CORES MEASURING AT LEAST 2x3 INCHES. DO NOT INTERRUPT GROUTING FOR MORE THAN ONE HOUR. MECHANICALLY VIBRATE GROUT IN VERTICAL SPACES IMMEDIATELY AFTER POURING AND AGAIN ABOUT 5 MINUTES LATER. RODDING OF GROUT IS NOT ACCEPTABLE.

CONTRACTOR IS RESPONSIBLE FOR BRACING OF WALLS DURING CONSTRUCTION SINCE WALLS ARE NOT STABLE UNTIL FLOORS OR ROOFS HAD BEEN COMPLETED. GROUT AND MORTAR MUST BE TESTED PER ACI 530-13; ACI 530.1 ;ASCE 6 AND TMS 602.

UNLESS NOTED OTHERWISE ON THE DRAWINGS, PROVIDE VERTICAL MASONRY CONTROL JOINTS SUCH THAT NO STRAIGHT RUN OF WALL EXCEEDS 24'-0". COORDINATE LOCATIONS WITH ARCHITECT.

CONTRACTOR SHALL PROVIDE LINTELS AS REQUIRED AT ALL OPENINGS. REFER TO TYPICAL DETAILS AND LINTEL SCHEDULES. NOTE: ALL WALL OPENINGS MAY NOT BE SHOWN ON STRUCTURAL DRAWINGS REFER TO ARCHITECTURAL DRAWINGS. MECHANICAL AND ELECTRICAL DRAWINGS FOR ALL OPENINGS SIZE AND LOCATIONS.

05055 WELDING:

ALL WELDING TO BE IN ACCORDANCE WITH THE AMERICAN WELDING SOCIETY (AWS) "STRUCTURAL WELDING CODE-STEEL," D1.1 – 2010 AND AS INDICATED ON THE STRUCTURAL DRAWINGS. WELDING ELECTRODES, WELDING PROCESS, MINIMUM PREHEAT AND INTERPOSE TEMPERATURES TO BE IN ACCORDANCE WITH THE A.W.S. SPECIFICATIONS. ANY STRUCTURAL STEEL DAMAGED IN WELDING TO BE REPLACED OR ACCEPTABLY REINFORCED. ALL FULL PENETRATION GROOVE WELDS TO BE SUBJECT TO RADIOGRAPHIC, MAGNETIC PARTICLE, ULTRASONIC, AND LIQUID PENETRATE INSPECTION CONDUCTED BY AN INDEPENDENT TESTING AGENCY PAID BY THE OWNER. ONE HUNDRED PER CENT OF ALL FIELD WELDS MUST BE VISUALLY INSPECTED BY AN INDEPENDENT LABORATORY, AND A REPORT SUBMITTED TO SEOR. A MINIMUM OF 20% OF ALL WELDS MUST BE TESTED BY ANY AISC APPROVED METHOD, OTHER THAN VISUAL.

WELDING ELECTRODES SHALL CONFORM TO AWS D1.1, GRADE E70XX. E90 SERIES ELECTRODES SHALL BE USED FOR ASTM A706 REINFORCING BARS. ALL WELDING SHALL BE DONE BY WELDERS HOLDING VALID CERTIFICATES ISSUED BY AN ACCEPTED TESTING AGENCY AND HAVING CURRENT EXPERIENCE IN TYPE OF WELDS SHOWN ON THE DRAWINGS OR NOTES. ALL WELDING PER AMERICAN WELDING SOCIETY STANDARDS. ALL WELDS ON DRAWINGS ARE SHOWN AS SHOP WELDS. CONTRACTOR MAY SHOP WELD OR FIELD WELD AT THEIR DISCRETION. SHOP WELDS OR FIELD WELDS SHALL BE SHOWN ON SHOP DRAWINGS.

ALL WELDS SHALL BE PRE QUALIFIED AND ALL WELDERS AND INSPECTORS SHALL BE INSTRUCTED IN THE WPS AND SHALL RETAIN A COPY.

WPS, AS A MINIMUM, SHALL STATE THE WELD POSITION, ELECTRODE TYPE AND SIZE, TRAVEL SPEED, ELECTRODE STICK-OUT, VOLTAGE AND AMPERAGE WITH ACCEPTABLE LIMITS, BEAD SIZE, WELD SEQUENCE, STRESS RELIEVING, AND OTHER RELEVANT DATA.

WELDERS SHALL BE QUALIFIED FOR THE WORK THEY ARE DOING WITH A CURRENT CERTIFICATION ACCORDING TO THE CHAPTER 5, PART C.

05120 STRUCTURAL STEEL:

ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) "SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS". STRUCTURAL STEEL TO CONFORM TO:

SHAPES ----- ASTM A992

PLATES ----- ASTM A36

PIPE ----- ASTM A 53 GRADE B

TUBES ----- ASTM A 500 GRADE B

ALL SHOP AND FIELD CONNECTIONS SHALL BE MADE WITH WELDING. ANCHOR RODS (ANCHOR BOLTS) SHALL BE ASTM F1554 GRADE 36 (FY = 36 KSI). ALL ANCHOR BOLTS IN CONCRETE OR CMU SHALL BE TIED IN PLACE PRIOR TO ANY REQUIRED INSPECTION. BOLTS, ANCHOR RODS, EXPANSION BOLTS, ETC., SHALL BE INSTALLED WITH STEEL WASHERS AND TIGHTENED NUTS. INSPECTION OF CONNECTED PARTS MUST BE AS PER AISC, BUT NOT LESS THAN 100% OF ALL CONNECTED PARTS.

ALL WELDING ELECTRODES SHALL CONSIST OF THE E70-XX TYPE. ANY CONNECTION NOT SPECIFICALLY DETAILED SHALL BE DESIGNED BY THE SPECIALTY ENGINEER FOR THE FORCES SHOWN ON THE STRUCTURAL CONSTRUCTION DOCUMENTS. WHERE FORCES ARE NOT PROVIDED DESIGN SHALL BE BASED ON THE MAXIMUM LOAD CAPACITIES OF THE CONNECTING MEMBERS. ALL STRUCTURAL SUBMITTALS REQUIRING ENGINEERING INPUT SHALL BE ACCOMPANIED BY DESIGN CALCULATIONS AND BE SIGNED AND SEALED BY THE SPECIALTY ENGINEER. ALL STEEL AT AND BELOW FINISHED GRADE TO BE FIELD PAINTED AND COVERED WITH A MINIMUM OF 2" CONCRETE. PROVIDE FABRICATOR'S STANDARD RUST-INHIBITING PRIMER SHOP PAINT FOR ALL STEEL SURFACES THAT WOULD BE EXPOSED TO WEATHER AT ITS FINAL INSTALLATION. SURFACES PERMANENTLY PROTECTED FROM THE WEATHER, ENCASED IN CONCRETE, OR TO RECEIVE SPRAY-APPLIED FIREPROOFING SHALL NOT BE SHOP PAINTED. DEFORMED BAR ANCHOR STUDS SHALL BE NELSON D2L ASTM A-496 GRANULAR FLUX-FILLED REBAR STUDS OR APPROVED EQUAL MADE FROM LOW CARBON COLD ROLLED STEEL WITH A MINIMUM TENSILE STRENGTH OF 70,000 PSI. STUD WELDING INSPECTION AND TESTING SHALL CONFORM TO AWS D1.1.

06410 PRE-MANUFACTURED PEDESTRIAN GUARDRAILS / HANDRAILS:

THE RAILING MANUFACTURER SHALL BE RESPONSIBLE FOR THE COMPLETE DESIGN AND FABRICATION OF THE RAILS. THE MANUFACTURER SHALL SUBMIT DESIGN CALCULATIONS AND SHOP DRAWINGS FOR ALL GUARDRAILS AND HANDRAILS FOR REVIEW PRIOR TO MANUFACTURE. CALCULATIONS AND SHOP DRAWINGS SHALL BE SEALED BY A CIVIL OR STRUCTURAL ENGINEER REGISTERED IN THE STATE OF THE LOCAL JURISDICTION. ALL SHOP MADE ASSEMBLIES SHALL USE STRUCTURAL STEEL PIPES OR TUBES. USE ALL WELDED CONSTRUCTION.

RAILING SHALL BE DESIGNED FOR DEAD, WIND, AND SEISMIC LOADS, AND THE FOLLOWING LIVE LOADS:

HANDRAILS, GUARDRAILS AND VERTICAL RAILS: DESIGNED TO RESIST A UNIFORM LOAD OF 50 PLF APPLIED IN ANY DIRECTION OR A CONCENTRATED LOAD OF 200 POUNDS APPLIED IN ANY DIRECTION. THESE TWO SEPARATE LOADING CASES (UNIFORM AND CONCENTRATED) NEED NOT BE ASSUMED TO ACT CONCURRENTLY. LOADS FROM RAILS SHALL TRANSFER TO THE MAIN BUILDING STRUCTURE FOR RAIL DIMENSIONS, SEE ARCHITECTURAL DRAWINGS.

THIS SET OF STRUCTURAL DOCUMENTS MUST BE SUBMITTED TO BUILDING DEPARTMENT FOR APPROVAL BEFORE CONSTRUCTION. THIS SET IS NOT IN ITS FINAL FORM UNTIL IT BEARS THE APPROVAL SEAL FROM THE CORRESPONDING AUTHORITIES AS REQUIRED BY THE BUILDING CODE. THIS SET OF PLANS IS NOT VALID UNLESS SIGNED AND SEALED BY THE SEOR.

TERMITE PROTECTION.

TERMITE PROTECTION SHALL BE PROVIDED BY REGISTERED TERMITICIDES, INCLUDING SOIL APPLIED PESTICIDES, BAITING SYSTEMS, AND PESTICIDES APPLIED TO WOOD, OR OTHER APPROVED METHODS OF TERMITE PROTECTION LABELED FOR USE AS A PREVENTATIVE TREATMENT TO NEW CONSTRUCTION. SEE SECTION FBC 202, REGISTERED TERMITICIDE. UPON COMPLETION OF THE APPLICATION OF THE TERMITE PROTECTIVE TREATMENT, A CERTIFICATE OF COMPLIANCE SHALL BE ISSUED TO THE BUILDING DEPARTMENT BY THE LICENSED PEST CONTROL COMPANY THAT CONTAINS THE FOLLOWING STATEMENT: "THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES. TREATMENT IS IN ACCORDANCE WITH RULES AND LAWS ESTABLISHED BY THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES." REFER TO FBC SECTION 1816, CHAPTER 18 FOR ALL TERMITE APPLICATION RELATED INFORMATION AS WELL AS ARCHITECTURAL DRAWINGS..

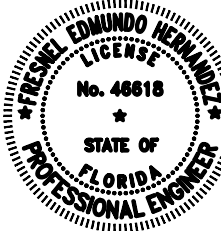
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CASA AT 63RD AVENUE

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PHASE

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FILE NAME:
File Path

PROJECT NO:
24-115

DRAWN BY:
FH

CHECKED BY:
FEH

ISSUE DATE:
11-14-2024

REVISIONS

GENERAL NOTES

SHEET TITLE

S003

SHEET: OF: