



MARTINEZ TRUSS CO., INC.

9280 NW SOUTH RIVER DR. - MEDLEY, FL 33166

Office: (305)883-6261

Fax: (305)885-2925

info@MartinezTruss.com

Order #: 23081008

Customer Information

Name:

SOL-ARCH, INC.

Address:

9485 SUNSET DRIVE A292

City, State, Zip:

Miami, FL 33173

Job Information

Name:

5900 RES.

Address:

5900 S.W. 114 TER.

City, State, Zip:

Pinecrest, FL 33156

Building Code: FBC 8th Ed. 2023

Roof Loading:

TC LL: 30 psf

TC DL: 15 psf

BC LL: 0 psf

BC DL: 10 psf

Wind Loading:

Wind Standard: ASCE 7-22

Wind Speed: 175

WindLoadType: MWFRS & C&C

Wind Usage: Residential - CAT II

Project Engineer of Record

- ☒ REVIEWED AND APPROVED
☐ REVIEWED, MARKED AND APPROVED
RESUBMIT CORRECTED COPY FOR FILING
☐ REVISE AND RESUBMIT
☐ NOT REVIEWED

REVIEWED ONLY FOR THE LIMITED PURPOSE OF THE CHECKING FOR CONFORMANCE WITH THE INFORMATION GIVEN AND THE DESIGN CONCEPT EXPRESSED IN THE FULL SET OF CONTRACT DOCUMENTS. IT DOES NOT AUTHORIZE CHANGES IN THE CONTRACT SUM, REVIEW OF SHOP DRAWINGS IS NOT CONDUCTED FOR DETERMINING THE ACCURACY AND COMPLETENESS OF DETAILS, QUANTITIES OR FOR SUBSTANTIATING FABRICATION, INSTALLATION, INSTRUCTION OR PERFORMANCE OF EQUIPMENT OF SYSTEM, ALL OF WHICH SHALL REMAIN THE SOLE RESPONSIBILITY OF THE CONTRACTOR. REVIEW OF SHOP DRAWINGS DOES NOT CONSTITUTE APPROVAL OF SAFETY PRECAUTIONS, CONSTRUCTION MEANS, METHODS, SEQUENCES OR PROCEDURES, NOR DOES IT INDICATE APPROVAL OF ASSEMBLY OF WHICH THE ITEM IS A COMPONENT. COORDINATION OF ALL TRADES SHALL REMAIN THE SOLE RESPONSIBILITY OF THE CONTRACTOR. CONTRACTOR MUST REVIEW ALL QUANTITIES AND DIMENSIONS AT THE SITE.

GF CONSULTING ENGINEERS, INC.

BY:  DATE: 8-16-24

This cover sheet provided as per Florida Statute 61G15-31.003 in lieu of signing and sealing each individual sheet. An Index sheet of truss designs is attached which is numbered and with the identification and date of each drawing.

Engineer of Truss Design Package

JOSE MARTINEZ

FL Reg. Eng. No 031509

24 EAST 5th St. #1D

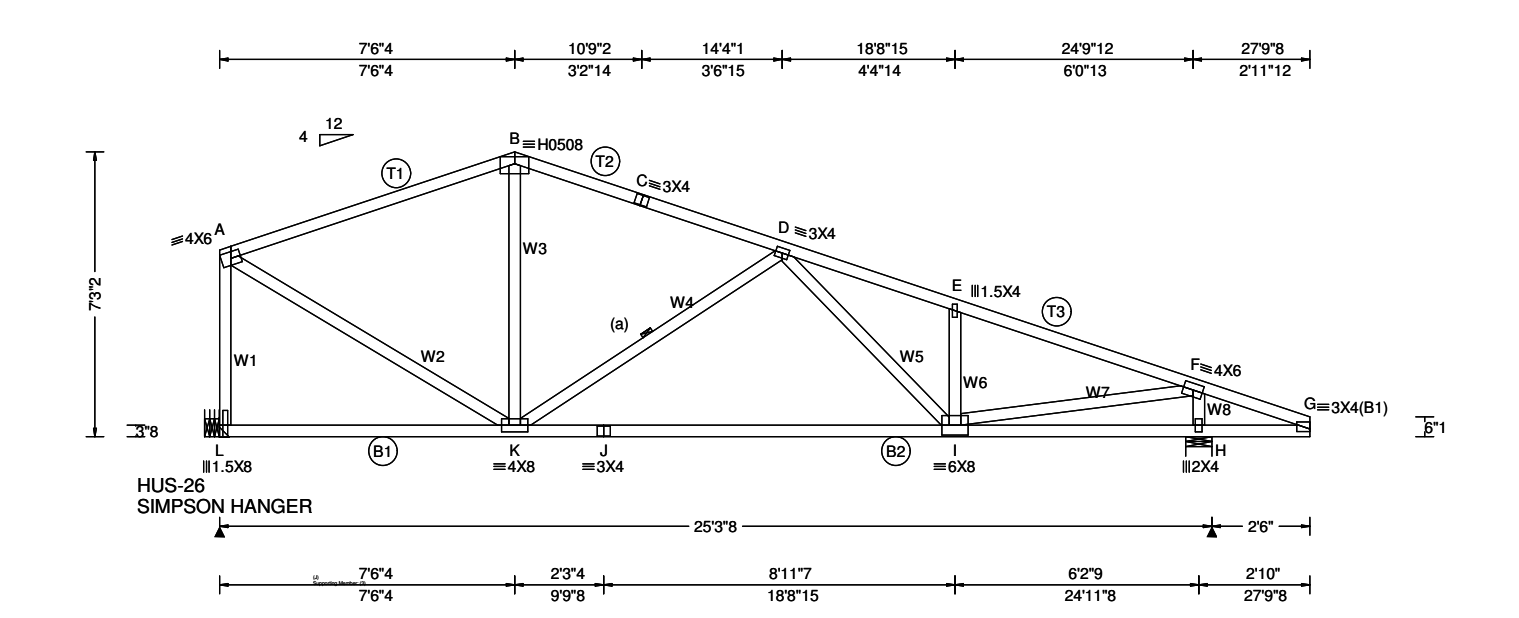
HALEAH, FL 33010

#	SEQN	Item Desc	Date	#	SEQN	Item Desc	Date	#	SEQN	Item Desc	Date
1	136770	1T	7/25/2024	27	136782	2H10	7/25/2024	53	201655	6TB	7/25/2024
2	136773	1H20	7/25/2024	28	139974	2MHG10	7/25/2024	54	139977	6MHG	7/25/2024
3	136674	1H18	7/25/2024	29	140013	2HG8	7/25/2024	55	201683	7T	7/25/2024
4	136753	1H16	7/25/2024	30	201396	3TG	7/25/2024	56	201680	7H12	7/25/2024
5	136738	1H15	7/25/2024	31	136855	3T	7/25/2024	57	201677	7H10	7/25/2024
6	136727	1TH14	7/25/2024	32	136847	3TA	7/25/2024	58	139980	7HG8	7/25/2024
7	136741	1TH14A	7/25/2024	33	136858	3TB	7/25/2024	59	139983	7MHG	7/25/2024
8	136724	1TH12	7/25/2024	34	136864	3TC	7/25/2024	60	201269	8T	7/25/2024
9	136744	1TH12A	7/25/2024	35	136844	3H16	7/25/2024	61	201272	8H10	7/25/2024
10	136695	1TH10	7/25/2024	36	140010	3HG14	7/25/2024	62	201275	8H10A	7/25/2024
11	136747	1TH10A	7/25/2024	37	136879	4T	7/25/2024	63	139986	8HG8	7/25/2024
12	136701	1TH10B	7/25/2024	38	136882	4TA	7/25/2024	64	139989	8MHG8	7/25/2024
13	136692	1TH8	7/25/2024	39	136873	4H14	7/25/2024	65	201235	9M	7/25/2024
14	139968	1THG8	7/25/2024	40	136870	4H12	7/25/2024	66	201238	9MA	7/25/2024
15	139971	1THG8A	7/25/2024	41	136867	4H10	7/25/2024	67	201241	9MH14	7/25/2024
16	136683	1TH6	7/25/2024	42	140007	4HG8	7/25/2024	68	201244	9MH14A	7/25/2024
17	201692	1THG4	7/25/2024	43	201206	5H12	7/25/2024	69	201247	9MH12	7/25/2024
18	136835	2T	7/25/2024	44	201197	5H10	7/25/2024	70	201250	9MH12A	7/25/2024
19	136826	2H18	7/25/2024	45	140004	5HG8	7/25/2024	71	201253	9MH10	7/25/2024
20	140016	2HG17	7/25/2024	46	201166	5TH6	7/25/2024	72	139992	9MHG8	7/25/2024
21	136808	2H16	7/25/2024	47	201172	5TH6A	7/25/2024	73	200902	1FRT	7/25/2024
22	136829	2H16A	7/25/2024	48	139998	5THG4	7/25/2024	74	139944	CJ10	7/25/2024
23	136798	2H14	7/25/2024	49	140001	5THG4A	7/25/2024	75	139947	CJ8	7/25/2024
24	136832	2H14A	7/25/2024	50	139995	5THG4B	7/25/2024	76	139950	CJ8A	7/25/2024
25	136788	2H12	7/25/2024	51	201643	6T	7/25/2024	77	139953	CJ8P	7/25/2024
26	136795	2H12A	7/25/2024	52	201649	6TA	7/25/2024	78	139956	CJ8AP	7/25/2024

Order #: 23081008

INDEX SHEET

#	SEQN	Item Desc	Date	#	SEQN	Item Desc	Date	#	SEQN	Item Desc	Date
79	139959	CJ6	7/25/2024								
80	139962	CJ6A	7/25/2024								
81	139965	CJ6B	7/25/2024								
82	200908	J10	7/25/2024								
83	200911	JB8	7/25/2024								
84	200914	J8	7/25/2024								
85	200917	J8P	7/25/2024								
86	200920	J8AP	7/25/2024								
87	201512	J8G	7/25/2024								
88	201517	J8GA	7/25/2024								
89	201261	J8GB	7/25/2024								
90	200923	JB6	7/25/2024								
91	200935	JB6A	7/25/2024								
92	200947	JB6B	7/25/2024								
93	200955	JB6P	7/25/2024								
94	200960	JB6AP	7/25/2024								
95	200963	JB4	7/25/2024								
96	200966	JB4A	7/25/2024								
97	200984	JB4B	7/25/2024								
98	200987	JB4P	7/25/2024								
99	200990	JB4AP	7/25/2024								
100	200998	JB2	7/25/2024								
101	201007	JB2A	7/25/2024								
102	201089	V10	7/25/2024								
103	201092	V6	7/25/2024								
104	201095	V4	7/25/2024								
105	201107	VM12	7/25/2024								
106	201104	VM10	7/25/2024								
107	201110	VM8	7/25/2024								
108	201113	VM6	7/25/2024								
109	201119	VM4	7/25/2024								
110	201122	VM2	7/25/2024								



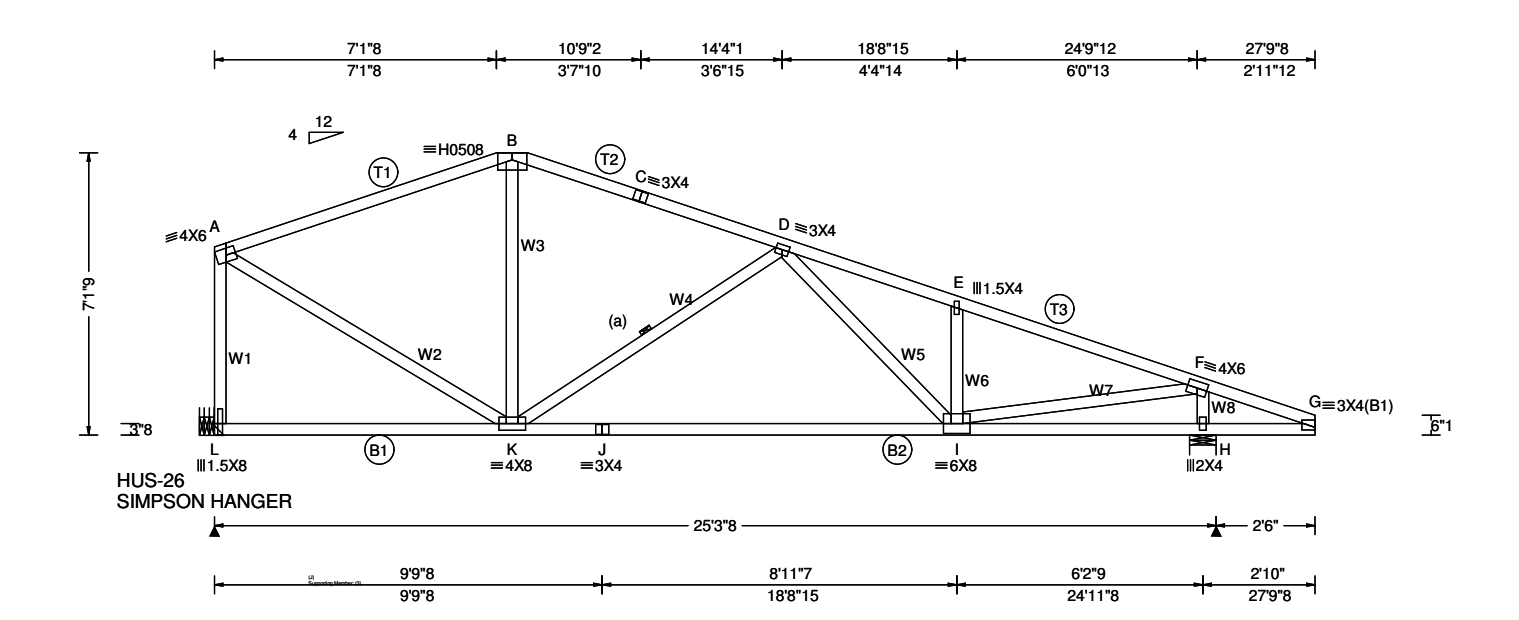
Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria		▲ Maximum Reactions (lbs)					
									Gravity			Non-Gravity		
									Loc			R+ / R- / Rh / Rw / U / RL		
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#		L			1339 - / - / - /886 /472 /297		
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.079 D 999 360		H			1702 - / - / - /1062 /638 - /-		
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.146 D 999 240		Wind reactions based on C&C					
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.027 B - -		L Brg Wid = - Min Req = -					
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS			HORZ(TL): 0.050 B - -		H Brg Wid = 8.0 Min Req = 2.0 (Truss)					
NCBCLL: 10.00		Mean Height: 27.30 ft					Creep Factor: 2.0		Bearing H is a rigid surface.					
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.865		Maximum Top Chord Forces Per Ply (lbs)					
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.923		Chords Tens.Comp. Chords Tens. Comp.					
Spacing: 24.0 "		MWFRS Parallel Dist: h to 2h					Max Web CSI: 0.554		A - B 469 - 1269 D - E 718 - 2213					
		C&C Dist a: 3.00 ft					Mfg Specified Camber:		B - C 508 - 1165 E - F 621 - 2249					
		Loc. from endwall: not in 9.00 ft					VIEW Ver: 23.02.01A.1204.18							
		GCpi: 0.18												
		Wind Duration: 1.60												

Lumber
Top chord: 2x4 SP #2; T1 2x4 SP SS Dense;
Bot chord: 2x4 SP #1; B2 2x4 SP #2;
Webs: 2x4 SP #2;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Wind
Member design based on both MWFRS and C&C.
Left end vertical not exposed to wind pressure.
Right cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSi (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSi. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSi sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org



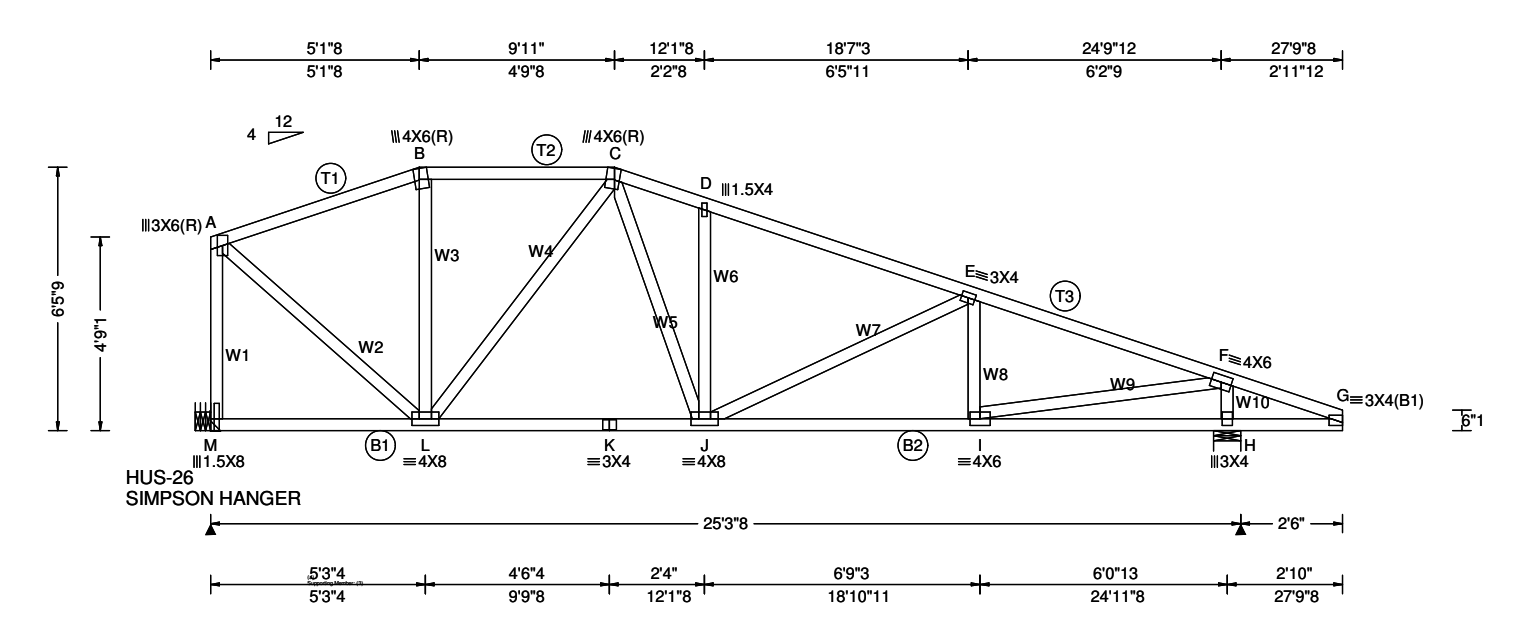
Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)								
TCLL:	30.00	Wind Std:	ASCE 7-22	Pg: NA	Ct: NA	CAT: NA	PP Deflection in	loc L/defl	L/#	Gravity			Non-Gravity					
TCDL:	15.00	Speed:	175 mph	Pf: NA		Ce: NA	VERT(LL):	0.079	D	999	360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL):	0.147	D	999	240	L	1339	/-	/-	/859	/453	/294
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA		HORZ(LL):	0.026	B	-	-	H	1702	/-	/-	/1050	/628	/-
		EXP:	C Kzt: NA				HORZ(TL):	0.049	B	-	-	Wind reactions based on C&C						
Des Ld:	55.00	Mean Height:	27.23 ft			Building Code:	Creep Factor:	2.0				L	Brg Wid = -			Min Req = -		
NCBCLL:	10.00	TCDL:	6.0 psf			FBC 8th Ed. 2023 Res. HVHZ	Max TC CSI:	0.975				H	Brg Wid = 8.0			Min Req = 2.0 (Truss)		
Soffit:	2.00	BCDL:	6.0 psf			TPI Std: 2014	Max BC CSI:	0.923				Bearing H is a rigid surface.						
Load Duration:	1.25	MWFRS Parallel Dist:	h to 2h			Rep Fac: Yes	Max Web CSI:	0.533				Maximum Top Chord Forces Per Ply (lbs)						
Spacing:	24.0 "	C&C Dist a:	3.00 ft			FT/RT:20(0)/10(0)	Mfg Specified Camber:					Chords	Tens.Comp.	Chords	Tens.	Comp.		
		Loc. from endwall:	not in 9.00 ft			Plate Type(s):						A - B	443	- 1270	D - E	698	- 2213	
		GCpi:	0.18			WAVE, HS						B - C	481	- 1166	E - F	600	- 2249	
		Wind Duration:	1.60															
				</														

Lumber Top chord: 2x4 SP #2; T1 2x4 SP #1; Bot chord: 2x4 SP #1; B2 2x4 SP #2; Webs: 2x4 SP #2;	Bracing (a) Continuous lateral restraint equally spaced on member.	Wind Member design based on both MWFRS and C&C. Left end vertical not exposed to wind pressure. Right cantilever is exposed to wind Wind loading based on both gable and hip roof types.
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Maximum Bot Chord Forces Per Ply (lbs)
Chords Tens.Comp. Chords Tens. Comp.
L - K 294 -87 I - H 202 -213
K - J 1785 -408 H - G 207 -257
J - I 1785 -408

Maximum Web Forces Per Ply (lbs)
Webs Tens.Comp. Webs Tens. Comp.
A - L 483 -1290 D - I 378 -92
A - K 1296 -403 I - E 280 -451
K - D 416 -803 I - F 2015 -552
B - K 248 0 F - H 612 -1610

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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)							
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity				
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.077 D 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.142 D 999 240			M	1365	/-	/-	/644	/787	/277	
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.028 B - -			H	1702	/-	/-	/904	/745	/-	
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.051 B - -			Wind reactions based on C&C							
NCBCLL: 10.00		Mean Height: 26.90 ft					Creep Factor: 2.0			M Brg Wid = - Min Req = -							
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.657			H Brg Wid = 8.0 Min Req = 2.0 (Truss)							
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.583			Bearing H is a rigid surface.							
Spacing: 24.0 "		MWFRS Parallel Dist: h to 2h					Max Web CSI: 0.636			Maximum Top Chord Forces Per Ply (lbs)							
		C&C Dist a: 3.00 ft					Mfg Specified Camber:			Chords		Tens.Comp.		Chords		Tens. Comp.	
		Loc. from endwall: not in 9.00 ft								A - B		677 - 1075		D - E		908 - 1800	
		GCpi: 0.18								B - C		710 - 958		E - F		893 - 2226	
		Wind Duration: 1.60															

Lumber				Maximum Bot Chord Forces Per Ply (lbs)			
Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2;				Chords		Tens. Comp.	
Wind				M - L		277 - 85	
Member design based on both MWFRS and C&C.				L - K		1325 - 542	
Left end vertical not exposed to wind pressure.				K - J		1325 - 542	
Right cantilever is exposed to wind				J - I		2052 - 740	
Wind loading based on both gable and hip roof types.				I - H		190 - 193	
				H - G		219 - 243	
Maximum Web Forces Per Ply (lbs)				Webs		Tens. Comp.	
				A - M		809 - 1325	
				A - L		1260 - 770	
				B - L		349 - 197	
				L - C		338 - 610	
				C - J		852 - 316	
				J - D		275 - 444	
				J - E		243 - 487	
				E - I		252 - 274	
				I - F		2003 - 832	
				F - H		735 - 1577	

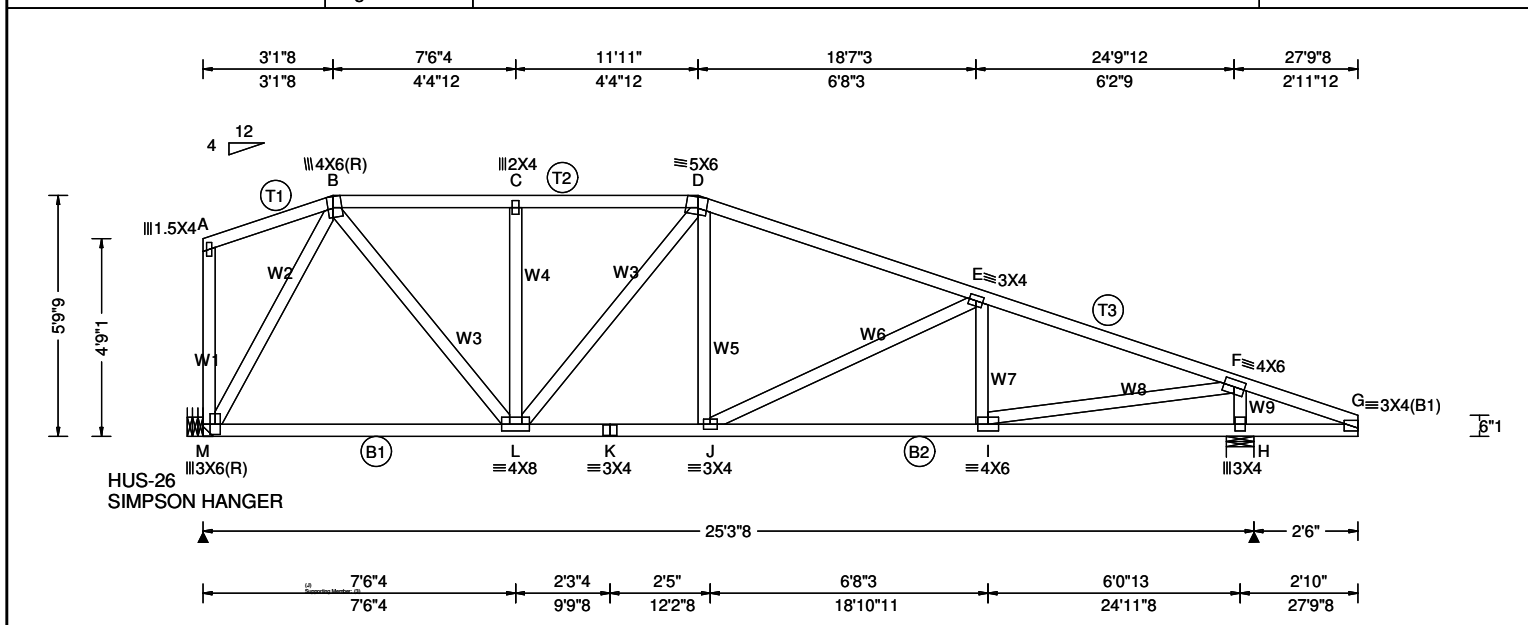
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity			Non-Gravity			
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): 0.075 E 999 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.139 E 999 240	M	1365	/-	/-	/644	/1038	/268
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.031 B - -	H	1702	/-	/-	/918	/839	/-
	EXP: C Kzt: NA		HORZ(TL): 0.057 B - -	Wind reactions based on C&C						
Des Ld: 55.00	Mean Height: 26.57 ft	Building Code:	Creep Factor: 2.0	M	Brg Wid = -			Min Req = -		
NCBCLL: 10.00	TCDL: 6.0 psf	FBC 8th Ed. 2023 Res. HVHZ	Max TC CSI: 0.706	H	Brg Wid = 8.0			Min Req = 2.0 (Truss)		
Soffit: 2.00	BCDL: 6.0 psf	TPI Std: 2014	Max BC CSI: 0.677	Bearing H is a rigid surface.						
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.903	Maximum Top Chord Forces Per Ply (lbs)						
Spacing: 24.0 "	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	Mfg Specified Camber:	Chords	Tens.Comp.		Chords	Tens. Comp.		
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	VIEW Ver: 23.02.01A.1204.18	A - B	53 -45		D - E	1152 -1789		
	GCpi: 0.18	WAVE		B - C	1115 -1343		E - F	1100 -2233		
	Wind Duration: 1.60									

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2;		C - D	1115	-1343	F - G	313	-221
Wind Member design based on both MWFRS and C&C. Left end vertical not exposed to wind pressure. Right cantilever is exposed to wind Wind loading based on both gable and hip roof types.		Maximum Bot Chord Forces Per Ply (lbs)					
		Chords	Tens.Comp.		Chords	Tens.	Comp.
		M - L	660	-336	J - I	2060	-939
		L - K	1583	-842	I - H	189	-192
		K - J	1583	-842	H - G	213	-247
		Maximum Web Forces Per Ply (lbs)					
		Webs	Tens.Comp.		Webs	Tens.	Comp.
		A - M	128	-140	D - J	359	-60
		M - B	1088	-1335	J - E	260	-581
		B - L	1095	-806	E - I	291	-271
		C - L	689	-412	I - F	2018	-1000
		L - D	188	-405	F - H	826	-1578

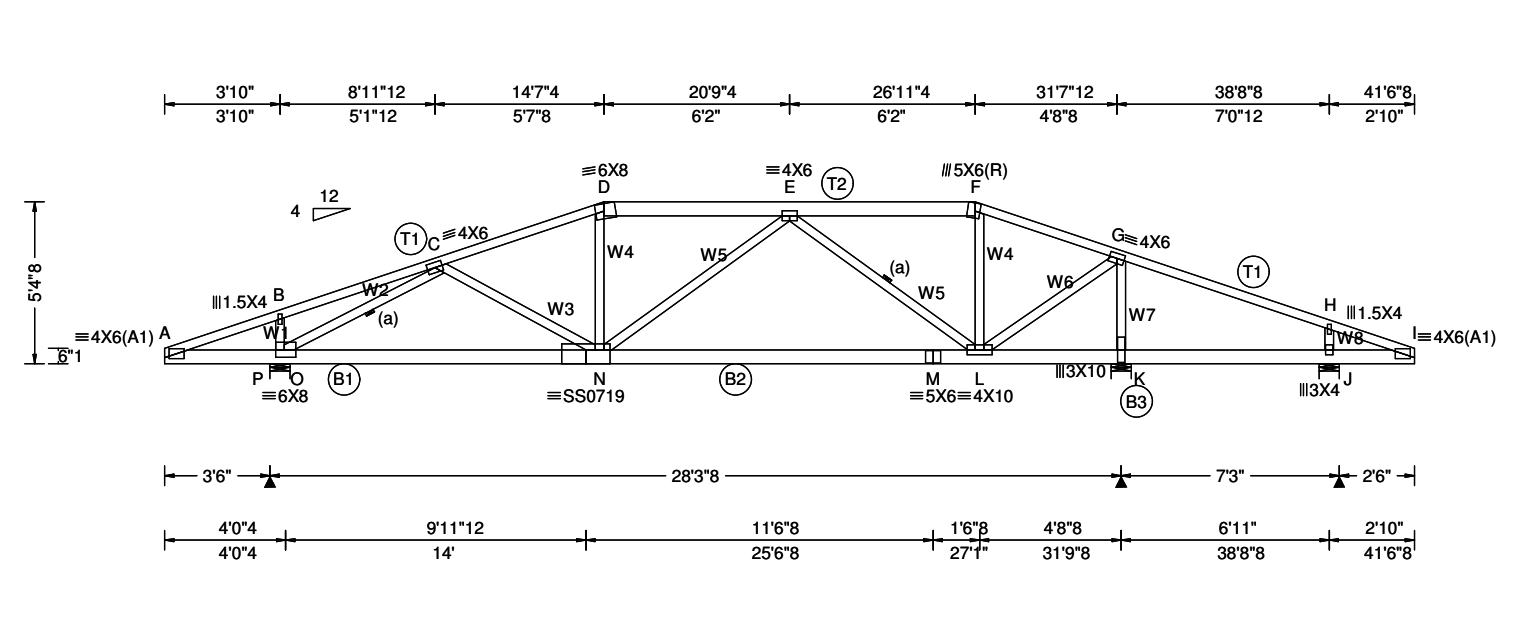
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)					
				Gravity			Non-Gravity		
				Loc	R+	R- / Rh	Rw	U	RL
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	P	1978	- / -	1528	1438	153
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): -0.077 D 999 360	K	2029	- / -	1467	1681	-
BCLL: 0.00	Enclosure: Part. Enc.	Lu: NA Cs: NA	VERT(LL): 0.138 N 999 240	J	659	- / -	602	431	-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.025 L - -	Wind reactions based on C&C					
Des Ld: 55.00	EXP: C Kzt: NA	Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, 18SS	HORZ(TL): 0.046 L - -	P Brg Wid = 8.0 Min Req = 2.3 (Truss)					
NCBCLL: 10.00	Mean Height: 27.19 ft		Creep Factor: 2.0	K Brg Wid = 8.0 Min Req = 2.4 (Truss)					
Soffit: 2.00	TCDL: 6.0 psf		Max TC CSI: 0.781	J Brg Wid = 8.0 Min Req = 1.5 (Truss)					
Load Duration: 1.25	BCDL: 6.0 psf		Max BC CSI: 0.630	Bearings P, K, & J are a rigid surface.					
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h		Max Web CSI: 0.708	Maximum Top Chord Forces Per Ply (lbs)					
	C&C Dist a: 4.15 ft		Mfg Specified Camber:	Chords	Tens.Comp.	Chords	Tens. Comp.		
	Loc. from endwall: not in 13.00 ft		VIEW Ver: 23.02.01A.1204.18	A - B	456 -401	E - F	1351 -1151		
	GCpi: 0.55			B - C	522 -398	F - G	1334 -1289		
	Wind Duration: 1.60			C - D	2124 -2164	G - H	306 -96		
				D - E	2123 -1978	H - I	299 -185		

Lumber Top chord: 2x4 SP #2; T2 2x6 SP #2; Bot chord: 2x6 SP #2; Webs: 2x4 SP #2; Bracing (a) Continuous lateral restraint equally spaced on member. Wind Member design based on both MWFRS and C&C. Left and right cantilevers are exposed to wind Wind loading based on both gable and hip roof types. Right bottom chord exposed to wind.	Maximum Bot Chord Forces Per Ply (lbs)			
	Chords	Tens.Comp.	Chords	Tens. Comp.
	A - O	373 -390	L - K	152 -219
	O - N	1885 -1692	K - J	199 -245
	Maximum Web Forces Per Ply (lbs)			
	Webs	Tens.Comp.	Webs	Tens. Comp.
	B - O	377 -436	E - L	1331 -1076
	O - C	2052 -2527	F - L	185 -99
	C - N	302 -368	L - G	1527 -1279
	N - D	254 -44	G - K	1683 -1900
	N - E	546 -68	J - H	459 -560

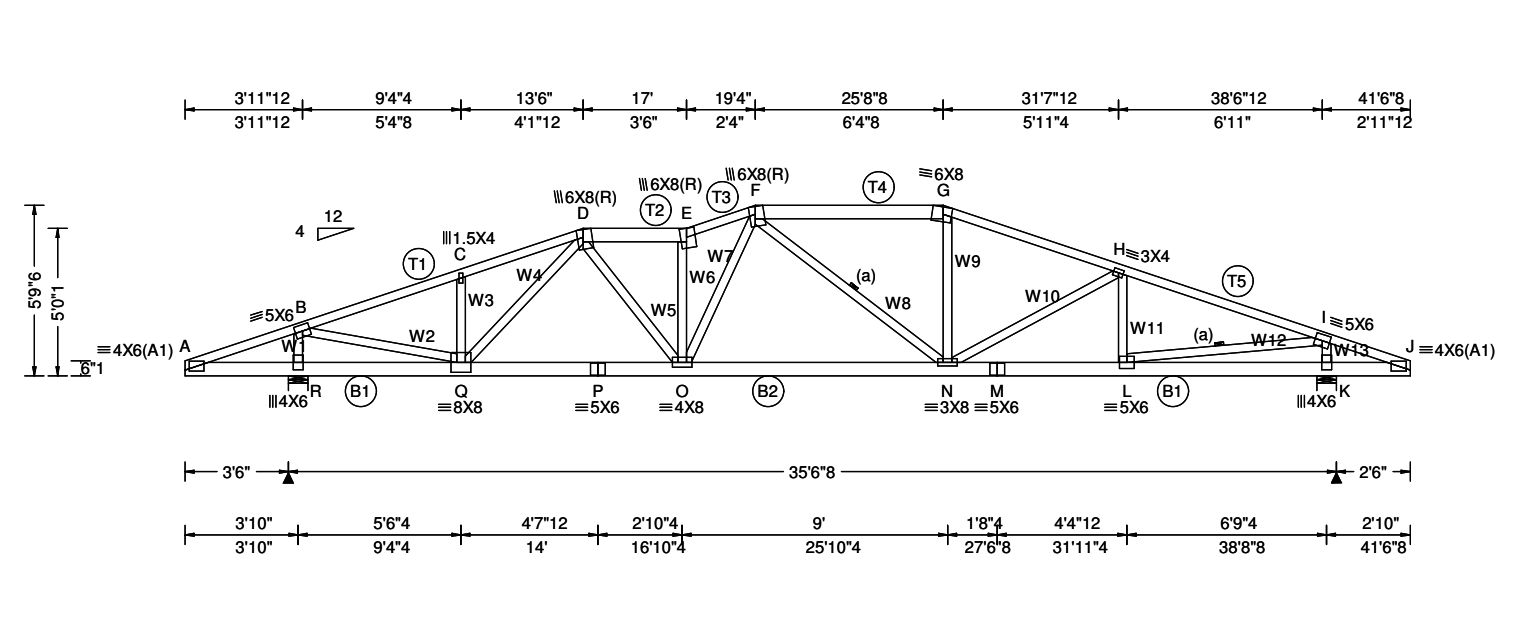
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)						
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity			
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.188 E 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.350 E 999 240			R	2357	-	-	/1259	/1165	/167
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.040 D - -			K	2232	-	-	/1161	/1062	-
Des Ld: 55.00		EXP: C Kzt: NA					HORZ(TL): 0.076 D - -			Wind reactions based on C&C						
NCBCLL: 10.00		Mean Height: 27.39 ft					Creep Factor: 2.0			R Brg Wid = 8.0 Min Req = 2.8 (Truss)						
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.878			K Brg Wid = 8.0 Min Req = 2.6 (Truss)						
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.584			Bearings R & K are a rigid surface.						
Spacing: 24.0 "		MWFRS Parallel Dist: > 2h					Max Web CSI: 0.795			Maximum Top Chord Forces Per Ply (lbs)						
		C&C Dist a: 4.15 ft					Mfg Specified Camber:			Chords Tens.Comp. Chords Tens. Comp.						
		Loc. from endwall: not in 13.00 ft								A - B 478 -297 F - G 1871 -2898						
		GCpi: 0.18								B - C 1704 -2900 G - H 1887 -3148						
		Wind Duration: 1.60														

Lumber
Top chord: 2x4 SP #2; T2,T4 2x6 SP #2;
Bot chord: 2x6 SP #2;
Webs: 2x4 SP #2;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Wind
Member design based on both MWFRS and C&C.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

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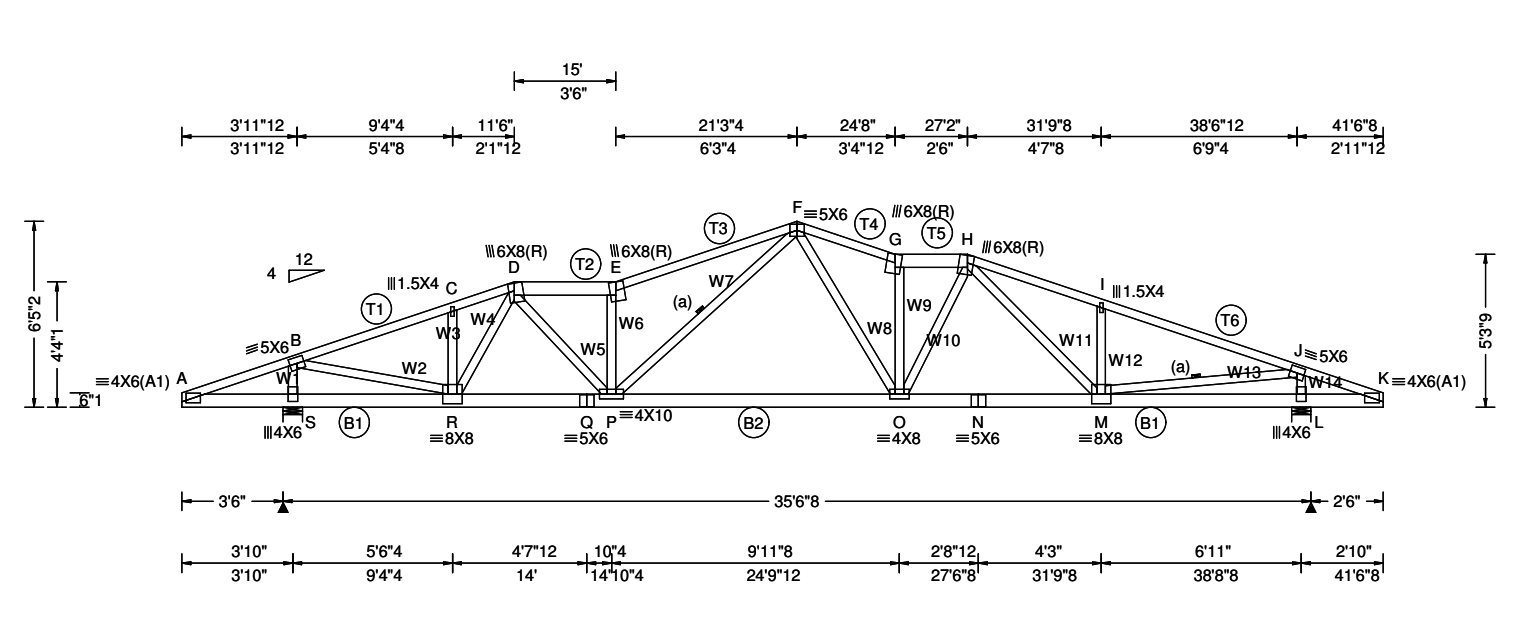
▲ Maximum Reactions (lbs)

Gravity			Non-Gravity					
Loc	R+	/ R-	/ Rh	/ Rw	/ U / RL			
R	2357	-	-	1259	1165 /167			
K	2232	-	-	1161	1062 -			
Wind reactions based on C&C								
R	Brg Wid = 8.0	Min Req = 2.8 (Truss)						
K	Brg Wid = 8.0	Min Req = 2.6 (Truss)						
Bearings R & K are a rigid surface.								
Maximum Top Chord Forces Per Ply (lbs)								
Chords	Tens.Comp.		Chords	Tens. Comp.				
A - B	478	-297	F - G	1871	-2898			
B - C	1704	-2900	G - H	1887	-3148			
C - D	1807	-2871	H - I	1749	-3366			
D - E	2205	-3336	I - J	231	-281			
E - F	2367	-3567						
Maximum Bot Chord Forces Per Ply (lbs)								
Chords	Tens.Comp.		Chords	Tens. Comp.				
A - R	288	-396	N - M	3129	-1561			
R - Q	287	-315	M - L	3129	-1561			
Q - P	2839	-1743	L - K	452	-207			
P - O	2839	-1743	K - J	285	-209			
O - N	3022	-1818						
Maximum Web Forces Per Ply (lbs)								
Webs	Tens.Comp.		Webs	Tens. Comp.				
R - B	1111	-2162	F - N	356	-185			
B - Q	2796	-1464	G - N	418	-108			
C - Q	282	-416	N - H	235	-465			
Q - D	380	-331	H - L	347	-413			
D - O	867	-535	L - I	2821	-1428			
O - E	907	-1266	I - K	1026	-2014			
O - F	846	-500						

The diagram illustrates a roof truss system with the following components and dimensions:

- Members:**
 - Top Chords: $\parallel 1.5 \times 4$ (T1), $\parallel 6 \times 8$ (R) (D), $\equiv 5 \times 6$ (T2), $\parallel 6 \times 8$ (R) (F), $\parallel 1.5 \times 4$ (T1).
 - Bottom Chords: $\equiv 4 \times 6$ (A1), $\parallel 2 \times 10$ (B1), $\equiv 6 \times 8$ (P), $\equiv 5 \times 6$ (O), $\equiv 3 \times 4$ (N), $\equiv 3 \times 6$ (M), $\equiv 5 \times 6$ (L), $\parallel 4 \times 8$ (R) (K), $\parallel 3 \times 4$ (J), $\equiv 4 \times 6$ (A1).
 - Verticals: $\parallel 1.5 \times 4$ (W1), $\parallel 1.5 \times 4$ (W2), $\parallel 1.5 \times 4$ (W3), $\parallel 1.5 \times 4$ (W4), $\parallel 1.5 \times 4$ (W5), $\parallel 1.5 \times 4$ (W6), $\parallel 1.5 \times 4$ (W7).
 - Diagonals: $\parallel 1.5 \times 4$ (W1), $\parallel 1.5 \times 4$ (W2), $\parallel 1.5 \times 4$ (W3), $\parallel 1.5 \times 4$ (W4), $\parallel 1.5 \times 4$ (W5), $\parallel 1.5 \times 4$ (W6), $\parallel 1.5 \times 4$ (W7).
- Joints:** A, B, C, D, E, F, G, H, I, J, K, L, M, N, O, P, Q.
- Dimensions:**
 - Overall height: 5'0" (12'0" total).
 - Horizontal dimensions (top): 3'11"12, 9'7"12, 13'6", 20'9"4, 28'0"8, 31'10"12, 38'8", 41'6"8.
 - Horizontal dimensions (bottom): 3'6", 28'3"8, 7'3", 2'6".
 - Horizontal dimensions (bottom): 3'10", 5'9"12, 4'4"4, 3'1"10, 7'3"4, 3'1"10, 4'2"8, 6'11"8, 2'10".

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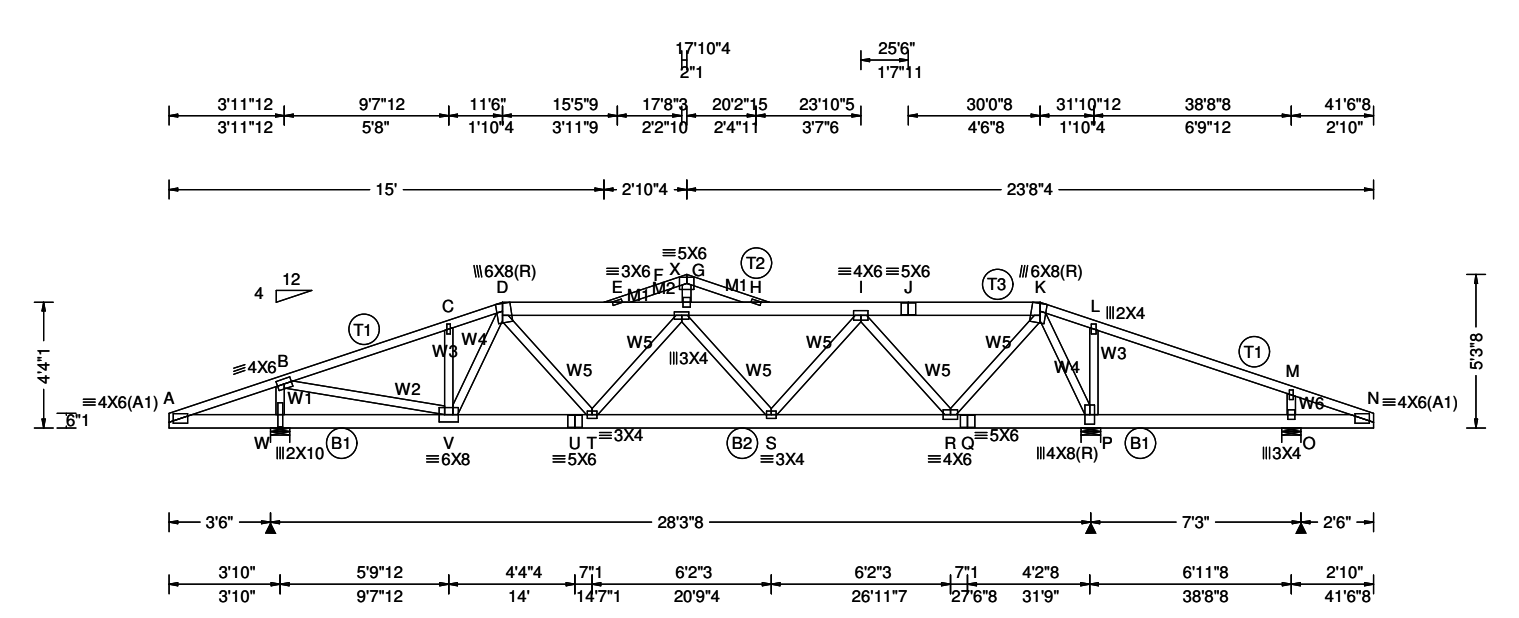
Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)									
TCLL:	30.00	Wind Std:	ASCE 7-22	Pg: NA	Ct: NA	CAT: NA	PP Deflection in	loc L/defl	L/#	Gravity			Non-Gravity						
TCDL:	15.00	Speed:	175 mph	Pf: NA		Ce: NA	VERT(LL):	0.216	G	999	360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL):	0.399	G	999	240	S	2357	-	-	-	/1232	/1097	/186
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA		HORZ(LL):	0.051	D	-	-	L	2232	-	-	-	/1132	/949	-
		EXP:	C Kzt: NA				HORZ(TL):	0.098	D	-	-	Wind reactions based on C&C							
Des Ld:	55.00	Mean Height:	27.72 ft				Creep Factor:	2.0				S	Brg Wid = 8.0	Min Req = 2.8	(Truss)				
NCBCLL:	10.00	TCDL:	6.0 psf				Max TC CSI:	0.882				L	Brg Wid = 8.0	Min Req = 2.6	(Truss)				
Soffit:	2.00	BCDL:	6.0 psf				Max BC CSI:	0.613				Bearings S & L are a rigid surface.							
Load Duration:	1.25	MWFRS Parallel Dist:	> 2h				Max Web CSI:	0.813				Maximum Top Chord Forces Per Ply (lbs)							
Spacing:	24.0 "	C&C Dist a:	4.15 ft				Mfg Specified Camber:					Chords	Tens.	Comp.	Chords	Tens.	Comp.		
		Loc. from endwall: not in	13.00 ft									A - B	471	-303	F - G	1925	-3476		
		GCpi:	0.18									B - C	1541	-2884	G - H	1751	-3226		
		Wind Duration:	1.60																
														</					

Lumber
Top chord: 2x4 SP #2; T2,T5 2x6 SP #2;
T3 2x4 SP #1;
Bot chord: 2x6 SP #2;
Webs: 2x4 SP #2;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Wind
Member design based on both MWFRS and C&C.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)							
				Gravity			Non-Gravity				
				Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	W	1946	-/-	-/-	/1471	/1335	/146	
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): -0.124 E 999 360	P	2136	-/-	-/-	/1518	/1862	-/-	
BCLL: 0.00	Enclosure: Part. Enc.	Lu: NA Cs: NA	VERT(CL): 0.210 E 999 240	O	605	-/-	-/-	/605	/441	-/-	
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.023 P - -	Wind reactions based on C&C							
	EXP: C Kzt: NA		HORZ(TL): 0.041 P - -	W	Brg Wid = 8.0		Min Req = 2.3 (Truss)				
Des Ld: 55.00	Mean Height: 26.67 ft	Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Creep Factor: 2.0	P	Brg Wid = 8.0		Min Req = 2.5 (Truss)				
NCBCLL: 10.00	TCDL: 6.0 psf		Max TC CSI: 0.762	O	Brg Wid = 8.0		Min Req = 1.5 (Truss)				
Soffit: 2.00	BCDL: 6.0 psf		Max BC CSI: 0.440	Bearings W, P, & O are a rigid surface.							
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h		Max Web CSI: 0.965	Maximum Top Chord Forces Per Ply (lbs)							
Spacing: 24.0 "	C&C Dist a: 4.15 ft		Mfg Specified Camber:	Chords		Tens.Comp.		Chords		Tens. Comp.	
	Loc. from endwall: not in 13.00 ft										
	GCpi: 0.55										
	Wind Duration: 1.60		VIEW Ver: 23.02.01A.1204.18								

Lumber
Top chord: 2x4 SP #2; T2,T3 2x6 SP #2;
Bot chord: 2x6 SP #2;
Webs: 2x4 SP #2;
Filler: 2x4 SP #2;

Plating Notes
All plates are 1.5X4 except as noted.

Purlins
Laterally brace TC below filler at 24" oc.

Wind
Member design based on both MWFRS and C&C.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.
Right bottom chord exposed to wind.

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▲ Maximum Reactions (lbs)

Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U / RL
W	1946	-	-	/1471	/1335 /146
P	2136	-	-	/1518	/1862 -
O	605	-	-	/605	/441 -

Wind reactions based on C&C

W Brg Wid = 8.0 Min Req = 2.3 (Truss)

P Brg Wid = 8.0 Min Req = 2.5 (Truss)

O Brg Wid = 8.0 Min Req = 1.5 (Truss)

Bearings W, P, & O are a rigid surface.

Maximum Top Chord Forces Per Ply (lbs)

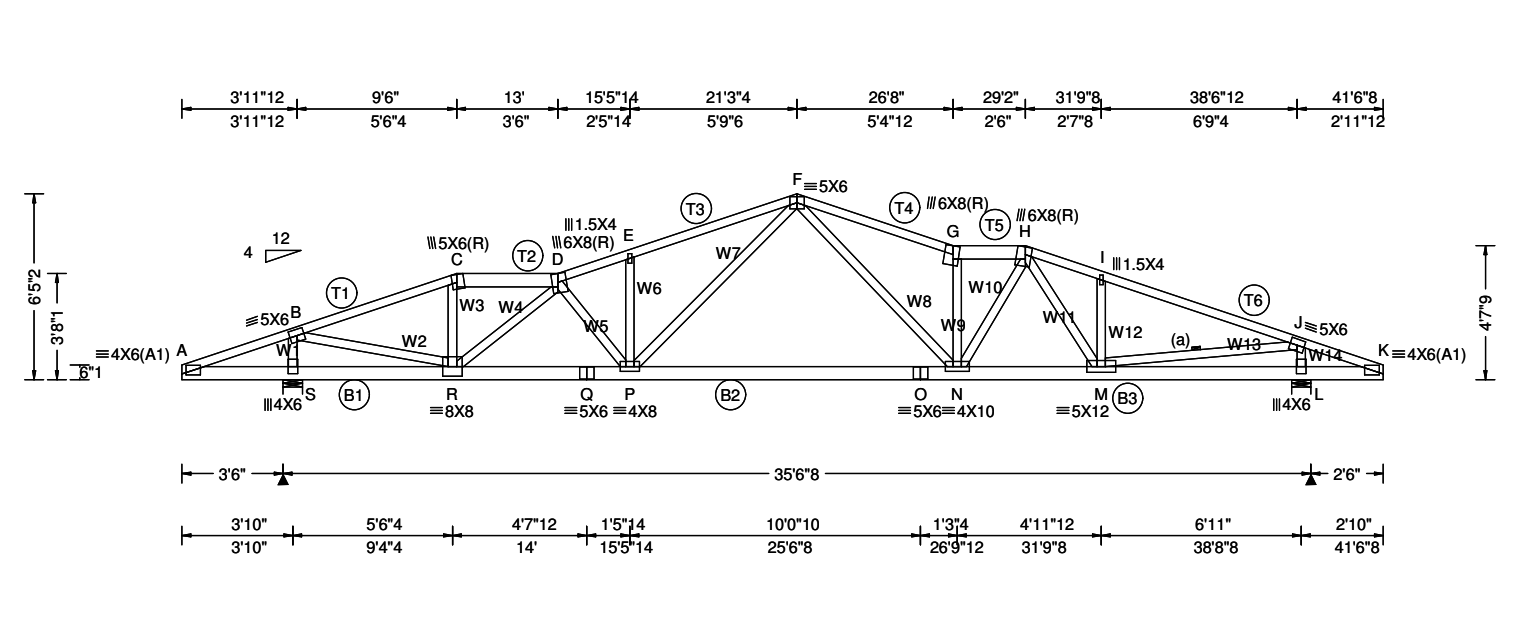
Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	477	-351	H - I	2437	-2364
B - C	2005	-2159	I - J	1344	-1223
C - D	2133	-2100	J - K	1343	-1222
D - E	2378	-2339	K - L	329	-132
E - F	2586	-2451	L - M	425	-157
F - G	2647	-2474	M - N	369	-284
G - H	2647	-2474			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - W	327	-406	S - R	2034	-1936
W - V	352	-327	R - Q	370	-288
V - U	1998	-1797	Q - P	370	-288
U - T	1998	-1797	P - O	296	-307
T - S	2589	-2387	O - N	277	-285

Maximum Web Forces Per Ply (lbs)

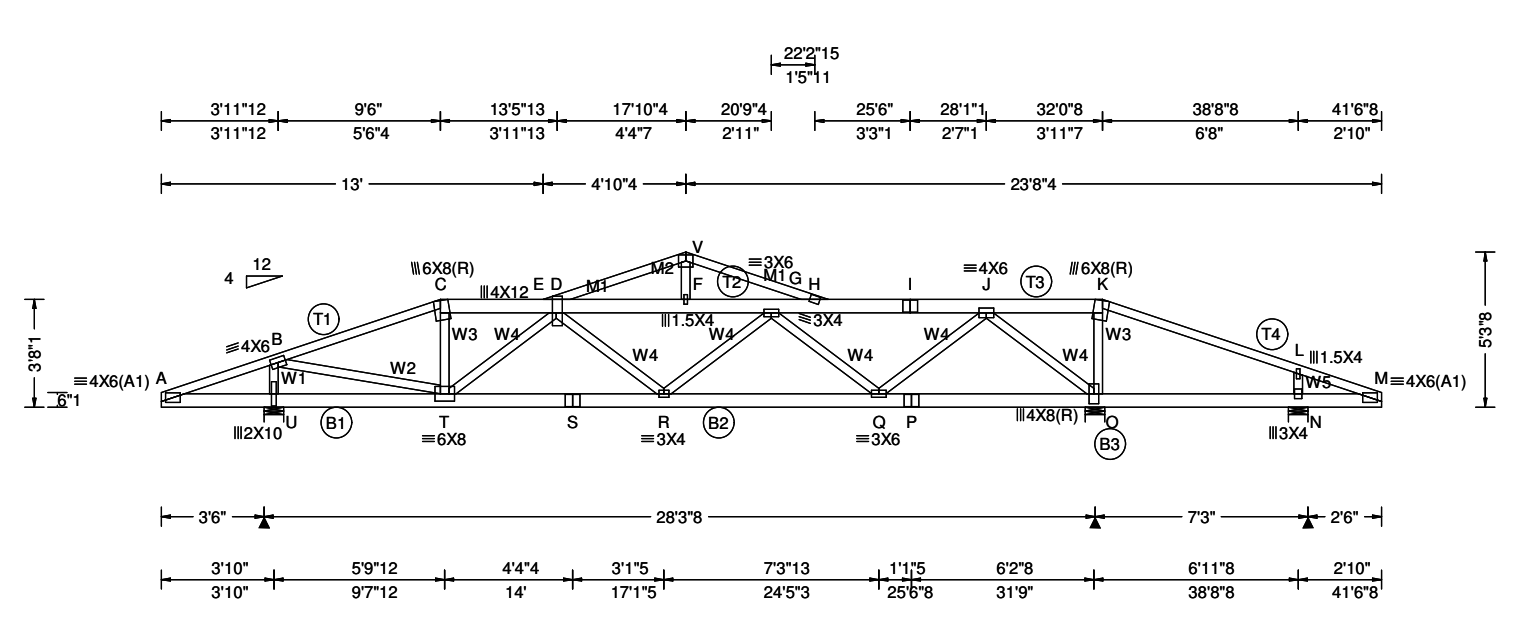
Webs	Tens.	Comp.	Webs	Tens.	Comp.
W - B	1391	-1756	X - G	456	-364
B - V	2116	-1604	X - H	177	-189
C - V	389	-354	S - I	573	-313
V - D	343	-157	I - R	1446	-1347
D - T	594	-442	R - K	1422	-1294
T - F	508	-431	K - P	1397	-1531
E - X	177	-190	P - L	640	-589
F - S	411	-407	O - M	440	-523



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): 0.234 G 999 360	S	2357	-/-	-/-	/1227	/1104	/186
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.434 G 964 240	L	2232	-/-	-/-	/1135	/935	-/-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.051 C - -	Wind reactions based on C&C						
	EXP: C Kzt: NA		HORZ(TL): 0.097 C - -	S	Brg Wid = 8.0		Min Req = 2.8 (Truss)			
Des Ld: 55.00	Mean Height: 27.72 ft		Creep Factor: 2.0	L	Brg Wid = 8.0		Min Req = 2.6 (Truss)			
NCBCLL: 10.00	TCDL: 6.0 psf	Building Code:	Max TC CSI: 0.903	Bearings S & L are a rigid surface.						
Soffit: 2.00	BCDL: 6.0 psf	FBC 8th Ed. 2023 Res. HVHZ	Max BC CSI: 0.714	Maximum Top Chord Forces Per Ply (lbs)						
Load Duration: 1.25	MWFRS Parallel Dist: > 2h	TPI Std: 2014	Max Web CSI: 0.907	Chords	Tens.Comp.		Chords	Tens. Comp.		
Spacing: 24.0 "	C&C Dist a: 4.15 ft	Rep Fac: Yes	Mfg Specified Camber:	A - B	470	-304	F - G	2060	-3855	
	Loc. from endwall: not in 13.00 ft	FT/RT:20(0)/10(0)		B - C	1527	-2904	G - H	1842	-3529	
	GCpi: 0.18	Plate Type(s):								
	Wind Duration: 1.60	WAVE	VIEW Ver: 23.02.01A.1204.18							

Lumber Top chord: 2x4 SP #2; T2,T5 2x6 SP #2; Bot chord: 2x6 SP #2; Webs: 2x4 SP #2;	Bracing (a) Continuous lateral restraint equally spaced on member.	Wind Member design based on both MWFRS and C&C. Left and right cantilevers are exposed to wind Wind loading based on both gable and hip roof types.
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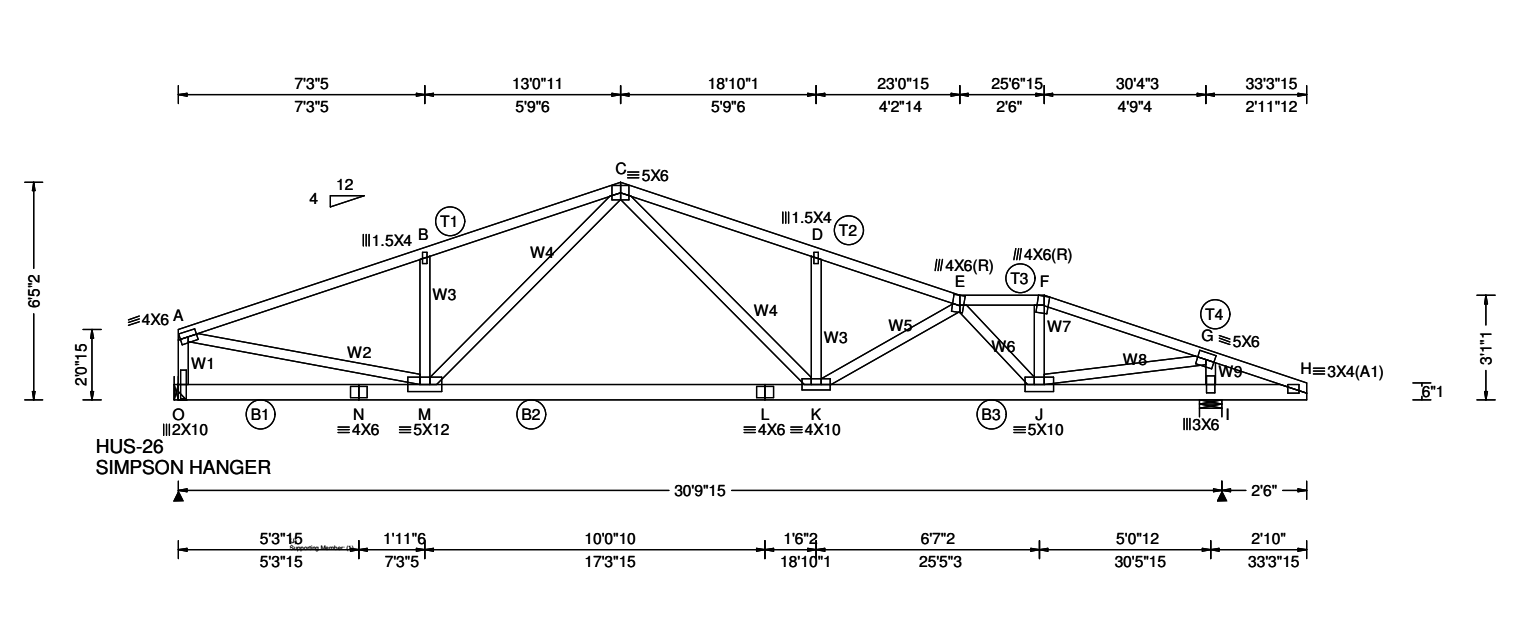
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)					
				Gravity			Non-Gravity		
	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL		
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	U	1954	-/-	-/-	/1450 /1335 /146	
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): -0.152 H 999 360	O	2100	-/-	-/-	/1476 /1888 -/-	
BCLL: 0.00	Enclosure: Part. Enc.	Lu: NA Cs: NA	VERT(CL): 0.256 H 999 240	N	650	-/-	-/-	/593 /450 -/-	
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.030 O - -	Wind reactions based on C&C					
	EXP: C Kzt: NA		HORZ(TL): 0.053 O - -	U	Brg Wid = 8.0		Min Req = 2.3 (Truss)		
Des Ld: 55.00	Mean Height: 26.34 ft		Creep Factor: 2.0	O	Brg Wid = 8.0		Min Req = 2.5 (Truss)		
NCBCLL: 10.00	TCDL: 6.0 psf	Building Code:	Max TC CSI: 0.716	N	Brg Wid = 8.0		Min Req = 1.5 (Truss)		
Soffit: 2.00	BCDL: 6.0 psf	FBC 8th Ed. 2023 Res. HVHZ	Max BC CSI: 0.529	Bearings U, O, & N are a rigid surface.					
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max Web CSI: 0.980	Maximum Top Chord Forces Per Ply (lbs)					
Spacing: 24.0 "	C&C Dist a: 4.15 ft	Rep Fac: Yes	Mfg Specified Camber:	Chords	Tens.Comp.		Chords	Tens. Comp.	
	Loc. from endwall: not in 13.00 ft	FT/RT:20(0)/10(0)							
	GCpi: 0.55	Plate Type(s):							
	Wind Duration: 1.60	WAVE	VIEW Ver: 23.02.01A.1204.18						

Lumber Top chord: 2x6 SP #2; T1 2x4 SP #2; T4 2x4 SP #1; Bot chord: 2x6 SP #2; Webs: 2x4 SP #2; Filler: 2x4 SP #2;	Plating Notes All plates are 5X6 except as noted.	Purlins Laterally brace TC below filler at 24" oc.	Wind Member design based on both MWFRS and C&C. Left and right cantilevers are exposed to wind Wind loading based on both gable and hip roof types. Right bottom chord exposed to wind.	Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - U 321 -411 Q - P 1364 -1307 U - T 352 -334 P - O 1364 -1307 T - S 2756 -2475 O - N 307 -332 S - R 2756 -2475 N - M 284 -320 R - Q 2904 -2627	Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. U - B 1433 -1774 V - F 257 -179 B - T 2192 -1630 V - H 396 -544 C - T 254 -68 G - Q 981 -999 T - D 875 -999 Q - J 1159 -776 D - V 387 -544 J - O 2193 -2140 E - R 282 -114 O - K 423 -450 R - G 264 -20 N - L 599 -605
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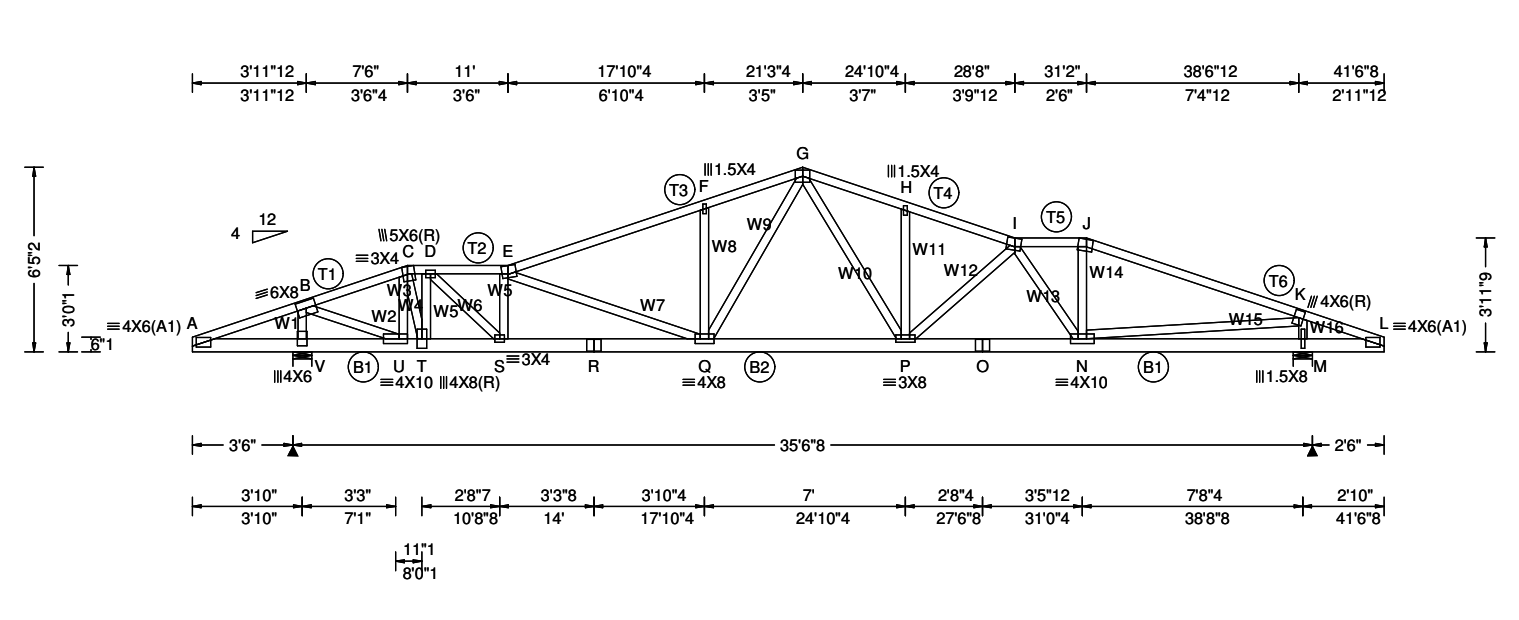
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)						
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity			
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.165 D 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.306 D 999 240			O	1671	-	-	/1028	/527	/210
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.033 B - -			I	2003	-	-	/1013	/826	-
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.061 B - -			Wind reactions based on C&C						
NCBCLL: 10.00		Mean Height: 27.72 ft					Creep Factor: 2.0			O Brg Wid = - Min Req = -						
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.615			I Brg Wid = 8.0 Min Req = 2.4 (Truss)						
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.681			Bearing I is a rigid surface.						
Spacing: 24.0 "		MWFRS Parallel Dist: h to 2h					Max Web CSI: 0.685			Maximum Top Chord Forces Per Ply (lbs)						
		C&C Dist a: 3.33 ft					Mfg Specified Camber:			Chords	Tens.Comp.	Chords	Tens.	Comp.		
		Loc. from endwall: not in 9.00 ft					VIEW Ver: 23.02.01A.1204.18			A - B	860	-2600	E - F	1166	-2544	
		GCpi: 0.18								B - C	1000	-2597	F - G	1166	-2727	
		Wind Duration: 1.60														

Lumber
Top chord: 2x4 SP #2; T1 2x4 SP SS Dense;
Bot chord: 2x6 SP #2;
Webs: 2x4 SP #2;

Wind
Member design based on both MWFRS and C&C.
Left end vertical not exposed to wind pressure.
Right cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

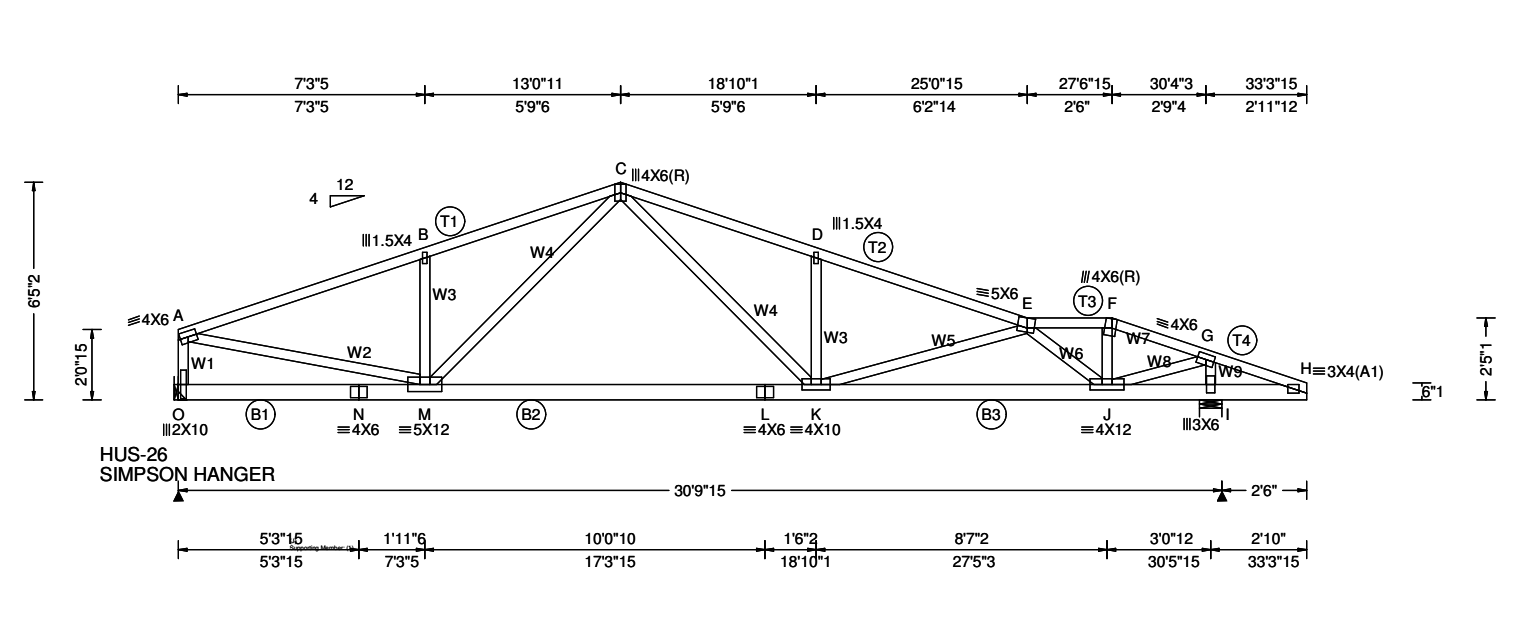


Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)		Defl/CSI Criteria		▲ Maximum Reactions (lbs)						
TCLL:	30.00	Wind Std:	ASCE 7-22	Pg: NA	Ct: NA	CAT: NA	PP Deflection in loc L/defl L/#	Gravity		Non-Gravity				
TCDL:	15.00	Speed:	175 mph	Pf: NA		Ce: NA	VERT(LL): 0.199 F 999 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL): 0.364 F 999 240	V	5252	-/-	-/-	-/-	/2304	-/-
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA		HORZ(LL): 0.054 C - -	M	2629	-/-	-/-	-/-	/1313	-/-
Des Ld:	55.00	EXP: C	Kzt: NA				HORZ(TL): 0.100 C - -	Wind reactions based on MWFRS						
NCBCLL:	0.00	Mean Height:	27.72 ft	Building Code:			Creep Factor: 2.0	V		Brg Wid = 8.0		Min Req = 3.1 (Truss)		
Soffit:	2.00	TCDL:	6.0 psf	FBC 8th Ed. 2023 Res. HVHZ			Max TC CSI: 0.649	M		Brg Wid = 8.0		Min Req = 1.6 (Truss)		
Load Duration:	1.25	BCDL:	6.0 psf	TPI Std: 2014			Max BC CSI: 0.877	Bearings V & M are a rigid surface.						
Spacing:	24.0 "	MWFRS Parallel Dist:	0 to h/2	Rep Fac: Varies by Ld Case			Max Web CSI: 0.711	Maximum Top Chord Forces Per Ply (lbs)						
		C&C Dist a:	4.15 ft	FT/RT:20(0)/10(0)			Mfg Specified Camber:	Chords	Tens.Comp.	Chords	Tens.	Comp.		
		Loc. from endwall:	not in 13.00 ft	Plate Type(s):				A - B	254	-105	G - H	1129	-2336	
		GCpi:	0.18	WAVE			VIEW Ver: 23.02.01A.1204.18	B - C	1324	-3120	H - I	1125	-2333	
		Wind Duration:	1.60					C - D	1449	-3459	I - J	968	-1987	
								D - E	1775	-4058	J - K	1057	-2134	
								E - F	1256	-2662	K - L	83	-182	
								F - G	1246	-2649				
								Maximum Bot Chord Forces Per Ply (lbs)						
								Chords	Tens.Comp.	Chords	Tens.	Comp.		
								A - V	77	-212	Q - P	1923	-917	
								V - U	44	-172	P - O	2394	-1166	
								U - T	3066	-1291	O - N	2394	-1166	
								T - S	3515	-1481	N - M	270	-117	
								S - R	4110	-1812	M - L	186	-57	
								R - Q	4110	-1812				
								Maximum Web Forces Per Ply (lbs)						
								Webs	Tens.Comp.	Webs	Tens.	Comp.		
								V - B	1132	-2502	Q - G	1132	-488	
								B - U	3148	-1344	G - P	549	-262	
								U - C	522	-1243	P - H	142	-188	
								C - T	2079	-836	P - I	157	-310	
								T - D	291	-678	I - N	361	-741	
								D - S	954	-435	N - J	399	-125	
								S - E	333	-666	N - K	1760	-854	
								E - Q	699	-1743	K - M	663	-1194	
								F - Q	204	-272				
Lumber														
Top chord: 2x4 SP #2;														
Bot chord: 2x6 SP #2;														
Webs: 2x4 SP #2;														
Nailnote														
Nail Schedule:0.128"x3", min. nails														
Top Chord: 1 Row @12.00" o.c.														
Bot Chord: 1 Row @12.00" o.c.														
Webs : 1 Row @ 4" o.c.														
Use equal spacing between rows and stagger nails														
in each row to avoid splitting.														
Special Loads														
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)														
TC: From 90 plf at 0.00 to 90 plf at 41.54														
BC: From 20 plf at 0.00 to 20 plf at 7.46														
BC: From 10 plf at 7.46 to 10 plf at 8.15														
BC: From 20 plf at 8.15 to 20 plf at 41.54														
BC: -5 lb Conc. Load at 7.46														
BC: 3323 lb Conc. Load at 8.15														
Plating Notes														
All plates are 5X6 except as noted.														
Wind														
Wind loads and reactions based on MWFRS.														
Left and right cantilevers are exposed to wind														
Wind loading based on both gable and hip roof types.														

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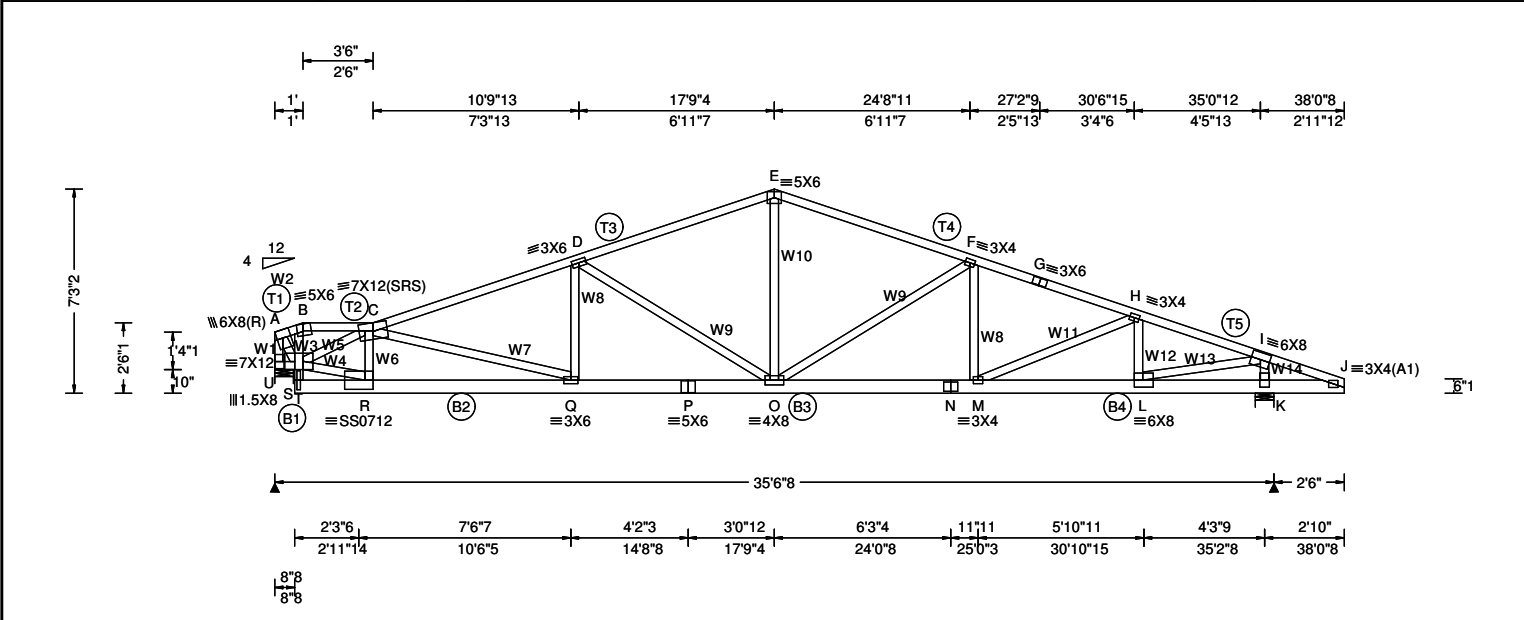
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)					
TCLL:	30.00	Wind Std:	ASCE 7-22	Pg: NA	Ct: NA	CAT: NA	PP Deflection in	loc L/defl	L/#	Gravity			Non-Gravity		
TCDL:	15.00	Speed:	175 mph	Pf: NA		Ce: NA	VERT(LL):	0.168	D 999 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U / RL
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL):	0.313	D 999 240	O	1671	-	-	/1048	/514 /210
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA		HORZ(LL):	0.034	B - -	I	2003	-	-	/994	/839 -
		EXP:	C Kzt: NA				HORZ(TL):	0.063	B - -	Wind reactions based on C&C					
Des Ld:	55.00	Mean Height:	27.72 ft				Creep Factor:	2.0		O	Brg Wid = -	Min Req = -			
NCBCLL:	10.00	TCDL:	6.0 psf				Max TC CSI:	0.727		I	Brg Wid = 8.0	Min Req = 2.4 (Truss)			
Soffit:	2.00	BCDL:	6.0 psf				Max BC CSI:	0.718		Bearing I is a rigid surface.					
Load Duration:	1.25	MWFRS Parallel Dist:	h to 2h				Max Web CSI:	0.665		Maximum Top Chord Forces Per Ply (lbs)					
Spacing:	24.0 "	C&C Dist a:	3.33 ft				Mfg Specified Camber:			Chords	Tens.Comp.	Chords	Tens.	Comp.	
		Loc. from endwall:	not in 9.00 ft							A - B	837 -2600	E - F	919	-2150	
		GCpi:	0.18							B - C	978 -2598	F - G	929	-2273	
		Wind Duration:	1.60												

Lumber
Top chord: 2x4 SP #2; T1 2x4 SP SS Dense;
Bot chord: 2x6 SP #2;
Webs: 2x4 SP #2;

Wind
Member design based on both MWFRS and C&C.
Left end vertical not exposed to wind pressure.
Right cantilever is exposed to wind
Wind loading based on both gable and hip roof types.



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): 0.203 O 999 360	U	6180	-	-	-	-	/3032 -
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.373 O 999 240	K	5234	-	-	-	-	/2339 -
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.041 K - -	Wind reactions based on MWFRS						
Des Ld: 55.00	EXP: C Kzt: NA	Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE, 18SS	HORZ(TL): 0.077 K - -	U Brg Wid = 8.0 Min Req = 3.6 (Truss)						
NCBCLL: 0.00	Mean Height: 27.30 ft		Creep Factor: 2.0	K Brg Wid = 8.0 Min Req = 3.1 (Truss)						
Soffit: 2.00	TCDL: 6.0 psf		Max TC CSI: 0.971	Bearings U & K are a rigid surface.						
Load Duration: 1.25	BCDL: 6.0 psf		Max BC CSI: 0.750	Maximum Top Chord Forces Per Ply (lbs)						
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2		Max Web CSI: 0.803	Chords Tens.Comp. Chords Tens. Comp.						
	C&C Dist a: 3.80 ft	Mfg Specified Camber:	A - B 1207 -2461 F - G 1544 -3352							
	Loc. from endwall: not in 9.00 ft	VIEW Ver: 23.02.01A.1204.18	B - C 1087 -2223 G - H 1582 -3400							
	GCpi: 0.18									
	Wind Duration: 1.60									

Lumber Top chord: 2x4 SP #2; Bot chord: 2x6 SP #2; B1 2x4 SP #2; B2 2x6 SP M-26; Webs: 2x4 SP #2; W4 2x4 SP #1;	Nailnote Nail Schedule: 0.128"x3", min. nails Top Chord: 1 Row @ 12.00" o.c. Bot Chord: 1 Row @ 7.50" o.c. Webs : 1 Row @ 4" o.c. Use equal spacing between rows and stagger nails in each row to avoid splitting.	Special Loads ----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC: From 90 plf at 0.00 to 90 plf at 38.04 BC: From 20 plf at 0.00 to 20 plf at 38.04 BC: 1155 lb Conc. Load at 1.98 BC: 1146 lb Conc. Load at 3.98, 5.98, 7.98 BC: 2347 lb Conc. Load at 30.89 BC: 284 lb Conc. Load at 32.95	Plating Notes All plates are 4X6 except as noted.	Wind Wind loads and reactions based on MWFRS. Left end vertical not exposed to wind pressure. Right cantilever is exposed to wind Wind loading based on both gable and hip roof types.
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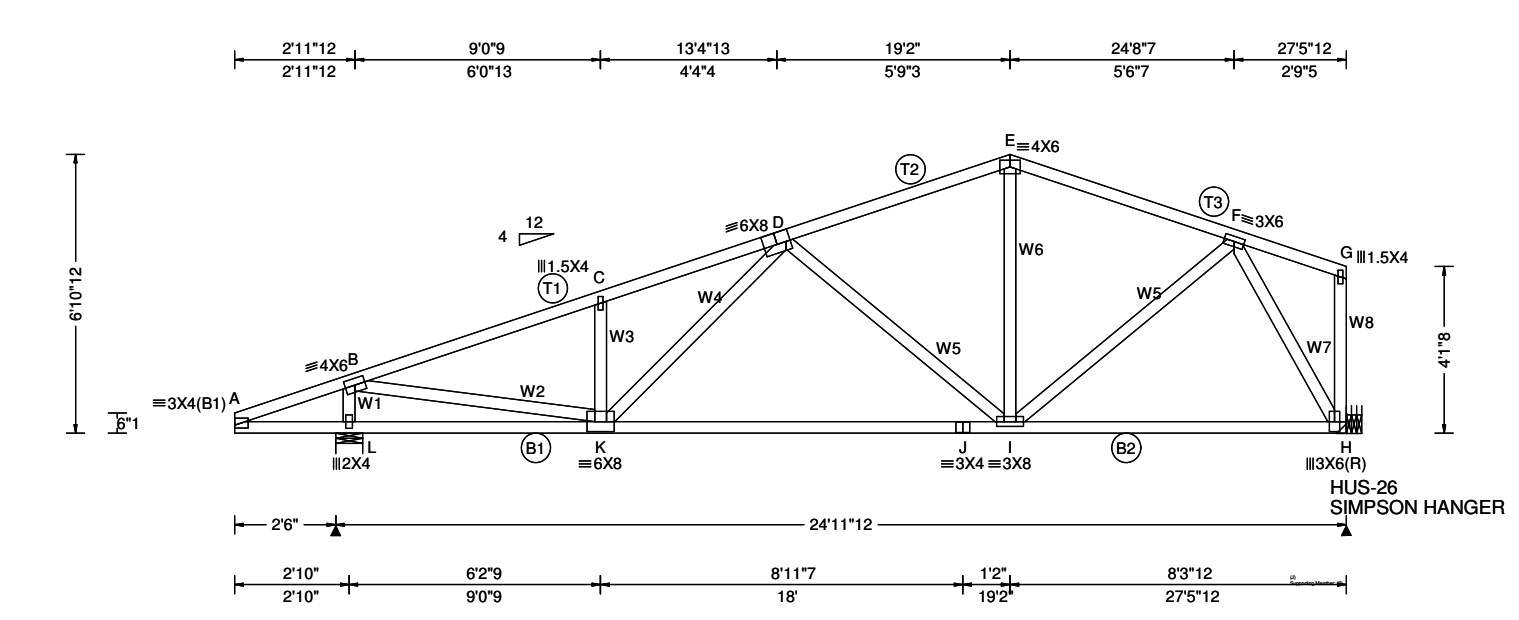
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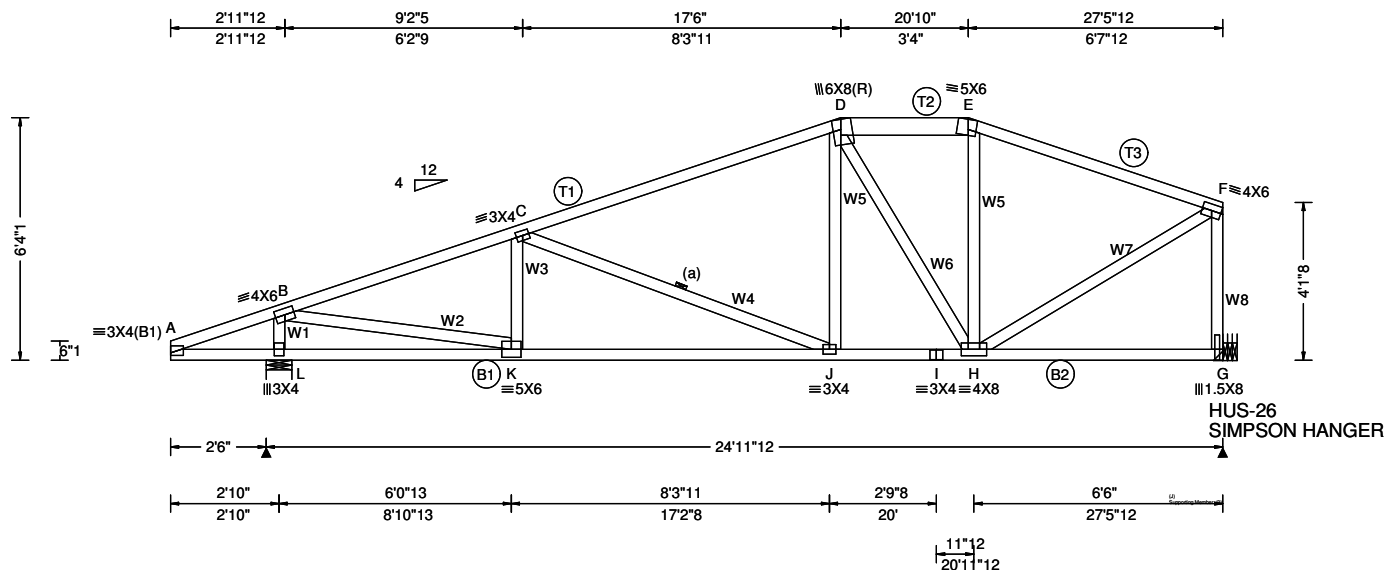
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria		▲ Maximum Reactions (lbs)							
		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#		Gravity			Non-Gravity				
TCLL: 30.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.074 D 999 360		Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
TCDL: 15.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.137 D 999 240		L	1685	-	-	-	/1053	/645	/269
BCLL: 0.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.021 H - -		H	1347	-	-	-	/888	/477	-
BCDL: 10.00		EXP: C Kzt: NA					HORZ(TL): 0.040 H - -		Wind reactions based on C&C							
Des Ld: 55.00		Mean Height: 27.12 ft		Building Code:			Creep Factor: 2.0		L Brg Wid = 8.0 Min Req = 2.0 (Truss)							
NCBCLL: 10.00		TCDL: 6.0 psf		FBC 8th Ed. 2023 Res. HVHZ			Max TC CSI: 0.630		H Brg Wid = - Min Req = -							
Soffit: 2.00		BCDL: 6.0 psf		TPI Std: 2014			Max BC CSI: 0.949		Bearing L is a rigid surface.							
Load Duration: 1.25		MWFRS Parallel Dist: h/2 to h		Rep Fac: Yes			Max Web CSI: 0.717		Maximum Top Chord Forces Per Ply (lbs)							
Spacing: 24.0 "		C&C Dist a: 3.00 ft		FT/RT:20(0)/10(0)			Mfg Specified Camber:		Chords	Tens.Comp.		Chords	Tens. Comp.			
		Loc. from endwall: not in 9.00 ft		Plate Type(s):			VIEW Ver: 23.02.01A.1204.18		A - B	319	- 225	D - E	553	- 1323		
		GCpi: 0.18		WAVE					B - C	710	- 2208	E - F	531	- 1323		
		Wind Duration: 1.60							C - D	820	- 2180	F - G	52	- 26		
Lumber																

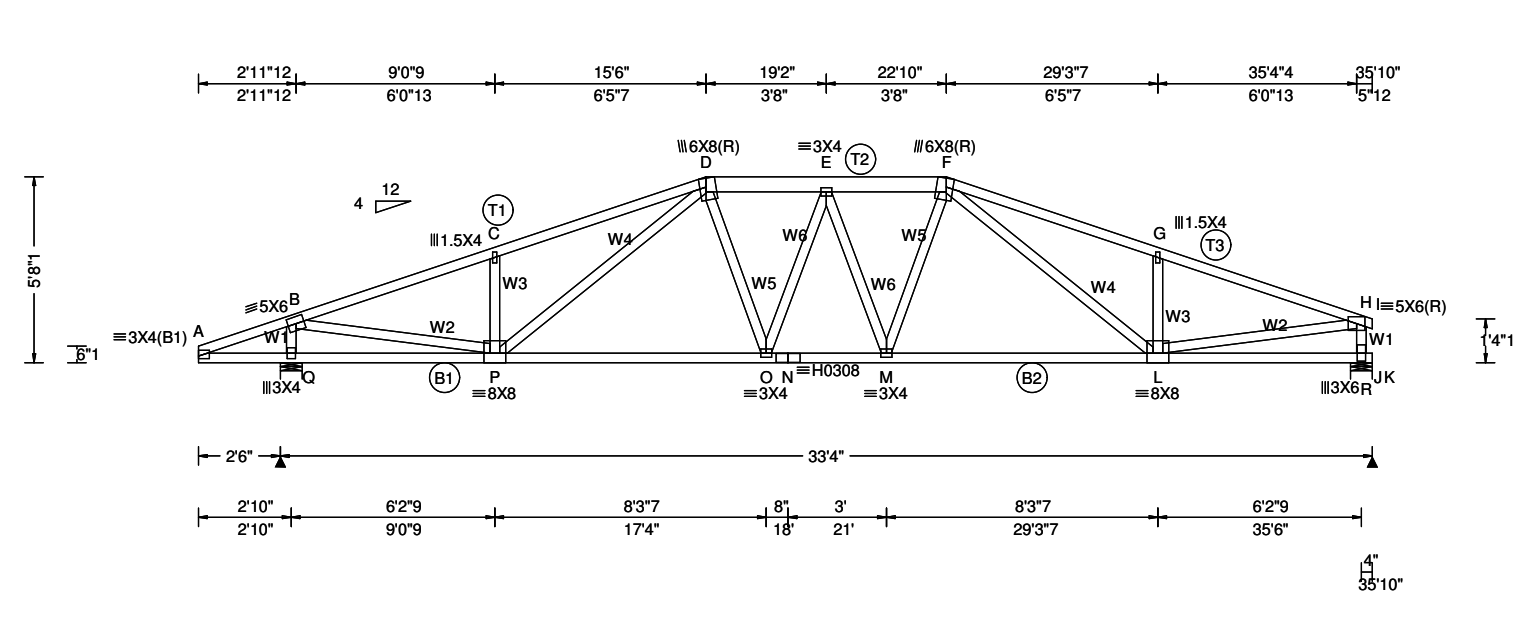
Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2;	Hangers / Ties (J) Hanger Support Required, by others	Wind Member design based on both MWFRS and C&C. Right end vertical not exposed to wind pressure. Left cantilever is exposed to wind Wind loading based on both gable and hip roof types.	Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - L 215 -250 J - I 1725 -664 L - K 276 -315 I - H 711 -285 K - J 1725 -664	Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. L - B 628 -1581 E - I 341 -39 B - K 1975 -609 I - F 618 -134 C - K 302 -479 F - H 577 -1409 K - D 404 -138 G - H 32 -62 D - I 381 -712
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): 0.078 C 999 360	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.144 C 999 240	L 1685 -/- /- /897 /734 /253
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.020 G - -	G 1347 -/- /- /636 /676 -/-
Des Ld: 55.00	EXP: C Kzt: NA		HORZ(TL): 0.038 G - -	Wind reactions based on C&C
NCBCLL: 10.00	Mean Height: 26.84 ft	Building Code:	Creep Factor: 2.0	L Brg Wid = 8.0 Min Req = 2.0 (Truss)
Soffit: 2.00	TCDL: 6.0 psf	FBC 8th Ed. 2023 Res. HVHZ	Max TC CSI: 0.980	G Brg Wid = - Min Req = -
Load Duration: 1.25	BCDL: 6.0 psf	TPI Std: 2014	Max BC CSI: 0.711	Bearing L is a rigid surface.
Spacing: 24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.651	Maximum Top Chord Forces Per Ply (lbs)
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	Mfg Specified Camber:	Chords Tens.Comp. Chords Tens. Comp.
	Loc. from endwall: not in 9.00 ft	Plate Type(s):		A - B 329 -233 D - E 774 -1140
	GCpi: 0.18	WAVE	VIEW Ver: 23.02.01A.1204.18	B - C 860 -2233 E - F 718 -1278
	Wind Duration: 1.60			C - D 842 -1626
Lumber				Maximum Bot Chord Forces Per Ply (lbs)
Top chord: 2x4 SP #2; T2 2x6 SP #2;				Chords Tens.Comp. Chords Tens. Comp.
Bot chord: 2x4 SP #2; B2 2x4 SP #1;				A - L 224 -263 J - I 1414 -705
Webs: 2x4 SP #2;				L - K 238 -277 I - H 1414 -705
Bracing				K - J 2066 -923 H - G 22 -11
(a) Continuous lateral restraint equally spaced on member.				Maximum Web Forces Per Ply (lbs)
Hangers / Ties				Webs Tens.Comp. Webs Tens. Comp.
(J) Hanger Support Required, by others				L - B 715 -1567 D - H 244 -546
Wind				B - K 2062 -833 E - H 262 -90
Member design based on both MWFRS and C&C.				K - C 268 -259 H - F 1294 -694
Right end vertical not exposed to wind pressure.				C - J 343 -683 F - G 709 -1287
Left cantilever is exposed to wind				J - D 403 -56
Wind loading based on both gable and hip roof types.				

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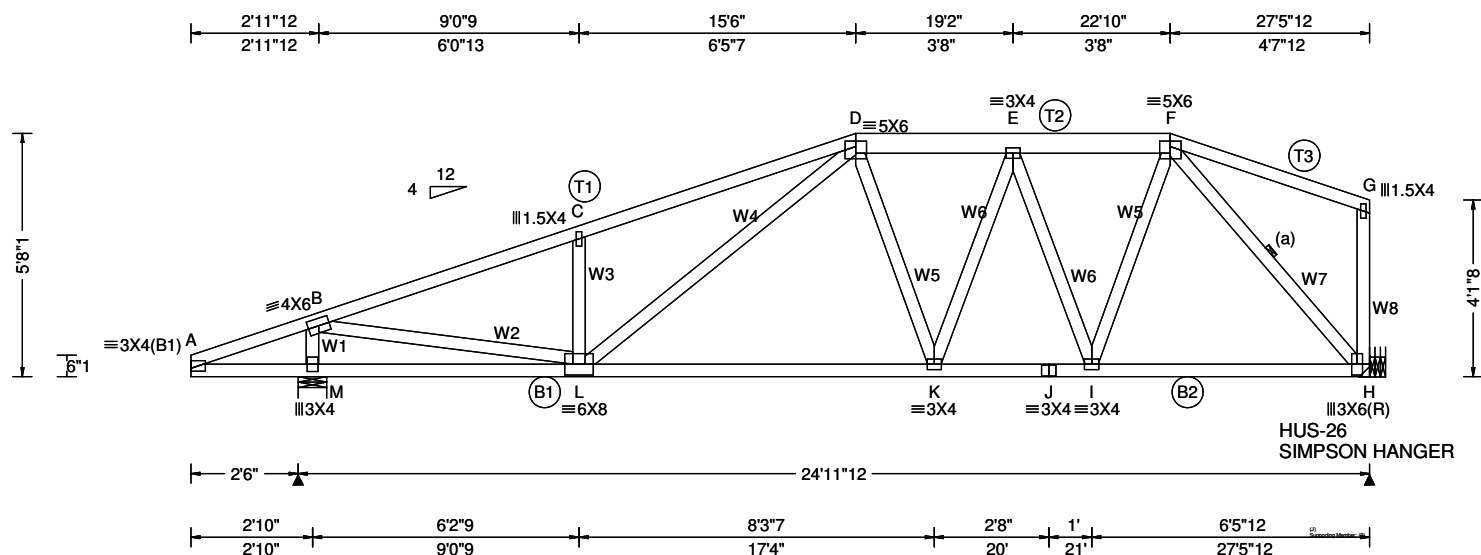


Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity			Non-Gravity			
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): 0.180 E 999 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.332 E 999 240	Q	2130	/-	/-	/1085	/960	/174
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.049 K - -	R	1819	/-	/-	/859	/800	/-
	EXP: C Kzt: NA		HORZ(TL): 0.092 K - -	Wind reactions based on C&C						
Des Ld: 55.00	Mean Height: 26.51 ft		Creep Factor: 2.0	Q	Brg Wid = 8.0	Min Req = 2.5	(Truss)			
NCBCLL: 10.00	TCDL: 6.0 psf	Building Code:	Max TC CSI: 0.730	R	Brg Wid = 8.0	Min Req = 2.1	(Truss)			
Soffit: 2.00	BCDL: 6.0 psf	FBC 8th Ed. 2023 Res. HVHZ	Max BC CSI: 0.808	Bearings Q & K are a rigid surface.						
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max Web CSI: 0.953	Maximum Top Chord Forces Per Ply (lbs)						
Spacing: 24.0 "	C&C Dist a: 3.58 ft	Rep Fac: Yes	Mfg Specified Camber:	Chords	Tens.Comp.	Chords	Tens.	Comp.		
	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)		A - B	309	-210	E - F	1690	-2745	
	GCpi: 0.18	Plate Type(s):		B - C	1529	-3127	F - G	1677	-3243	
	Wind Duration: 1.60	WAVE, HS	VIEW Ver: 23.02.01A.1204.18							

Lumber
Top chord: 2x4 SP #2; T2 2x6 SP #2;
T3 2x4 SP #1;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Wind
Member design based on both MWFRS and C&C.
Right end vertical not exposed to wind pressure.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

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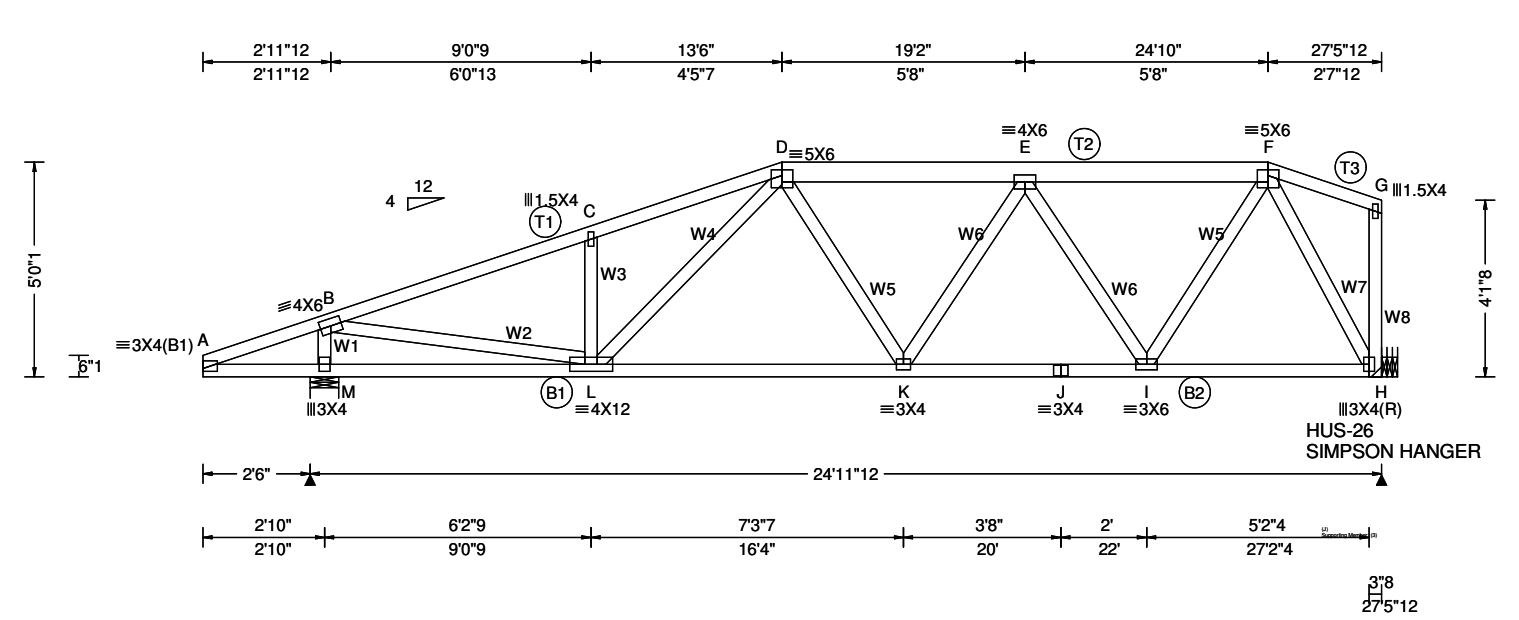


Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)		Defl/CSI Criteria		▲ Maximum Reactions (lbs)							
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA		PP Deflection in loc L/defl L/#		Gravity				Non-Gravity			
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA		VERT(LL): 0.077 C 999 360		Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA		VERT(CL): 0.143 C 999 240		M	1685	/-	/-	/901	/841	/233	
BCDL: 10.00		Risk Category: II		Snow Duration: NA		HORZ(LL): 0.022 H - -		H	1347	/-	/-	/636	/914	/-	
Des Ld: 55.00		EXP: C Kzt: NA				HORZ(TL): 0.042 H - -		Wind reactions based on C&C							
NCBCLL: 10.00		Mean Height: 26.51 ft		Building Code:		Creep Factor: 2.0		M Brg Wid = 8.0 Min Req = 2.0 (Truss)							
Soffit: 2.00		TCDL: 6.0 psf		FBC 8th Ed. 2023 Res. HVHZ		Max TC CSI: 0.637		H Brg Wid = - Min Req = -							
Load Duration: 1.25		BCDL: 6.0 psf		TPI Std: 2014		Max BC CSI: 0.649		Bearing M is a rigid surface.							
Spacing: 24.0 "		MWFRS Parallel Dist: h/2 to h		Rep Fac: Yes		Max Web CSI: 0.686		Maximum Top Chord Forces Per Ply (lbs)							
		C&C Dist a: 3.00 ft		FT/RT:20(0)/10(0)		Mfg Specified Camber:		Chords		Tens.Comp.		Chords		Tens. Comp.	
		Loc. from endwall: not in 9.00 ft		Plate Type(s):		VIEW Ver: 23.02.01A.1204.18		A - B		318 -223		D - E		1140 -1525	
		GCpi: 0.18		WAVE				B - C		1089 -2223		E - F		972 -1197	
		Wind Duration: 1.60						C - D		1215 -2223		F - G		76 -83	

Webs	Tens.Comp.	Webs	Tens.	Comp.
M - B	825 - 1567	K - E	392	- 123
B - L	2014 - 1006	E - I	575	- 673
C - L	367 - 576	I - F	774	- 507
L - D	649 - 258	F - H	1046	- 1413
D - K	200 - 260	G - H	158	- 222

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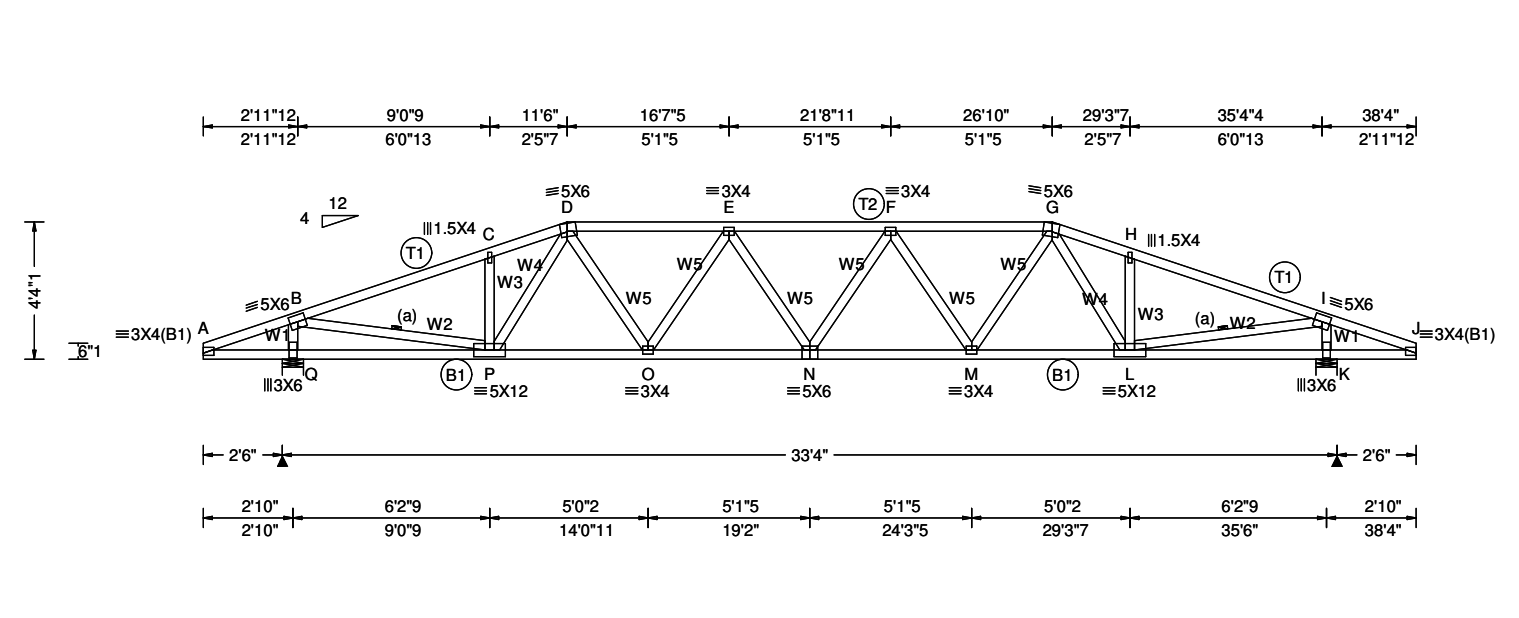
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)						
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity			
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.073 D 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.134 D 999 240			M	1685	-	-	/913	/946	/241
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): -0.023 H - -			H	1342	-	-	/630	/1144	-
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.044 H - -			Wind reactions based on C&C						
NCBCLL: 10.00		Mean Height: 26.17 ft					Creep Factor: 2.0			M Brg Wid = 8.0 Min Req = 2.0 (Truss)						
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.610			H Brg Wid = - Min Req = -						
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.549			Bearing M is a rigid surface.						
Spacing: 24.0 "		MWFRS Parallel Dist: h/2 to h					Max Web CSI: 0.801			Maximum Top Chord Forces Per Ply (lbs)						
		C&C Dist a: 3.00 ft					Mfg Specified Camber:			Chords	Tens.Comp.		Chords	Tens. Comp.		
		Loc. from endwall: not in 9.00 ft								A - B	305	-218	D - E	1550	-1744	
		GCpi: 0.18								B - C	1428	-2194	E - F	1111	-1199	
		Wind Duration: 1.60														

Lumber Top chord: 2x4 SP #2; T2 2x6 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2;	Hangers / Ties (J) Hanger Support Required, by others	Wind Member design based on both MWFRS and C&C. Right end vertical not exposed to wind pressure. Left cantilever is exposed to wind Wind loading based on both gable and hip roof types.
Maximum Bot Chord Forces Per Ply (lbs)		
Chords	Tens.Comp.	Chords Tens. Comp.
A - M	209 -238	K - J 1656 -1553
M - L	212 -333	J - I 1656 -1553
L - K	1745 -1487	I - H 671 -611
Maximum Web Forces Per Ply (lbs)		
Webs	Tens.Comp.	Webs Tens. Comp.
M - B	936 -1564	K - E 406 -50
B - L	1953 -1176	E - I 950 -881
C - L	308 -478	I - F 1012 -862
L - D	603 -189	F - H 1236 -1351
D - K	156 -263	G - H 113 -124

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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria		▲ Maximum Reactions (lbs)						
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#		Gravity			Non-Gravity			
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.237 N 999 360		Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.437 N 897 240		Q	2116	/-	/-	/1119	/1192	/119
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.057 K - -		K	2116	/-	/-	/1119	/1192	/-
Des Ld: 55.00		EXP: C Kzt: NA					HORZ(TL): 0.107 K - -		Wind reactions based on C&C						
NCBCLL: 10.00		Mean Height: 25.84 ft		Building Code:			Creep Factor: 2.0		Q Brg Wid = 8.0 Min Req = 2.5 (Truss)						
Soffit: 2.00		TCDL: 6.0 psf		FBC 8th Ed. 2023 Res. HVHZ			Max TC CSI: 0.805		K Brg Wid = 8.0 Min Req = 2.5 (Truss)						
Load Duration: 1.25		BCDL: 6.0 psf		TPI Std: 2014			Max BC CSI: 0.806		Bearings Q & K are a rigid surface.						
Spacing: 24.0 "		MWFRS Parallel Dist: h to 2h		Rep Fac: Yes			Max Web CSI: 0.912		Maximum Top Chord Forces Per Ply (lbs)						
		C&C Dist a: 3.83 ft		FT/RT:20(0)/10(0)			Mfg Specified Camber:		Chords	Tens.Comp.	Chords	Tens.	Comp.		
		Loc. from endwall: not in 9.00 ft		Plate Type(s):			VIEW Ver: 23.02.01A.1204.18		A - B	285	-201	F - G	2445	-3175	
		GCpi: 0.18		WAVE					B - C	2059	-3036	G - H	2152	-2987	
		Wind Duration: 1.60													

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

Member design based on both MWFRS and C&C.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

C - D	2152	- 2967	H - I	2059	- 5036
D - E	2444	- 3175	I - J	285	- 201
E - F	2716	- 3529			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - Q	191	- 222	N - M	3503	- 2517
Q - P	264	- 199	M - L	2779	- 1913
P - O	2779	- 1961	L - K	264	- 197
O - N	3503	- 2565	K - J	191	- 219

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
Q - B	1190	- 1981	N - F	121	- 9
B - P	2700	- 1716	F - M	695	- 632
C - P	275	- 408	M - G	759	- 636
P - D	509	- 150	G - L	509	- 149
D - O	759	- 636	L - H	274	- 408
O - E	695	- 632	L - I	2700	- 1715
E - N	121	- 9	I - K	1190	- 1981

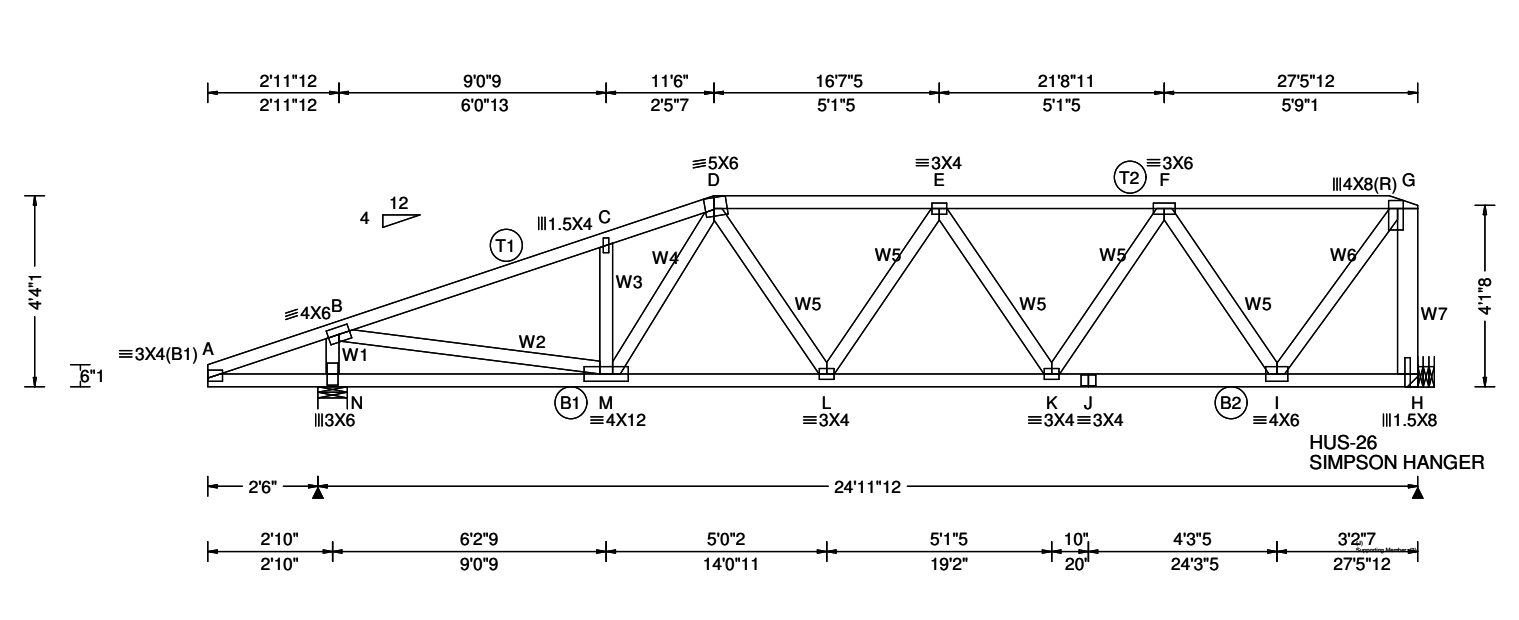
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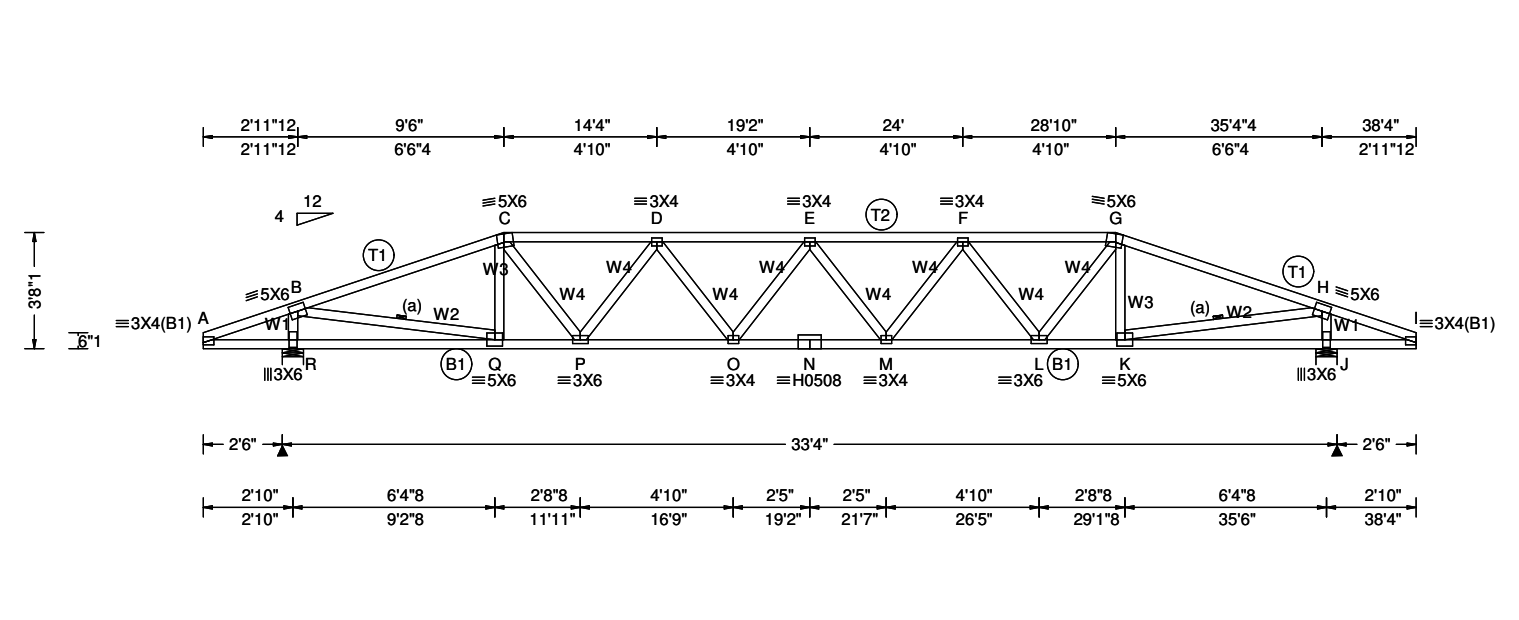
Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)							
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity				
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.088 L 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.162 L 999 240			N	1685	/ -	/ -	/ 919	/ 1050	/ 346	
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): -0.028 H - -			H	1347	/ -	/ -	/ 615	/ 1312	/ -	
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.048 H - -			Wind reactions based on C&C							
NCBCLL: 10.00		Mean Height: 25.84 ft					Creep Factor: 2.0			N Brg Wid = 8.0 Min Req = 2.0 (Truss)							
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.870			H Brg Wid = - Min Req = -							
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.497			Bearing N is a rigid surface.							
Spacing: 24.0 "		MWFRS Parallel Dist: 0 to h/2					Max Web CSI: 0.932			Maximum Top Chord Forces Per Ply (lbs)							
		C&C Dist a: 3.00 ft					Mfg Specified Camber:			Chords		Tens.Comp.		Chords		Tens. Comp.	
		Loc. from endwall: not in 9.00 ft								A - B		294 -217		D - E		1895 -1993	
		GCpi: 0.18								B - C		1671 -2177		E - F		1805 -1821	
		Wind Duration: 1.60															

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2; W7 2x6 SP #2;

Hangers / Ties
(J) Hanger Support Required, by others

Wind
Member design based on both MWFRS and C&C.
Right end vertical not exposed to wind pressure.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

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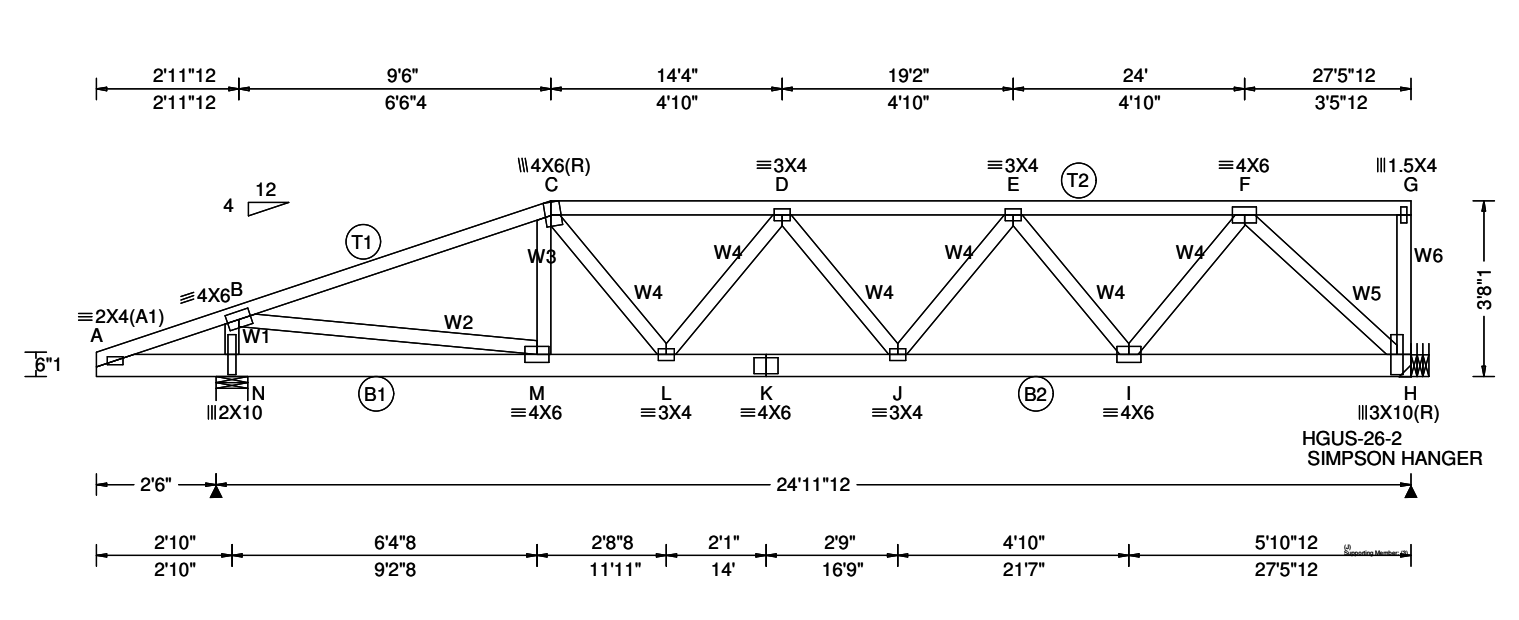
Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria		▲ Maximum Reactions (lbs)								
TCLL:	30.00	Wind Std:	ASCE 7-22	Pg: NA	Ct: NA	CAT: NA	PP Deflection in	loc L/defl L/#	Gravity			Non-Gravity					
TCDL:	15.00	Speed:	175 mph	Pf: NA		Ce: NA	VERT(LL):	0.305 E	999	360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL):	0.562 E	697	240	R	2116	/-	/-	/1126	/1317	/98
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA		HORZ(LL):	0.066 J	-	-	J	2116	/-	/-	/1126	/1317	/-
		EXP:	C Kzt: NA				HORZ(TL):	0.125 J	-	-	Wind reactions based on C&C						
Des Ld:	55.00	Mean Height:	25.51 ft				Creep Factor:	2.0			R	Brg Wid = 8.0	Min Req = 2.5	(Truss)			
NCBCLL:	10.00	TCDL:	6.0 psf				Max TC CSI:	0.853			J	Brg Wid = 8.0	Min Req = 2.5	(Truss)			
Soffit:	2.00	BCDL:	6.0 psf				Max BC CSI:	0.969			Bearings R & J are a rigid surface.						
Load Duration:	1.25	MWFRS Parallel Dist:	h to 2h				Max Web CSI:	0.818			Maximum Top Chord Forces Per Ply (lbs)						
Spacing:	24.0 "	C&C Dist a:	3.83 ft				Mfg Specified Camber:				Chords	Tens.Comp.	Chords	Tens. Comp.			
		Loc. from endwall:	not in 9.00 ft								A - B	274	-201	E - F	3403	-4130	
		GCpi:	0.18								B - C	2322	-3031	F - G	2790	-3387	
		Wind Duration:	1.60														

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Wind
Member design based on both MWFRS and C&C.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)							
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity				
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): -0.132 D 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.187 D 999 240			N	2714	-	-	-	-	/2038	-
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): -0.027 H - -			H	2716	-	-	-	-	/2028	-
		EXP: C Kzt: NA					HORZ(TL): 0.038 H - -			Wind reactions based on MWFRS							
Des Ld: 55.00		Mean Height: 25.51 ft					Creep Factor: 2.0			N Brg Wid = 8.0 Min Req = 1.6 (Truss)							
NCBCLL: 0.00		TCDL: 6.0 psf		Building Code:			Max TC CSI: 0.554			H Brg Wid = - Min Req = -							
Soffit: 2.00		BCDL: 6.0 psf		FBC 8th Ed. 2023 Res. HVHZ			Max BC CSI: 0.622			Bearing N is a rigid surface.							
Load Duration: 1.25		MWFRS Parallel Dist: 0 to h/2		TPI Std: 2014			Max Web CSI: 0.429			Maximum Top Chord Forces Per Ply (lbs)							
Spacing: 24.0 "		C&C Dist a: 3.00 ft		Rep Fac: Varies by Ld Case			Mfg Specified Camber:			Chords	Tens.Comp.		Chords	Tens. Comp.			
		Loc. from endwall: not in 9.00 ft		FT/RT:20(0)/10(0)						A - B	130	-79	D - E	1948	-2555		
		GCpi: 0.18		Plate Type(s):						B - C	1715	-2134	E - F	1464	-1950		
		Wind Duration: 1.60		WAVE			VIEW Ver: 23.02.01A.1204.18										

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x6 SP #2;
Webs: 2x4 SP #2;

Nailnote
Nail Schedule:0.128"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 90 plf at 0.00 to 90 plf at 9.50
TC: From 45 plf at 9.50 to 45 plf at 27.48
BC: From 20 plf at 0.00 to 20 plf at 9.46
BC: From 10 plf at 9.46 to 10 plf at 27.48
TC: 173 lb Conc. Load at 10.15,12.15,14.15,16.15
18.15,20.15,22.15,24.15,26.15
BC: 296 lb Conc. Load at 9.46
BC: 171 lb Conc. Load at 10.15,12.15,14.15,16.15
18.15,20.15,22.15,24.15,26.15

Hangers / Ties
(J) Hanger Support Required, by others

Wind
Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)
Chords Tens.Comp. Chords Tens. Comp.
A - N 84 -99 K - J 2513 -1945
N - M 182 -156 J - I 2294 -1741
M - L 1986 -1591 I - H 1196 -902
L - K 2513 -1945

Maximum Web Forces Per Ply (lbs)
Webs Tens.Comp. Webs Tens. Comp.
N - B 1001 -1241 J - E 441 -349
B - M 1821 -1452 E - I 468 -583
M - C 45 -116 I - F 1275 -951
C - L 708 -501 F - H 1236 -1639
L - D 98 -183 G - H 27 -32
D - J 71 -59

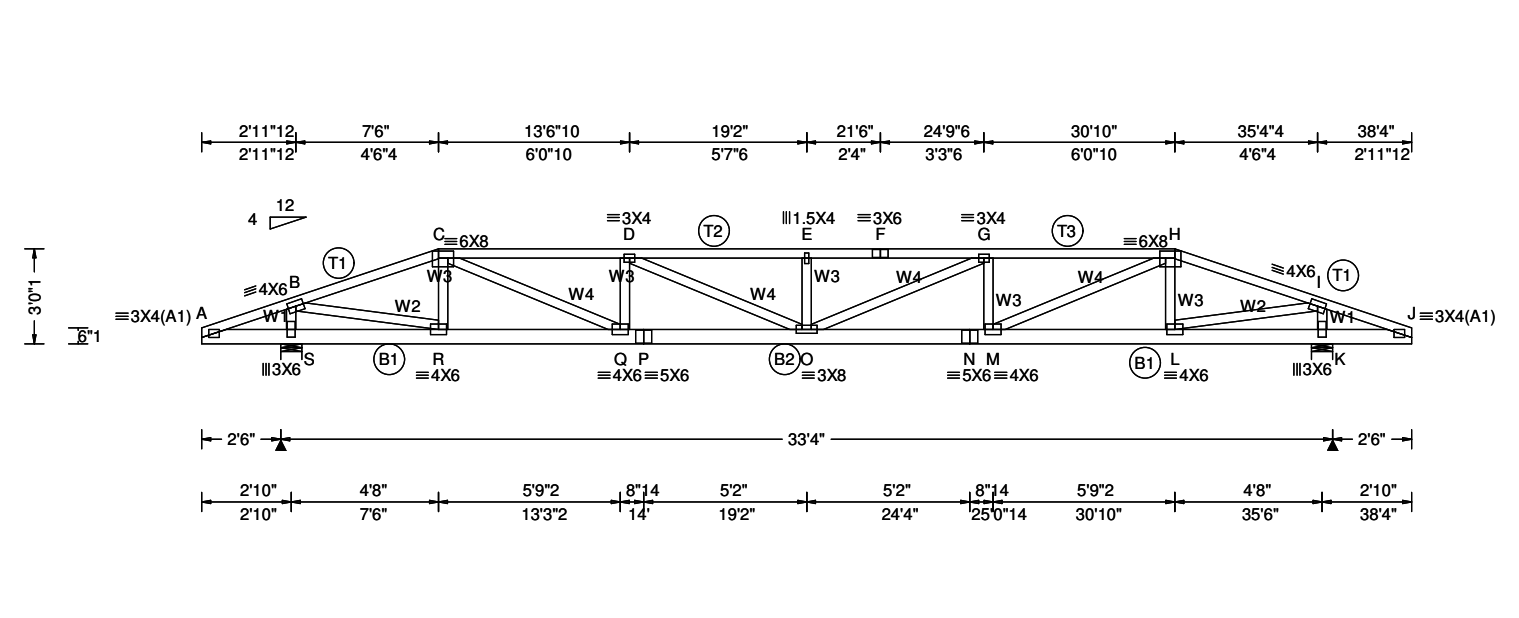
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)							
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity				
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.330 E 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.606 E 647 240			S	2887	-	-	-	-	/1674 -	
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.059 C - -			K	2887	-	-	-	-	/1674 -	
Des Ld: 55.00		EXP: C Kzt: NA					HORZ(TL): 0.108 C - -			Wind reactions based on MWFRS							
NCBCLL: 0.00		Mean Height: 25.17 ft					Creep Factor: 2.0			S Brg Wid = 8.0 Min Req = 1.7 (Truss)							
Soffit: 2.00		TCDL: 6.0 psf		Building Code:			Max TC CSI: 0.485			K Brg Wid = 8.0 Min Req = 1.7 (Truss)							
Load Duration: 1.25		BCDL: 6.0 psf		FBC 8th Ed. 2023 Res. HVHZ			Max BC CSI: 0.808			Bearings S & K are a rigid surface.							
Spacing: 24.0 "		MWFRS Parallel Dist: 0 to h/2		TPI Std: 2014			Max Web CSI: 0.447			Maximum Top Chord Forces Per Ply (lbs)							
		C&C Dist a: 3.83 ft		Rep Fac: Varies by Ld Case			Mfg Specified Camber:			Chords		Tens.Comp.		Chords		Tens. Comp.	
		Loc. from endwall: not in 9.00 ft		FT/RT:(20(0)/10(0)						A - B	159	-31	F - G	2336	-4097		
		GCpi: 0.18		Plate Type(s):						B - C	1298	-2161	G - H	2059	-3584		
		Wind Duration: 1.60		WAVE			VIEW Ver: 23.02.01A.1204.18										

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x6 SP #2;
Webs: 2x4 SP #2;

Nailnote

Nail Schedule:0.128"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

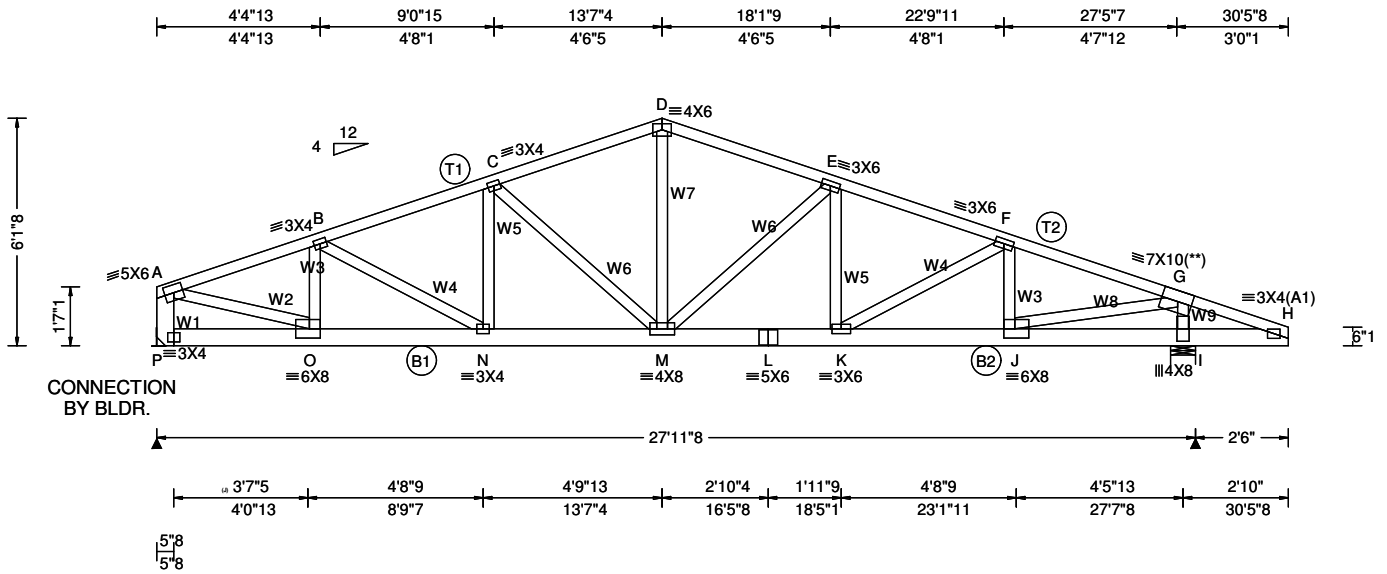
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From 90 plf at 0.00 to 90 plf at 7.50
TC: From 45 plf at 7.50 to 45 plf at 30.83
TC: From 90 plf at 30.83 to 90 plf at 38.33
BC: From 20 plf at 0.00 to 20 plf at 7.46
BC: From 10 plf at 7.46 to 10 plf at 30.88
BC: From 20 plf at 30.88 to 20 plf at 38.33
TC: 132 lb Conc. Load at 8.09,10.09,12.09,14.09
16.09,18.09,20.24,22.24,24.24,26.24,28.24,30.24
BC: 124 lb Conc. Load at 7.46,30.88
BC: 84 lb Conc. Load at 8.09,10.09,12.09,14.09
16.09,18.09,20.24,22.24,24.24,26.24,28.24,30.24

Wind

Wind loads and reactions based on MWFRS.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

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Loading Criteria (psf)
TCLL: 30.00
TCDL: 15.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 55.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 175 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 26.73 ft
TCDL: 6.0 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.05 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA
Building Code:
FBC 8th Ed. 2023 Res. HVHZ
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria
PP Deflection in loc L/defl L/#
VERT(LL): 0.167 K 999 360
VERT(CR): 0.306 K 999 240
HORZ(LL): 0.039 I - -
HORZ(TL): 0.073 I - -
Creep Factor: 2.0
Max TC CSI: 0.759
Max BC CSI: 0.878
Max Web CSI: 0.904
Mfg Specified Camber:

VIEW Ver: 23.02.01A.1204.18

▲ Maximum Reactions (lbs)					
Gravity				Non-Gravity	
Loc	R+	/ R-	/ Rh	/ Rw	/ U / RL
P	1819	/-	/-	/-	/1057 /-
I	3685	/-	/-	/-	/1858 /-
Wind reactions based on MWFRS					
P	Brg Wid = -		Min Req = -		
I	Brg Wid = 8.0		Min Req = 4.3 (Truss)		
Bearing I is a rigid surface.					
Maximum Top Chord Forces Per Ply (lbs)					
Chords		Tens.Comp.		Chords Tens. Comp.	
A - B		1591 -2765		E - F 2070 -3912	
B - C		1760 -3104		F - G 2594 -5373	
C - D		1549 -2796		G - H 110 -369	
D - E		1549 -2797			

Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.		Tens. Comp.
P - O	87	-59	3602 -1897
O - N	2632	-1509	4954 -2395
N - M	2885	-1622	621 -186
M - L	3602	-1897	348 -87

Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.		Tens. Comp.
A - P	1051	-1763	D - M 1325 -599
A - O	2587	-1464	E - K 856 -241
O - B	485	-679	K - F 565 -1519
B - N	291	-125	F - J 1105 -190
N - C	153	-52	J - G 4638 -2287
C - M	282	-405	G - I 1693 -3140
M - E	656	-1376	

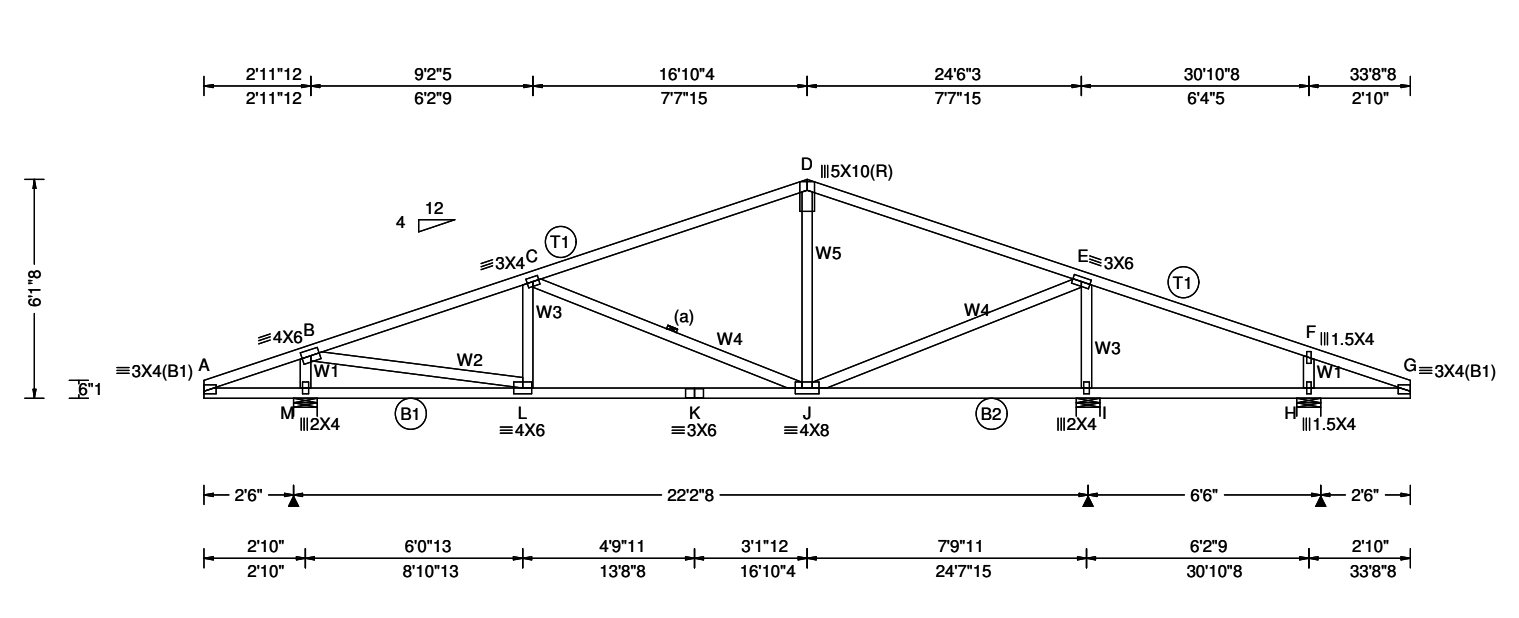
Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x6 SP #2;
Webs: 2x4 SP #2; W1 2x6 SP #2; W8 2x4 SP #1;

Special Loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 90 plf at 0.00 to 90 plf at 30.46
BC: From 20 plf at 0.46 to 20 plf at 23.31
BC: From 10 plf at 23.31 to 10 plf at 25.37
BC: From 20 plf at 25.37 to 20 plf at 30.46
BC: 1890 lb Conc. Load at 23.31
BC: 284 lb Conc. Load at 25.37

Plating Notes
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind
Wind loads and reactions based on MWFRS.
Left end vertical not exposed to wind pressure.
Right cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

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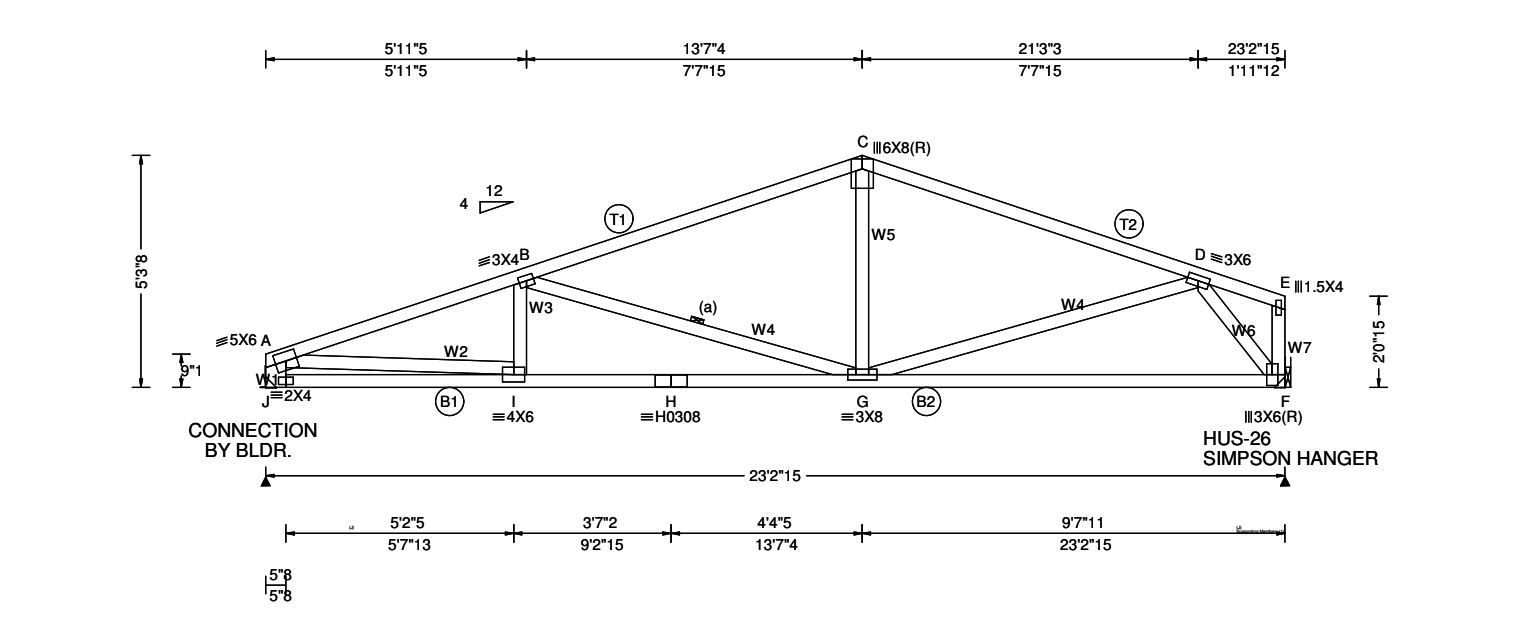
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
				Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	M	1506	/-	/-	/957	/545	/167
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): 0.059 C 999 360	I	1692	/-	/-	/1078	/555	/-
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.109 C 999 240	H	606	/-	/-	/342	/256	/-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.012 J - -	Wind reactions based on C&C						
	EXP: C Kzt: NA		HORZ(TL): 0.023 J - -	M	Brg Wid = 8.0		Min Req = 1.8 (Truss)			
Des Ld: 55.00	Mean Height: 26.73 ft	Building Code: FBC 8th Ed. 2023 Res. HVHZ	Creep Factor: 2.0	I	Brg Wid = 8.0		Min Req = 2.0 (Truss)			
NCBCLL: 10.00	TCDL: 6.0 psf		Max TC CSI: 0.880	H	Brg Wid = 8.0		Min Req = 1.5 (Truss)			
Soffit: 2.00	BCDL: 6.0 psf	TPI Std: 2014	Max BC CSI: 0.654	Bearings M, I, & H are a rigid surface.						
Load Duration: 1.25	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.485	Maximum Top Chord Forces Per Ply (lbs)						
Spacing: 24.0 "	C&C Dist a: 3.37 ft	FT/RT:20(0)/10(0)	Mfg Specified Camber:	Chords	Tens.Comp.		Chords	Tens. Comp.		
	Loc. from endwall: not in 9.00 ft	Plate Type(s):								
	GCpi: 0.18	WAVE	VIEW Ver: 23.02.01A.1204.18							
	Wind Duration: 1.60									

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Wind
Member design based on both MWFRS and C&C.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

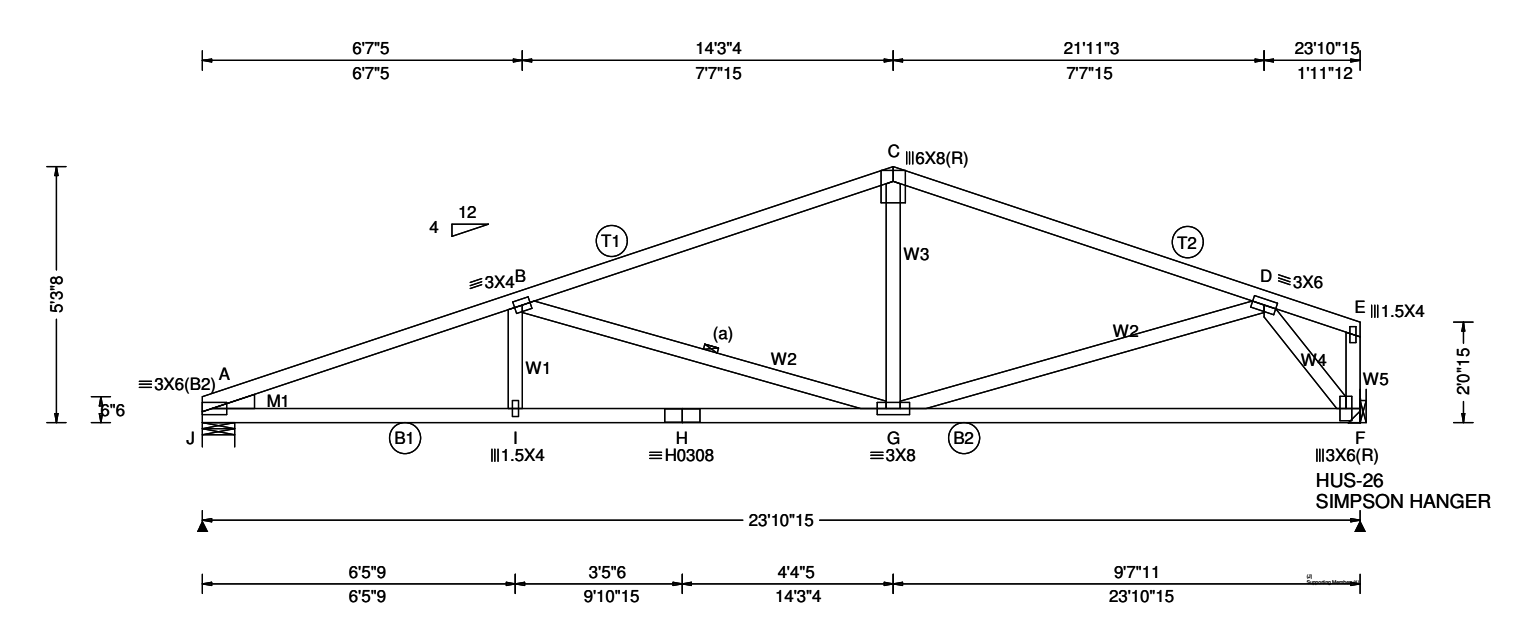
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria		▲ Maximum Reactions (lbs)							
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#		Gravity			Non-Gravity				
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.104 B 999 360		Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.190 B 999 240		J	1269	/-	/-	/821	/552	/161	
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.026 F - -		F	1278	/-	/-	/833	/568	/-	
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS			HORZ(TL): 0.048 F - -		Wind reactions based on C&C							
NCBCLL: 10.00		Mean Height: 27.27 ft					Creep Factor: 2.0		J Brg Wid = - Min Req = -							
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.878		F Brg Wid = - Min Req = -							
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.721		Maximum Top Chord Forces Per Ply (lbs)							
Spacing: 24.0 "		MWFRS Parallel Dist: h to 2h					Max Web CSI: 0.483		Chords	Tens.Comp.		Chords	Tens. Comp.			
		C&C Dist a: 3.00 ft					Mfg Specified Camber:		A - B	1163 -2681		C - D	829 -1757			
		Loc. from endwall: not in 9.00 ft							B - C	849 -1758		D - E	66 -44			
		GCpi: 0.18														
		Wind Duration: 1.60														

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; B2 2x4 SP #1; Webs: 2x4 SP #2; W1 2x6 SP #2;	Maximum Bot Chord Forces Per Ply (lbs)							
	Chords		Tens.Comp.		Chords		Tens. Comp.	
	J - I		341 -262		H - G		2481 -1116	
Bracing (a) Continuous lateral restraint equally spaced on member.	I - H		2481 -1116		G - F		1046 -556	
	Maximum Web Forces Per Ply (lbs)							
	Webs		Tens.Comp.		Webs		Tens. Comp.	
Hangers / Ties (J) Hanger Support Required, by others	A - J		569 -1223		G - D		545 -103	
	A - I		2142 -856		C - G		439 -18	
	I - B		157 -52		D - F		914 -1647	
	B - G		524 -959		E - F		91 -97	
Wind								

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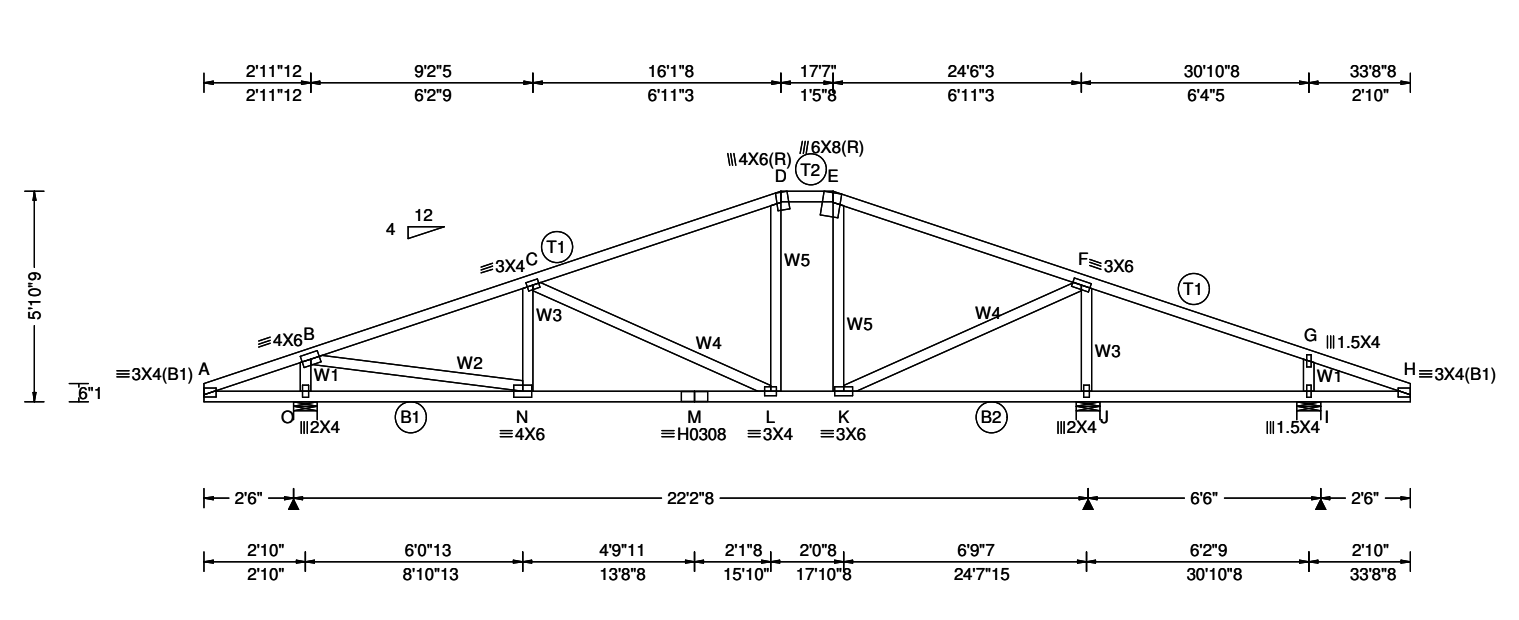


Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria		▲ Maximum Reactions (lbs)							
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#		Gravity			Non-Gravity				
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.140 B 999 360		Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.257 B 999 240		J	1321	/-	/-	/858	/539	/173	
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.040 F - -		F	1310	/-	/-	/853	/557	/-	
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS			HORZ(TL): 0.074 F - -		Wind reactions based on C&C							
NCBCLL: 10.00		Mean Height: 27.16 ft					Creep Factor: 2.0		J Brg Wid = 8.0 Min Req = 1.6 (Truss)							
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.912		F Brg Wid = - Min Req = -							
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.920		Bearing J is a rigid surface.							
Spacing: 24.0 "		MWFRS Parallel Dist: h to 2h					Max Web CSI: 0.339		Maximum Top Chord Forces Per Ply (lbs)							
		C&C Dist a: 3.00 ft					Mfg Specified Camber:		Chords		Tens.Comp.		Chords		Tens. Comp.	
		Loc. from endwall: not in 9.00 ft							A - B		1210 -2888		C - D		823 -1825	
		GCpi: 0.18							B - C		842 -1823		D - E		66 -43	
		Wind Duration: 1.60														

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; B2 2x4 SP #1; Webs: 2x4 SP #2; Lt Wedge: 2x4 SP #2;	<table><tr><th colspan="4">Maximum Bot Chord Forces Per Ply (lbs)</th></tr><tr><th>Chords</th><th>Tens.Comp.</th><th>Chords</th><th>Tens. Comp.</th></tr><tr><td>A - I</td><td>2669 - 1138</td><td>H - G</td><td>2665 - 1142</td></tr><tr><td>I - H</td><td>2665 - 1142</td><td>G - F</td><td>1069 - 544</td></tr></table>	Maximum Bot Chord Forces Per Ply (lbs)				Chords	Tens.Comp.	Chords	Tens. Comp.	A - I	2669 - 1138	H - G	2665 - 1142	I - H	2665 - 1142	G - F	1069 - 544				
Maximum Bot Chord Forces Per Ply (lbs)																					
Chords	Tens.Comp.	Chords	Tens. Comp.																		
A - I	2669 - 1138	H - G	2665 - 1142																		
I - H	2665 - 1142	G - F	1069 - 544																		
Bracing (a) Continuous lateral restraint equally spaced on member.	<table><tr><th colspan="4">Maximum Web Forces Per Ply (lbs)</th></tr><tr><th>Webs</th><th>Tens.Comp.</th><th>Webs</th><th>Tens. Comp.</th></tr><tr><td>I - B</td><td>220 0</td><td>G - D</td><td>589 - 111</td></tr><tr><td>B - G</td><td>549 - 1084</td><td>D - F</td><td>895 - 1685</td></tr><tr><td>C - G</td><td>461 - 25</td><td>E - F</td><td>88 - 93</td></tr></table>	Maximum Web Forces Per Ply (lbs)				Webs	Tens.Comp.	Webs	Tens. Comp.	I - B	220 0	G - D	589 - 111	B - G	549 - 1084	D - F	895 - 1685	C - G	461 - 25	E - F	88 - 93
Maximum Web Forces Per Ply (lbs)																					
Webs	Tens.Comp.	Webs	Tens. Comp.																		
I - B	220 0	G - D	589 - 111																		
B - G	549 - 1084	D - F	895 - 1685																		
C - G	461 - 25	E - F	88 - 93																		
Hangers / Ties (J) Hanger Support Required, by others																					

Wind
Member design based on both MWFRS and C&C.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

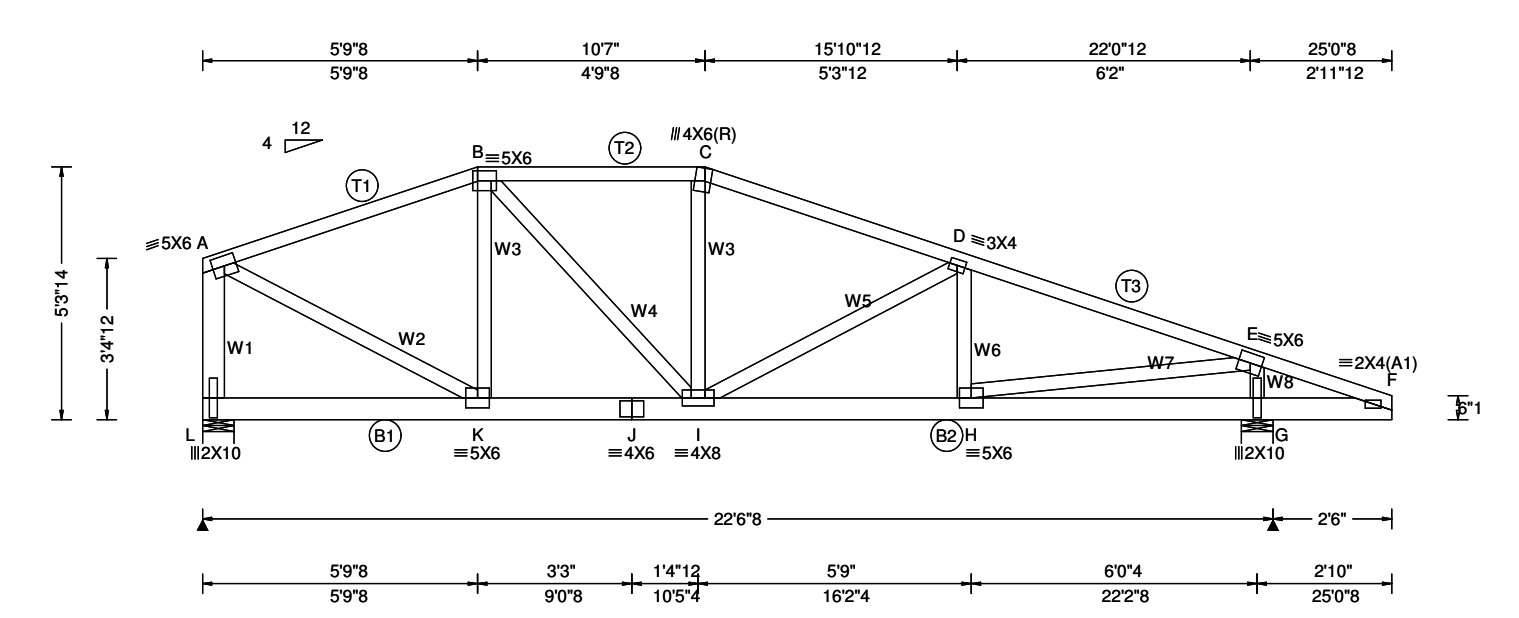
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)							
TCLL:	30.00	Wind Std:	ASCE 7-22	Pg: NA	Ct: NA	CAT: NA	PP Deflection in	loc L/defl	L/#	Gravity			Non-Gravity				
TCDL:	15.00	Speed:	175 mph	Pf: NA		Ce: NA	VERT(LL):	0.115	D 999 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL):	0.238	D 999 240	O	1523	/-	/-	/902	/570	/160	
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA		HORZ(LL):	0.031	D - -	J	1611	/-	/-	/911	/540	/-	
		EXP:	C Kzt: NA				HORZ(TL):	0.064	D - -	I	679	/-	/-	/380	/270	/-	
Des Ld:	55.00	Mean Height:	26.61 ft				Creep Factor:	2.0		Wind reactions based on C&C							
NCBCLL:	10.00	TCDL:	6.0 psf	Building Code:			Max TC CSI:	0.932		O	Brg Wid = 8.0	Min Req = 1.8	(Truss)				
Soffit:	2.00	BCDL:	6.0 psf	FBC 8th Ed. 2023 Res.	HVHZ		Max BC CSI:	0.785		J	Brg Wid = 8.0	Min Req = 1.9	(Truss)				
Load Duration:	1.25	MWFRS Parallel Dist:	h/2 to h	TPI Std:	2014		Max Web CSI:	0.758		I	Brg Wid = 8.0	Min Req = 1.5	(Truss)				
Spacing:	24.0 "	C&C Dist a:	3.37 ft	Rep Fac:	Yes		Mfg Specified Camber:			Bearings O, J, & I are a rigid surface.							
		Loc. from endwall:	not in 9.00 ft	FT/RT:	20(0)/10(0)					Maximum Top Chord Forces Per Ply (lbs)							
		GCpi:	0.18	Plate Type(s):						Chords	Tens.Comp.	Chords	Tens. Comp.				
		Wind Duration:	1.60	WAVE, HS			VIEW Ver:	23.02.01A.1204.18									

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

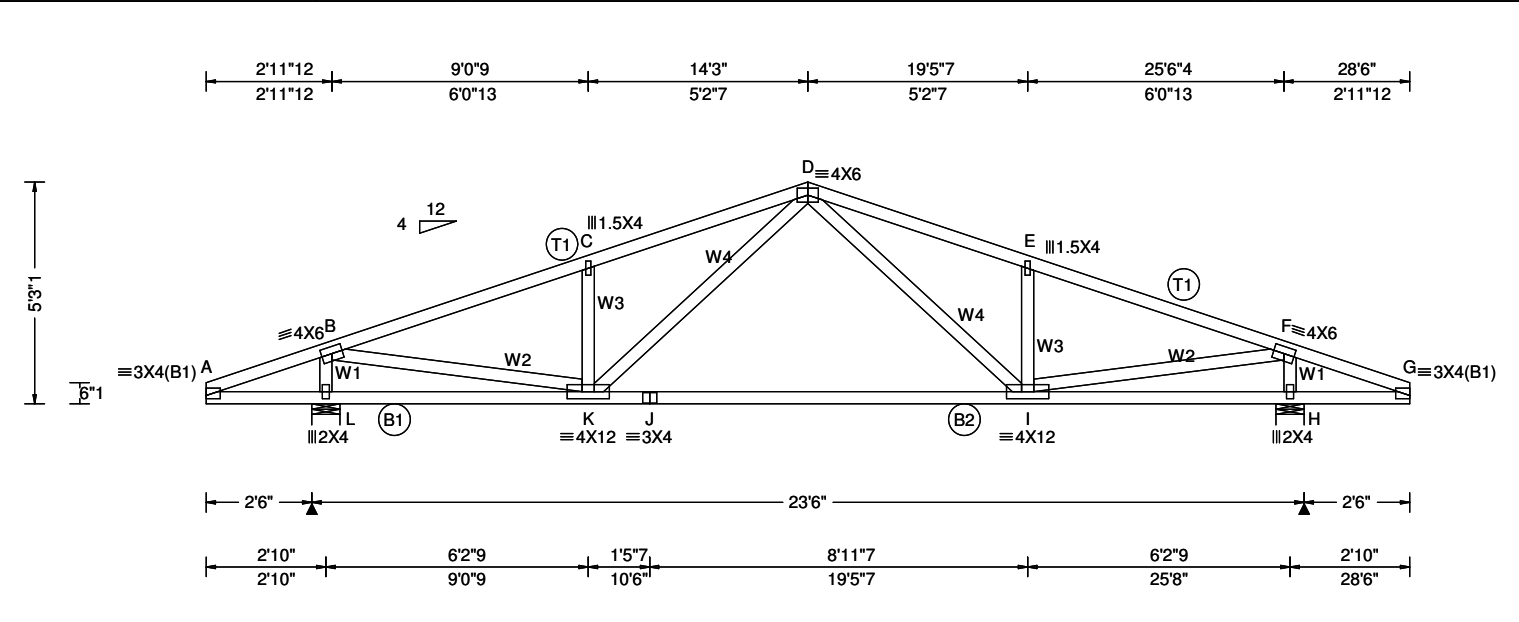
Wind
Member design based on both MWFRS and C&C.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.



Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria		▲ Maximum Reactions (lbs)						
TCLL:	30.00	Wind Std:	ASCE 7-22	Pg: NA	Ct: NA	CAT: NA	PP Deflection in	loc L/defl L/#	Gravity			Non-Gravity			
TCDL:	15.00	Speed:	175 mph	Pf: NA		Ce: NA	VERT(LL):	0.095 H 999 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL):	0.175 H 999 240	L	4490	-/-	-/-	-/-	/2402	-/-
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA		HORZ(LL):	0.028 B - -	G	3943	-/-	-/-	-/-	/2292	-/-
		EXP:	C Kzt: NA				HORZ(TL):	0.051 B - -	Wind reactions based on MWFRS						
Des Ld:	55.00	Mean Height:	26.33 ft				Creep Factor:	2.0	L	Brg Wid = 8.0	Min Req = 2.6	(Truss)			
NCBCLL:	0.00	TCDL:	6.0 psf				Max TC CSI:	0.545	G	Brg Wid = 8.0	Min Req = 2.3	(Truss)			
Soffit:	2.00	BCDL:	6.0 psf				Max BC CSI:	0.969	Bearings L & G are a rigid surface.						
Load Duration:	1.25	MWFRS Parallel Dist:	0 to h/2				Max Web CSI:	0.632	Maximum Top Chord Forces Per Ply (lbs)						
Spacing:	24.0 "	C&C Dist a:	3.00 ft				Mfg Specified Camber:		Chords	Tens.Comp.	Chords	Tens.	Comp.		
		Loc. from endwall:	not in 9.00 ft						A - B	1125 -2142	D - E	1897	-3289		
		GCpi:	0.18						B - C	1309 -2436	E - F	145	-180		
		Wind Duration:	1.60												
														</	

Lumber Top chord: 2x4 SP #2; Bot chord: 2x6 SP #2; Webs: 2x4 SP #2; W1 2x6 SP #2; Nailnote Nail Schedule:0.128"x3", min. nails Top Chord: 1 Row @12.00" o.c. Bot Chord: 1 Row @ 5.50" o.c. Webs : 1 Row @ 4" o.c. Use equal spacing between rows and stagger nails in each row to avoid splitting. Special Loads ----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) <table><tr><td>TC: From</td><td>45 plf at</td><td>0.00 to</td><td>45 plf at</td><td>17.48</td></tr><tr><td>TC: From</td><td>90 plf at</td><td>17.48 to</td><td>90 plf at</td><td>25.04</td></tr><tr><td>BC: From</td><td>10 plf at</td><td>0.00 to</td><td>10 plf at</td><td>17.48</td></tr><tr><td>BC: From</td><td>20 plf at</td><td>17.48 to</td><td>20 plf at</td><td>25.04</td></tr></table> BC: 1196 lb Conc. Load at 1.90 BC: 658 lb Conc. Load at 3.48 BC: 659 lb Conc. Load at 5.48 BC: 647 lb Conc. Load at 7.48 BC: 622 lb Conc. Load at 9.60,11.48,13.48,15.48 BC: 992 lb Conc. Load at 17.48 Wind Wind loads and reactions based on MWFRS. Left end vertical not exposed to wind pressure. Right cantilever is exposed to wind Wind loading based on both gable and hip roof types.	TC: From	45 plf at	0.00 to	45 plf at	17.48	TC: From	90 plf at	17.48 to	90 plf at	25.04	BC: From	10 plf at	0.00 to	10 plf at	17.48	BC: From	20 plf at	17.48 to	20 plf at	25.04	
TC: From	45 plf at	0.00 to	45 plf at	17.48																	
TC: From	90 plf at	17.48 to	90 plf at	25.04																	
BC: From	10 plf at	0.00 to	10 plf at	17.48																	
BC: From	20 plf at	17.48 to	20 plf at	25.04																	

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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria		▲ Maximum Reactions (lbs)						
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#		Gravity			Non-Gravity			
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.079 D 999 360		Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.142 D 999 240		L	1620	- / -	- / -	/ 998	/ 532	/ 140
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.018 C - -		H	1620	- / -	- / -	/ 998	/ 532	- / -
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.033 C - -		Wind reactions based on C&C						
NCBCLL: 10.00		Mean Height: 15.63 ft					Creep Factor: 2.0		L Brg Wid = 8.0 Min Req = 1.9 (Truss)						
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.636		H Brg Wid = 8.0 Min Req = 1.9 (Truss)						
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.947		Bearings L & H are a rigid surface.						
Spacing: 24.0 "		MWFRS Parallel Dist: h/2 to h					Max Web CSI: 0.437		Maximum Top Chord Forces Per Ply (lbs)						
		C&C Dist a: 3.00 ft					Mfg Specified Camber:		Chords	Tens.Comp.		Chords	Tens. Comp.		
		Loc. from endwall: not in 9.00 ft							A - B	350	- 188	D - E	839	- 2082	
		GCpi: 0.18							B - C	714	- 2096	E - F	713	- 2096	
		Wind Duration: 1.60					VIEW Ver: 23.02.01A.1204.18								

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2;	Loading Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.	Wind Member design based on both MWFRS and C&C. Left and right cantilevers are exposed to wind Wind loading based on both gable and hip roof types.
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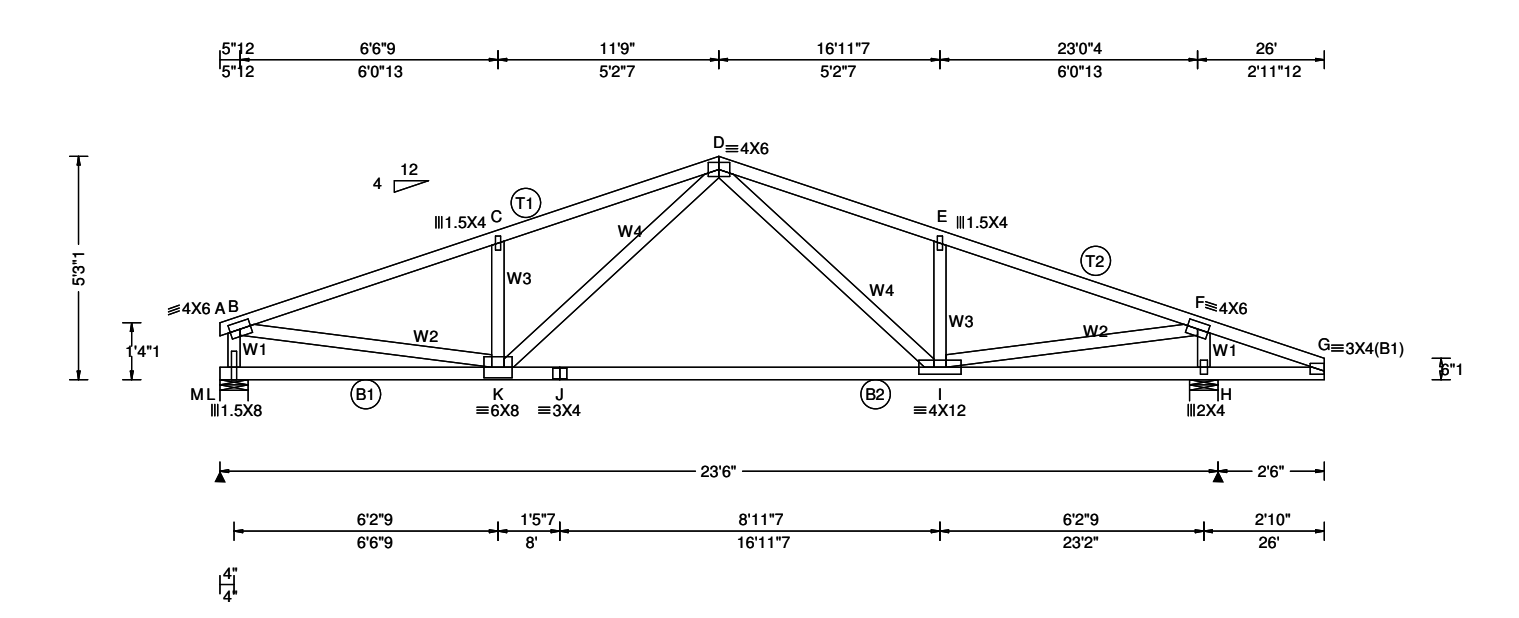
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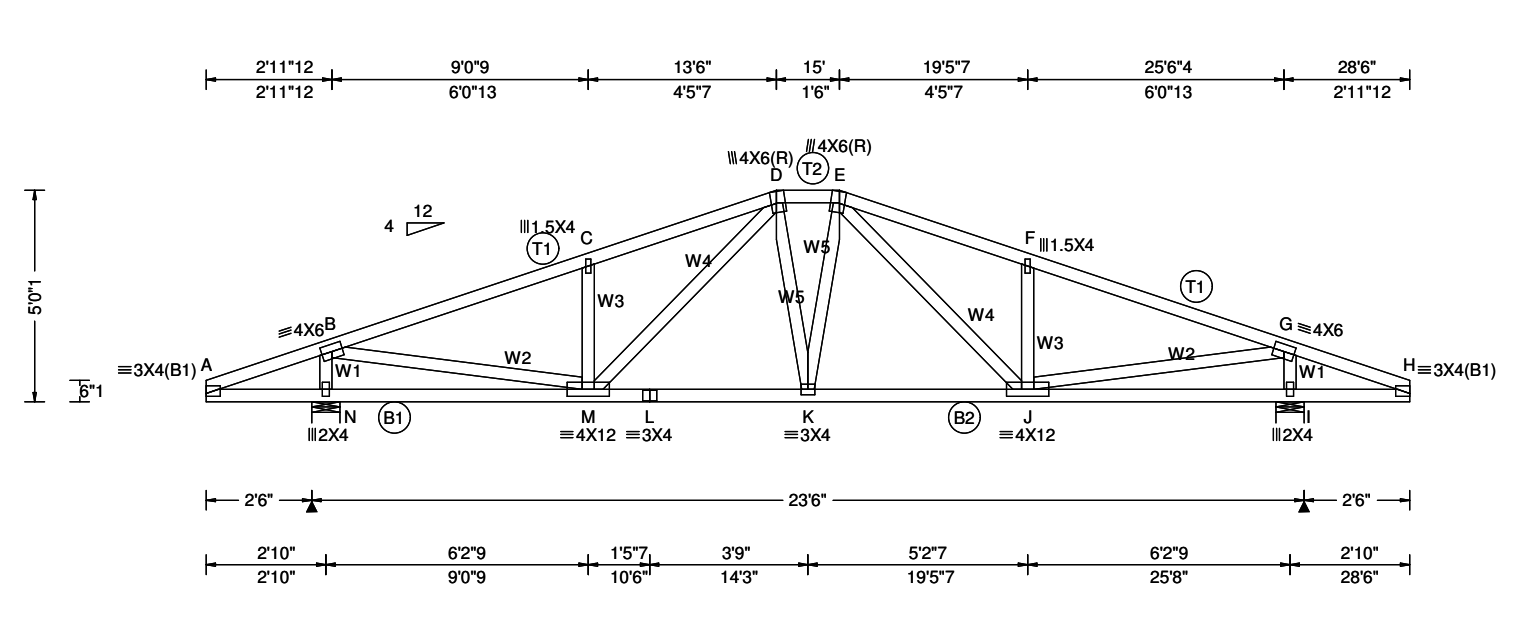
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria		▲ Maximum Reactions (lbs)						
		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#		Gravity			Non-Gravity			
TCLL: 30.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.087 C 999 360		Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
TCDL: 15.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.157 C 999 240		M	1317	/-	/-	/835	/392	/154
BCLL: 0.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.018 C - -		H	1637	/-	/-	/1004	/586	/-
BCDL: 10.00		EXP: C Kzt: NA					HORZ(TL): 0.033 C - -		Wind reactions based on C&C						
Des Ld: 55.00		Mean Height: 15.63 ft		Building Code:			Creep Factor: 2.0		M Brg Wid = 8.0 Min Req = 1.6 (Truss)						
NCBCLL: 10.00		TCDL: 6.0 psf		FBC 8th Ed. 2023 Res. HVHZ			Max TC CSI: 0.881		H Brg Wid = 8.0 Min Req = 1.9 (Truss)						
Soffit: 2.00		BCDL: 6.0 psf		TPI Std: 2014			Max BC CSI: 0.957		Bearings M & H are a rigid surface.						
Load Duration: 1.25		MWFRS Parallel Dist: h to 2h		Rep Fac: Yes			Max Web CSI: 0.451		Maximum Top Chord Forces Per Ply (lbs)						
Spacing: 24.0 "		C&C Dist a: 3.00 ft		FT/RT:20(0)/10(0)			Mfg Specified Camber:		Chords	Tens.Comp.	Chords	Tens.	Comp.		
		Loc. from endwall: not in 9.00 ft		Plate Type(s):			VIEW Ver: 23.02.01A.1204.18		A - B	5	0	D - E	765	-2139	
		GCpi: 0.18		WAVE					B - C	658	-2223	E - F	649	-2152	
		Wind Duration: 1.60													

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2;	Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. M - L 0 0 J - I 1516 -374 L - K 167 -123 I - H 174 -204 K - J 1516 -374 H - G 190 -279
Loading Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.	Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B - L 417 -1283 D - I 603 -168 B - K 1997 -534 I - E 316 -528 C - K 346 -584 I - F 1966 -546 K - D 710 -204 F - H 572 -1566
Wind Member design based on both MWFRS and C&C. Left end vertical not exposed to wind pressure. Right cantilever is exposed to wind Wind loading based on both gable and hip roof types.	

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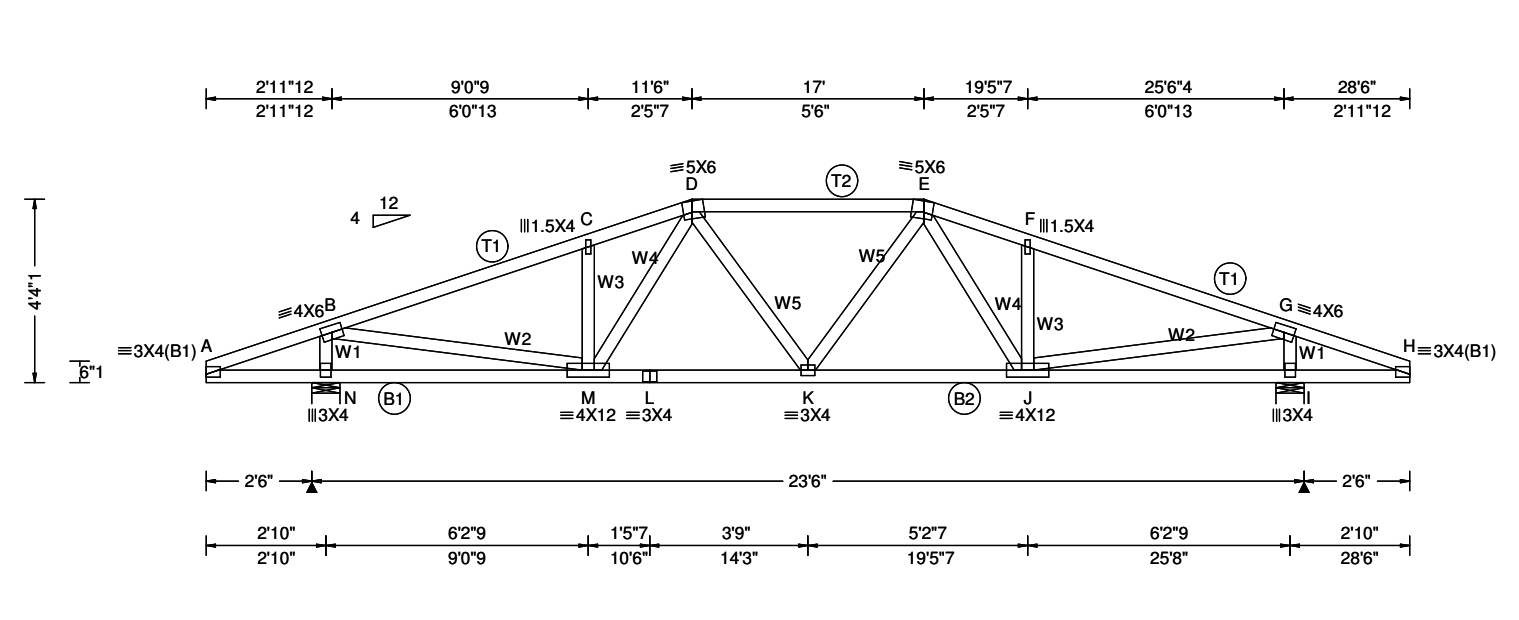


Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)								
TCLL:	30.00	Wind Std:	ASCE 7-22	Pg: NA	Ct: NA	CAT: NA	PP Deflection in	loc L/defl	L/#	Gravity			Non-Gravity					
TCDL:	15.00	Speed:	175 mph	Pf: NA		Ce: NA	VERT(LL):	0.073	K	999	360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL):	0.134	K	999	240	N	1578	/-	/-	/904	/562	/133
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA		HORZ(LL):	0.016	C	-	-	I	1578	/-	/-	/904	/562	/-
		EXP:	C Kzt: NA				HORZ(TL):	0.030	C	-	-	Wind reactions based on C&C						
Des Ld:	55.00	Mean Height:	15.51 ft	Building Code:			Creep Factor:	2.0				N	Brg	Wid = 8.0	Min	Req = 1.9	(Truss)	
NCBCLL:	10.00	TCDL:	6.0 psf	FBC 8th Ed. 2023 Res.	HVHZ		Max TC CSI:	0.595				I	Brg	Wid = 8.0	Min	Req = 1.9	(Truss)	
Soffit:	2.00	BCDL:	6.0 psf	TPI Std:	2014		Max BC CSI:	0.398				Bearings N & I are a rigid surface.						
Load Duration:	1.25	MWFRS Parallel Dist:	h/2 to h	Rep Fac:	Yes		Max Web CSI:	0.405				Maximum Top Chord Forces Per Ply (lbs)						
Spacing:	24.0 "	C&C Dist a:	3.00 ft	FT/RT:	20(0)/10(0)		Mfg Specified	Camber:				Chords	Tens.Comp.	Chords	Tens.	Comp.		
		Loc. from endwall:	not in 9.00 ft	Plate Type(s):								A - B	300	-200	E - F	847	-1931	
		GCpi:	0.18	WAVE								B - C	728	-1948	F - G	728	-1948	
		Wind Duration:	1.60															

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Wind
Member design based on both MWFRS and C&C.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria		▲ Maximum Reactions (lbs)							
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#		Gravity			Non-Gravity				
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.076 K 999 360		Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.140 K 999 240		N	1578	/-	/-	/859	/706	/113	
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.015 I - -		I	1578	/-	/-	/859	/706	/-	
Des Ld: 55.00		EXP: C Kzt: NA					HORZ(TL): 0.030 I - -		Wind reactions based on C&C							
NCBCLL: 10.00		Mean Height: 15.17 ft		Building Code:			Creep Factor: 2.0		N Brg Wid = 8.0 Min Req = 1.9 (Truss)							
Soffit: 2.00		TCDL: 6.0 psf		FBC 8th Ed. 2023 Res. HVHZ			Max TC CSI: 0.641		I Brg Wid = 8.0 Min Req = 1.9 (Truss)							
Load Duration: 1.25		BCDL: 6.0 psf		TPI Std: 2014			Max BC CSI: 0.419		Bearings N & I are a rigid surface.							
Spacing: 24.0 "		MWFRS Parallel Dist: h/2 to h		Rep Fac: Yes			Max Web CSI: 0.570		Maximum Top Chord Forces Per Ply (lbs)							
		C&C Dist a: 3.00 ft		FT/RT:20(0)/10(0)			Mfg Specified Camber:		Chords		Tens.Comp.		Chords		Tens. Comp.	
		Loc. from endwall: not in 9.00 ft		Plate Type(s):			VIEW Ver: 23.02.01A.1204.18		A - B		297 -187		E - F		1064 -1884	
		GCpi: 0.18		WAVE					B - C		997 -1937		F - G		997 -1937	
		Wind Duration: 1.60														

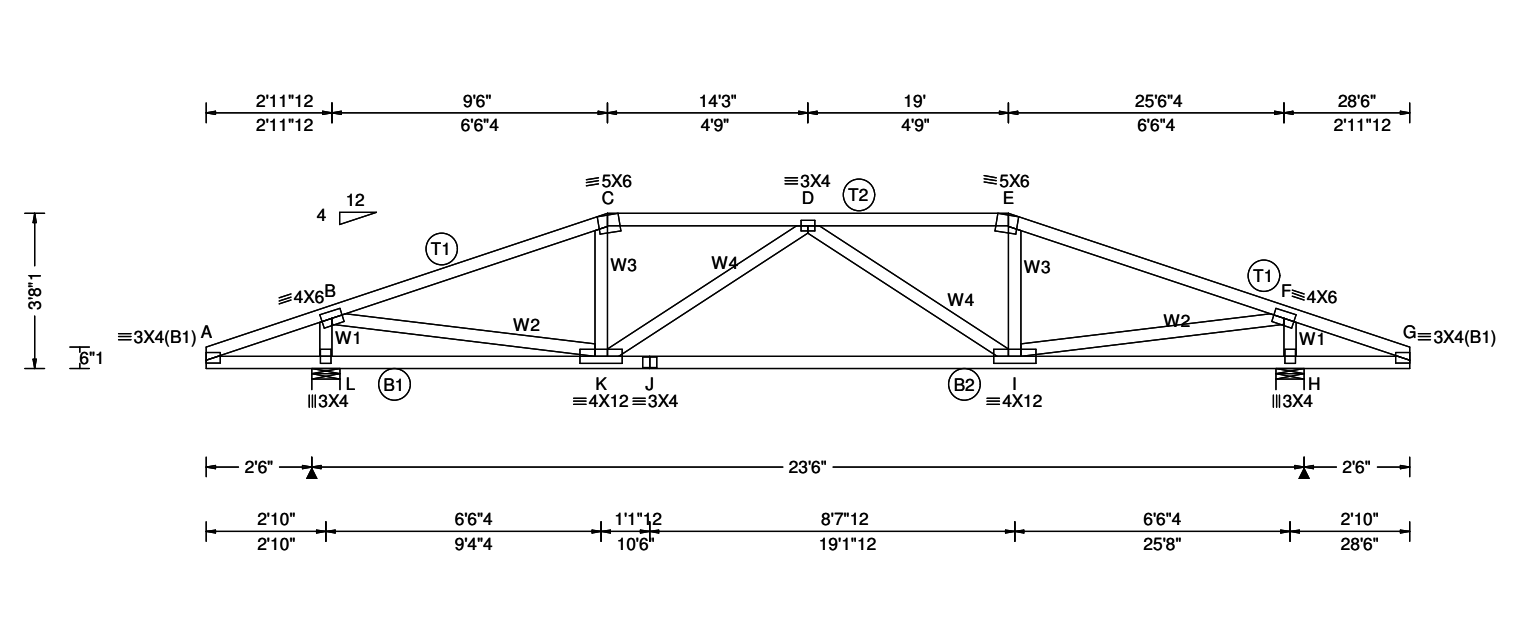
Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Wind
Member design based on both MWFRS and C&C.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)			
Chords		Tens. Comp.	
A - N	177	-	230
N - M	195	-	164
M - L	1625	-	884
L - K	1625	-	884

Maximum Web Forces Per Ply (lbs)			
Webs		Tens. Comp.	
N - B	709	-	1451
B - M	1721	-	836
C - M	264	-	396
M - D	453	-	128
D - K	120	-	23

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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)						
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity			
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.090 D 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.167 D 999 240			L	1578	/-	/-	/868	/849	/93
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.018 H - -			H	1578	/-	/-	/868	/849	/-
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.035 H - -			Wind reactions based on C&C						
NCBCLL: 10.00		Mean Height: 15.00 ft					Creep Factor: 2.0			L Brg Wid = 8.0 Min Req = 1.9 (Truss)						
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.790			H Brg Wid = 8.0 Min Req = 1.9 (Truss)						
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.868			Bearings L & H are a rigid surface.						
Spacing: 24.0 "		MWFRS Parallel Dist: h/2 to h					Max Web CSI: 0.833			Maximum Top Chord Forces Per Ply (lbs)						
		C&C Dist a: 3.00 ft					Mfg Specified Camber:			Chords	Tens.Comp.		Chords	Tens. Comp.		
		Loc. from endwall: not in 9.00 ft								A - B	307	-183	D - E	1309	-1767	
		GCpi: 0.18								B - C	1308	-1962	E - F	1308	-1962	
		Wind Duration: 1.60														

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Wind

Member design based on both MWFRS and C&C.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

C - D		1310	- 1767	F - G		308	- 183
Maximum Bot Chord Forces Per Ply (lbs)							
Chords	Tens.Comp.		Chords	Tens. Comp.			
A - L	175	- 238	J - I	2029	- 1512		
L - K	209	- 171	I - H	209	- 186		
K - J	2029	- 1512	H - G	175	- 238		
Maximum Web Forces Per Ply (lbs)							
Webs	Tens.Comp.		Webs	Tens. Comp.			
L - B	842	- 1476	D - I	565	- 358		
B - K	1727	- 1109	E - I	209	- 72		
K - C	208	- 72	I - F	1727	- 1109		
K - D	564	- 359	F - H	842	- 1476		

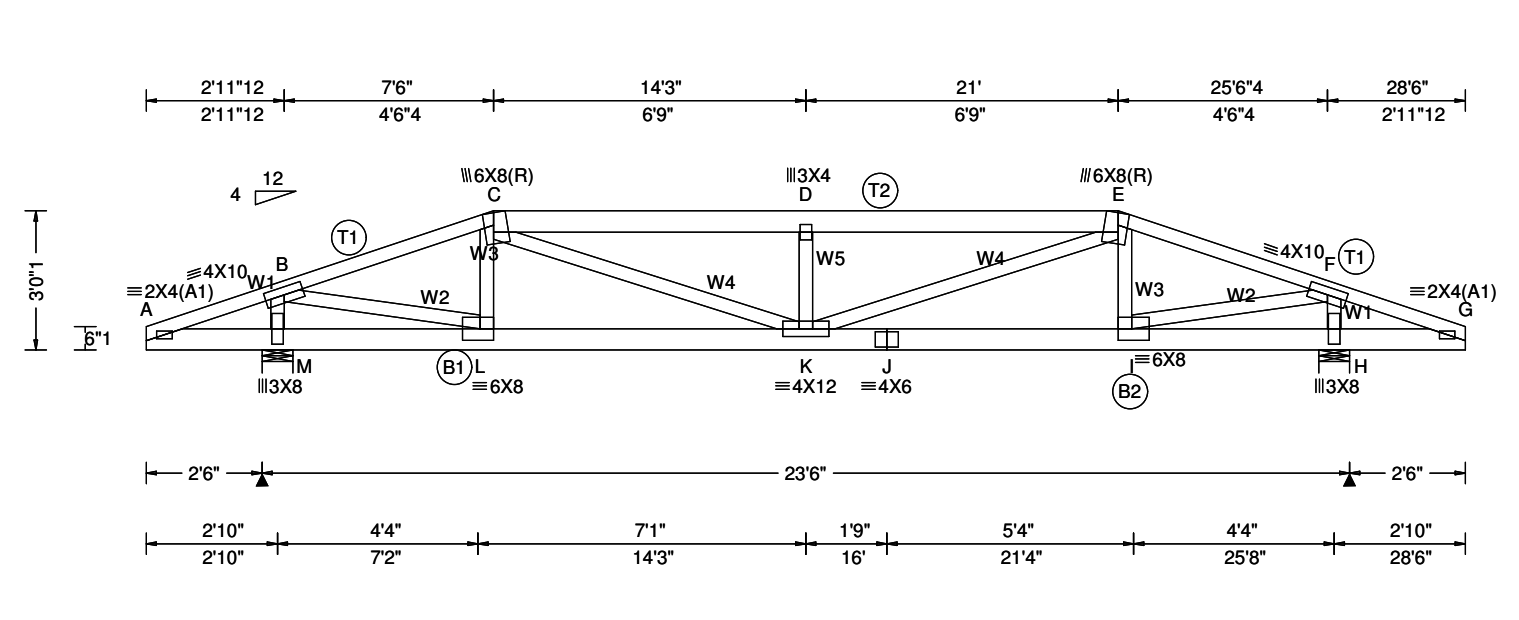
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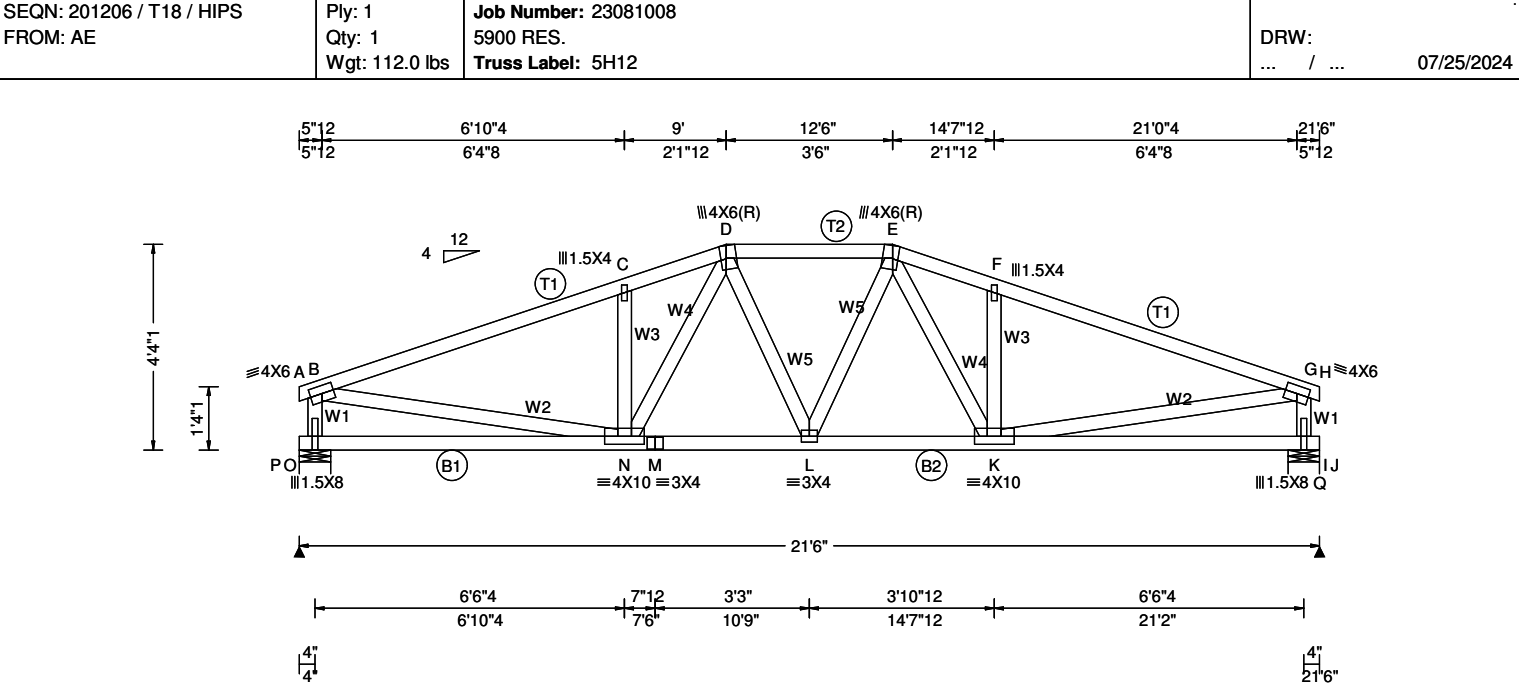
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)							
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity				
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.185 D 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.340 D 806 240			M	2191	-	-	-	-	/1126 -	
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.030 C - -			H	2191	-	-	-	-	/1126 -	
Des Ld: 55.00		EXP: C Kzt: NA					HORZ(TL): 0.055 C - -			Wind reactions based on MWFRS							
NCBCLL: 10.00		Mean Height: 15.00 ft		Building Code:			Creep Factor: 2.0			M Brg Wid = 8.0 Min Req = 2.6 (Truss)							
Soffit: 2.00		TCDL: 6.0 psf		FBC 8th Ed. 2023 Res. HVHZ			Max TC CSI: 0.655			H Brg Wid = 8.0 Min Req = 2.6 (Truss)							
Load Duration: 1.25		BCDL: 6.0 psf		TPI Std: 2014			Max BC CSI: 0.774			Bearings M & H are a rigid surface.							
Spacing: 24.0 "		MWFRS Parallel Dist: 0 to h/2		Rep Fac: Varies by Ld Case			Max Web CSI: 0.936			Maximum Top Chord Forces Per Ply (lbs)							
		C&C Dist a: 3.00 ft		FT/RT:20(0)/10(0)			Mfg Specified Camber:			Chords		Tens.Comp.		Chords		Tens. Comp.	
		Loc. from endwall: not in 4.50 ft		Plate Type(s):			VIEW Ver: 23.02.01A.1204.18			A - B		334 -71		D - E		2574 -4699	
		GCpi: 0.18		WAVE						B - C		1721 -3011		E - F		1721 -3012	
		Wind Duration: 1.60															

Lumber Top chord: 2x4 SP #2; T2 2x6 SP #2; Bot chord: 2x6 SP #2; Webs: 2x4 SP #2;	Special Loads ----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC: From 90 plf at 0.00 to 90 plf at 7.50 TC: From 45 plf at 7.50 to 45 plf at 21.00 TC: From 90 plf at 21.00 to 90 plf at 28.50 BC: From 20 plf at 0.00 to 20 plf at 8.09 BC: From 10 plf at 8.09 to 10 plf at 20.41 BC: From 20 plf at 20.41 to 20 plf at 28.50 TC: 132 lb Conc. Load at 8.09,10.09,12.09,14.09 14.41,16.41,18.41,20.41 BC: 124 lb Conc. Load at 7.46,21.04 BC: 84 lb Conc. Load at 8.09,10.09,12.09,14.09 14.41,16.41,18.41,20.41
Wind Wind loads and reactions based on MWFRS. Left and right cantilevers are exposed to wind Wind loading based on both gable and hip roof types.	

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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)								
TCLL:	30.00	Wind Std:	ASCE 7-22	Pg: NA	Ct: NA	CAT: NA	PP Deflection in	loc L/defl	L/#	Gravity			Non-Gravity					
TCDL:	15.00	Speed:	175 mph	Pf: NA		Ce: NA	VERT(LL):	0.070	L	999	360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL):	0.128	L	999	240	P	1182	-	-	/557	/632	/89
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA		HORZ(LL):	0.015	H	-	-	Q	1182	-	-	/557	/632	-
		EXP:	C Kzt: NA				HORZ(TL):	0.028	H	-	-	Wind reactions based on C&C						
Des Ld:	55.00	Mean Height:	15.59 ft				Creep Factor:	2.0				P	Brg Wid = 8.0	Min Req = 1.5	(Truss)			
NCBCLL:	10.00	TCDL:	6.0 psf				Max TC CSI:	0.956				Q	Brg Wid = 8.0	Min Req = 1.5	(Truss)			
Soffit:	2.00	BCDL:	6.0 psf				Max BC CSI:	0.473				Bearings P & J are a rigid surface.						
Load Duration:	1.25	MWFRS Parallel Dist:	> 2h				Max Web CSI:	0.713				Maximum Top Chord Forces Per Ply (lbs)						
Spacing:	24.0 "	C&C Dist a:	3.00 ft				Mfg Specified Camber:					Chords	Tens.	Comp.	Chords	Tens.	Comp.	
		Loc. from endwall:	not in 9.00 ft									A - B	5	0	E - F	1249	-1869	
		GCpi:	0.18									B - C	1114	-1886	F - G	1113	-1886	
		Wind Duration:	1.60															

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Wind
Member design based on both MWFRS and C&C.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

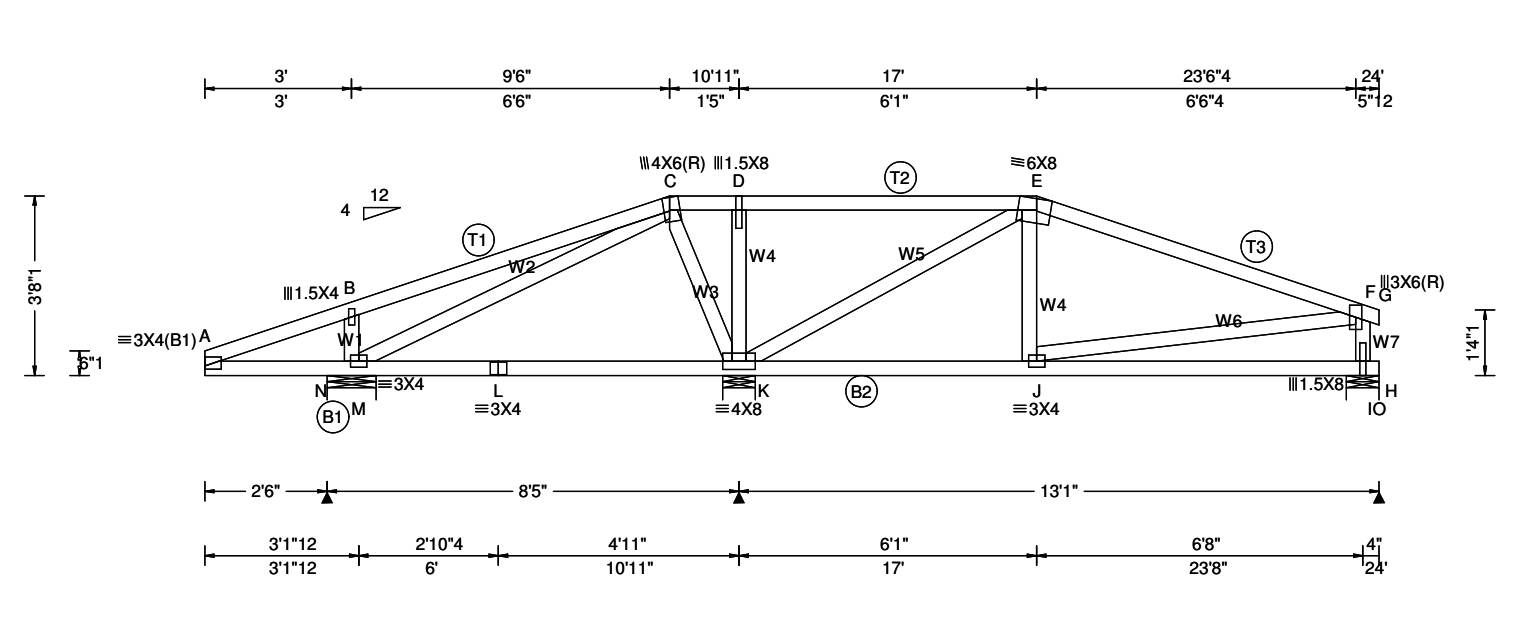
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)					
				Gravity			Non-Gravity		
				Loc	R+	R- / Rh	Rw	U	RL
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	N	763	- / -	/567	/409	/127
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): 0.022 G 100 360	K	1281	- / -	/894	/1277	-
BCLL: 0.00	Enclosure: Part. Enc.	Lu: NA Cs: NA	VERT(CL): -0.037 G 999 240	O	663	- / -	/499	/473	-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.010 G - -	Wind reactions based on C&C					
Des Ld: 55.00	EXP: C Kzt: NA	Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	HORZ(TL): 0.018 G - -	N Brg Wid = 12.0 Min Req = 1.5 (Truss)					
NCBCLL: 10.00	Mean Height: 15.00 ft		Creep Factor: 2.0	K Brg Wid = 8.0 Min Req = 1.5 (Truss)					
Soffit: 2.00	TCDL: 6.0 psf		Max TC CSI: 0.945	O Brg Wid = 8.0 Min Req = 1.5 (Truss)					
Load Duration: 1.25	BCDL: 6.0 psf		Max BC CSI: 0.421	Bearings N, K, & I are a rigid surface.					
Spacing: 24.0 "	MWFRS Parallel Dist: > 2h		Max Web CSI: 0.651	Maximum Top Chord Forces Per Ply (lbs)					
	C&C Dist a: 3.00 ft		Mfg Specified Camber:	Chords	Tens.Comp.	Chords	Tens. Comp.		
	Loc. from endwall: not in 9.00 ft		VIEW Ver: 23.02.01A.1204.18	A - B	331 -314	D - E	181 -86		
	GCpi: 0.55			B - C	407 -339	E - F	562 -694		
	Wind Duration: 1.60			C - D	182 -86	F - G	5 0		

Lumber Top chord: 2x4 SP #2; T3 2x4 SP #1; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2; Wind Member design based on both MWFRS and C&C. Right end vertical not exposed to wind pressure. Left cantilever is exposed to wind Wind loading based on both gable and hip roof types.	Maximum Bot Chord Forces Per Ply (lbs)			
	Chords	Tens.Comp.	Chords	Tens. Comp.
	A - M	294 -288	K - J	562 -411
	M - L	220 -160	J - I	109 -85
Maximum Web Forces Per Ply (lbs)				
Webs	Tens.Comp.	Webs	Tens. Comp.	
B - M	552 -585	K - E	705 -820	
M - C	226 -291	E - J	222 0	
C - K	109 -302	J - F	459 -330	
D - K	1034 -621	F - I	509 -599	

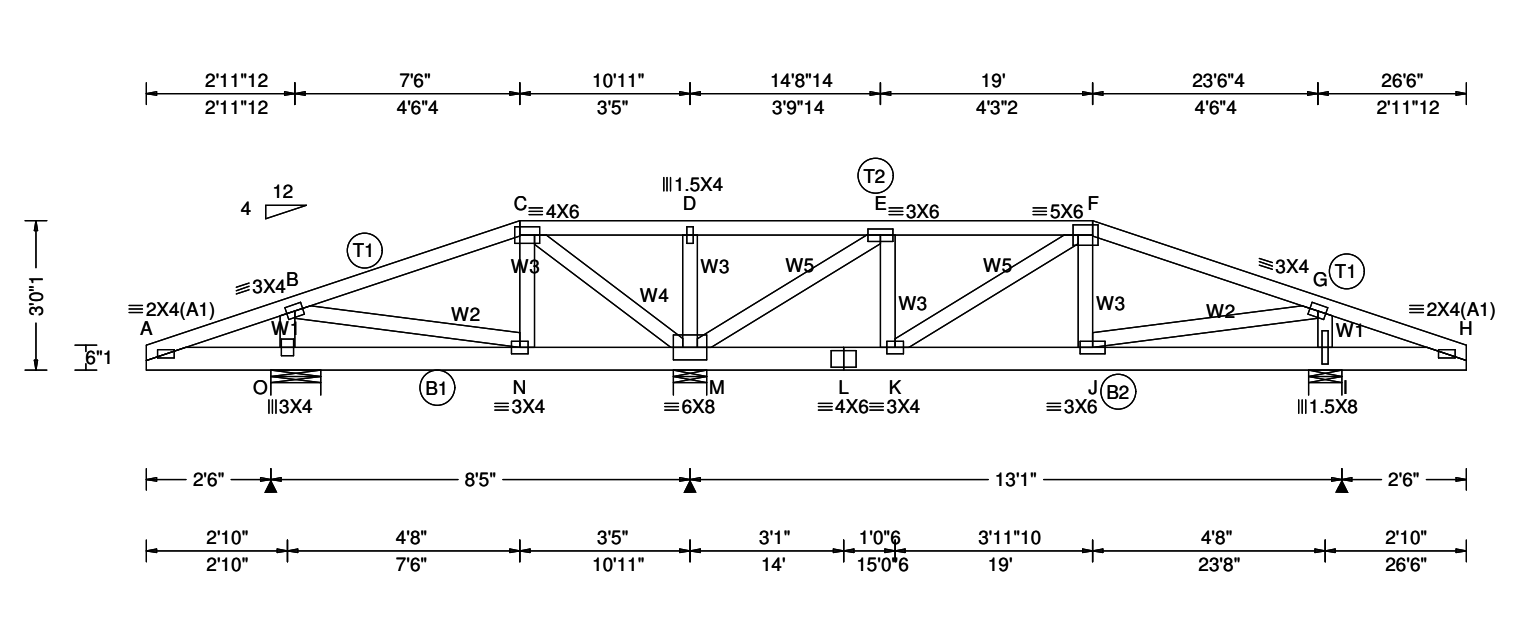
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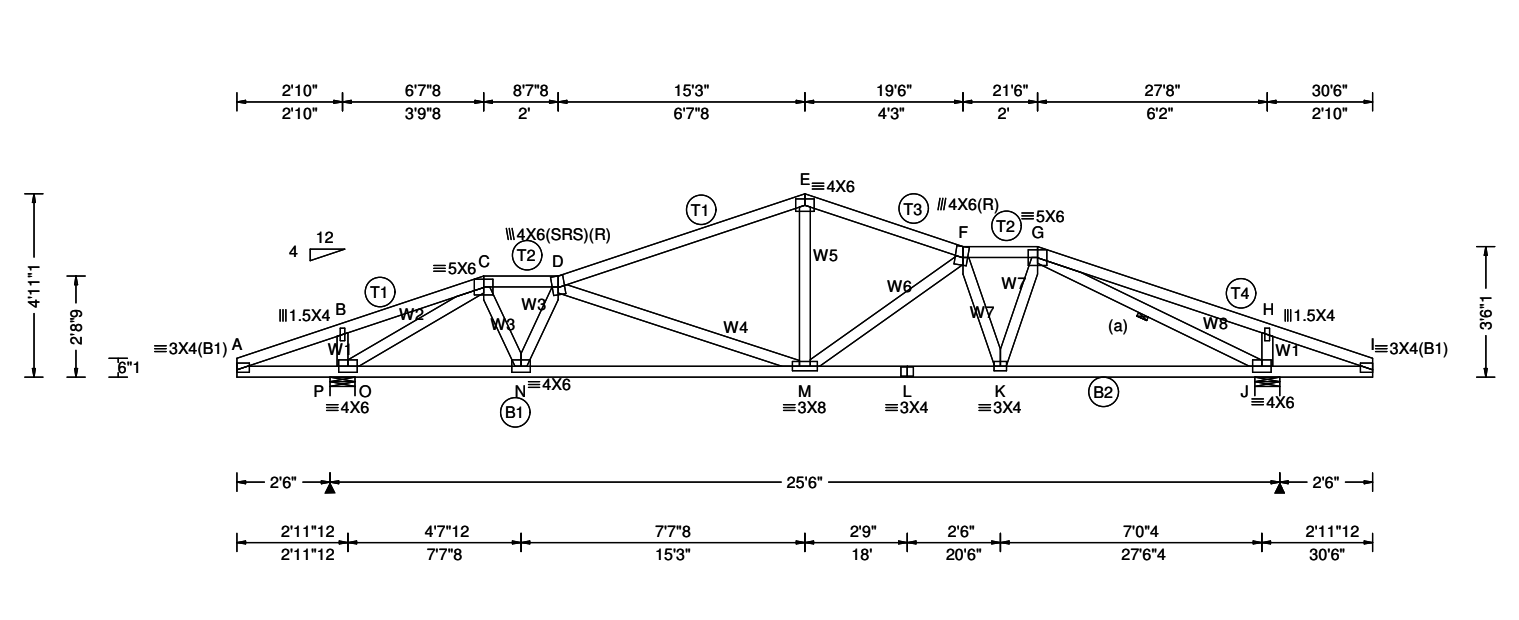
Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)											
		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity								
		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): -0.023 J 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL					
		Enclosure: Part. Enc.		Lu: NA Cs: NA			VERT(CL): 0.039 J 999 240			O	713	/-	/-	/-	/519	/-					
		Risk Category: II		Snow Duration: NA			HORZ(LL): -0.005 E - -			M	2180	/-	/-	/-	/1568	/-					
		EXP: C Kzt: NA					HORZ(TL): 0.008 E - -			I	1180	/-	/-	/-	/755	/-					
		Mean Height: 15.00 ft					Creep Factor: 2.0			Wind reactions based on MWFRS											
		TCDL: 6.0 psf		Building Code:			Max TC CSI: 0.514			O Brg Wid = 12.0 Min Req = 1.5 (Truss)											
		BCDL: 6.0 psf		FBC 8th Ed. 2023 Res. HVHZ			Max BC CSI: 0.316			M Brg Wid = 8.0 Min Req = 2.6 (Truss)											
		MWFRS Parallel Dist: 0 to h/2		TPI Std: 2014			Max Web CSI: 0.584			I Brg Wid = 8.0 Min Req = 1.5 (Truss)											
		C&C Dist a: 3.00 ft		Rep Fac: Varies by Ld Case			Mfg Specified Camber:			Bearings O, M, & I are a rigid surface.											
		Loc. from endwall: not in 9.00 ft		FT/RT:20(0)/10(0)						Maximum Top Chord Forces Per Ply (lbs)											
		GCpi: 0.55		Plate Type(s):						Chords			Tens.Comp.			Chords			Tens. Comp.		
		Wind Duration: 1.60		WAVE			VIEW Ver: 23.02.01A.1204.18														

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x6 SP #2;
Webs: 2x4 SP #2;

Special Loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 90 plf at 0.00 to 90 plf at 7.50
TC: From 45 plf at 7.50 to 45 plf at 19.00
TC: From 90 plf at 19.00 to 90 plf at 26.50
BC: From 20 plf at 0.00 to 20 plf at 7.46
BC: From 10 plf at 7.46 to 10 plf at 18.41
BC: From 20 plf at 18.41 to 20 plf at 26.50
TC: 132 lb Conc. Load at 8.09,10.09,12.09,13.25
14.41,16.41,18.41
BC: 124 lb Conc. Load at 7.46,19.04
BC: 84 lb Conc. Load at 8.09,10.09,12.09,13.25
14.41,16.41,18.41

Wind
Wind loads and reactions based on MWFRS.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

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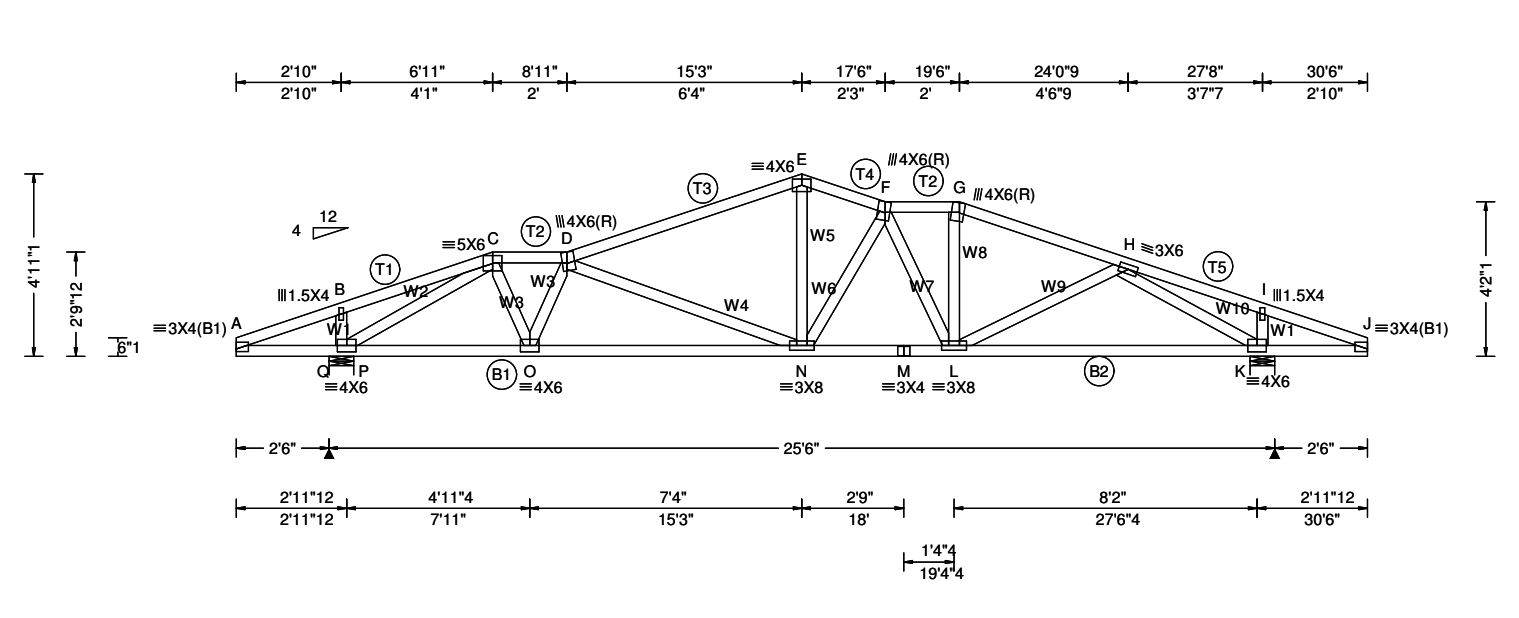
Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)						
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity			
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.107 M 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.198 M 999 240			P	1688	/-	/-	/895	/677	/131
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.043 J - -			J	1688	/-	/-	/892	/637	/-
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.081 J - -			Wind reactions based on C&C						
NCBCLL: 10.00		Mean Height: 15.46 ft					Creep Factor: 2.0			P Brg Wid = 8.0 Min Req = 2.0 (Truss)						
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.854			J Brg Wid = 8.0 Min Req = 2.0 (Truss)						
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.703			Bearings P & J are a rigid surface.						
Spacing: 24.0 "		MWFRS Parallel Dist: h to 2h					Max Web CSI: 0.757			Maximum Top Chord Forces Per Ply (lbs)						
		C&C Dist a: 3.05 ft					Mfg Specified Camber:			Chords	Tens.Comp.	Chords	Tens.	Comp.		
		Loc. from endwall: not in 9.00 ft								A - B	340	-182	E - F	828	-1928	
		GCpi: 0.18								B - C	338	-161	F - G	966	-2016	
		Wind Duration: 1.60														

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Wind
Member design based on both MWFRS and C&C.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

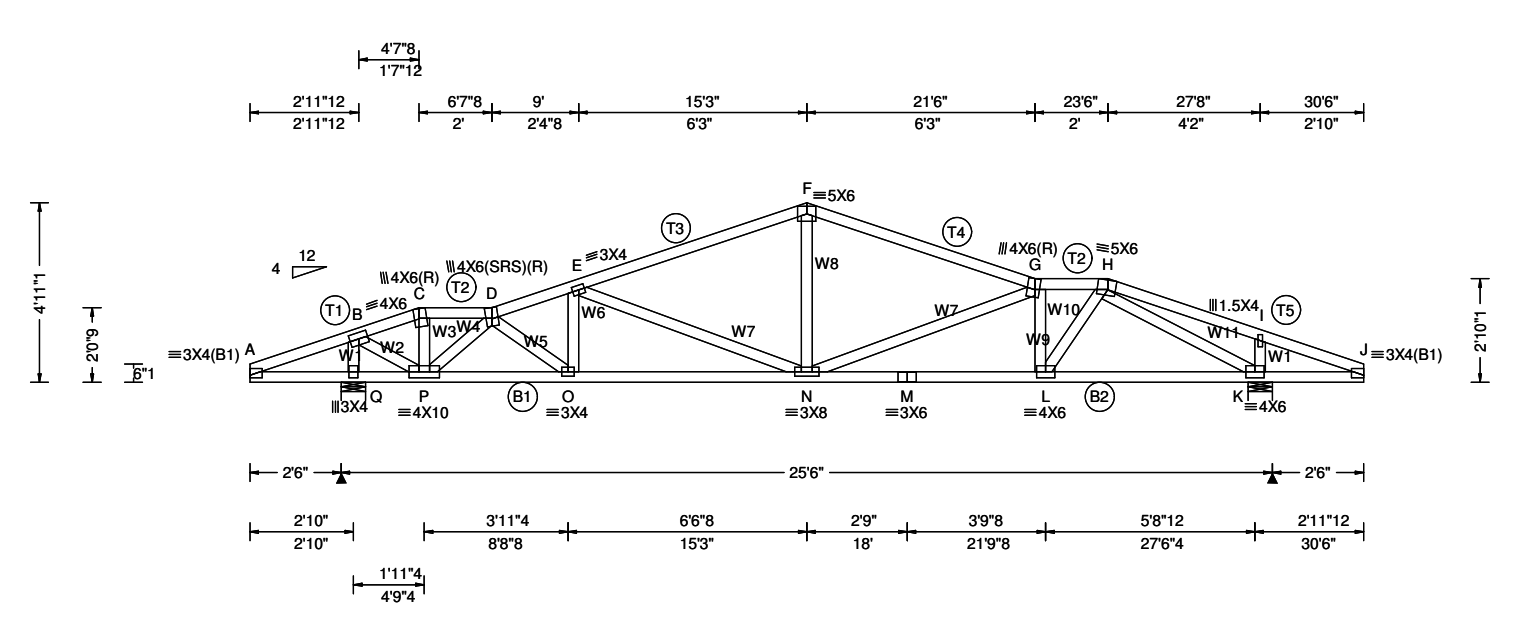
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria		▲ Maximum Reactions (lbs)											
TCLL:	30.00	Wind Std:	ASCE 7-22	Pg:	NA	Ct:	NA	CAT:	NA	Gravity			Non-Gravity							
TCDL:	15.00	Speed:	175 mph	Pf:	NA			Ce:	NA	Loc	R+	/ R-	/ Rh	/ Rw	/ U / RL					
BCLL:	0.00	Enclosure:	Closed	Lu:	NA	Cs:	NA			Q	1688	/-	/-	/899	/685 /131					
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA				VERT(LL):	0.098	N	999	360	K	1688	/-	/-	/894	/628	/-
		EXP:	C Kzt:	NA						HORZ(LL):	0.040	K	-	-	Wind reactions based on C&C					
Des Ld:	55.00	Mean Height:	15.46 ft							HORZ(TL):	0.076	K	-	-	Q Brg Wid = 8.0 Min Req = 2.0 (Truss)					
NCBCLL:	10.00	TCDL:	6.0 psf		Building Code:										K Brg Wid = 8.0 Min Req = 2.0 (Truss)					
Soffit:	2.00	BCDL:	6.0 psf		FBC 8th Ed. 2023 Res.	HVHZ				Creep Factor:	2.0				Bearings Q & K are a rigid surface.					
Load Duration:	1.25	MWFRS Parallel Dist:	h to 2h		TPI Std:	2014				Max TC CSI:	0.775				Maximum Top Chord Forces Per Ply (lbs)					
Spacing:	24.0 "	C&C Dist a:	3.05 ft		Rep Fac:	Yes				Max BC CSI:	0.690									
		Loc. from endwall:	not in 9.00 ft		FT/RT:	20(0)/10(0)				Max Web CSI:	0.863									
		GCpi:	0.18		Plate Type(s):					Mfg Specified Camber:										
		Wind Duration:	1.60		WAVE															
										VIEW Ver:	23.02.01A.1204.18									

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Wind
Member design based on both MWFRS and C&C.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.



Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria		▲ Maximum Reactions (lbs)								
TCLL:	30.00	Wind Std:	ASCE 7-22	Pg: NA	Ct: NA	CAT: NA	PP Deflection in	loc L/defl L/#	Gravity			Non-Gravity					
TCDL:	15.00	Speed:	175 mph	Pf: NA		Ce: NA	VERT(LL):	0.112 N	999	360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL):	0.206 N	999	240	Q	1678	/-	/-	/-	/981	/-
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA		HORZ(LL):	0.043 K	-	-	K	1697	/-	/-	/-	/657	/-
		EXP:	C Kzt: NA				HORZ(TL):	0.081 K	-	-	Wind reactions based on MWFRS						
Des Ld:	55.00	Mean Height:	15.46 ft				Creep Factor:	2.0			Q	Brg Wid = 8.0	Min Req = 2.0	(Truss)			
NCBCLL:	10.00	TCDL:	6.0 psf				Max TC CSI:	0.715			K	Brg Wid = 8.0	Min Req = 2.0	(Truss)			
Soffit:	2.00	BCDL:	6.0 psf				Max BC CSI:	0.697			Bearings Q & K are a rigid surface.						
Load Duration:	1.25	MWFRS Parallel Dist:	0 to h/2				Max Web CSI:	0.925			Maximum Top Chord Forces Per Ply (lbs)						
Spacing:	24.0 "	C&C Dist a:	3.05 ft				Mfg Specified Camber:				Chords	Tens.Comp.	Chords	Tens.	Comp.		
		Loc. from endwall:	not in 9.00 ft								A - B	368	-128	F - G	827	-2004	
		GCpi:	0.18								B - C	835	-1329	G - H	877	-2298	
		Wind Duration:	1.60														

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Special Loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 90 plf at 0.00 to 90 plf at 30.50
BC: From 20 plf at 0.00 to 20 plf at 30.50
TC: -45 lb Conc. Load at 4.58
BC: -167 lb Conc. Load at 4.58

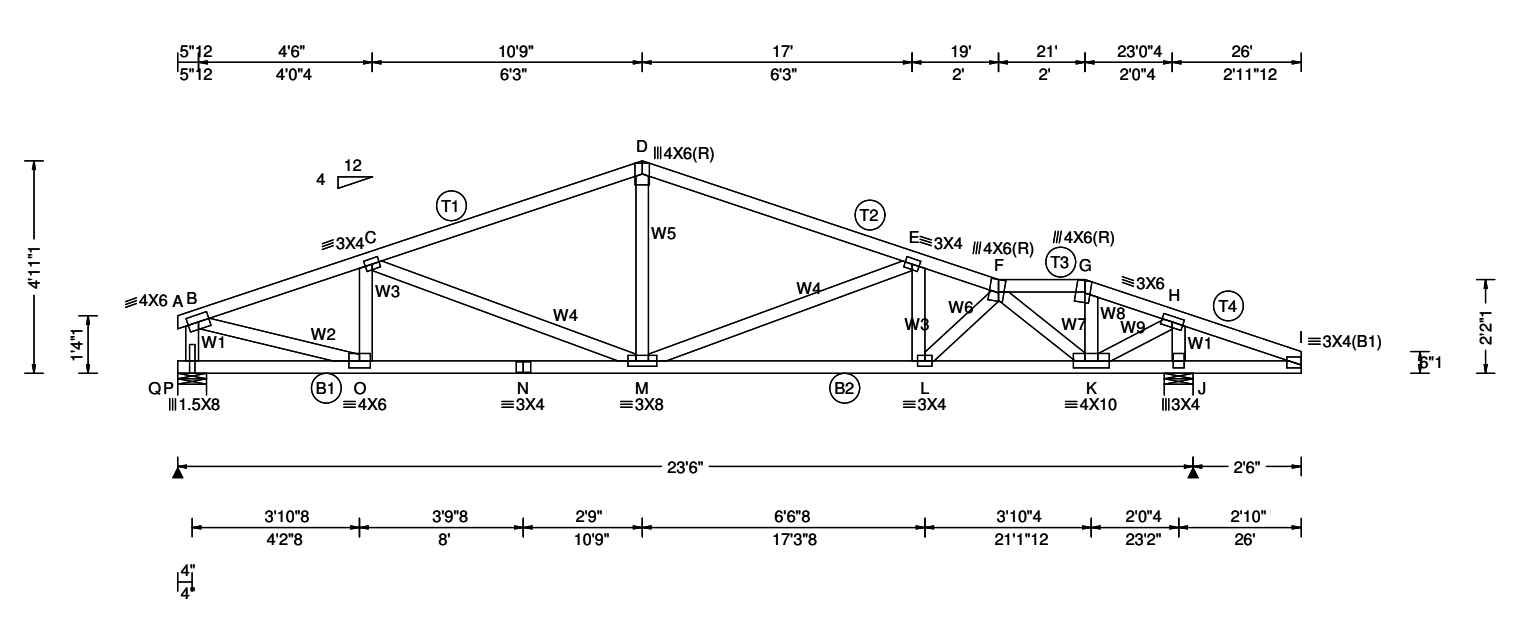
Wind
Wind loads and reactions based on MWFRS.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

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The drawing shows a roof truss system with the following details:

- Top Chord:** Members include $\equiv 3X4(B1)$ (A-B), $\equiv 4X6(B)$ (B-C), $\equiv 4X6(R)$ (C-D), $\equiv 4X6(SRS)(R)$ (D-E), $\equiv 3X4$ (E-F), $\equiv 4X6(R)$ (F-G), $\equiv 4X6(R)$ (G-H), $\equiv 1.5X4$ (H-I), and $\equiv 4X6$ (I-J).
- Bottom Chord:** Members include $\equiv 3X4$ (Q-P), $\equiv 4X10$ (P-O), $\equiv 3X4$ (O-N), $\equiv 3X8$ (N-M), $\equiv 3X4$ (M-L), $\equiv 4X12$ (L-K), and $\equiv 2X4$ (K-J).
- Vertical Members:** $W8$ (F-N), $W9$ (G-M), $W10$ (H-L), and $W11$ (I-K).
- Diagonal Members:** $W2$ (B-C), $W3$ (C-D), $W4$ (D-E), $W5$ (E-F), $W6$ (F-G), $W7$ (G-H), $W12$ (I-J), and $W1$ (K-J).
- Joints:** Labeled A, B, C, D, E, F, G, H, I, J, K, L, M, N, O, P, Q.
- Dimensions:**
 - Horizontal:**
 - Top: 2'11"12, 6'11", 9', 15', 17'6", 21'10"9, 27'6"4, 30'6"
 - Bottom: 2'6", 25'6", 2'6"
 - Bottom (Detailed): 2'10", 3'11"4 (8'8"), 6'5"4 (15'1"12), 2'10"4 (18"), 3'10"9 (21'10"9), 5'9"7 (27'8"), 2'10" (30'6")
 - Bottom (Vertical): 1'11"4 (4'9"4)
 - Vertical:** 4'10"1 (Total), 2'1"12 (Partial), 6"1 (Partial)
 - Angles:** 4/12 slope at joint B.

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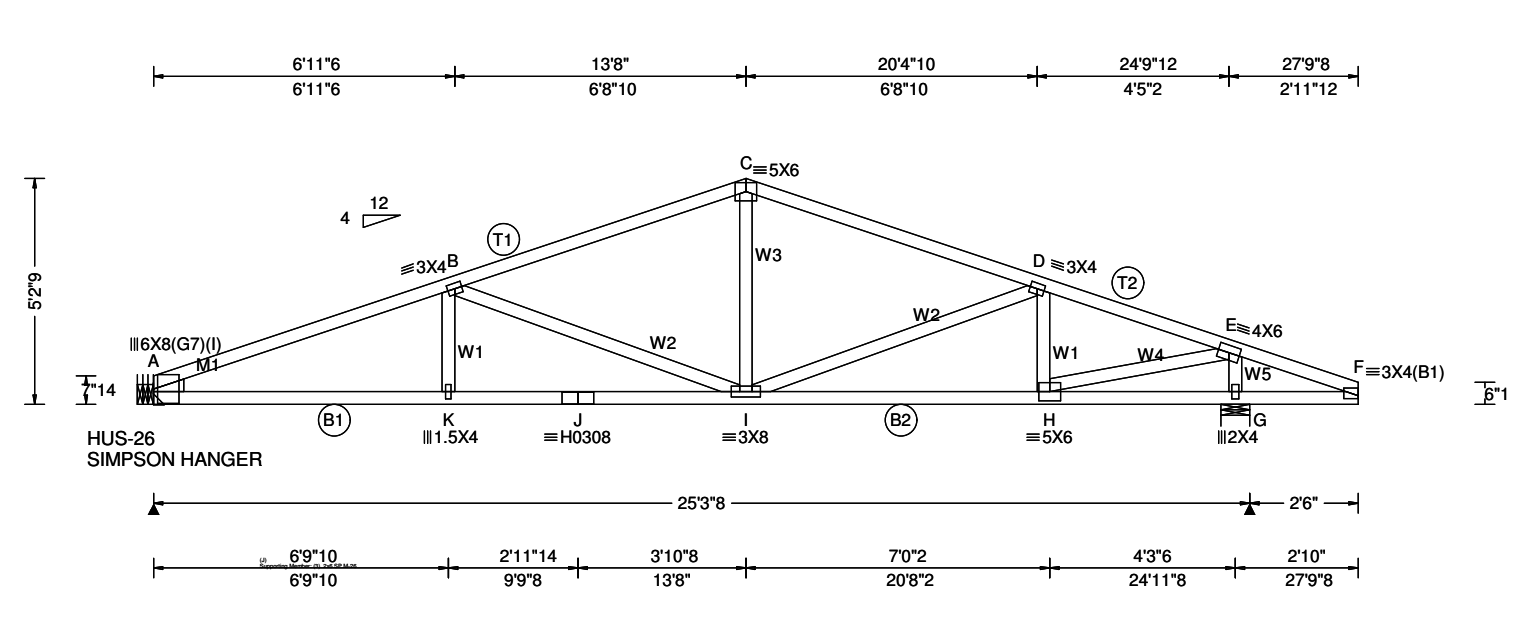
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)																																																																																																																								
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 175 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.46 ft TCDL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.084 M 999 360 VERT(CL): 0.158 E 999 240 HORZ(LL): 0.031 K - - HORZ(TL): 0.058 J - - Creep Factor: 2.0 Max TC CSI: 0.668 Max BC CSI: 0.667 Max Web CSI: 0.577 Mfg Specified Camber: VIEW Ver: 23.02.01A.1204.18	<table><tr><th colspan="2">Gravity</th><th colspan="3">Non-Gravity</th></tr><tr><th>Loc</th><th>R+ / R-</th><th>/ Rh</th><th>/ Rw</th><th>/ U / RL</th></tr><tr><td>Q</td><td>1276</td><td>- / -</td><td>- / -</td><td>/505 - / -</td></tr><tr><td>J</td><td>1470</td><td>- / -</td><td>- / -</td><td>/882 - / -</td></tr></table> Wind reactions based on MWFRS Q Brg Wid = 8.0 Min Req = 1.5 (Truss) J Brg Wid = 8.0 Min Req = 1.7 (Truss) Bearings Q & J are a rigid surface. <table><tr><th colspan="5">Maximum Top Chord Forces Per Ply (lbs)</th></tr><tr><th>Chords</th><th>Tens.</th><th>Comp.</th><th>Chords</th><th>Tens. Comp.</th></tr><tr><td>A - B</td><td>5</td><td>-3</td><td>E - F</td><td>1075 -2391</td></tr><tr><td>B - C</td><td>773</td><td>-1929</td><td>F - G</td><td>703 -1163</td></tr><tr><td>C - D</td><td>749</td><td>-1795</td><td>G - H</td><td>766 -1275</td></tr><tr><td>D - E</td><td>748</td><td>-1794</td><td>H - I</td><td>370 -138</td></tr></table> <table><tr><th colspan="5">Maximum Bot Chord Forces Per Ply (lbs)</th></tr><tr><th>Chords</th><th>Tens.</th><th>Comp.</th><th>Chords</th><th>Tens. Comp.</th></tr><tr><td>Q - P</td><td>0</td><td>0</td><td>M - L</td><td>2281 -1028</td></tr><tr><td>P - O</td><td>24</td><td>-10</td><td>L - K</td><td>2181 -1066</td></tr><tr><td>O - N</td><td>1824</td><td>-734</td><td>K - J</td><td>28 -235</td></tr><tr><td>N - M</td><td>1824</td><td>-734</td><td>J - I</td><td>103 -305</td></tr></table> <table><tr><th colspan="5">Maximum Web Forces Per Ply (lbs)</th></tr><tr><th>Webs</th><th>Tens.</th><th>Comp.</th><th>Webs</th><th>Tens. Comp.</th></tr><tr><td>B - P</td><td>517</td><td>-1237</td><td>E - L</td><td>119 -81</td></tr><tr><td>B - O</td><td>1828</td><td>-725</td><td>L - F</td><td>248 0</td></tr><tr><td>O - C</td><td>254</td><td>-383</td><td>F - K</td><td>481 -1391</td></tr><tr><td>C - M</td><td>76</td><td>-235</td><td>G - K</td><td>195 -5</td></tr><tr><td>D - M</td><td>495</td><td>-112</td><td>K - H</td><td>1381 -859</td></tr><tr><td>M - E</td><td>393</td><td>-712</td><td>H - J</td><td>888 -1364</td></tr></table>	Gravity		Non-Gravity			Loc	R+ / R-	/ Rh	/ Rw	/ U / RL	Q	1276	- / -	- / -	/505 - / -	J	1470	- / -	- / -	/882 - / -	Maximum Top Chord Forces Per Ply (lbs)					Chords	Tens.	Comp.	Chords	Tens. Comp.	A - B	5	-3	E - F	1075 -2391	B - C	773	-1929	F - G	703 -1163	C - D	749	-1795	G - H	766 -1275	D - E	748	-1794	H - I	370 -138	Maximum Bot Chord Forces Per Ply (lbs)					Chords	Tens.	Comp.	Chords	Tens. Comp.	Q - P	0	0	M - L	2281 -1028	P - O	24	-10	L - K	2181 -1066	O - N	1824	-734	K - J	28 -235	N - M	1824	-734	J - I	103 -305	Maximum Web Forces Per Ply (lbs)					Webs	Tens.	Comp.	Webs	Tens. Comp.	B - P	517	-1237	E - L	119 -81	B - O	1828	-725	L - F	248 0	O - C	254	-383	F - K	481 -1391	C - M	76	-235	G - K	195 -5	D - M	495	-112	K - H	1381 -859	M - E	393	-712	H - J	888 -1364
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Special Loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 90 plf at 0.00 to 90 plf at 26.00
BC: From 20 plf at 0.00 to 20 plf at 26.00
TC: -21 lb Conc. Load at 21.04
BC: -116 lb Conc. Load at 21.04

Wind
Wind loads and reactions based on MWFRS.
Left end vertical not exposed to wind pressure.
Right cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

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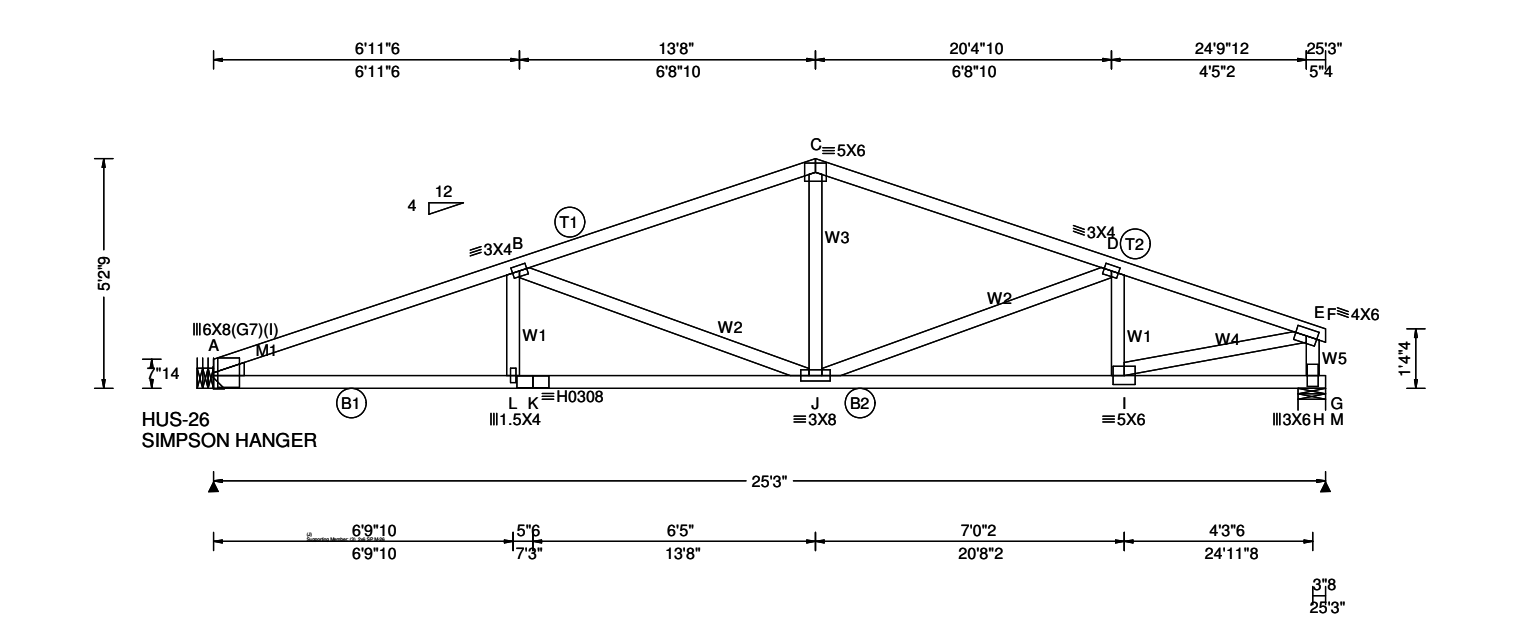


Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)								
TCLL:	30.00	Wind Std:	ASCE 7-22	Pg: NA	Ct: NA	CAT: NA	PP Deflection in	loc L/defl	L/#	Gravity			Non-Gravity					
TCDL:	15.00	Speed:	175 mph	Pf: NA		Ce: NA	VERT(LL):	0.138	B	999	360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL):	0.255	B	999	240	A	1365	/-	/-	/893	/443	/138
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA		HORZ(LL):	0.042	H	-	-	G	1702	/-	/-	/1071	/667	/-
		EXP:	C Kzt: NA				HORZ(TL):	0.080	G	-	-	Wind reactions based on C&C						
Des Ld:	55.00	Mean Height:	26.28 ft				Creep Factor:	2.0				A	Brg Wid = -			Min Req = -		
NCBCLL:	10.00	TCDL:	6.0 psf			Building Code:						G	Brg Wid = 8.0			Min Req = 2.0 (Truss)		
Soffit:	2.00	BCDL:	6.0 psf			FBC 8th Ed. 2023 Res.	HVHZ					Bearing G is a rigid surface.						
Load Duration:	1.25	MWFRS Parallel Dist:	h to 2h			TPI Std:	2014					Maximum Top Chord Forces Per Ply (lbs)						
Spacing:	24.0 "	C&C Dist a:	3.00 ft			Rep Fac:	Yes					Chords	Tens.Comp.	Chords	Tens.	Comp.		
		Loc. from endwall:	not in 9.00 ft			FT/RT:	20(0)/10(0)					A - B	986	- 2945	D - E	735	- 2070	
		GCpi:	0.18			Plate Type(s):						B - C	785	- 1983	E - F	361	- 230	
		Wind Duration:	1.60			WAVE, HS						VIEW Ver: 23.02.01A.1204.18						

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;
Lt Stub Wedge: 2x4 SP #2;

Plating Notes
(I) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.

Wind
Member design based on both MWFRS and C&C.
Right cantilever is exposed to wind
Wind loading based on both gable and hip roof types.



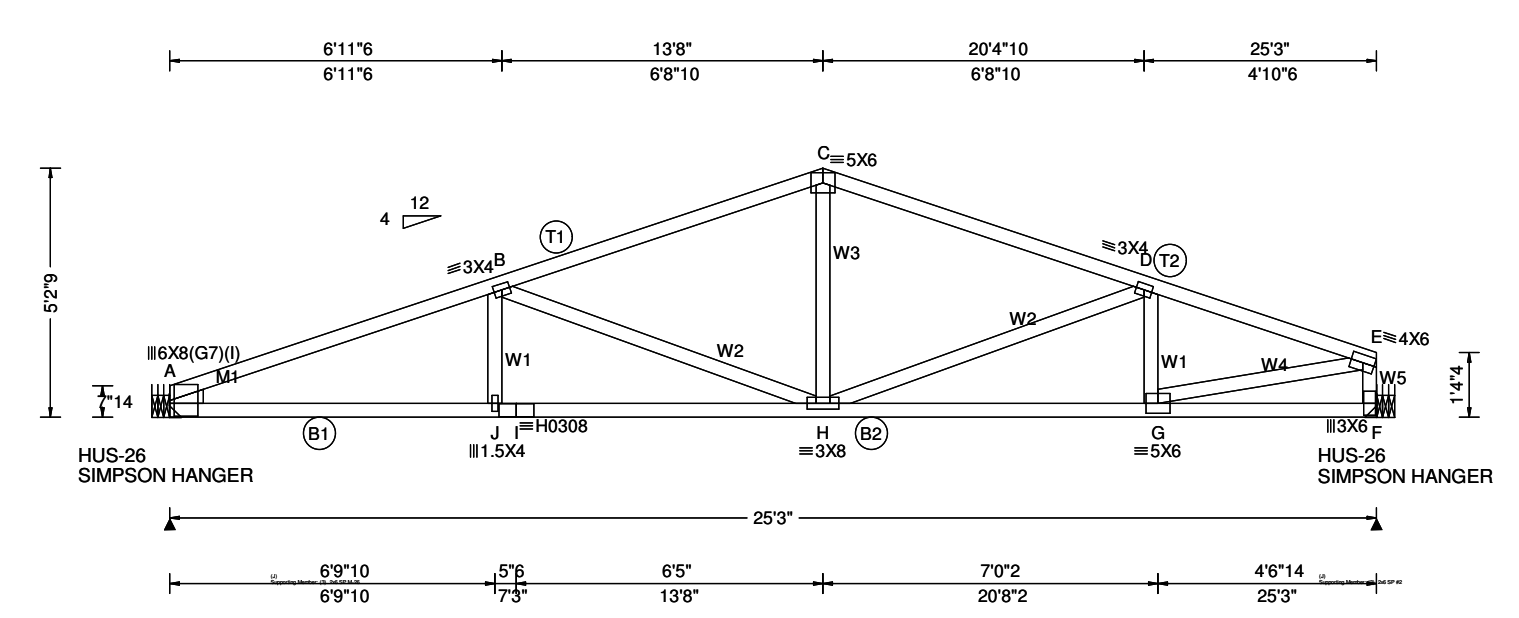
Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)								
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity					
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.143 B 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL		
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.263 B 999 240			A	1381	/-	/-	/898	/520	/147		
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.045 H - -			M	1397	/-	/-	/909	/535	/-		
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS			HORZ(TL): 0.082 H - -			Wind reactions based on C&C								
NCBCLL: 10.00		Mean Height: 26.35 ft					Creep Factor: 2.0			A Brg Wid = - Min Req = -								
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.787			M Brg Wid = 7.5 Min Req = 1.6 (Truss)								
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.821			Bearing H is a rigid surface.								
Spacing: 24.0 "		MWFRS Parallel Dist: h to 2h					Max Web CSI: 0.890			Maximum Top Chord Forces Per Ply (lbs)								
		C&C Dist a: 3.00 ft					Mfg Specified Camber:			Chords	Tens.Comp.	Chords	Tens.	Comp.				
		Loc. from endwall: not in 9.00 ft								A - B	1153	- 2988	D - E	876	- 2211			
		GCpi: 0.18								B - C	893	- 2029	E - F	4		0		
		Wind Duration: 1.60																

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2; Lt Stub Wedge: 2x4 SP #2;	Plating Notes (I) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.	Wind Member design based on both MWFRS and C&C. Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.
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Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
A - L	2727 - 1079	J - I	2087 - 809
L - K	2723 - 1083	I - H	30 - 14
K - J	2723 - 1083	H - G	0 - 0

Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.	Webs	Tens. Comp.
L - B	234 - 0	D - I	275 - 393
B - J	452 - 955	I - E	2082 - 796
C - J	608 - 140	E - H	551 - 1355
J - D	232 - 273		

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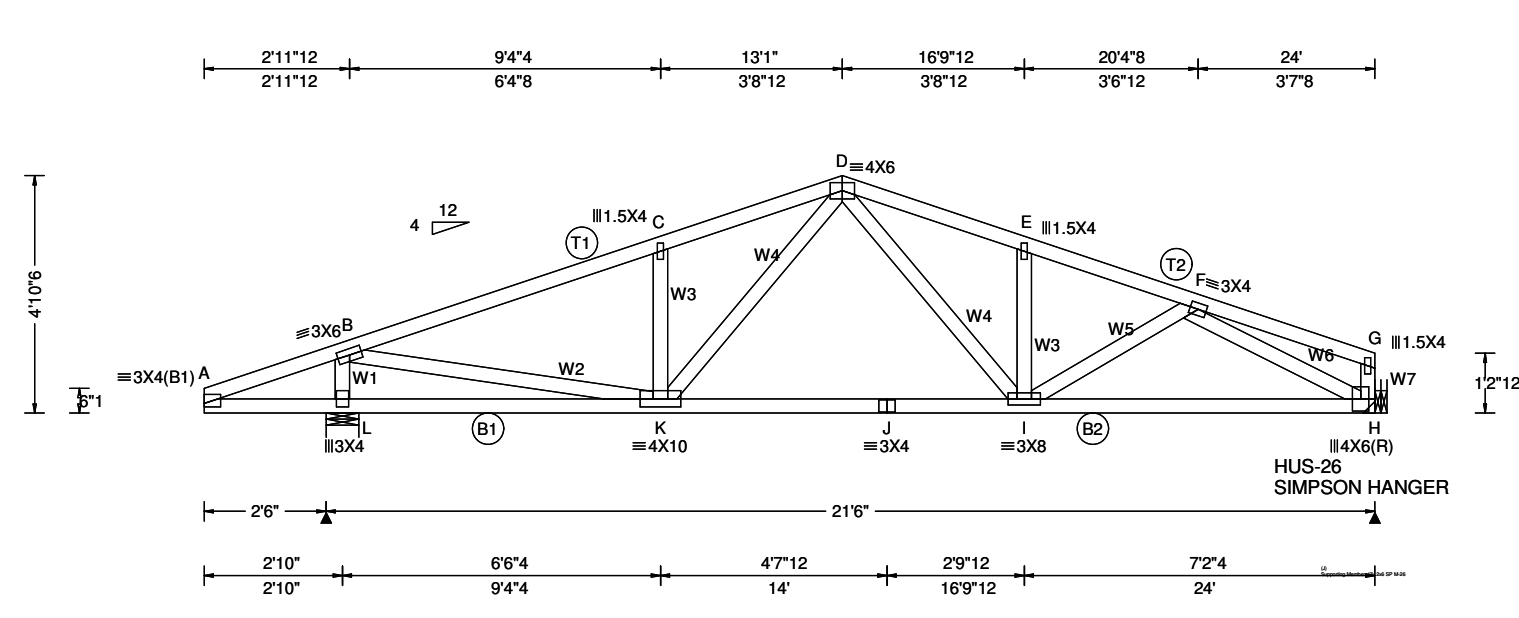
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity			Non-Gravity			
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): 0.146 B 999 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.268 B 999 240	A	1389	/-	/-	/903	/523	/147
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.045 F - -	F	1389	/-	/-	/904	/532	/-
	EXP: C Kzt: NA		HORZ(TL): 0.083 F - -	Wind reactions based on C&C						
Des Ld: 55.00	Mean Height: 26.35 ft	Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Creep Factor: 2.0	A Brg Wid = -		Min Req = -				
NCBCLL: 10.00	TCDL: 6.0 psf		Max TC CSI: 0.791	F Brg Wid = -		Min Req = -				
Soffit: 2.00	BCDL: 6.0 psf		Max BC CSI: 0.829	Maximum Top Chord Forces Per Ply (lbs)						
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h		Max Web CSI: 0.888	Chords	Tens.Comp.		Chords	Tens. Comp.		
Spacing: 24.0 "	C&C Dist a: 3.00 ft		Mfg Specified Camber:	A - B	1161 - 3010		C - D	900 - 2051		
	Loc. from endwall: not in 9.00 ft			B - C	902 - 2053		D - E	902 - 2283		
	GCpi: 0.18									
	Wind Duration: 1.60		VIEW Ver: 23.02.01A.1204.18							

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;
Lt Stub Wedge: 2x4 SP #2;

Plating Notes
(I) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.

Wind
Member design based on both MWFRS and C&C.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

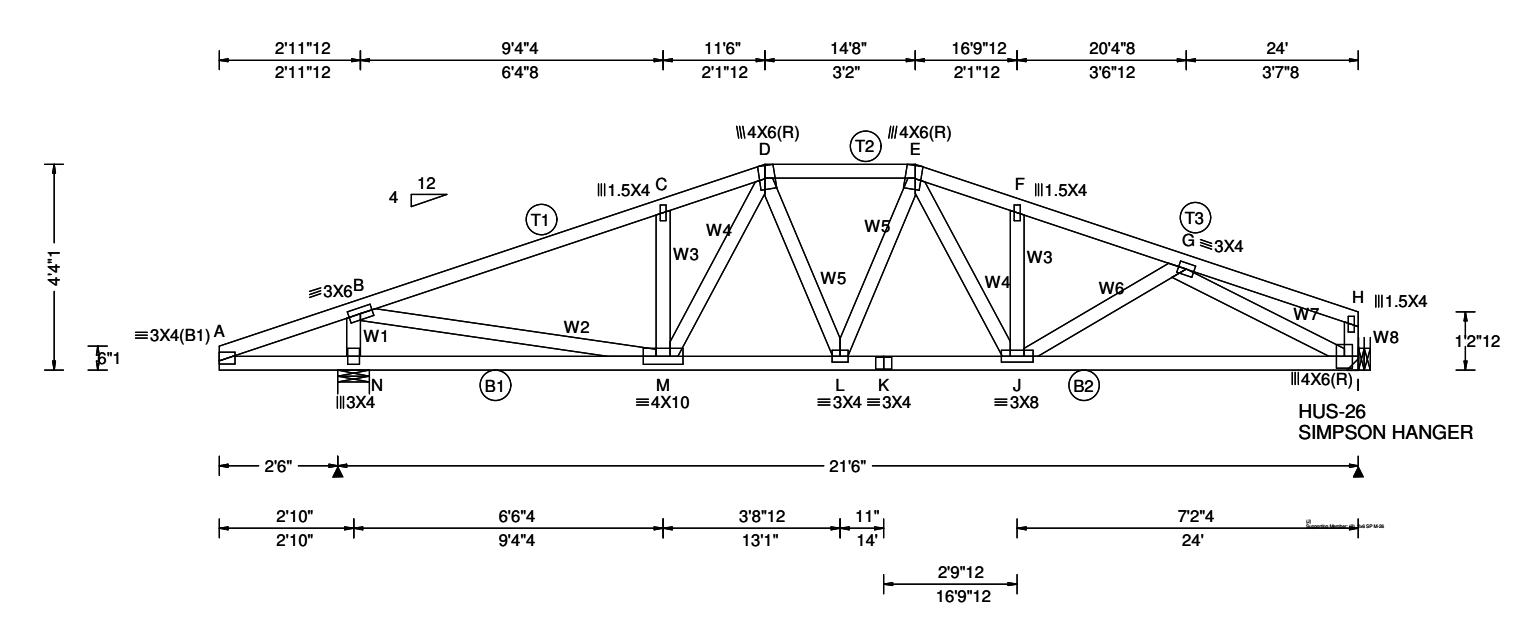
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria		▲ Maximum Reactions (lbs)							
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#		Gravity			Non-Gravity				
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.067 E 999 360		Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.123 E 999 240		L	1497	- / -	- / -	/ 936	/ 728	/ 142	
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.019 H - -		H	1155	- / -	- / -	/ 758	/ 475	- / -	
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.037 H - -		Wind reactions based on C&C							
NCBCLL: 10.00		Mean Height: 26.10 ft					Creep Factor: 2.0		L Brg Wid = 8.0 Min Req = 1.8 (Truss)							
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.695		H Brg Wid = - Min Req = -							
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.634		Bearing L is a rigid surface.							
Spacing: 24.0 "		MWFRS Parallel Dist: h/2 to h					Max Web CSI: 0.572		Maximum Top Chord Forces Per Ply (lbs)							
		C&C Dist a: 3.00 ft					Mfg Specified Camber:		Chords		Tens. Comp.		Chords		Tens. Comp.	
		Loc. from endwall: not in 9.00 ft							A - B		296 - 241		D - E		898 - 1845	
		GCpi: 0.18							B - C		786 - 1812		E - F		815 - 1844	
		Wind Duration: 1.60														

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2; Hangers / Ties (J) Hanger Support Required, by others Wind Member design based on both MWFRS and C&C. Right end vertical not exposed to wind pressure. Left cantilever is exposed to wind Wind loading based on both gable and hip roof types.	
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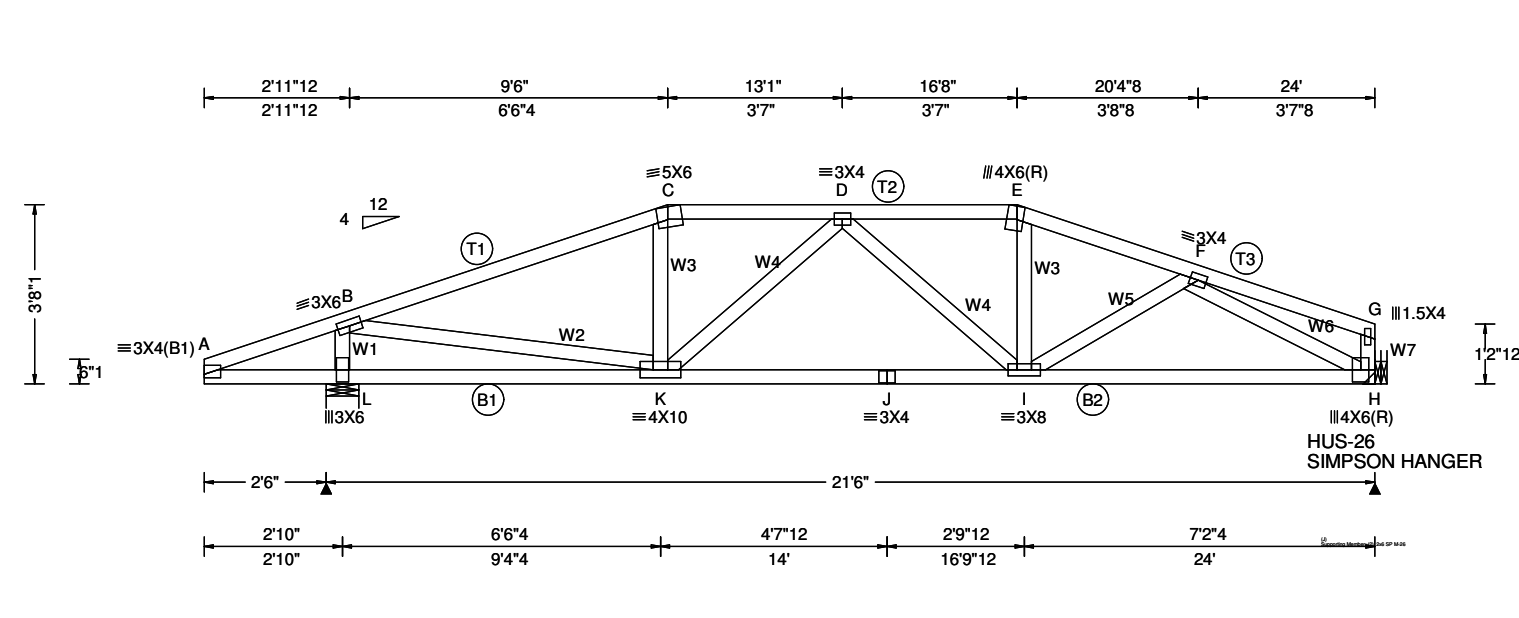


Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)								
TCLL:	30.00	Wind Std:	ASCE 7-22	Pg: NA	Ct: NA	CAT: NA	PP Deflection in	loc L/defl	L/#	Gravity			Non-Gravity					
TCDL:	15.00	Speed:	175 mph	Pf: NA		Ce: NA	VERT(LL):	0.065	L	999	360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL):	0.120	L	999	240	N	1497	-	-	/767	/863	/127
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA		HORZ(LL):	0.020	I	-	-	I	1155	-	-	/544	/687	-
		EXP:	C Kzt: NA				HORZ(TL):	0.039	I	-	-	Wind reactions based on C&C						
Des Ld:	55.00	Mean Height:	25.84 ft			Building Code:	Creep Factor:	2.0				N	Brg Wid = 8.0	Min Req = 1.8	(Truss)			
NCBCLL:	10.00	TCDL:	6.0 psf			FBC 8th Ed. 2023 Res. HVHZ	Max TC CSI:	0.677				I	Brg Wid = -	Min Req = -				
Soffit:	2.00	BCDL:	6.0 psf			TPI Std: 2014	Max BC CSI:	0.614				Bearing N is a rigid surface.						
Load Duration:	1.25	MWFRS Parallel Dist:	0 to h/2			Rep Fac: Yes	Max Web CSI:	0.689				Maximum Top Chord Forces Per Ply (lbs)						
Spacing:	24.0 "	C&C Dist a:	3.00 ft			FT/RT:20(0)/10(0)	Mfg Specified Camber:					Chords	Tens.Comp.	Chords	Tens.	Comp.		
		Loc. from endwall:	not in 9.00 ft			Plate Type(s):						A - B	288	-235	E - F	1216	-1800	
		GCpi:	0.18			WAVE						B - C	1088	-1802	F - G	1133	-1826	
		Wind Duration:	1.60															

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Hangers / Ties
(J) Hanger Support Required, by others

Wind
Member design based on both MWFRS and C&C.
Right end vertical not exposed to wind pressure.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.



Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)						
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity			
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.073 D 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.134 D 999 240			L	1497	-	-	/790	/1032	/107
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.022 H - -			H	1155	-	-	/543	/778	-
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.043 H - -			Wind reactions based on C&C						
NCBCLL: 10.00		Mean Height: 25.51 ft					Creep Factor: 2.0			L Brg Wid = 8.0 Min Req = 1.8 (Truss)						
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.795			H Brg Wid = - Min Req = -						
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.645			Bearing L is a rigid surface.						
Spacing: 24.0 "		MWFRS Parallel Dist: 0 to h/2					Max Web CSI: 0.909			Maximum Top Chord Forces Per Ply (lbs)						
		C&C Dist a: 3.00 ft					Mfg Specified Camber:			Chords	Tens.Comp.		Chords	Tens. Comp.		
		Loc. from endwall: not in 9.00 ft					VIEW Ver: 23.02.01A.1204.18			A - B	291	-225	D - E	1457	-1677	
		GCpi: 0.18								B - C	1440	-1792	E - F	1485	-1833	
		Wind Duration: 1.60														

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2;	Hangers / Ties (J) Hanger Support Required, by others	Wind Member design based on both MWFRS and C&C. Right end vertical not exposed to wind pressure. Left cantilever is exposed to wind Wind loading based on both gable and hip roof types.
Maximum Bot Chord Forces Per Ply (lbs)		
Chords	Tens.Comp.	Chords Tens. Comp.
A - L	215 -224	J - I 1791 -1551
L - K	219 -203	I - H 1628 -1223
K - J	1791 -1551	
Maximum Web Forces Per Ply (lbs)		
Webs	Tens.Comp.	Webs Tens. Comp.
L - B	1027 -1378	E - I 253 -115
B - K	1534 -1211	I - F 189 -281
K - C	178 -88	F - H 1413 -1843
K - D	461 -272	G - H 129 -154
D - I	415 -154	

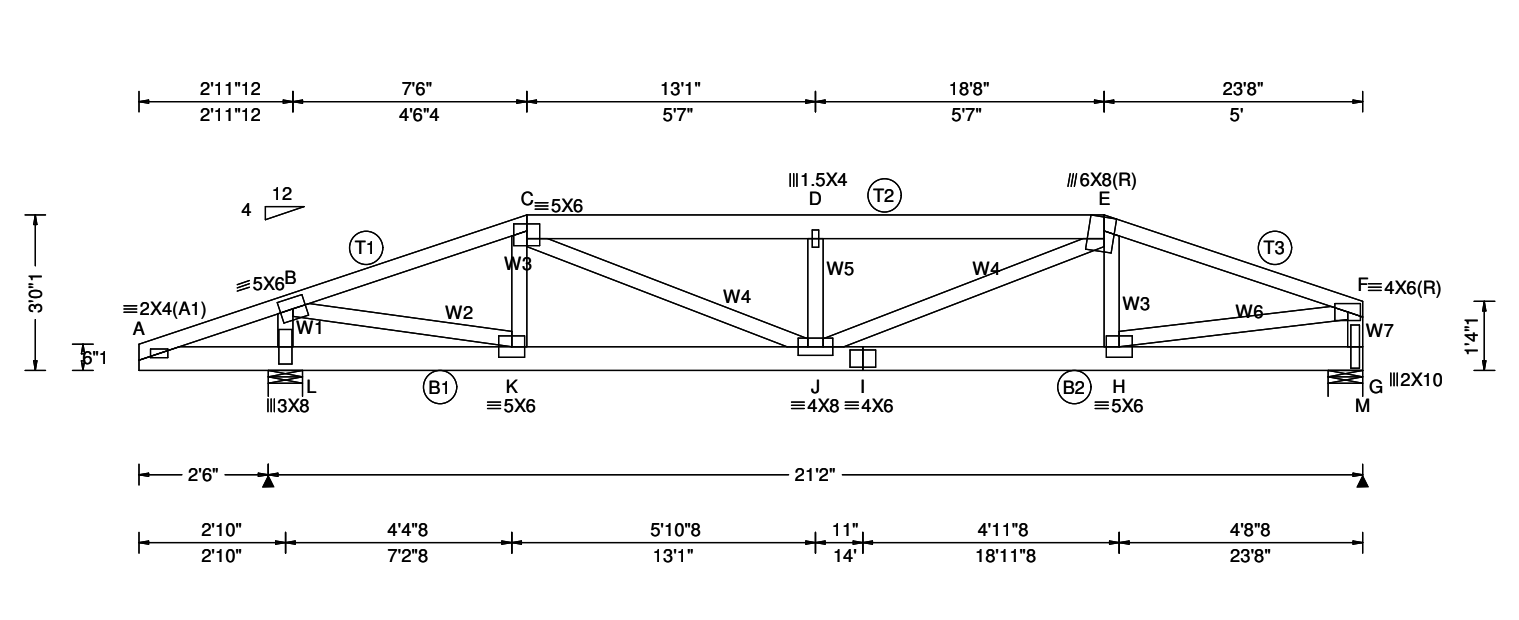
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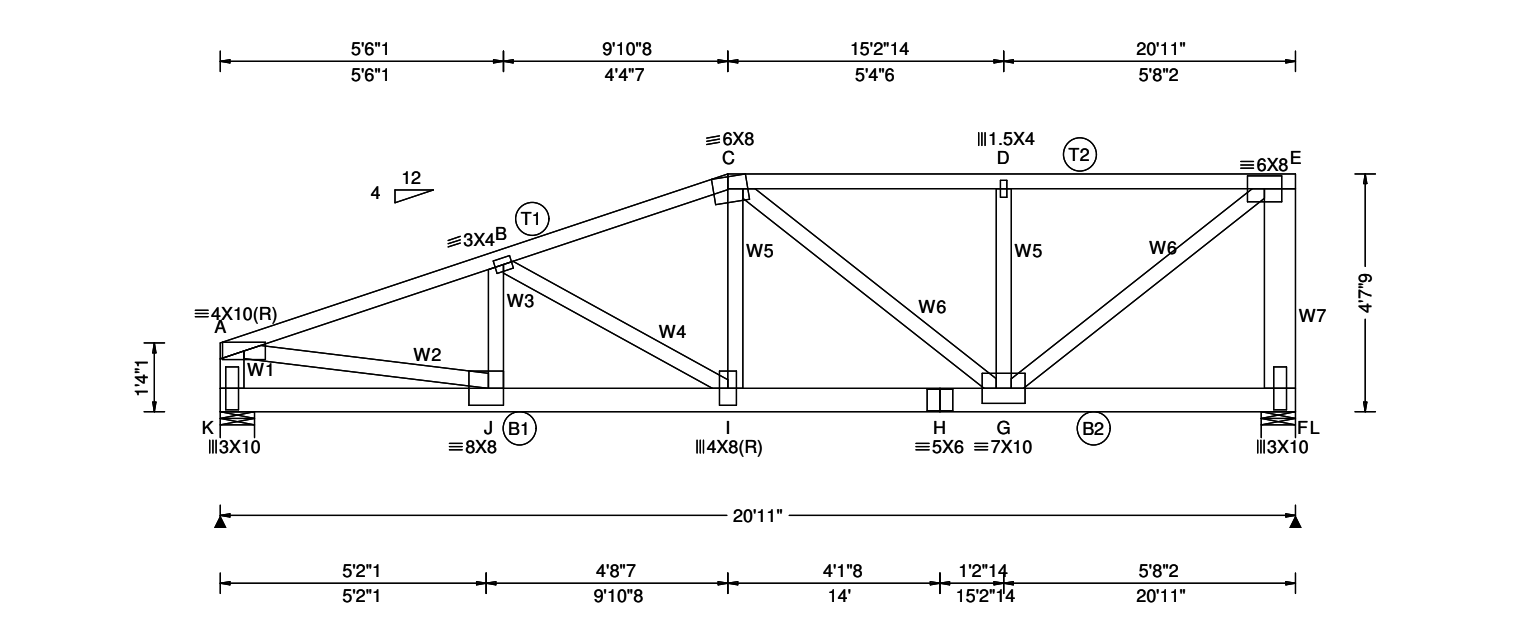
Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)							
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity				
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): -0.125 D 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.224 D 999 240			L	1951	- / -	- / -	- / -	1267	- / -	
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): -0.022 C - -			M	1582	- / -	- / -	- / -	1045	- / -	
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.038 C - -			Wind reactions based on MWFRS							
NCBCLL: 10.00		Mean Height: 25.17 ft					Creep Factor: 2.0			L Brg Wid = 8.0 Min Req = 2.3 (Truss)							
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.733			M Brg Wid = 8.0 Min Req = 1.9 (Truss)							
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.803			Bearings L & M are a rigid surface.							
Spacing: 24.0 "		MWFRS Parallel Dist: 0 to h/2					Max Web CSI: 0.673			Maximum Top Chord Forces Per Ply (lbs)							
		C&C Dist a: 3.00 ft					Mfg Specified Camber:			Chords		Tens.Comp.		Chords		Tens. Comp.	
		Loc. from endwall: not in 4.50 ft								A - B	333	- 135	D - E	2311	- 3592		
		GCpi: 0.18								B - C	1725	- 2549	E - F	1880	- 2797		
		Wind Duration: 1.60															

Lumber
Top chord: 2x4 SP #2; T2 2x6 SP #2;
Bot chord: 2x6 SP #2;
Webs: 2x4 SP #2;

Special Loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 90 plf at 0.00 to 90 plf at 7.50
TC: From 45 plf at 7.50 to 45 plf at 18.67
TC: From 90 plf at 18.67 to 90 plf at 23.67
BC: From 20 plf at 0.00 to 20 plf at 7.46
BC: From 10 plf at 7.46 to 10 plf at 18.71
BC: From 20 plf at 18.71 to 20 plf at 23.67
TC: 132 lb Conc. Load at 8.09,10.09,12.09,14.08
16.08,18.08
BC: 124 lb Conc. Load at 7.46,18.71
BC: 84 lb Conc. Load at 8.09,10.09,12.09,14.08
16.08,18.08

Wind
Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)				
	Wind Std: ASCE 7-22 Speed: 175 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 26.40 ft TCDL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA	PP Deflection in loc L/defl L/# VERT(LL): 0.134 I 999 360 VERT(CL): 0.244 I 999 240 HORZ(LL): -0.027 E - - HORZ(TL): 0.049 E - - Creep Factor: 2.0 Max TC CSI: 0.461 Max BC CSI: 0.383 Max Web CSI: 0.927 Mfg Specified Camber: VIEW Ver: 23.02.01A.1204.18	Gravity		Non-Gravity		
				Loc	R+ / R- / Rh	/ Rw / U / RL		
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "				K 7620	- / -	- / -	- / -	/2855 - / -
				L 8180	- / -	- / -	- / -	/3158 - / -
				Wind reactions based on MWFRS K Brg Wid = 8.0 Min Req = 2.1 (Truss) L Brg Wid = 8.0 Min Req = 2.3 (Truss) Bearings K & L are a rigid surface.				
				Maximum Top Chord Forces Per Ply (lbs)				
				Chords	Tens.Comp.	Chords	Tens.	Comp.
				A - B	1684 -4507	C - D	1167	-2986
				B - C	1506 -3950	D - E	1167	-2986

Lumber Top chord: 2x4 SP #2; Bot chord: 2x6 SP M-26; Webs: 2x4 SP #2; W1 2x6 SP #2; W7 2x8 SP #2; Nailnote Nail Schedule:0.128"x3", min. nails Top Chord: 1 Row @12.00" o.c. Bot Chord: 2 Rows @ 4.50" o.c. (Each Row) Webs : 1 Row @ 4" o.c. Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting. Special Loads ----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC: From 90 plf at 0.00 to 90 plf at 20.92 BC: From 20 plf at 0.00 to 20 plf at 2.50 BC: From 10 plf at 2.50 to 10 plf at 20.92 BC: 1389 lb Conc. Load at 2.50, 4.50, 6.50 BC: 1381 lb Conc. Load at 8.50 BC: 1365 lb Conc. Load at 10.50,11.44,13.44,15.44 BC: 1339 lb Conc. Load at 17.44,19.44 Wind Wind loads and reactions based on MWFRS. End verticals not exposed to wind pressure. Wind loading based on both gable and hip roof types.	
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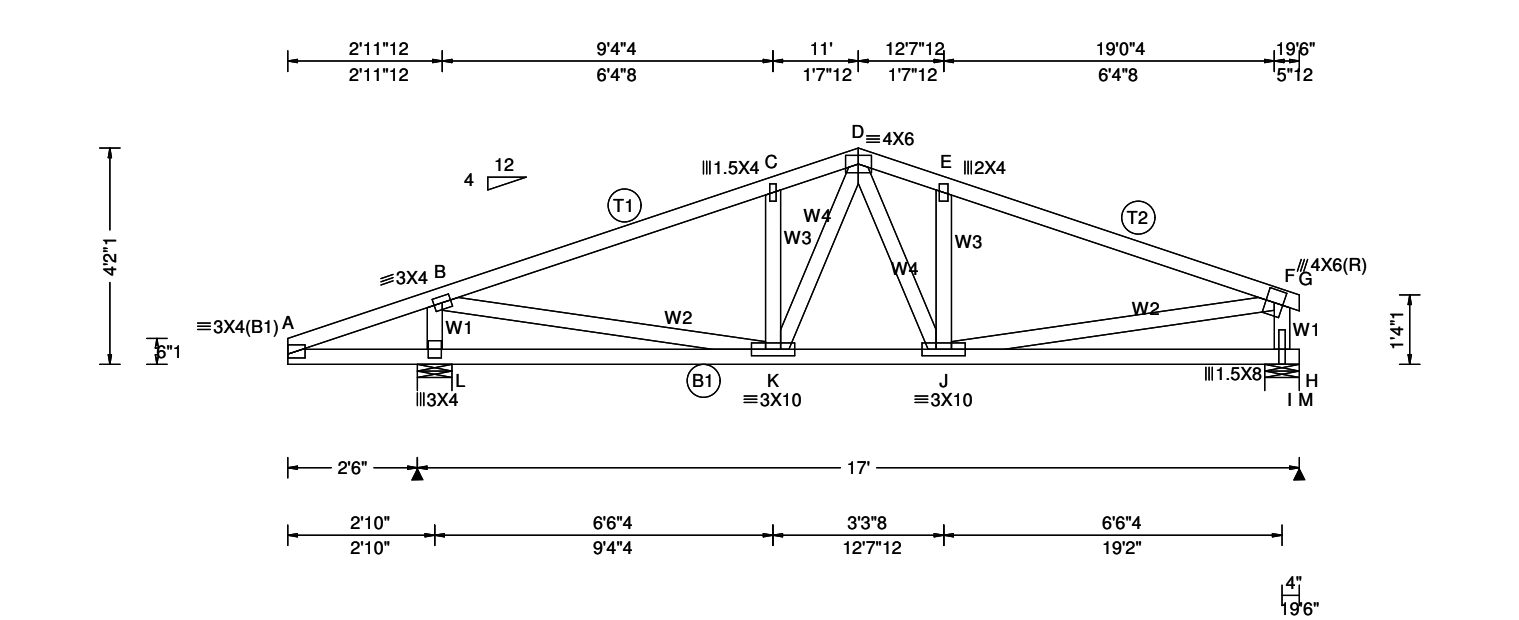
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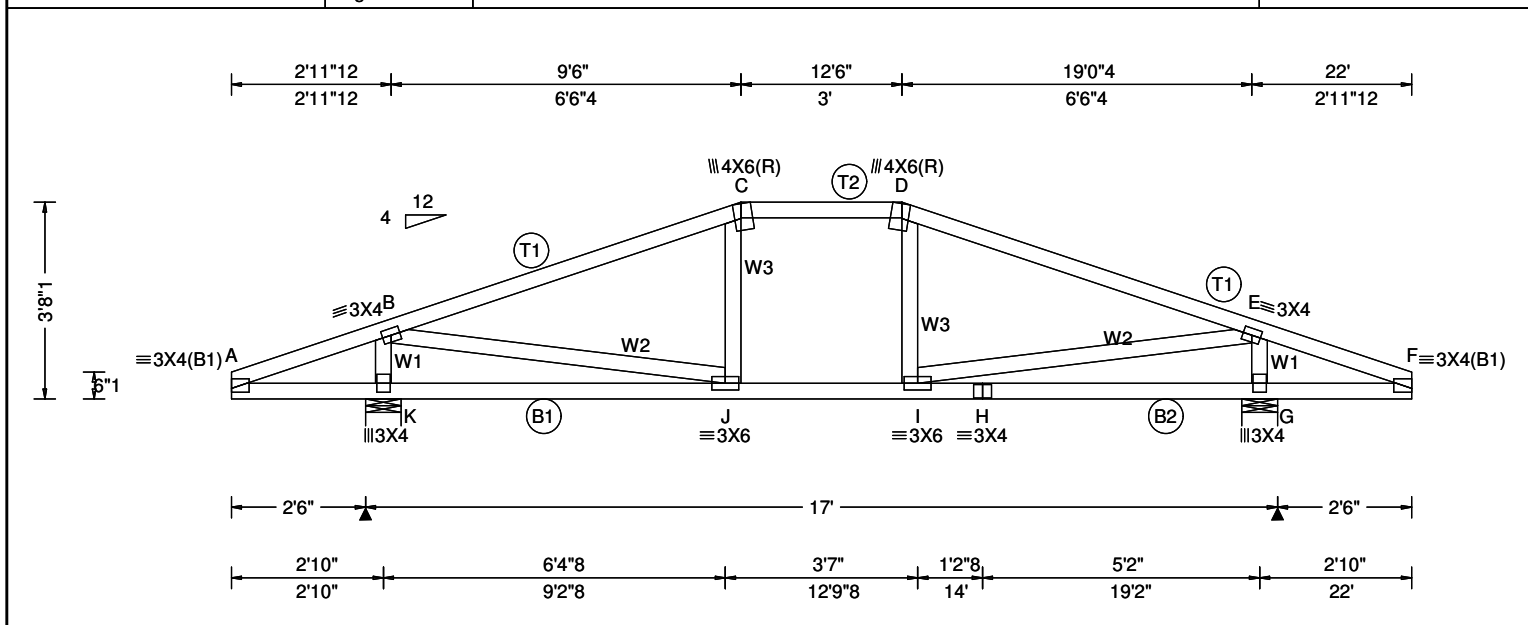
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TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity				
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.039 E 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.072 E 999 240			L	1245	/-	/-	/771	/797	/124	
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): -0.010 G - -			M	915	/-	/-	/604	/540	/-	
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.017 G - -			Wind reactions based on C&C							
NCBCLL: 10.00		Mean Height: 25.76 ft					Creep Factor: 2.0			L Brg Wid = 8.0 Min Req = 1.5 (Truss)							
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.784			M Brg Wid = 8.0 Min Req = 1.5 (Truss)							
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.413			Bearings L & I are a rigid surface.							
Spacing: 24.0 "		MWFRS Parallel Dist: 0 to h/2					Max Web CSI: 0.474			Maximum Top Chord Forces Per Ply (lbs)							
		C&C Dist a: 3.00 ft					Mfg Specified Camber:			Chords		Tens.Comp.		Chords		Tens. Comp.	
		Loc. from endwall: not in 9.00 ft								A - B		290 -268		D - E		988 -1328	
		GCpi: 0.18								B - C		770 -1278		E - F		794 -1327	
		Wind Duration: 1.60															

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Wind
Member design based on both MWFRS and C&C.
Right end vertical not exposed to wind pressure.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

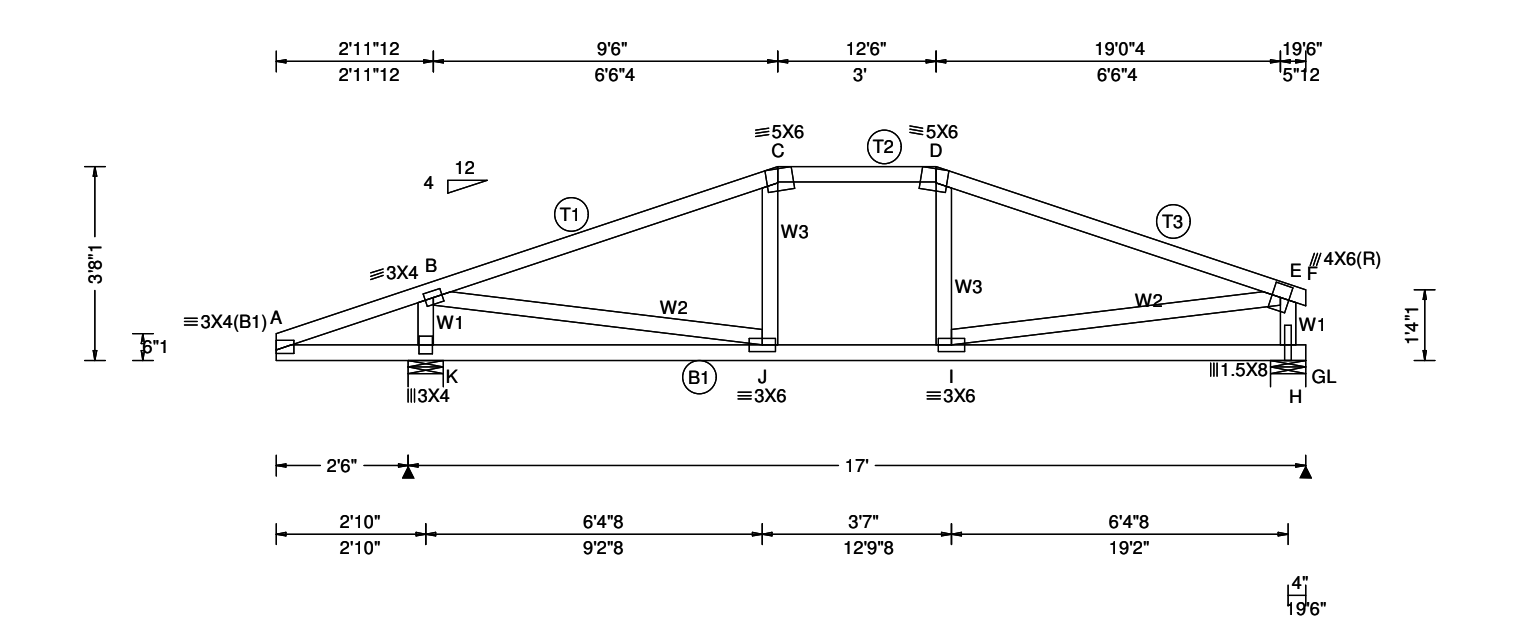
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SEQN: 201272 / T62 / HIPS FROM: AE	Ply: 1 Qty: 1 Wgt: 105.0 lbs	Job Number: 23081008 5900 RES. Truss Label: 8H10	DRW: ... / ... 07/25/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)																												
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	<table><thead><tr><th colspan="4">Gravity</th><th colspan="3">Non-Gravity</th></tr><tr><th>Loc</th><th>R+</th><th>/ R-</th><th>/ Rh</th><th>/ Rw</th><th>/ U</th><th>/ RL</th></tr></thead><tbody><tr><td>K</td><td>1225</td><td>/-</td><td>/-</td><td>/660</td><td>/812</td><td>/91</td></tr><tr><td>G</td><td>1225</td><td>/-</td><td>/-</td><td>/660</td><td>/812</td><td>/-</td></tr></tbody></table>	Gravity				Non-Gravity			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	K	1225	/-	/-	/660	/812	/91	G	1225	/-	/-	/660	/812	/-
Gravity				Non-Gravity																												
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL																										
K	1225	/-	/-	/660	/812	/91																										
G	1225	/-	/-	/660	/812	/-																										
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): 0.064 D 999 360	Wind reactions based on C&C K Brg Wid = 8.0 Min Req = 1.5 (Truss) G Brg Wid = 8.0 Min Req = 1.5 (Truss) Bearings K & G are a rigid surface.																												
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.143 D 999 240																													
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.025 C - -																													
Des Ld: 55.00	EXP: C Kzt: NA		HORZ(TL): 0.055 C - -	Maximum Top Chord Forces Per Ply (lbs)																												
NCBCLL: 10.00	Mean Height: 25.51 ft	Building Code:	Creep Factor: 2.0	<table><thead><tr><th>Chords</th><th>Tens.Comp.</th><th>Chords</th><th>Tens. Comp.</th></tr></thead><tbody><tr><td>A - B</td><td>329 -267</td><td>D - E</td><td>838 -1166</td></tr><tr><td>B - C</td><td>839 -1166</td><td>E - F</td><td>328 -268</td></tr><tr><td>C - D</td><td>877 -1019</td><td></td><td></td></tr></tbody></table>	Chords	Tens.Comp.	Chords	Tens. Comp.	A - B	329 -267	D - E	838 -1166	B - C	839 -1166	E - F	328 -268	C - D	877 -1019														
Chords	Tens.Comp.	Chords	Tens. Comp.																													
A - B	329 -267	D - E	838 -1166																													
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C - D	877 -1019																															
Soffit: 2.00	TCDL: 6.0 psf	FBC 8th Ed. 2023 Res. HVHZ	Max TC CSI: 0.846	Maximum Bot Chord Forces Per Ply (lbs)																												
Load Duration: 1.25	BCDL: 6.0 psf	TPI Std: 2014	Max BC CSI: 0.455		<table><thead><tr><th>Chords</th><th>Tens.Comp.</th><th>Chords</th><th>Tens. Comp.</th></tr></thead><tbody><tr><td>A - K</td><td>256 -267</td><td>I - H</td><td>208 -201</td></tr><tr><td>K - J</td><td>233 -201</td><td>H - G</td><td>208 -201</td></tr><tr><td>J - I</td><td>1019 -648</td><td>G - F</td><td>256 -267</td></tr></tbody></table>	Chords	Tens.Comp.	Chords	Tens. Comp.	A - K	256 -267	I - H	208 -201	K - J	233 -201	H - G	208 -201	J - I	1019 -648	G - F	256 -267											
Chords	Tens.Comp.	Chords	Tens. Comp.																													
A - K	256 -267	I - H	208 -201																													
K - J	233 -201	H - G	208 -201																													
J - I	1019 -648	G - F	256 -267																													
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.576	Maximum Web Forces Per Ply (lbs)																												
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	Mfg Specified Camber:		<table><thead><tr><th>Webs</th><th>Tens.Comp.</th><th>Webs</th><th>Tens. Comp.</th></tr></thead><tbody><tr><td>K - B</td><td>827 -1106</td><td>D - I</td><td>190 -123</td></tr><tr><td>B - J</td><td>1014 -767</td><td>I - E</td><td>1013 -767</td></tr><tr><td>J - C</td><td>190 -124</td><td>E - G</td><td>827 -1106</td></tr></tbody></table>	Webs	Tens.Comp.	Webs	Tens. Comp.	K - B	827 -1106	D - I	190 -123	B - J	1014 -767	I - E	1013 -767	J - C	190 -124	E - G	827 -1106											
Webs	Tens.Comp.	Webs	Tens. Comp.																													
K - B	827 -1106	D - I	190 -123																													
B - J	1014 -767	I - E	1013 -767																													
J - C	190 -124	E - G	827 -1106																													
	Loc. from endwall: not in 9.00 ft	Plate Type(s):																														
	GCpi: 0.18																															
	Wind Duration: 1.60	WAVE	VIEW Ver: 23.02.01A.1204.18																													
Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2;																																
Wind Member design based on both MWFRS and C&C. Left and right cantilevers are exposed to wind Wind loading based on both gable and hip roof types.																																

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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)							
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity				
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.089 D 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.206 D 957 240			K	1245	-	-	/666	/920	/110	
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): -0.035 D - -			L	915	-	-	/429	/628	-	
Des Ld: 55.00		EXP: C Kzt: NA					HORZ(TL): 0.082 D - -			Wind reactions based on C&C							
NCBCLL: 10.00		Mean Height: 25.51 ft		Building Code:			Creep Factor: 2.0			K Brg Wid = 8.0 Min Req = 1.5 (Truss)							
Soffit: 2.00		TCDL: 6.0 psf		FBC 8th Ed. 2023 Res. HVHZ			Max TC CSI: 0.881			L Brg Wid = 8.0 Min Req = 1.5 (Truss)							
Load Duration: 1.25		BCDL: 6.0 psf		TPI Std: 2014			Max BC CSI: 0.593			Bearings K & H are a rigid surface.							
Spacing: 24.0 "		MWFRS Parallel Dist: h/2 to h		Rep Fac: Yes			Max Web CSI: 0.670			Maximum Top Chord Forces Per Ply (lbs)							
		C&C Dist a: 3.00 ft		FT/RT:20(0)/10(0)			Mfg Specified Camber:			Chords		Tens.Comp.		Chords		Tens. Comp.	
		Loc. from endwall: not in 9.00 ft		Plate Type(s):			VIEW Ver: 23.02.01A.1204.18			A - B		358 -289		D - E		992 -1259	
		GCpi: 0.18		WAVE						B - C		985 -1267		E - F		5 0	
		Wind Duration: 1.60															

Lumber				Maximum Bot Chord Forces Per Ply (lbs)			
Top chord: 2x4 SP #2; T3 2x4 SP #1; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2;				Chords	Tens.Comp.		Tens. Comp.
Wind				A - K	278	-296	I - H 115 -75
Member design based on both MWFRS and C&C.				K - J	218	-227	H - G 0 0
Right end vertical not exposed to wind pressure.				J - I	1115	-858	
Left cantilever is exposed to wind				Maximum Web Forces Per Ply (lbs)			
Wind loading based on both gable and hip roof types.				Webs	Tens.Comp.		Tens. Comp.
				K - B	944	-1138	D - I 247 -133
				B - J	1093	-892	I - E 1025 -799
				J - C	187	-117	E - H 658 -844

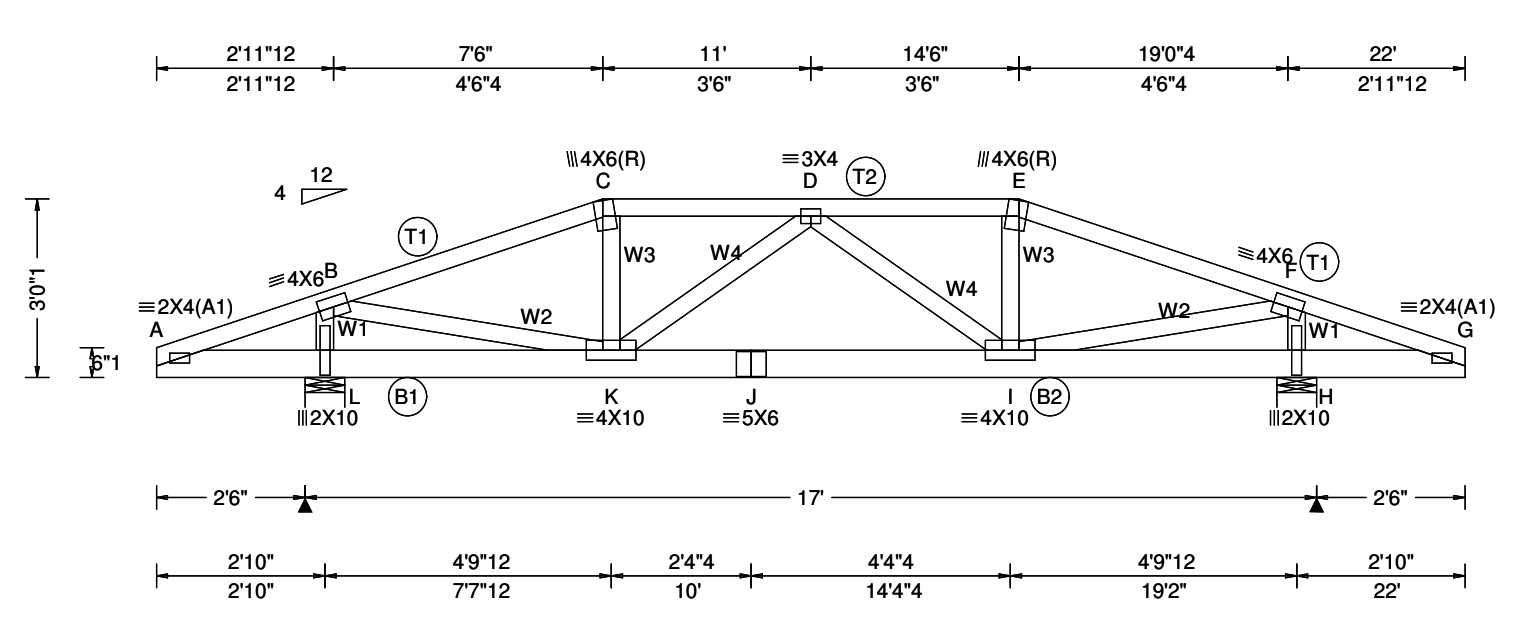
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
				Loc	R+	R- / Rh	Rw / U	/ RL		
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	L	1573	-	-	-	/1048	-
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): -0.058 D 999 360	H	1573	-	-	-	/1048	-
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.097 D 999 240	Wind reactions based on MWFRS						
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.010 C - -	L	Brg Wid = 8.0 Min Req = 1.9 (Truss)					
Des Ld: 55.00	EXP: C Kzt: NA	Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	HORZ(TL): 0.016 C - -	H	Brg Wid = 8.0 Min Req = 1.9 (Truss)					
NCBCLL: 10.00	Mean Height: 25.17 ft		Creep Factor: 2.0	Bearings L & H are a rigid surface.						
Soffit: 2.00	BCDL: 6.0 psf		Max TC CSI: 0.484	Maximum Top Chord Forces Per Ply (lbs)						
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2		Max BC CSI: 0.791	Chords			Tens. Comp.			
Spacing: 24.0 "	C&C Dist a: 3.00 ft		Max Web CSI: 0.518	A - B	332	-150	D - E	1242	-1772	
	Loc. from endwall: not in 4.50 ft		Mfg Specified Camber:	B - C	1360	-1925	E - F	1359	-1923	
	GCpi: 0.18			C - D	1243	-1774	F - G	332	-149	
	Wind Duration: 1.60		VIEW Ver: 23.02.01A.1204.18	Maximum Bot Chord Forces Per Ply (lbs)						
				Chords			Tens. Comp.			
				A - L	96	-271	J - I	1854	-1309	
				L - K	61	-222	I - H	61	-222	
				K - J	1854	-1309	H - G	95	-271	
				Maximum Web Forces Per Ply (lbs)						
				Webs			Tens. Comp.			
				L - B	1080	-1501	D - I	85	-124	
				B - K	1859	-1264	I - E	311	-181	
				C - K	312	-181	I - F	1856	-1262	
				K - D	83	-124	F - H	1079	-1499	

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x6 SP #2;
Webs: 2x4 SP #2;

Special Loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 90 plf at 0.00 to 90 plf at 7.50
TC: From 45 plf at 7.50 to 45 plf at 14.50
TC: From 90 plf at 14.50 to 90 plf at 22.00
BC: From 20 plf at 0.00 to 20 plf at 7.46
BC: From 10 plf at 7.46 to 10 plf at 14.54
BC: From 20 plf at 14.54 to 20 plf at 22.00
TC: 132 lb Conc. Load at 8.09,10.09,11.91,13.91
BC: 124 lb Conc. Load at 7.46,14.54
BC: 84 lb Conc. Load at 8.09,10.09,11.91,13.91

Wind
Wind loads and reactions based on MWFRS.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

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Maximum Reactions (lbs)

Gravity			Non-Gravity		
Loc	R+	R- / Rh	Rw / U	/ RL	
L	1573	-	-	-	/1048 -
H	1573	-	-	-	/1048 -

Wind reactions based on MWFRS
L Brg Wid = 8.0 Min Req = 1.9 (Truss)
H Brg Wid = 8.0 Min Req = 1.9 (Truss)
Bearings L & H are a rigid surface.

Maximum Top Chord Forces Per Ply (lbs)

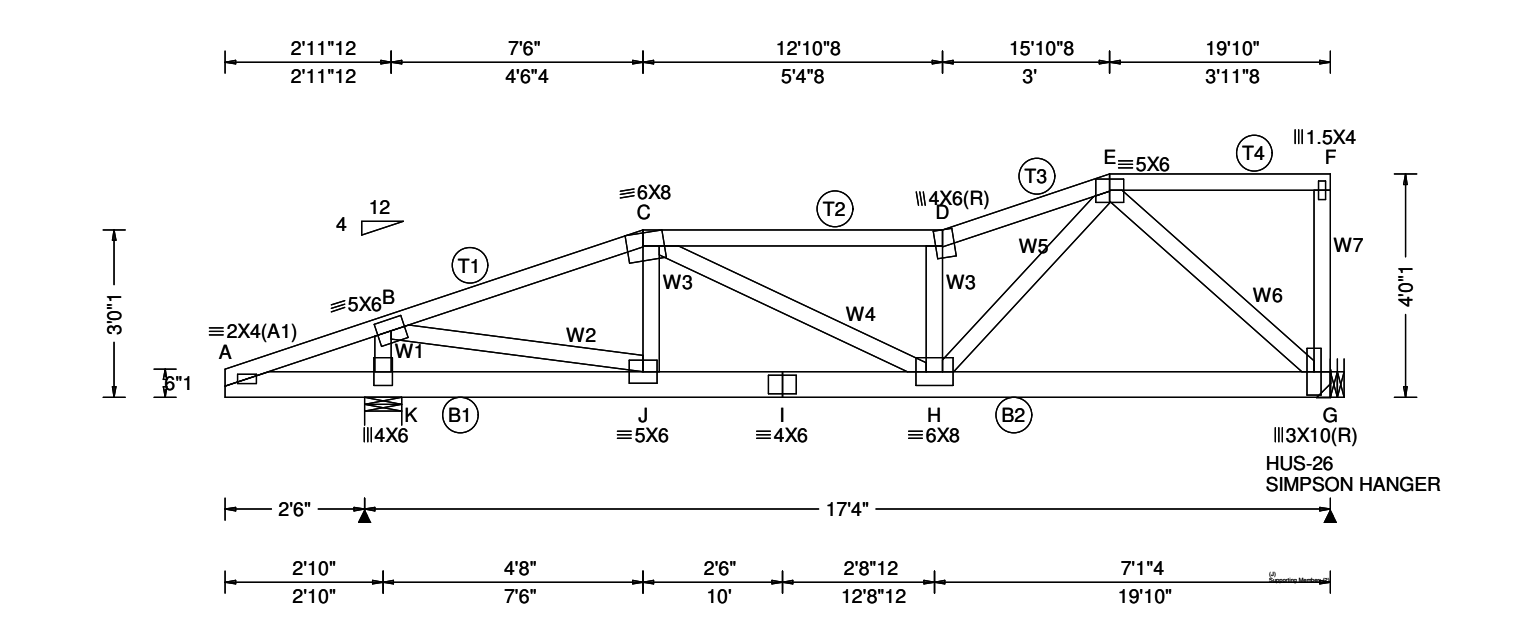
Chords	Tens. Comp.	Chords	Tens. Comp.
A - B	332 -150	D - E	1242 -1772
B - C	1360 -1925	E - F	1359 -1923
C - D	1243 -1774	F - G	332 -149

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens. Comp.	Chords	Tens. Comp.
A - L	96 -271	J - I	1854 -1309
L - K	61 -222	I - H	61 -222
K - J	1854 -1309	H - G	95 -271

Maximum Web Forces Per Ply (lbs)

Webs	Tens. Comp.	Webs	Tens. Comp.
L - B	1080 -1501	D - I	85 -124
B - K	1859 -1264	I - E	311 -181
C - K	312 -181	I - F	1856 -1262
K - D	83 -124	F - H	1079 -1499



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity			Non-Gravity			
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): 0.074 D 999 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.136 D 999 240	K	1973	-/-	-/-	-/-	/1153	-/-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.017 F - -	G	1196	-/-	-/-	-/-	/701	-/-
	EXP: C Kzt: NA		HORZ(TL): 0.030 F - -	Wind reactions based on MWFRS						
Des Ld: 55.00	Mean Height: 25.67 ft		Creep Factor: 2.0	K	Brg Wid = 8.0 Min Req = 2.3 (Truss)					
NCBCLL: 10.00	TCDL: 6.0 psf	Building Code:	Max TC CSI: 0.696	G	Brg Wid = - Min Req = -					
Soffit: 2.00	BCDL: 6.0 psf	FBC 8th Ed. 2023 Res. HVHZ	Max BC CSI: 0.653	Bearing K is a rigid surface.						
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max Web CSI: 0.664	Maximum Top Chord Forces Per Ply (lbs)						
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Varies by Ld Case	Mfg Specified Camber:	Chords	Tens.Comp.	Chords	Tens.	Comp.		
	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)		A - B	328	-137	D - E	1428	-2480	
	GCpi: 0.18	Plate Type(s):		B - C	1505	-2594	E - F	5	-9	
	Wind Duration: 1.60	WAVE	VIEW Ver: 23.02.01A.1204.18							

Lumber

Top chord: 2x4 SP #2;

Bot chord: 2x6 SP #2;

Webs: 2x4 SP #2;

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From 90 plf at 0.00 to 90 plf at 19.83

BC: From 20 plf at 0.00 to 20 plf at 7.46

BC: From 10 plf at 7.46 to 10 plf at 7.81

BC: From 20 plf at 7.81 to 20 plf at 19.83

BC: 124 lb Conc. Load at 7.46

BC: 867 lb Conc. Load at 7.81

Wind

Wind loads and reactions based on MWFRS.

C - D

1267 - 2234

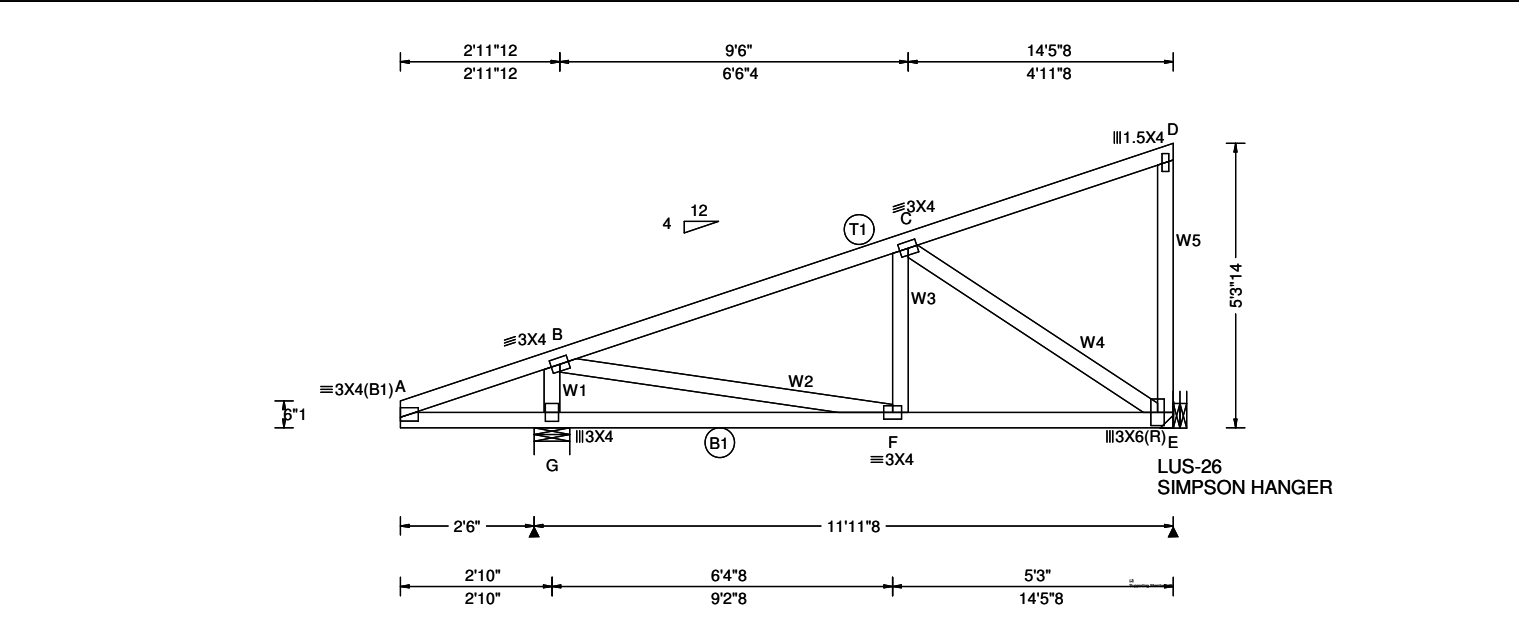
Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - K	85 - 268	I - H	2399 - 1377
K - J	86 - 210	H - G	1044 - 629
J - I	2399 - 1377		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
K - B	1140 - 1814	H - D	786 - 1179
B - J	2439 - 1397	H - E	1917 - 1040
C - J	430 - 206	E - G	846 - 1404
C - H	310 - 184	F - G	155 - 184

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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria		▲ Maximum Reactions (lbs)							
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#		Gravity			Non-Gravity				
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.013 F 999 360		Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.024 F 999 240		G	989	/-	/-	/590	/717	/486	
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): -0.006 D - -		E	622	/-	/-	/440	/533	/-	
Des Ld: 55.00		EXP: C Kzt: NA					HORZ(TL): 0.011 D - -		Wind reactions based on C&C							
NCBCLL: 10.00		Mean Height: 26.33 ft		Building Code:			Creep Factor: 2.0		G Brg Wid = 8.0 Min Req = 1.5 (Truss)							
Soffit: 2.00		TCDL: 6.0 psf		FBC 8th Ed. 2023 Res. HVHZ			Max TC CSI: 0.761		E Brg Wid = - Min Req = -							
Load Duration: 1.25		BCDL: 6.0 psf		TPI Std: 2014			Max BC CSI: 0.409		Bearing G is a rigid surface.							
Spacing: 24.0 "		MWFRS Parallel Dist: h/2 to h		Rep Fac: Yes			Max Web CSI: 0.456		Maximum Top Chord Forces Per Ply (lbs)							
		C&C Dist a: 3.00 ft		FT/RT:20(0)/10(0)			Mfg Specified Camber:		Chords		Tens.Comp.		Chords		Tens. Comp.	
		Loc. from endwall: not in 9.00 ft		Plate Type(s):					A - B		309 -313		C - D		52 -96	
		GCpi: 0.18		WAVE			VIEW Ver: 23.02.01A.1204.18		B - C		300 -750					
		Wind Duration: 1.60														

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Hangers / Ties
(J) Hanger Support Required, by others

Wind
Member design based on both MWFRS and C&C.
Right end vertical not exposed to wind pressure.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

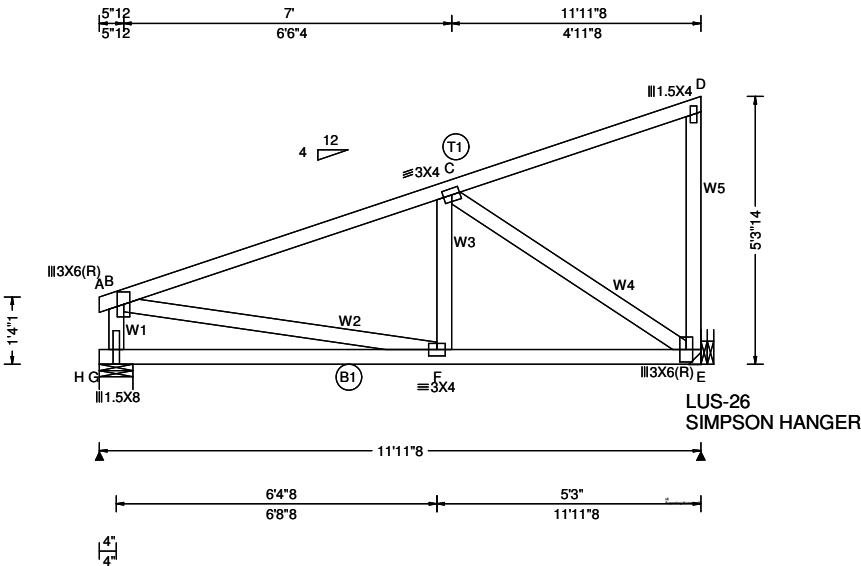
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)						
TCLL:	30.00	Wind Std:	ASCE 7-22	Pg:	NA	Ct:	NA	CAT:	NA	Gravity			Non-Gravity			
TCDL:	15.00	Speed:	175 mph	Pf:	NA			Ce:	NA	Loc	R+	/ R-	/ Rh	/ Rw	/ U / RL	
BCLL:	0.00	Enclosure:	Closed	Lu:	NA	Cs:	NA			H	668	/-	/-	/414	/422 /454	
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA						E	647	/-	/-	/442	/644 /-
		EXP:	C Kzt: NA							Wind reactions based on C&C						
Des Ld:	55.00	Mean Height:	26.75 ft	Building Code:					Creep Factor:	2.0	H	Brg Wid = 8.0 Min Req = 1.5 (Truss)				
NCBCLL:	10.00	TCDL:	6.0 psf	FBC 8th Ed. 2023 Res.	HVHZ				Max TC CSI:	0.923	E	Brg Wid = - Min Req = -				
Soffit:	2.00	BCDL:	6.0 psf	TPI Std:	2014				Max BC CSI:	0.487	Bearing H is a rigid surface.					
Load Duration:	1.25	MWFRS Parallel Dist:	h/2 to h	Rep Fac:	Yes				Max Web CSI:	0.500	Maximum Top Chord Forces Per Ply (lbs)					
Spacing:	24.0 "	C&C Dist a:	3.00 ft	FT/RT:	20(0)/10(0)				Mfg Specified Camber:		Chords	Tens.Comp.		Chords	Tens. Comp.	
		Loc. from endwall:	not in 9.00 ft	Plate Type(s):							A - B	5	0	C - D	47	- 95
		GCpi:	0.18								B - C	398	- 807			
		Wind Duration:	1.60	WAVE					VIEW Ver:	23.02.01A.1204.18						

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Hangers / Ties

(J) Hanger Support Required, by others

Wind

Member design based on both MWFRS and C&C.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

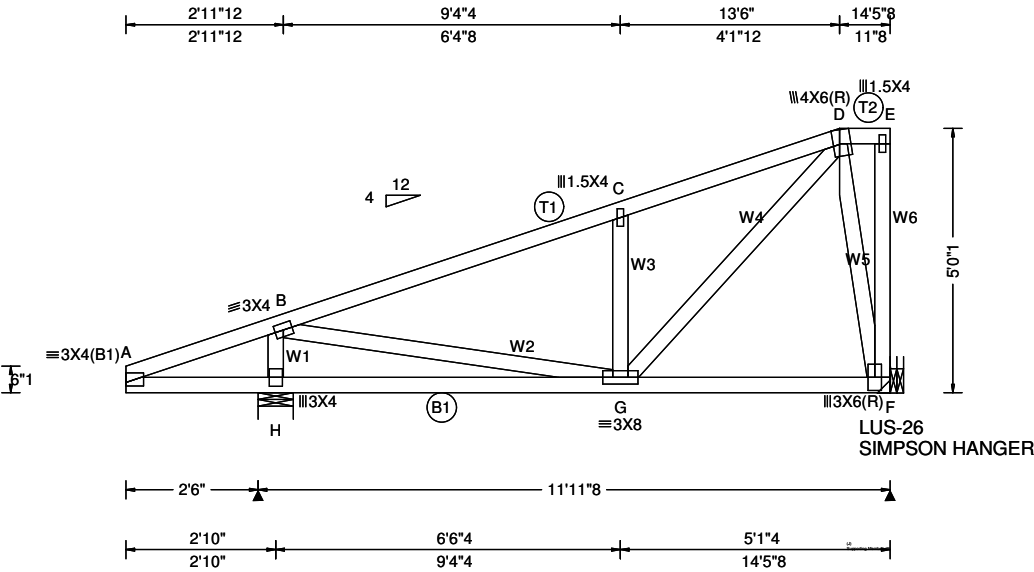
Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.		Chords	Tens. Comp.	
H - G	0	0	F - E	686	- 711
G - F	131	- 524			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.		Webs	Tens. Comp.	
B - G	455	- 608	C - E	856	- 826
B - F	618	- 180	D - E	188	- 149
F - C	220	0			

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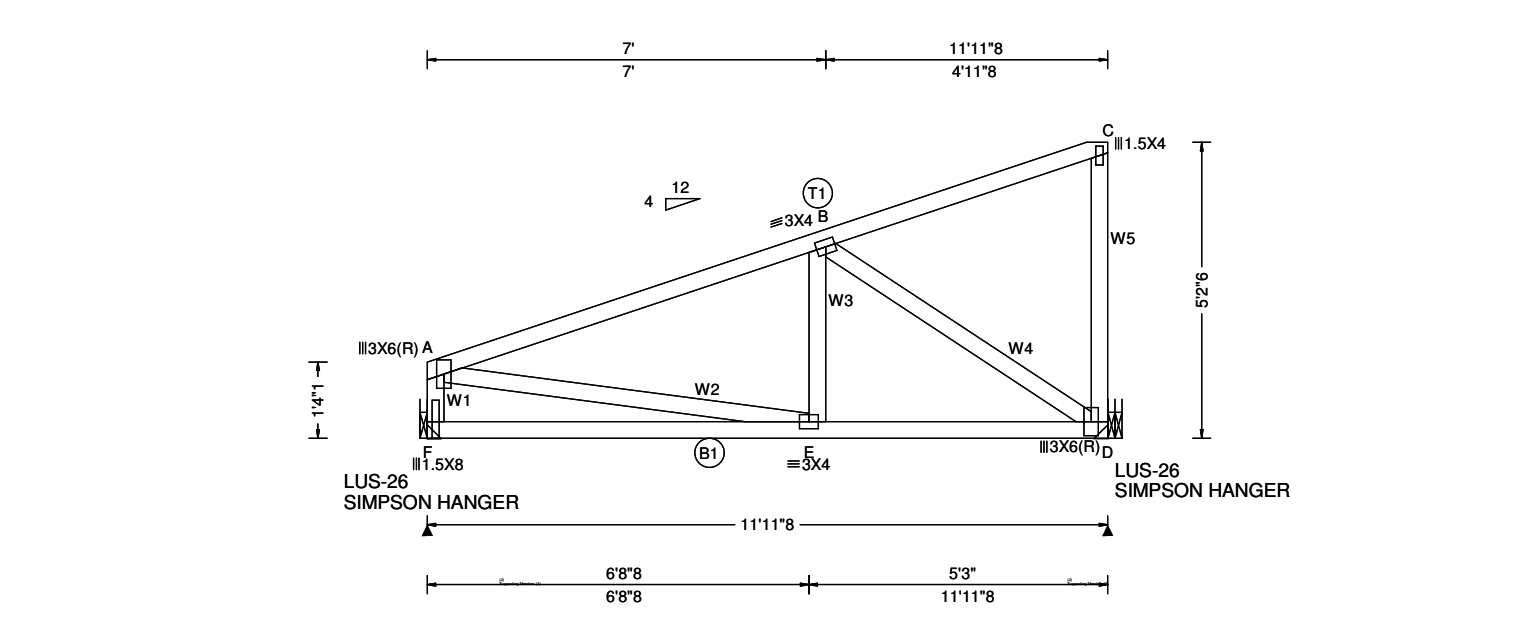
Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)							
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity				
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.017 C 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.031 C 999 240			H	989	/-	/-	/590	/733	/453	
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): -0.007 E - -			F	622	/-	/-	/291	/601	/-	
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.013 E - -			Wind reactions based on C&C							
NCBCLL: 10.00		Mean Height: 26.17 ft					Creep Factor: 2.0			H Brg Wid = 8.0 Min Req = 1.5 (Truss)							
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.614			F Brg Wid = - Min Req = -							
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.337			Bearing H is a rigid surface.							
Spacing: 24.0 "		MWFRS Parallel Dist: h/2 to h					Max Web CSI: 0.449			Maximum Top Chord Forces Per Ply (lbs)							
		C&C Dist a: 3.00 ft					Mfg Specified Camber:			Chords		Tens.Comp.		Chords		Tens. Comp.	
		Loc. from endwall: not in 9.00 ft								A - B		305 -313		C - D		533 -734	
		GCpi: 0.18								B - C		335 -750		D - E		0 0	
		Wind Duration: 1.60															

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Hangers / Ties
(J) Hanger Support Required, by others

Wind
Member design based on both MWFRS and C&C.
Right end vertical not exposed to wind pressure.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

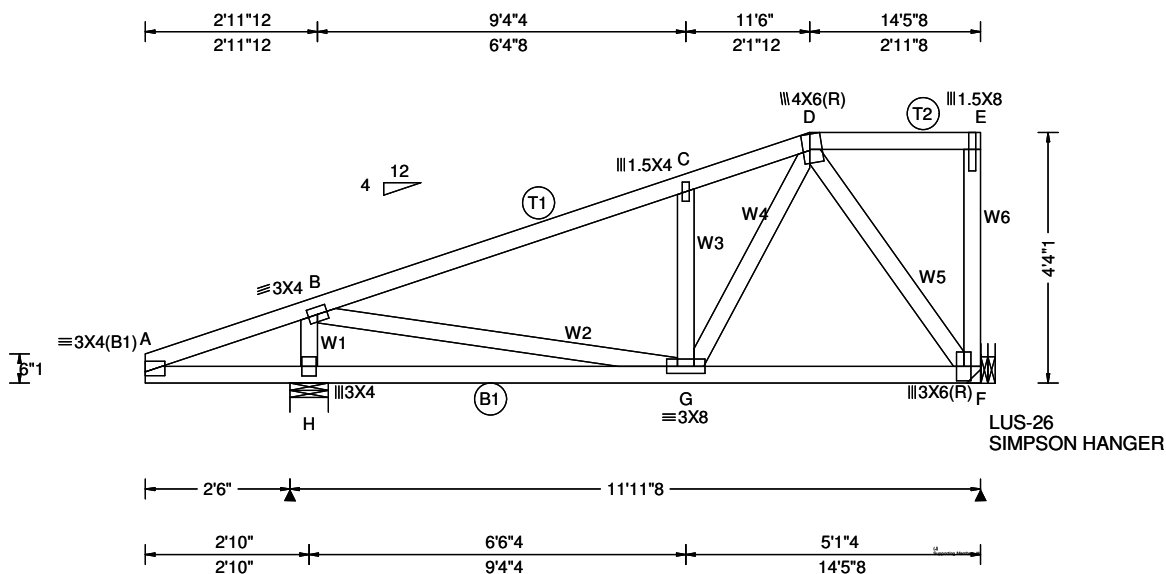
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)						
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity			
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.016 E 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.029 E 999 240			F	658	/-	/-	/408	/420	/440
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): -0.007 D - -			D	659	/-	/-	/429	/611	/-
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.011 C - -			Wind reactions based on C&C						
NCBCLL: 10.00		Mean Height: 26.69 ft					Creep Factor: 2.0			F Brg Wid = - Min Req = -						
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.959			D Brg Wid = - Min Req = -						
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.507			Maximum Top Chord Forces Per Ply (lbs)						
Spacing: 24.0 "		MWFRS Parallel Dist: h/2 to h					Max Web CSI: 0.515			Chords	Tens.Comp.		Chords	Tens. Comp.		
		C&C Dist a: 3.00 ft					Mfg Specified Camber:									
		Loc. from endwall: not in 9.00 ft					VIEW Ver: 23.02.01A.1204.18									
		GCpi: 0.18														
		Wind Duration: 1.60														

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2; Wind Member design based on both MWFRS and C&C. End verticals not exposed to wind pressure. Wind loading based on both gable and hip roof types.	
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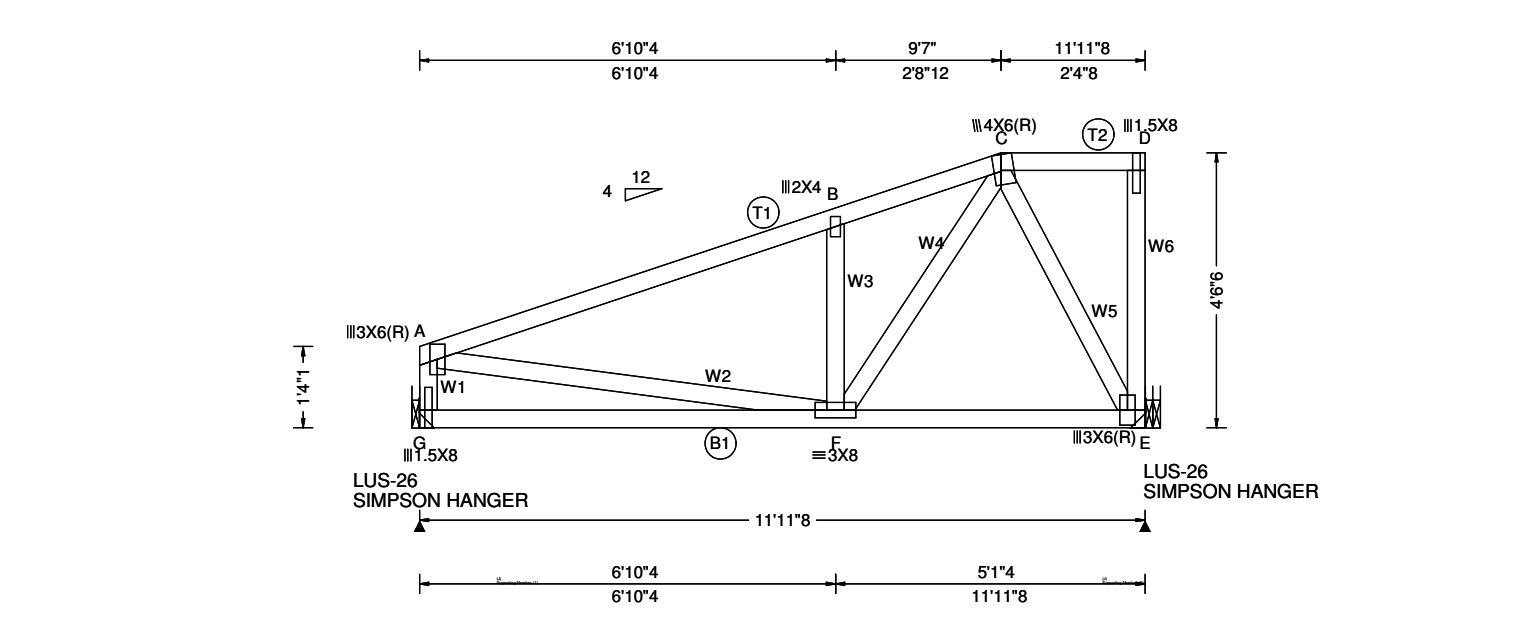
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)																																
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 175 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 25.84 ft TCDL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.014 C 999 360 VERT(CL): 0.025 C 999 240 HORZ(LL): -0.005 E - - HORZ(TL): 0.009 E - - Creep Factor: 2.0 Max TC CSI: 0.648 Max BC CSI: 0.370 Max Web CSI: 0.264 Mfg Specified Camber: VIEW Ver: 23.02.01A.1204.18	<table><tr><th colspan="2">Gravity</th><th colspan="3">Non-Gravity</th></tr><tr><th>Loc</th><th>R+ / R-</th><th>/ Rh</th><th>/ Rw</th><th>/ U / RL</th></tr><tr><td>H</td><td>989</td><td>- / -</td><td>/592</td><td>/786 /384</td></tr><tr><td>F</td><td>622</td><td>- / -</td><td>/291</td><td>/722 - / -</td></tr></table> Wind reactions based on C&C H Brg Wid = 8.0 Min Req = 1.5 (Truss) F Brg Wid = - Min Req = - Bearing H is a rigid surface. Maximum Top Chord Forces Per Ply (lbs) <table><tr><th>Chords</th><th>Tens.Comp.</th><th>Chords</th><th>Tens. Comp.</th></tr><tr><td>A - B</td><td>302 -305</td><td>C - D</td><td>615 -715</td></tr><tr><td>B - C</td><td>441 -745</td><td>D - E</td><td>0 0</td></tr></table>	Gravity		Non-Gravity			Loc	R+ / R-	/ Rh	/ Rw	/ U / RL	H	989	- / -	/592	/786 /384	F	622	- / -	/291	/722 - / -	Chords	Tens.Comp.	Chords	Tens. Comp.	A - B	302 -305	C - D	615 -715	B - C	441 -745	D - E	0 0
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Loc	R+ / R-	/ Rh	/ Rw	/ U / RL																																
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Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2; Hangers / Ties (J) Hanger Support Required, by others Wind Member design based on both MWFRS and C&C. Right end vertical not exposed to wind pressure. Left cantilever is exposed to wind Wind loading based on both gable and hip roof types.	
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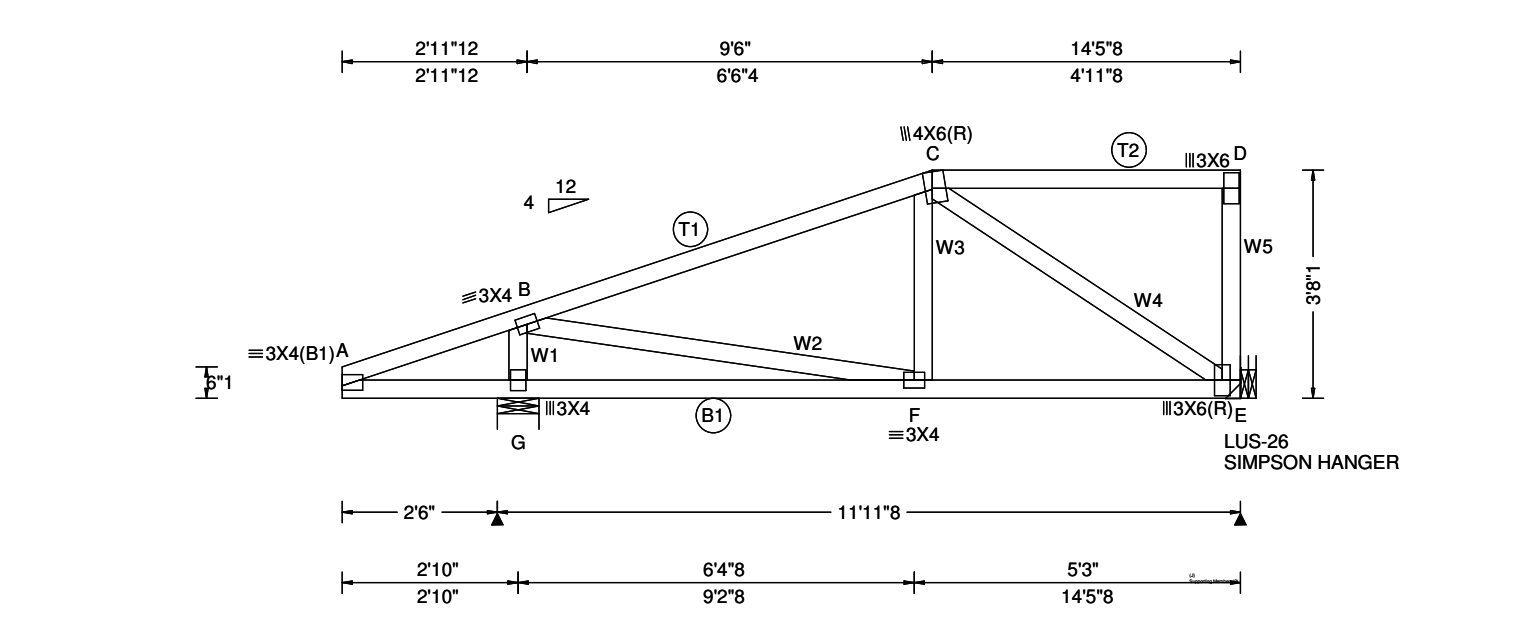
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)						
TCLL:	30.00	Wind Std:	ASCE 7-22	Pg: NA	Ct: NA	CAT: NA	PP Deflection in	loc L/defl	L/#	Gravity			Non-Gravity			
TCDL:	15.00	Speed:	175 mph	Pf: NA		Ce: NA	VERT(LL):	0.017	B 999 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL):	0.032	B 999 240	G	658	/-	/-	/367	/469	/362
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA		HORZ(LL):	-0.006	D - -	E	658	/-	/-	/322	/806	/-
		EXP:	C Kzt: NA				HORZ(TL):	0.011	D - -	Wind reactions based on C&C						
Des Ld:	55.00	Mean Height:	26.35 ft			Building Code:	Creep Factor:	2.0		G	Brg Wid = -		Min Req = -			
NCBCLL:	10.00	TCDL:	6.0 psf			FBC 8th Ed. 2023 Res. HVHZ	Max TC CSI:	0.883		E	Brg Wid = -		Min Req = -			
Soffit:	2.00	BCDL:	6.0 psf			TPI Std: 2014	Max BC CSI:	0.452		Maximum Top Chord Forces Per Ply (lbs)						
Load Duration:	1.25	MWFRS Parallel Dist:	h/2 to h			Rep Fac: Yes	Max Web CSI:	0.317		Chords	Tens.Comp.		Chords	Tens. Comp.		
Spacing:	24.0 "	C&C Dist a:	3.00 ft			FT/RT:20(0)/10(0)	Mfg Specified Camber:			A - B	534	- 835	C - D	0	0	
		Loc. from endwall:	not in 9.00 ft			Plate Type(s):	VIEW Ver:	23.02.01A.1204.18		B - C	774	- 821				
		GCpi:	0.18			WAVE										
		Wind Duration:	1.60													

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2;	<table><tr><th colspan="6">Maximum Bot Chord Forces Per Ply (lbs)</th></tr><tr><th colspan="2">Chords</th><th colspan="2">Tens.Comp.</th><th colspan="2">Chords</th><th colspan="2">Tens. Comp.</th></tr><tr><td colspan="2">G - F</td><td colspan="2">123 - 444</td><td colspan="2">F - E</td><td colspan="2">289 - 335</td></tr></table>	Maximum Bot Chord Forces Per Ply (lbs)						Chords		Tens.Comp.		Chords		Tens. Comp.		G - F		123 - 444		F - E		289 - 335																	
Maximum Bot Chord Forces Per Ply (lbs)																																							
Chords		Tens.Comp.		Chords		Tens. Comp.																																	
G - F		123 - 444		F - E		289 - 335																																	
Hangers / Ties (J) Hanger Support Required, by others	<table><tr><th colspan="6">Maximum Web Forces Per Ply (lbs)</th></tr><tr><th colspan="2">Webs</th><th colspan="2">Tens.Comp.</th><th colspan="2">Webs</th><th colspan="2">Tens. Comp.</th></tr><tr><td colspan="2">A - G</td><td colspan="2">503 - 598</td><td colspan="2">F - C</td><td colspan="2">766 - 736</td></tr><tr><td colspan="2">A - F</td><td colspan="2">626 - 303</td><td colspan="2">C - E</td><td colspan="2">685 - 591</td></tr><tr><td colspan="2">B - F</td><td colspan="2">756 - 594</td><td colspan="2">D - E</td><td colspan="2">227 - 107</td></tr></table>	Maximum Web Forces Per Ply (lbs)						Webs		Tens.Comp.		Webs		Tens. Comp.		A - G		503 - 598		F - C		766 - 736		A - F		626 - 303		C - E		685 - 591		B - F		756 - 594		D - E		227 - 107	
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)							
		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity				
TCLL: 30.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.011 A 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
TCDL: 15.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.017 A 999 240			G	989	/-	/-	/592	/865	/316	
BCLL: 0.00		Risk Category: II		Snow Duration: NA			HORZ(LL): -0.006 E - -			E	622	/-	/-	/290	/816	/-	
BCDL: 10.00		EXP: C Kzt: NA					HORZ(TL): 0.007 E - -			Wind reactions based on C&C							
Des Ld: 55.00		Mean Height: 25.51 ft		Building Code:			Creep Factor: 2.0			G Brg Wid = 8.0 Min Req = 1.5 (Truss)							
NCBCLL: 10.00		TCDL: 6.0 psf		FBC 8th Ed. 2023 Res. HVHZ			Max TC CSI: 0.936			E Brg Wid = - Min Req = -							
Soffit: 2.00		BCDL: 6.0 psf		TPI Std: 2014			Max BC CSI: 0.331			Bearing G is a rigid surface.							
Load Duration: 1.25		MWFRS Parallel Dist: 0 to h/2		Rep Fac: Yes			Max Web CSI: 0.376			Maximum Top Chord Forces Per Ply (lbs)							
Spacing: 24.0 "		C&C Dist a: 3.00 ft		FT/RT:20(0)/10(0)			Mfg Specified Camber:			Chords	Tens.Comp.	Chords	Tens. Comp.				
		Loc. from endwall: not in 9.00 ft		Plate Type(s):			VIEW Ver: 23.02.01A.1204.18			A - B	286	-247	C - D	25	-15		
		GCpi: 0.18		WAVE						B - C	526	-672					
		Wind Duration: 1.60															

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Hangers / Ties

(J) Hanger Support Required, by others

Wind

Member design based on both MWFRS and C&C.
Right end vertical not exposed to wind pressure.
Left cantilever is exposed to wind
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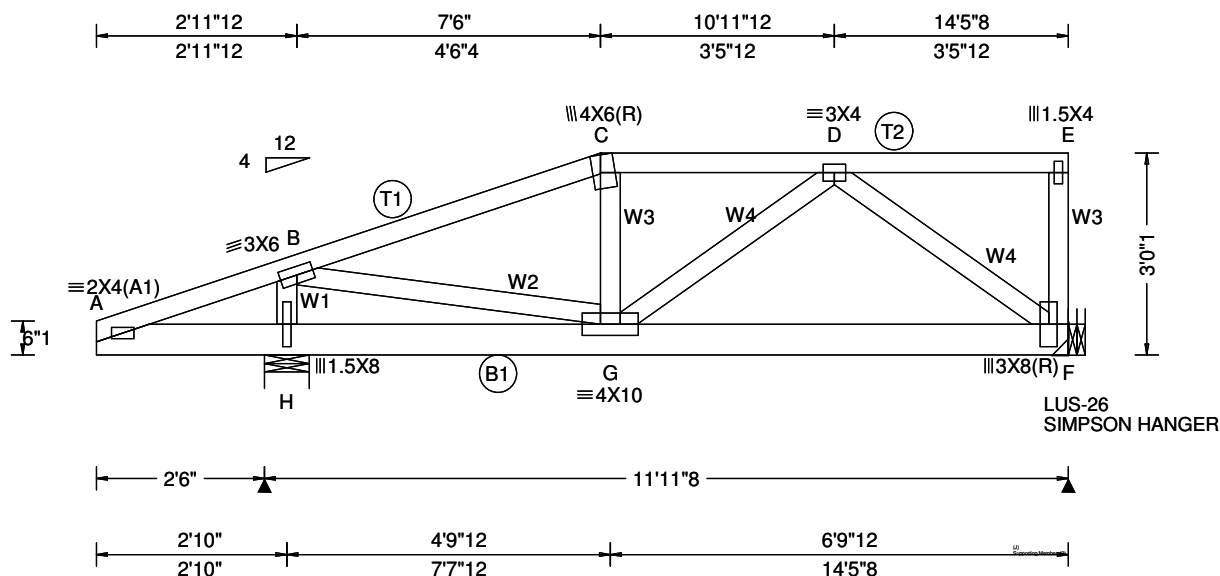
Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - G	232	-221	F - E	527	-589
G - F	276	-420			

Maximum Web Forces Per Ply (lbs)

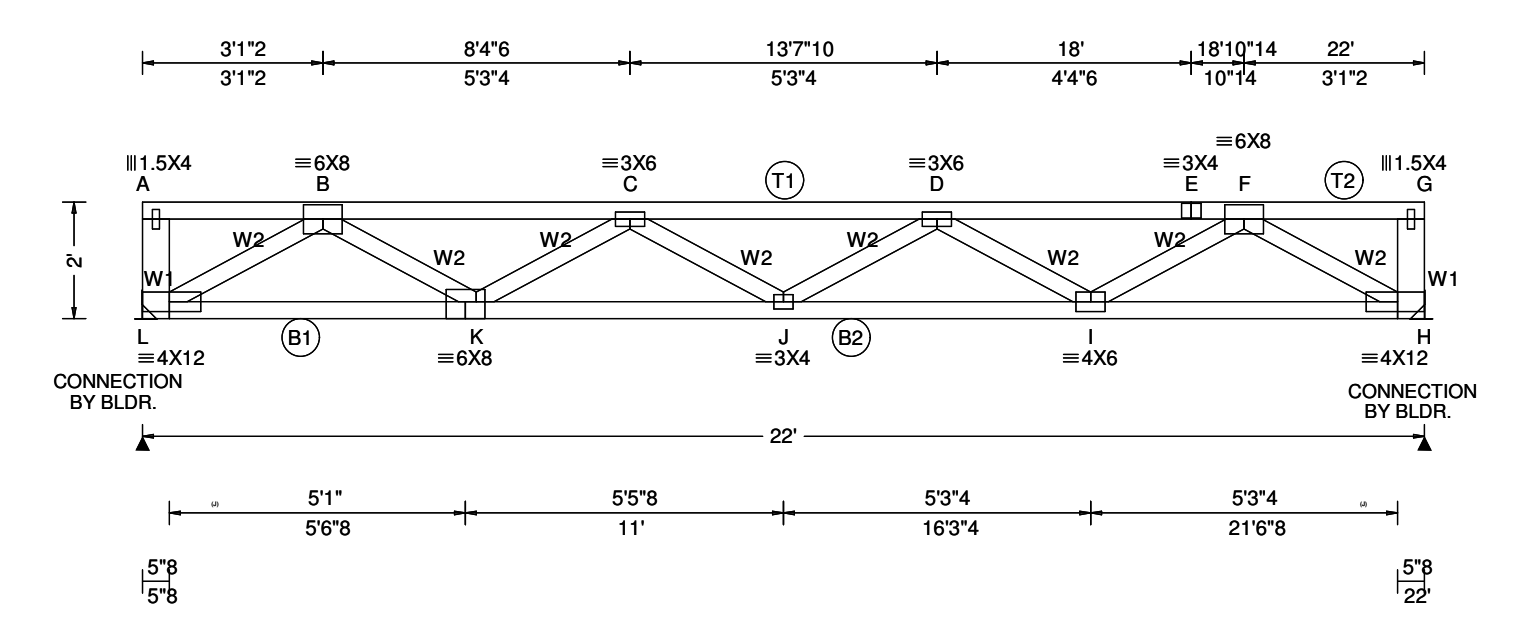
Webs	Tens.	Comp.	Webs	Tens.	Comp.
G - B	866	-882	C - E	682	-620
B - F	472	-381	D - E	454	-231
F - C	191	0			

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)																																																																																					
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 175 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 25.17 ft TCDL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): -0.023 G 999 360 VERT(CL): 0.039 G 999 240 HORZ(LL): -0.004 E - - HORZ(TL): 0.007 E - - Creep Factor: 2.0 Max TC CSI: 0.453 Max BC CSI: 0.785 Max Web CSI: 0.345 Mfg Specified Camber: VIEW Ver: 23.02.01A.1204.18	<table><tr><th colspan="2">Gravity</th><th colspan="3">Non-Gravity</th></tr><tr><th>Loc</th><th>R+ / R-</th><th>/ Rh</th><th>/ Rw</th><th>/ U / RL</th></tr><tr><td>H</td><td>1204</td><td>- / -</td><td>- / -</td><td>/783 - / -</td></tr><tr><td>F</td><td>992</td><td>- / -</td><td>- / -</td><td>/620 - / -</td></tr></table> Wind reactions based on MWFRS H Brg Wid = 8.0 Min Req = 1.5 (Truss) F Brg Wid = - Min Req = - Bearing H is a rigid surface. <table><tr><th colspan="5">Maximum Top Chord Forces Per Ply (lbs)</th></tr><tr><th>Chords</th><th>Tens.Comp.</th><th>Chords</th><th>Tens. Comp.</th><th></th></tr><tr><td>A - B</td><td>339 - 184</td><td>C - D</td><td>778 - 1141</td><td></td></tr><tr><td>B - C</td><td>878 - 1269</td><td>D - E</td><td>9 - 17</td><td></td></tr></table> <table><tr><th colspan="5">Maximum Bot Chord Forces Per Ply (lbs)</th></tr><tr><th>Chords</th><th>Tens.Comp.</th><th>Chords</th><th>Tens. Comp.</th><th></th></tr><tr><td>A - H</td><td>128 - 277</td><td>G - F</td><td>692 - 486</td><td></td></tr><tr><td>H - G</td><td>51 - 234</td><td></td><td></td><td></td></tr></table> <table><tr><th colspan="5">Maximum Web Forces Per Ply (lbs)</th></tr><tr><th>Webs</th><th>Tens.Comp.</th><th>Webs</th><th>Tens. Comp.</th><th></th></tr><tr><td>H - B</td><td>841 - 1180</td><td>G - D</td><td>571 - 373</td><td></td></tr><tr><td>B - G</td><td>1303 - 850</td><td>D - F</td><td>598 - 847</td><td></td></tr><tr><td>C - G</td><td>109 - 129</td><td>E - F</td><td>56 - 68</td><td></td></tr></table>	Gravity		Non-Gravity			Loc	R+ / R-	/ Rh	/ Rw	/ U / RL	H	1204	- / -	- / -	/783 - / -	F	992	- / -	- / -	/620 - / -	Maximum Top Chord Forces Per Ply (lbs)					Chords	Tens.Comp.	Chords	Tens. Comp.		A - B	339 - 184	C - D	778 - 1141		B - C	878 - 1269	D - E	9 - 17		Maximum Bot Chord Forces Per Ply (lbs)					Chords	Tens.Comp.	Chords	Tens. Comp.		A - H	128 - 277	G - F	692 - 486		H - G	51 - 234				Maximum Web Forces Per Ply (lbs)					Webs	Tens.Comp.	Webs	Tens. Comp.		H - B	841 - 1180	G - D	571 - 373		B - G	1303 - 850	D - F	598 - 847		C - G	109 - 129	E - F	56 - 68	
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Lumber Top chord: 2x4 SP #2; Bot chord: 2x6 SP #2; Webs: 2x4 SP #2; Special Loads ----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC: From 90 plf at 0.00 to 90 plf at 7.50 TC: From 45 plf at 7.50 to 45 plf at 14.46 BC: From 20 plf at 0.00 to 20 plf at 7.46 BC: From 10 plf at 7.46 to 10 plf at 14.46 TC: 132 lb Conc. Load at 8.15,10.15,12.15,13.65 BC: 124 lb Conc. Load at 7.46 BC: 84 lb Conc. Load at 8.15,10.15,12.15,13.65 Hangers / Ties (J) Hanger Support Required, by others Wind Wind loads and reactions based on MWFRS. Right end vertical not exposed to wind pressure. Left cantilever is exposed to wind Wind loading based on both gable and hip roof types.	
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)							
TCLL: 30.00		Wind Std: ASCE 7-16		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity				
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.224 J 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.407 J 648 240			L	1201	- / -	- / -	/561	/447	- / -	
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.050 H - -			H	1201	- / -	- / -	/561	/447	- / -	
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.091 H - -			Wind reactions based on MWFRS							
NCBCLL: 10.00		Mean Height: 15.00 ft					Creep Factor: 2.0			L Brg Wid = - Min Req = -							
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.665			H Brg Wid = - Min Req = -							
Load Duration: 1.00		BCDL: 6.0 psf					Max BC CSI: 0.779			Maximum Top Chord Forces Per Ply (lbs)							
Spacing: 24.0 "		MWFRS Parallel Dist: 0 to h/2					Max Web CSI: 0.653			Chords	Tens.Comp.		Chords	Tens. Comp.			
		C&C Dist a: 3.00 ft					Mfg Specified Camber:			A - B	16	- 22	D - E	2841	- 2928		
		Loc. from endwall: Any					VIEW Ver: 23.02.01A.1204.18			B - C	2843	- 2928	E - F	2841	- 2928		
		GCpi: 0.18								C - D	3665	- 3820	F - G	16	- 22		
		Wind Duration: 1.00															

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2; B2 2x4 SP #1;
Webs: 2x4 SP #2; W1 2x6 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Additional Notes

See detail STRBRIBR1014 for bracing and bridging recommendations.

Truss must be installed as shown with top chord up.

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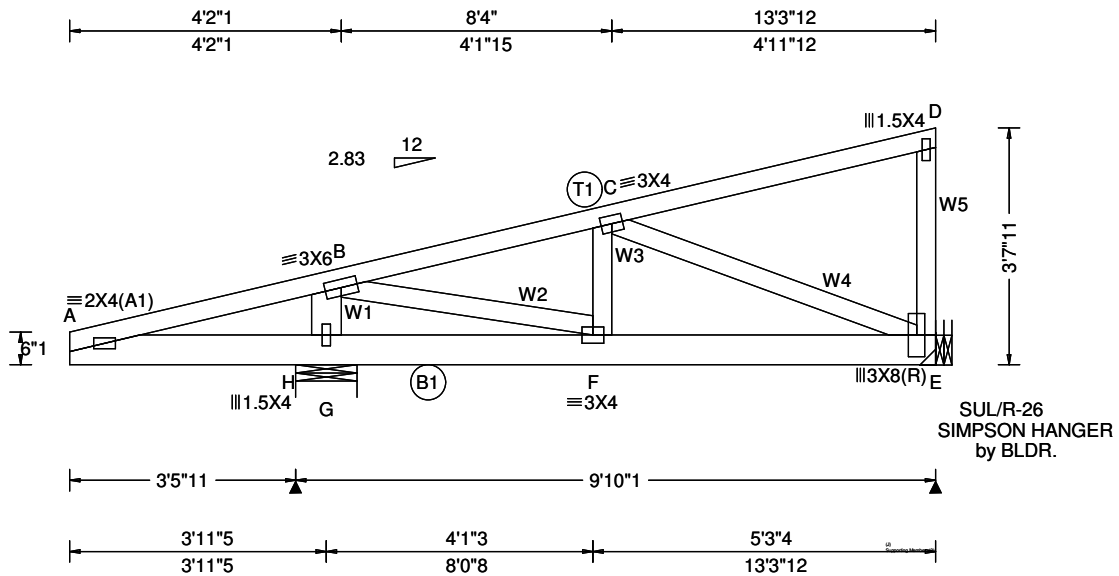
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SEQN: 139944 / T55 / HIP_ FROM: AE	Ply: 1 Qty: 1 Wgt: 291.2 lbs	Job Number: 23081008 5900 RES. Truss Label: CJ10	DRW: ... / ... 07/25/2024
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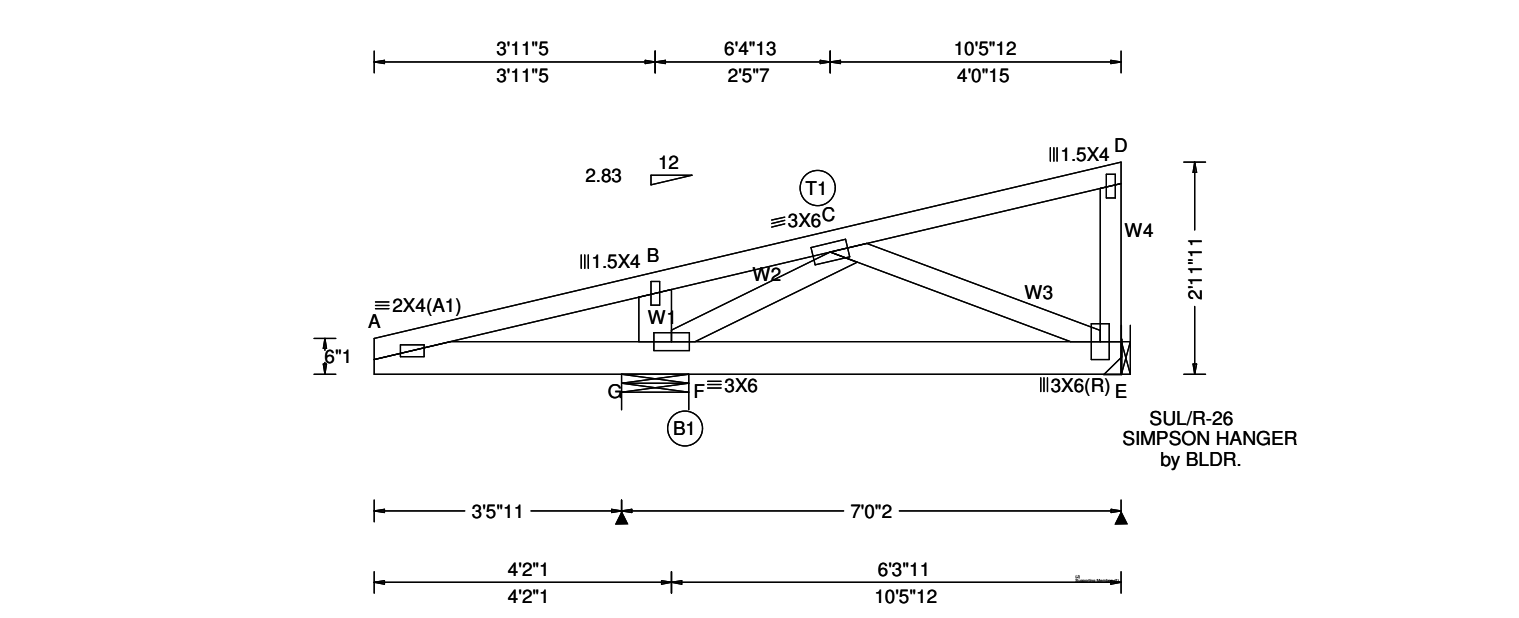


Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 175 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 25.49 ft TCDL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): -0.023 F 999 360 VERT(CL): 0.033 A 999 240 HORZ(LL): -0.004 E - - HORZ(TL): 0.004 E - - Creep Factor: 2.0 Max TC CSI: 0.196 Max BC CSI: 0.640 Max Web CSI: 0.188 Mfg Specified Camber: VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H 662 /- /- /- /1015 /- E 296 /- /- /- /475 /- Wind reactions based on MWFRS H Brg Wid = 11.3 Min Req = 1.5 (Truss) E Brg Wid = - Min Req = - Bearing H is a rigid surface. Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 445 0 C - D 10 -25 B - C 757 -296

Lumber Top chord: 2x4 SP #2; Bot chord: 2x6 SP #2; Webs: 2x4 SP #2; W1 2x6 SP #2; Special Loads ----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC: From 2 plf at 0.00 to 2 plf at 13.31 BC: From 2 plf at 0.00 to 2 plf at 13.31 TC: 262 lb Conc. Load at 2.83 TC: -129 lb Conc. Load at 5.66 TC: 303 lb Conc. Load at 11.31 BC: 178 lb Conc. Load at 2.83 BC: -194 lb Conc. Load at 5.66 BC: 13 lb Conc. Load at 8.49 BC: 188 lb Conc. Load at 11.31	Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - G 0 -435 F - E 270 -677 G - F 0 -387 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. G - B 327 -452 C - E 728 -287 B - F 574 -616 D - E 0 -65 F - C 28 -468
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Hangers / Ties (J) Hanger Support Required, by others Wind Wind loads and reactions based on MWFRS. Right end vertical not exposed to wind pressure. Left cantilever is exposed to wind Wind loading based on both gable and hip roof types.	
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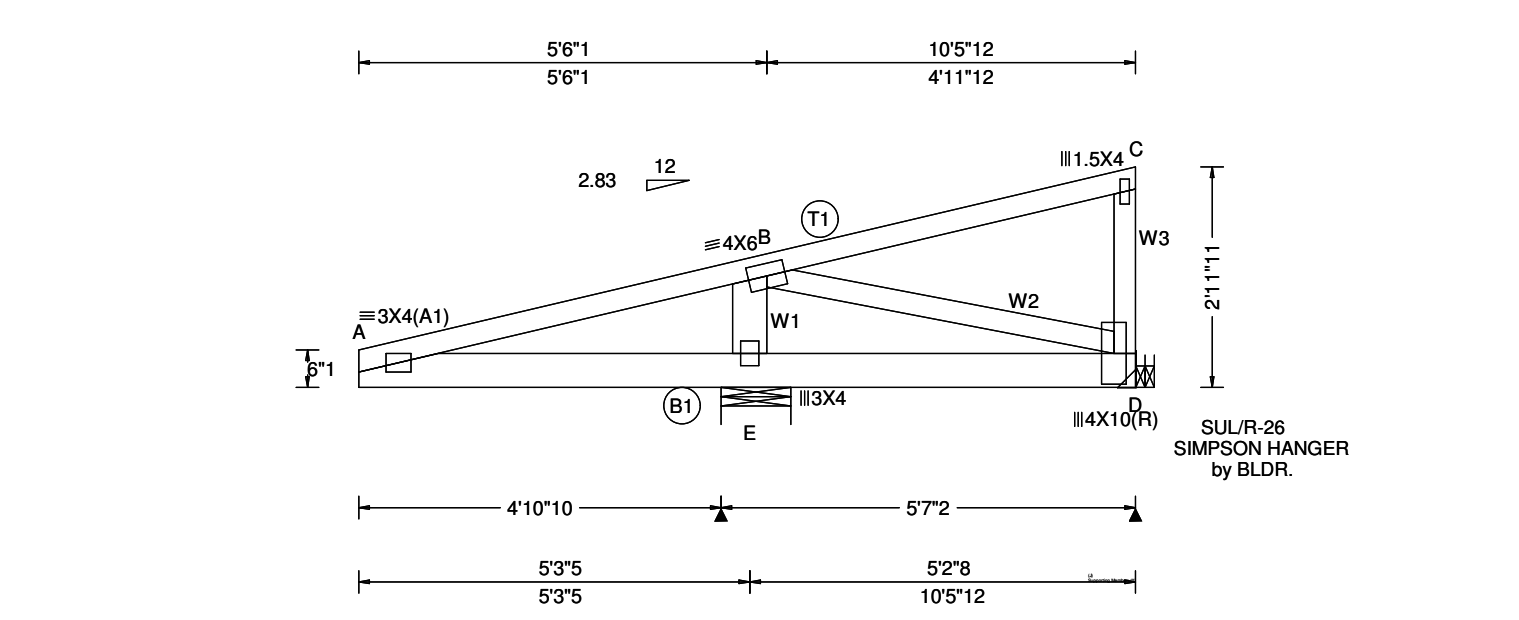
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)		Defl/CSI Criteria		▲ Maximum Reactions (lbs)					
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA		PP Deflection in loc L/defl L/#		Gravity			Non-Gravity		
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA		VERT(LL): 0.031 A 999 360		Loc	R+	/ R-	/ Rh	/ Rw	/ U / RL
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA		VERT(CL): 0.044 A 999 240		G	610	/-	/-	/-	/890 /-
BCDL: 10.00		Risk Category: II		Snow Duration: NA		HORZ(LL): -0.003 B - -		E	124	/-154	/-	/-	/205 /-
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE		HORZ(TL): 0.005 B - -		Wind reactions based on MWFRS					
NCBCLL: 10.00		Mean Height: 25.16 ft				Creep Factor: 2.0		G	Brg Wid = 11.3	Min Req = 1.5 (Truss)			
Soffit: 2.00		TCDL: 6.0 psf				Max TC CSI: 0.204		E	Brg Wid = -	Min Req = -			
Load Duration: 1.25		BCDL: 6.0 psf				Max BC CSI: 0.478		Bearing G is a rigid surface.					
Spacing: 24.0 "		MWFRS Parallel Dist: 0 to h/2				Max Web CSI: 0.063		Maximum Top Chord Forces Per Ply (lbs)					
		C&C Dist a: 3.00 ft				Mfg Specified Camber:		Chords	Tens.Comp.	Chords	Tens. Comp.		
		Loc. from endwall: Any				VIEW Ver: 23.02.01A.1204.18		A - B	546	0	C - D	10	- 21
		GCpi: 0.18						B - C	548	0			
		Wind Duration: 1.60											

Lumber Top chord: 2x4 SP #2; Bot chord: 2x6 SP #2; Webs: 2x4 SP #2; W1 2x6 SP #2;	
Special Loads ----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC: From 2 plf at 0.00 to 2 plf at 10.48 BC: From 2 plf at 0.00 to 2 plf at 10.48 TC: 262 lb Conc. Load at 2.83 TC: -129 lb Conc. Load at 5.66 BC: 178 lb Conc. Load at 2.83 BC: -194 lb Conc. Load at 5.66 BC: 13 lb Conc. Load at 8.49	
Hangers / Ties (J) Hanger Support Required, by others	
Wind Wind loads and reactions based on MWFRS. Right end vertical not exposed to wind pressure. Left cantilever is exposed to wind Wind loading based on both gable and hip roof types.	

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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)						
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity			
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.134 A 472 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.204 A 309 240			E	1165	/-	/-	/-	/1348	/-
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): -0.016 B - -			D	-	/-391	/-	/134	/96	/-
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.024 B - -			Wind reactions based on MWFRS						
NCBCLL: 10.00		Mean Height: 25.99 ft					Creep Factor: 2.0			E Brg Wid = 11.3 Min Req = 1.5 (Truss)						
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.815			D Brg Wid = - Min Req = -						
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.775			Bearing E is a rigid surface.						
Spacing: 24.0 "		MWFRS Parallel Dist: 0 to h/2					Max Web CSI: 0.395			Maximum Top Chord Forces Per Ply (lbs)						
		C&C Dist a: 3.00 ft					Mfg Specified Camber:			Chords Tens.Comp. Chords Tens. Comp.						
		Loc. from endwall: not in 4.50 ft					VIEW Ver: 23.02.01A.1204.18									
		GCpi: 0.18								A - B 1886 -954 B - C 9 -25						
		Wind Duration: 1.60														

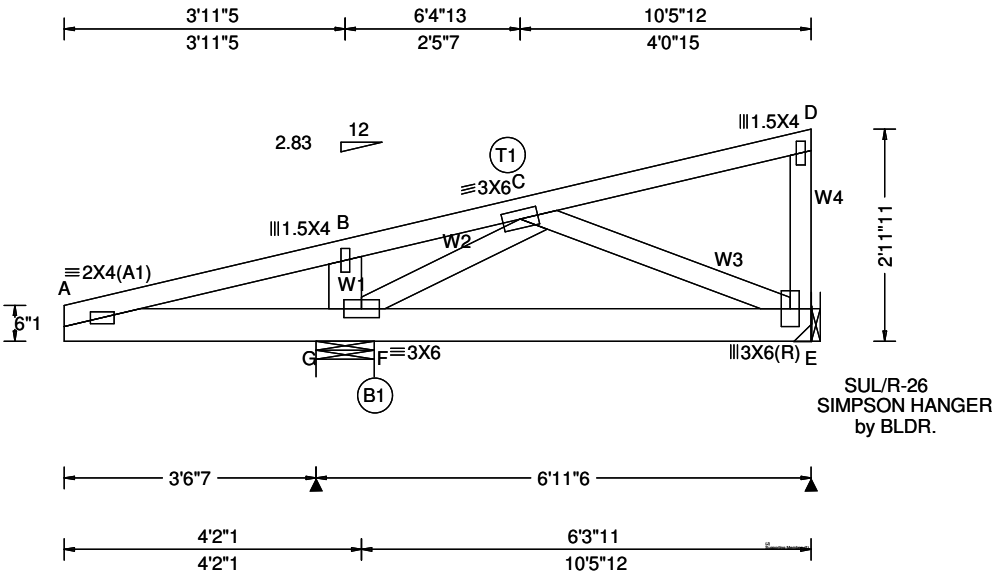
Lumber Top chord: 2x4 SP #2; Bot chord: 2x6 SP #2; Webs: 2x4 SP #2; W1 2x6 SP #2;	Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - E 926 -1829 E - D 856 -1688
Special Loads ----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC: From 2 plf at 0.00 to 2 plf at 10.48 BC: From 2 plf at 0.00 to 2 plf at 10.48 TC: 400 lb Conc. Load at 0.37 TC: 253 lb Conc. Load at 2.83 TC: -228 lb Conc. Load at 5.66 TC: 189 lb Conc. Load at 8.49 BC: 173 lb Conc. Load at 2.83 BC: -223 lb Conc. Load at 5.66 BC: 168 lb Conc. Load at 8.49	Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. E - B 493 -993 C - D 14 -54 B - D 1750 -890
Wind Wind loads and reactions based on MWFRS. Right end vertical not exposed to wind pressure. Left cantilever is exposed to wind Wind loading based on both gable and hip roof types.	
Additional Notes Negative reaction(s) of -391# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.	

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[illegible]

Lumber		Maximum Bot Chord Forces Per Ply (lbs)			
Top chord: 2x4 SP #2;		Chords	Tens.Comp.	Chords	Tens. Comp.
Bot chord: 2x6 SP #2;					
Webs: 2x4 SP #2; W1 2x6 SP #2;		A - F	0 -438	F - E	0 -415
Special Loads		Maximum Web Forces Per Ply (lbs)			
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)		Webs	Tens.Comp.	Webs	Tens. Comp.
TC: From 2 plf at 0.00 to 2 plf at 10.48		B - F	167 -184	C - E	434 0
BC: From 2 plf at 0.00 to 2 plf at 10.48		F - C	426 -359	D - E	74 -106
TC: 262 lb Conc. Load at 2.83					
TC: -138 lb Conc. Load at 5.66					
TC: 223 lb Conc. Load at 8.49					
BC: 178 lb Conc. Load at 2.83					
BC: -198 lb Conc. Load at 5.66					
BC: -3 lb Conc. Load at 8.49					
Plating Notes					
All plates are 1.5X4 except as noted.					
Hangers / Ties					
(J) Hanger Support Required, by others					
Wind					
Wind loads and reactions based on MWFRS.					
Right end vertical not exposed to wind pressure.					
Left cantilever is exposed to wind					
Wind loading based on both gable and hip roof types.					

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS
 Trusses require strict care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety) information, BCSI and SBCCI safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
 For more information see these web sites: Alpine: alpineitw.com; TPI: tjoinst.org; SBCCI: sbccacomponents.com; ICC: iccsafe.org; AWC: awc.org



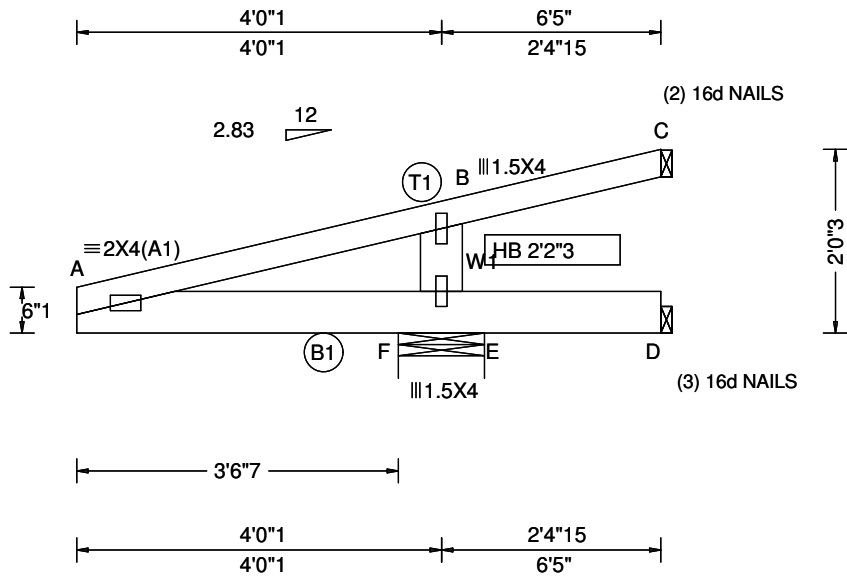
Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)							
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity				
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.032 A 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL: 0.00		Enclosure: Part. Enc.		Lu: NA Cs: NA			VERT(CL): 0.037 A 999 240			G 610	-	-	-	-	/1053	-	
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): -0.003 B - -			E 124	-11	-	-	-	/438	-	
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.004 B - -			Wind reactions based on MWFRS							
NCBCLL: 10.00		Mean Height: 25.99 ft					Creep Factor: 2.0			G Brg Wid = 9.8 Min Req = 1.5 (Truss)							
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.172			E Brg Wid = - Min Req = -							
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.645			Bearing G is a rigid surface.							
Spacing: 24.0 "		MWFRS Parallel Dist: 0 to h/2					Max Web CSI: 0.078			Maximum Top Chord Forces Per Ply (lbs)							
		C&C Dist a: 3.00 ft					Mfg Specified Camber:			Chords		Tens.Comp.		Chords		Tens. Comp.	
		Loc. from endwall: not in 4.50 ft								A - B		459 0		C - D		17 -21	
		GCpi: 0.55								B - C		461 0					
		Wind Duration: 1.60					VIEW Ver: 23.02.01A.1204.18										

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x6 SP #2;
Webs: 2x4 SP #2; W1 2x6 SP #2;

Special Loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 2 plf at 0.00 to 2 plf at 10.48
BC: From 2 plf at 0.00 to 2 plf at 10.48
TC: 262 lb Conc. Load at 2.83
TC: -129 lb Conc. Load at 5.66
TC: 224 lb Conc. Load at 8.49
BC: 178 lb Conc. Load at 2.83
BC: -194 lb Conc. Load at 5.66
BC: -3 lb Conc. Load at 8.49

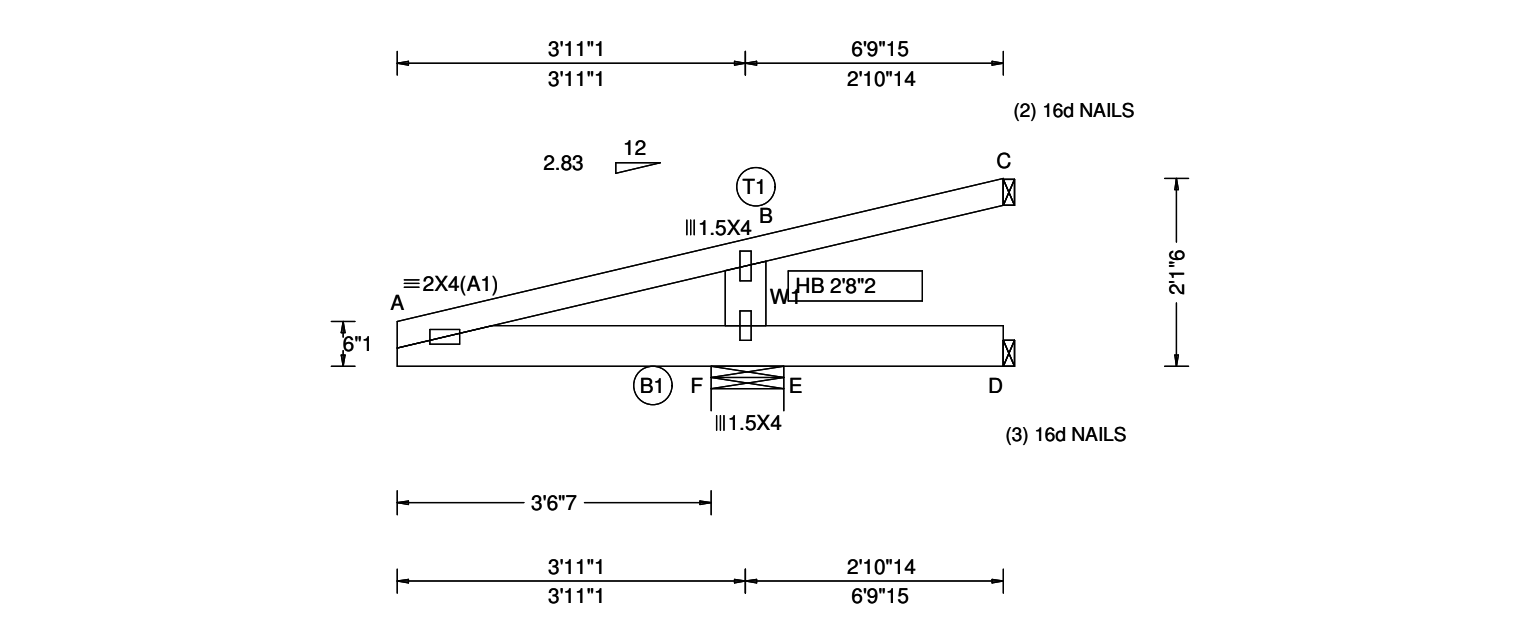
Wind
Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCEA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.
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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCEA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org



Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)						
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity			
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.149 A 341 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.217 A 234 240			F	661	/-	/-	/-	/619	/-
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): -0.039 C - -			D	-	/-331	/-	/-	/207	/-
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.057 C - -			C	-	/-177	/-	/-	/179	/-
NCBCLL: 10.00		Mean Height: 15.00 ft					Creep Factor: 2.0			Wind reactions based on MWFRS						
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.463			F Brg Wid = 11.3 Min Req = 1.5 (Truss)						
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.438			D Brg Wid = 1.5 Min Req = -						
Spacing: 24.0 "		MWFRS Parallel Dist: 0 to h/2		Max Web CSI: 0.050			Mfg Specified Camber:			C Brg Wid = 1.5 Min Req = -						
		C&C Dist a: 3.00 ft								Bearing F is a rigid surface.						
		Loc. from endwall: Any								Maximum Top Chord Forces Per Ply (lbs)						
		GCpi: 0.18								Chords Tens.Comp. Chords Tens. Comp.						
		Wind Duration: 1.60					VIEW Ver: 23.02.01A.1204.18									

Lumber Top chord: 2x4 SP #2; Bot chord: 2x6 SP #2; Webs: 2x6 SP #2; Special Loads ----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC: From 2 plf at 0.00 to 2 plf at 6.42 BC: From 2 plf at 0.00 to 2 plf at 6.42 TC: 262 lb Conc. Load at 2.83 TC: -129 lb Conc. Load at 5.66 BC: 178 lb Conc. Load at 2.83 BC: -194 lb Conc. Load at 5.66 Wind Wind loads and reactions based on MWFRS. Left cantilever is exposed to wind Wind loading based on both gable and hip roof types. Additional Notes Negative reaction(s) of -331# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.	
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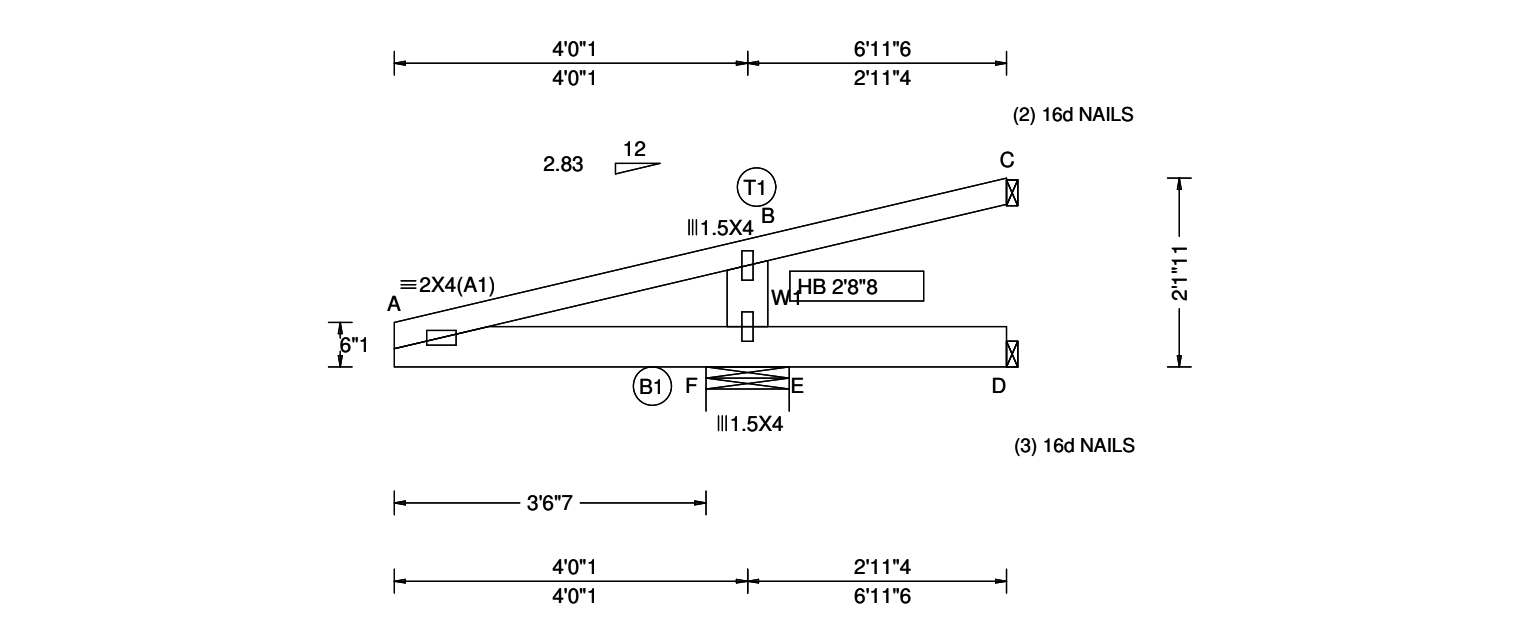
Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)						
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity			
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.157 A 317 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.214 A 232 240			F	610	/-	/-	/-	/660	/-
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): -0.041 C - -			D	-	/-269	/-	/-	/192	/-
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.058 C - -			C	-	/-138	/-	/-	/153	/-
NCBCLL: 10.00		Mean Height: 15.00 ft					Creep Factor: 2.0			Wind reactions based on MWFRS						
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.424			F Brg Wid = 9.8 Min Req = 1.5 (Truss)						
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.425			D Brg Wid = 1.5 Min Req = -						
Spacing: 24.0 "		MWFRS Parallel Dist: 0 to h/2		Max Web CSI: 0.053			Mfg Specified Camber:			C Brg Wid = 1.5 Min Req = -						
		C&C Dist a: 3.00 ft								Bearing F is a rigid surface.						
		Loc. from endwall: Any								Maximum Top Chord Forces Per Ply (lbs)						
		GCpi: 0.18								Chords Tens.Comp. Chords Tens. Comp.						
		Wind Duration: 1.60					VIEW Ver: 23.02.01A.1204.18									

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x6 SP #2;
Webs: 2x6 SP #2;

Special Loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 2 plf at 0.00 to 2 plf at 6.83
BC: From 2 plf at 0.00 to 2 plf at 6.83
TC: 262 lb Conc. Load at 2.83
TC: -129 lb Conc. Load at 5.66
BC: 178 lb Conc. Load at 2.83
BC: -194 lb Conc. Load at 5.66

Wind
Wind loads and reactions based on MWFRS.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

Additional Notes
Negative reaction(s) of -269# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.



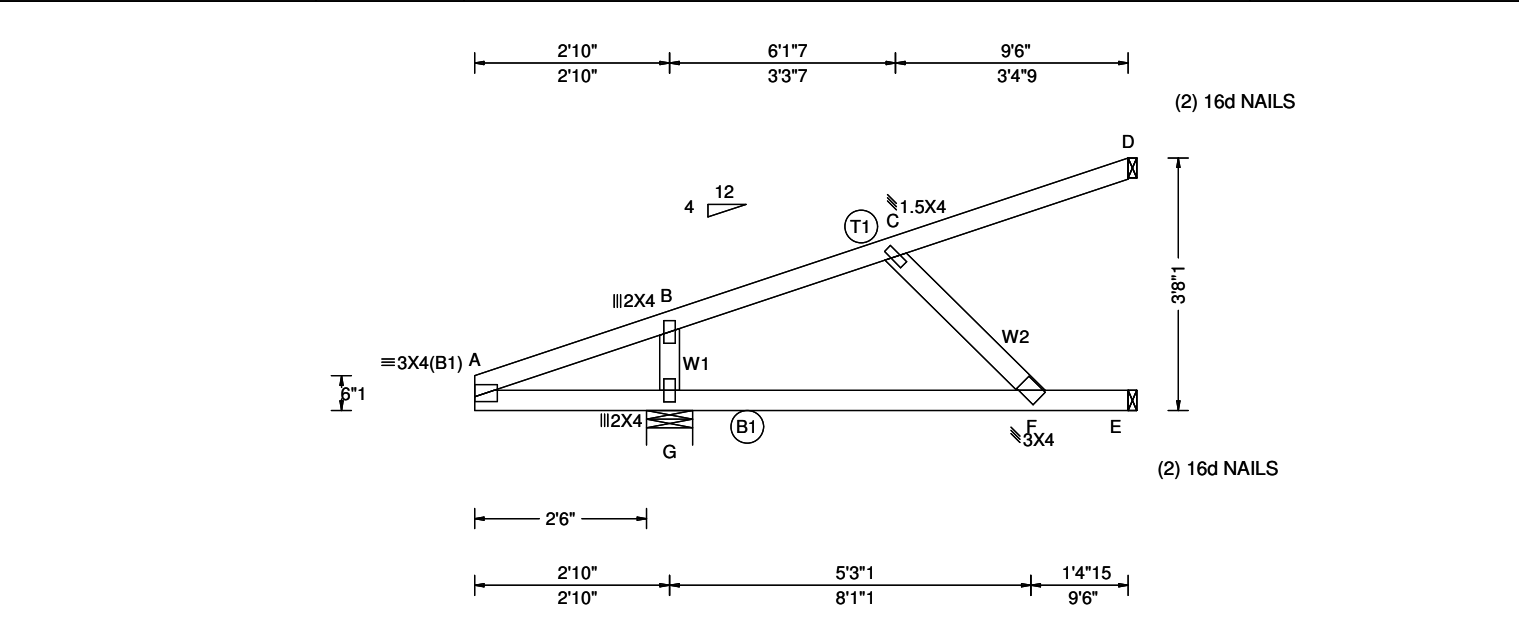
Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)								
TCLL:	30.00	Wind Std:	ASCE 7-22	Pg: NA	Ct: NA	CAT: NA	PP Deflection in	loc L/defl	L/#	Gravity			Non-Gravity					
TCDL:	15.00	Speed:	175 mph	Pf: NA		Ce: NA	VERT(LL):	0.173	A	293	360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL):	0.231	A	220	240	F	623	/-	/-	/-	/695	/-
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA		HORZ(LL):	-0.045	C	-	-	D	-	/-270	/-	/-	/172	/-
		EXP:	C Kzt: NA				HORZ(TL):	0.064	C	-	-	C	-	/-136	/-	/-	/137	/-
Des Ld:	55.00	Mean Height:	15.00 ft	Building Code:			Creep Factor:	2.0				Wind reactions based on MWFRS						
NCBCLL:	10.00	TCDL:	6.0 psf	FBC 8th Ed. 2023 Res.	HVHZ		Max TC CSI:	0.443				F	Brg Wid = 11.3	Min Req = 1.5	(Truss)			
Soffit:	2.00	BCDL:	6.0 psf	TPI Std:	2014		Max BC CSI:	0.452				D	Brg Wid = 1.5	Min Req = -				
Load Duration:	1.25	MWFRS Parallel Dist:	0 to h/2	Rep Fac:	Varies by Ld Case		Max Web CSI:	0.056				C	Brg Wid = 1.5	Min Req = -				
Spacing:	24.0 "	C&C Dist a:	3.00 ft	FT/RT:	20(0)/10(0)		Mfg Specified Camber:					Bearing F is a rigid surface.						
		Loc. from endwall:	Any	Plate Type(s):								Maximum Top Chord Forces Per Ply (lbs)						
		GCpi:	0.18									Chords	Tens.Comp.	Chords	Tens. Comp.			
		Wind Duration:	1.60	WAVE			VIEW Ver:	23.02.01A.1204.18										

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x6 SP #2;
Webs: 2x6 SP #2;

Special Loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 2 plf at 0.00 to 2 plf at 6.95
BC: From 2 plf at 0.00 to 2 plf at 6.95
TC: 262 lb Conc. Load at 2.83
TC: -129 lb Conc. Load at 5.66
BC: 178 lb Conc. Load at 2.83
BC: -194 lb Conc. Load at 5.66

Wind
Wind loads and reactions based on MWFRS.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

Additional Notes
Negative reaction(s) of -270# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

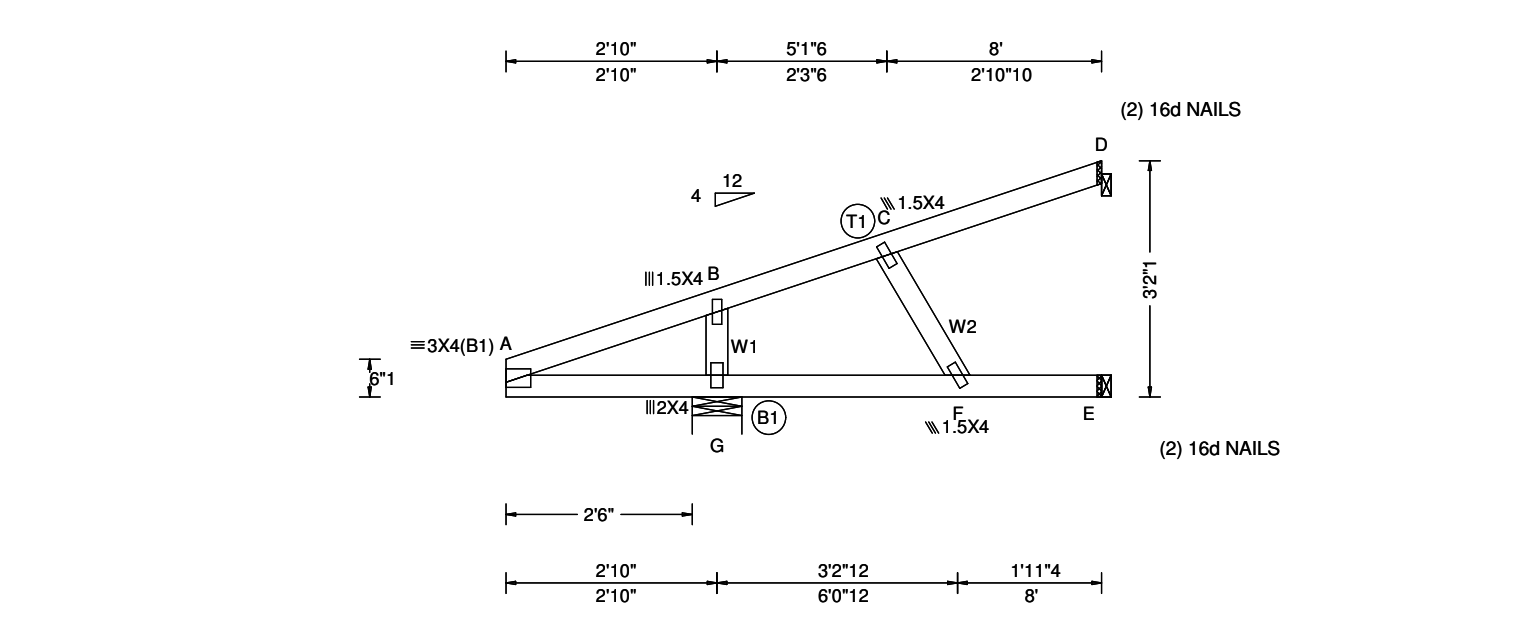


Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)							
TCLL:	30.00	Wind Std:	ASCE 7-22	Pg: NA	Ct: NA	CAT: NA	PP Deflection in loc L/defl L/#			Gravity			Non-Gravity				
TCDL:	15.00	Speed:	175 mph	Pf: NA		Ce: NA	VERT(LL): -0.128 A 264 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL): 0.194 A 175 240			G	745	/-	/-	/482	/758	/405	
BCDL:	10.00	Risk Category:	II	Snow Duration: NA			HORZ(LL): 0.051 D - -			E	171	/-	/-	/167	/190	/-	
		EXP: C	Kzt: NA				HORZ(TL): 0.088 D - -			D	173	/-	/-	/103	/221	/-	
Des Ld:	55.00	Mean Height:	25.51 ft				Creep Factor: 2.0			Wind reactions based on C&C							
NCBCLL:	10.00	TCDL:	6.0 psf	Building Code:			Max TC CSI: 0.872			G	Brg Wid = 8.0		Min Req = 1.5 (Truss)				
Soffit:	2.00	BCDL:	6.0 psf	TPI Std: 2014			Max BC CSI: 0.480			E	Brg Wid = 1.5		Min Req = -				
Load Duration:	1.25	MWFRS Parallel Dist:	0 to h/2	Rep Fac: Yes			Max Web CSI: 0.127			D	Brg Wid = 1.5		Min Req = -				
Spacing:	24.0 "	C&C Dist a:	3.00 ft	FT/RT:20(0)/10(0)			Mfg Specified Camber:			Bearing G is a rigid surface.							
		Loc. from endwall:	not in 9.00 ft	Plate Type(s):						Maximum Top Chord Forces Per Ply (lbs)							
		GCpi:	0.18	WAVE			VIEW Ver: 23.02.01A.1204.18			Chords		Tens.Comp.		Chords		Tens. Comp.	
		Wind Duration:	1.60							A - B		82 -169		C - D		55 -90	
										B - C		94 -234					
										D - E		103 -221					
										E - F		167 -190					
										F - G		221 -405					
										G - H		405 -745					
										H - I		745 -171					
										I - J		171 -173					
										J - K		173 -103					
										K - L		103 -482					
										L - M		482 -167					
										M - N		167 -171					
										N - O		171 -173					
										O - P		173 -103					
										P - Q		103 -482					
										Q - R		482 -167					
										R - S		167 -171					
										S - T		171 -173					
										T - U		173 -103					
										U - V		103 -482					
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										AR - AS		171 -173					
										AS - AT		173 -103					
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										AV - AW		167 -171					
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										AY - AZ		103 -482					
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										BL - BM		171 -173					
										BM - BN		173 -103					
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										CA - CB		171 -173					
										CB - CC		173 -103					
										CC - CD		103 -482					
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										CG - CH		173 -103					
										CH - CI		103 -482					
										CI - CJ		482 -167					
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										CO - CP		167 -171					
										CP - CQ		171 -173					
										CQ - CR		173 -103					
										CR - CS		103 -482					
										CS - CT		482 -167					
										CT - CU		167 -171					
										CU - CV		171 -173					
										CV - CW		173 -103					
										CW - CX		103 -482					
										CX - CY		482 -167					
										CY - CZ		167 -171					
										CZ - DA		171 -173					
										DA - DB		173 -103					
										DB - DC		103 -482					
										DC - DD		482 -167					
										DD - DE		167 -171					
										DE - DF		171 -173					
										DF - DG		173 -103					
										DG - DH		103 -482					
										DH - DI		482 -167					
										DI - DJ		167 -171					
										DJ - DK		171 -173					
										DK - DL		173 -103					
										DL - DM		103 -482					
										DM - DN		482 -167					
										DN - DO		167 -171					
										DO - DP		171 -173					
										DP - DQ		173 -103					
										DQ - DR		103 -482					
										DR - DS		482 -167					
										DS - DT		167 -171					
										DT - DU		171 -173					
										DU - DV		173 -103					
										DV - DW		103 -482					
										DW - DX		482 -167					
										DX - DY		167 -171					
										DY - DZ		171 -173					
										DZ - EA		173 -103					
										EA - EB		103 -482					
										EB - EC		482 -167					
										EC - ED		167 -171					
										ED - EE		171 -173					
										EE - EF		173 -103					
										EF - EG		103 -482					
										EG - EH		482 -167					
										EH - EI		167 -171					
										EI - EJ		171 -173					
										EJ - EK		173 -103					
										EK - EL		103 -482					
										EL - EM		482 -167					
										EM - EN		167 -171					
										EN - EO		171 -173					
										EO - EP		173 -103					
										EP - EQ		103 -482					
										EQ - ER		482 -167					
										ER - ES		167 -171					
										ES - ET		171 -173					
										ET - EU		173 -103					

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Wind
Member design based on both MWFRS and C&C.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

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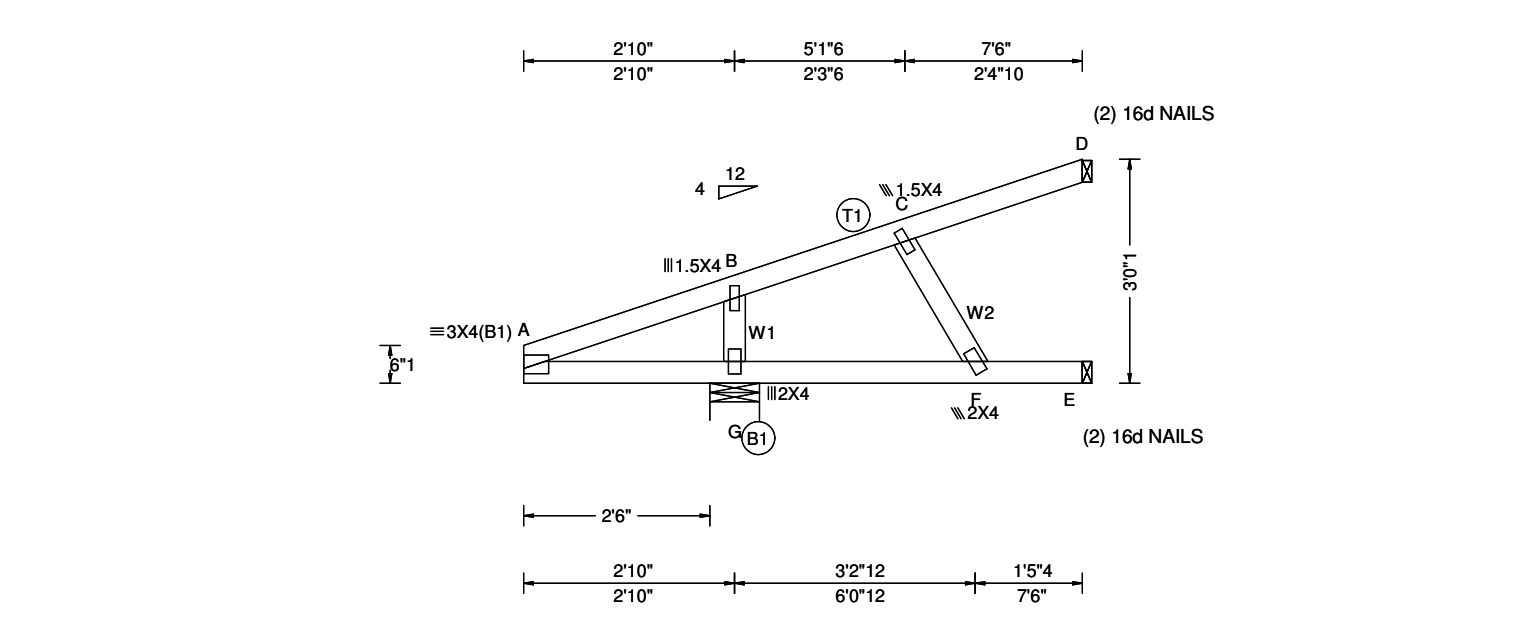


Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 175 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 25.26 ft TCDL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.123 A 275 360 VERT(CL): 0.213 A 159 240 HORZ(LL): -0.053 C - - HORZ(TL): 0.086 D - - Creep Factor: 2.0 Max TC CSI: 0.531 Max BC CSI: 0.364 Max Web CSI: 0.104 Mfg Specified Camber: VIEW Ver: 23.02.01A.1204.18	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
				G 681	/-	/-	/458	/780	/370	
				E 94	/-8	/-	/112	/110	/-	
				D 151	/-	/-	/99	/221	/-	
				Wind reactions based on C&C						
				G	Brg Wid = 8.0	Min Req = 1.5 (Truss)				
				E	Brg Wid = 1.5	Min Req = -				
				D	Brg Wid = 1.5	Min Req = -				
				Bearing G is a rigid surface.						
				Maximum Top Chord Forces Per Ply (lbs)						
				Chords	Tens.Comp.		Chords	Tens. Comp.		
				A - B	106	-236	C - D	48	-89	
				B - C	22	-118				
				Maximum Bot Chord Forces Per Ply (lbs)						
				Chords	Tens.Comp.		Chords	Tens. Comp.		
				A - G	258	-36	F - E	0	0	
				G - F	66	-137				
				Maximum Web Forces Per Ply (lbs)						
				Webs	Tens.Comp.		Webs	Tens. Comp.		
				B - G	587	-473	C - F	272	-118	

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Wind
Member design based on both MWFRS and C&C.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)							
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity			Non-Gravity				
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): 0.107 A 317 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.205 A 165 240	G	663	/-	/-	/462	/697	/322	
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.044 D - -	E	84	/-30	/-	/106	/100	/0	
	EXP: C Kzt: NA		HORZ(TL): 0.083 D - -	D	132	/-	/-	/81	/170	/-	
Des Ld: 55.00	Mean Height: 15.00 ft		Creep Factor: 2.0	Wind reactions based on C&C							
NCBCLL: 10.00	TCDL: 6.0 psf	Building Code:	Max TC CSI: 0.523	G	Brg	Wid = 8.0	Min	Req = 1.5 (Truss)			
Soffit: 2.00	BCDL: 6.0 psf	FBC 8th Ed. 2023 Res. HVHZ	Max BC CSI: 0.345	E	Brg	Wid = 1.5	Min	Req = -			
Load Duration: 1.25	MWFRS Parallel Dist: h/2 to h	TPI Std: 2014	Max Web CSI: 0.096	D	Brg	Wid = 1.5	Min	Req = -			
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Mfg Specified Camber:	Bearing G is a rigid surface.							
	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)							
	GCpi: 0.18	Plate Type(s):		Chords		Tens.Comp.		Chords		Tens. Comp.	
	Wind Duration: 1.60	WAVE	VIEW Ver: 23.02.01A.1204.18								

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Wind

Member design based on both MWFRS and C&C.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

WARNING

READ AND FOLLOW ALL NOTES ON THIS DRAWING!

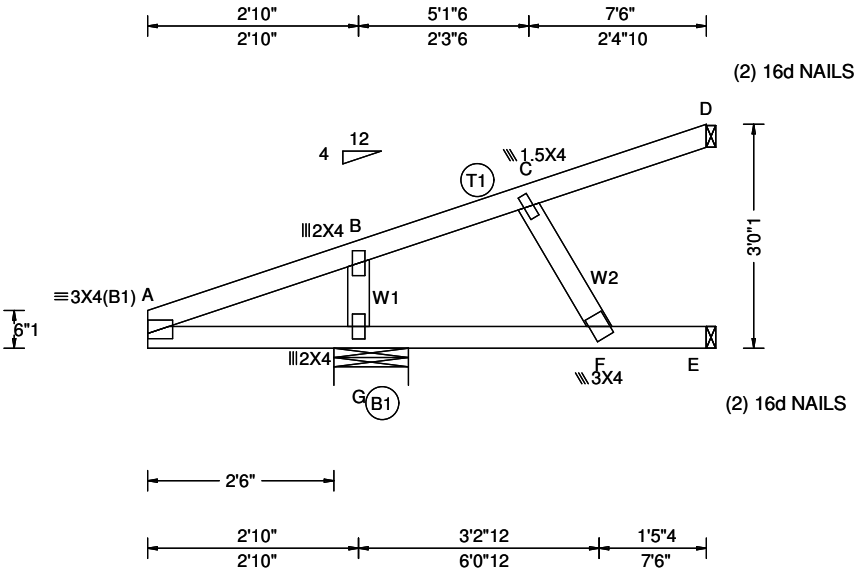
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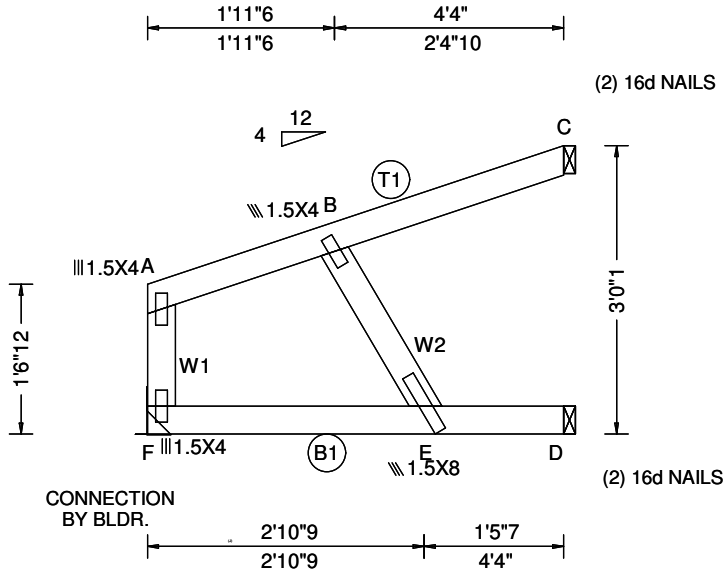


Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)								
TCLL:	30.00	Wind Std:	ASCE 7-22	Pg: NA	Ct: NA	CAT: NA	PP Deflection in	loc L/defl	L/#	Gravity			Non-Gravity					
TCDL:	15.00	Speed:	175 mph	Pf: NA		Ce: NA	VERT(LL):	0.107	A	317	360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL:	0.00	Enclosure:	Part. Enc.	Lu: NA	Cs: NA		VERT(CL):	0.203	A	166	240	G 663	/-		/-	/595	/881	/384
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA		HORZ(LL):	-0.044	D	-	-	E 84	/-29		/-	/79	/157	/0
		EXP:	C Kzt: NA				HORZ(TL):	0.082	D	-	-	D 132	/-		/-	/104	/218	/-
Des Ld:	55.00	Mean Height:	15.00 ft	Building Code:			Creep Factor:	2.0				Wind reactions based on C&C						
NCBCLL:	10.00	TCDL:	6.0 psf	FBC 8th Ed. 2023 Res.	HVHZ		Max TC CSI:	0.523				G	Brg Wid = 12.0	Min Req = 1.5	(Truss)			
Soffit:	2.00	BCDL:	6.0 psf	TPI Std:	2014		Max BC CSI:	0.436				E	Brg Wid = 1.5	Min Req = -				
Load Duration:	1.25	MWFRS Parallel Dist:	h/2 to h	Rep Fac:	Yes		Max Web CSI:	0.107				D	Brg Wid = 1.5	Min Req = -				
Spacing:	24.0 "	C&C Dist a:	3.00 ft	FT/RT:	20(0)/10(0)		Mfg Specified Camber:					Bearing G is a rigid surface.						
		Loc. from endwall:	not in 4.50 ft	Plate Type(s):								Maximum Top Chord Forces Per Ply (lbs)						
		GCpi:	0.55									Chords	Tens.Comp.		Chords	Tens. Comp.		
		Wind Duration:	1.60	WAVE			VIEW Ver:	23.02.01A.1204.18										

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Wind
Member design based on both MWFRS and C&C.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.
Left bottom chord exposed to wind.

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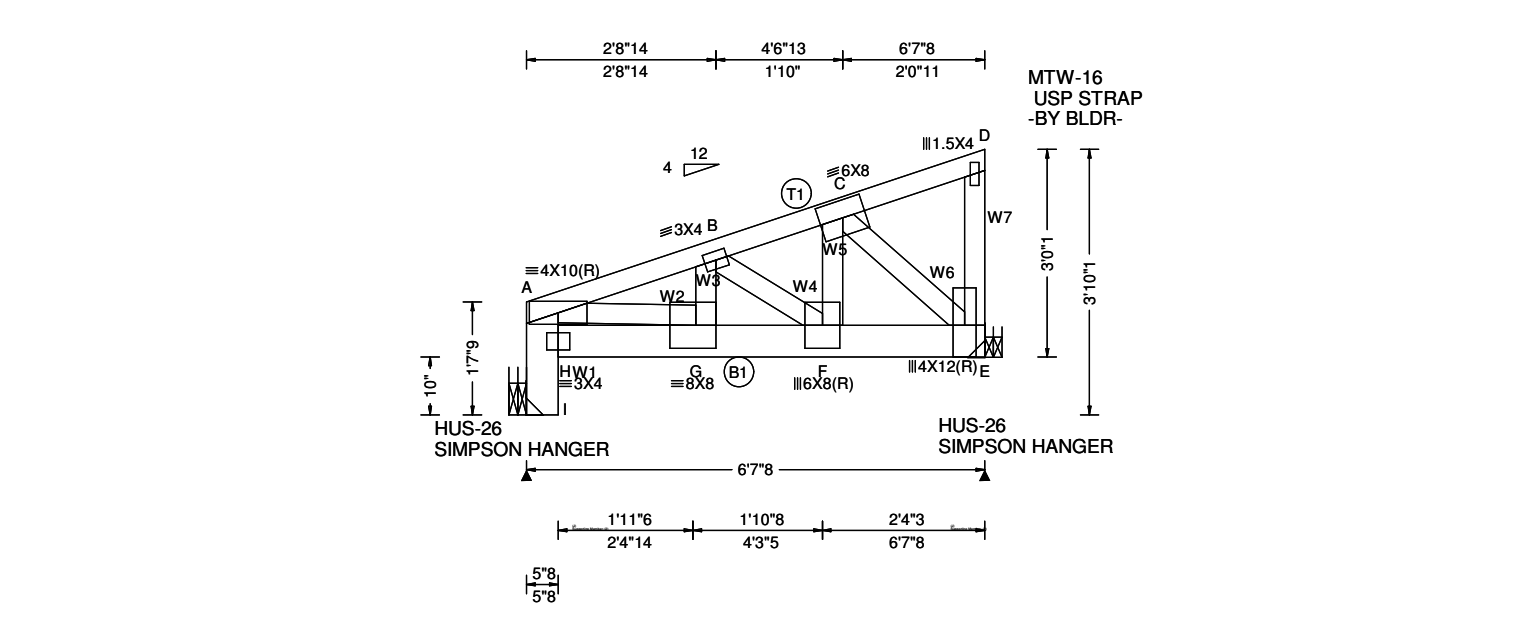


Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)							
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity				
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): -0.072 E 719 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL: 0.00		Enclosure: Part. Enc.		Lu: NA Cs: NA			VERT(CL): -0.081 E 642 240			F	238	/-	/-	/173	/232	/279	
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): -0.087 B - -			D	97	/-	/-	/117	/247	/-	
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.092 B - -			C	142	/-	/-	/113	/250	/-	
NCBCLL: 10.00		Mean Height: 26.53 ft					Creep Factor: 2.0			Wind reactions based on C&C							
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.268			F Brg Wid = - Min Req = -							
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.547			D Brg Wid = 1.5 Min Req = -							
Spacing: 24.0 "		MWFRS Parallel Dist: 0 to h/2					Max Web CSI: 0.173			C Brg Wid = 1.5 Min Req = -							
		C&C Dist a: 3.00 ft					Mfg Specified Camber:			Maximum Top Chord Forces Per Ply (lbs)							
		Loc. from endwall: not in 4.50 ft								Chords		Tens.Comp.		Chords		Tens. Comp.	
		GCpi: 0.55								A - B		22 -92		B - C		45 -89	
		Wind Duration: 1.60					VIEW Ver: 23.02.01A.1204.18										

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Wind
Member design based on both MWFRS and C&C.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
				Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	I	2347	-	-	-	-	/843 -
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): 0.041 G 999 360	E	3323	-	-	-	-	/1121 -
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.074 G 999 240	Wind reactions based on MWFRS						
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.043 D - -	I	Brg Wid = -		Min Req = -			
Des Ld: 55.00	EXP: C Kzt: NA		HORZ(TL): 0.078 D - -	E	Brg Wid = -		Min Req = -			
NCBCLL: 10.00	Mean Height: 26.15 ft		Creep Factor: 2.0	Maximum Top Chord Forces Per Ply (lbs)						
Soffit: 2.00	TCDL: 6.0 psf	Building Code:	Max TC CSI: 0.383	Chords	Tens.Comp.		Chords	Tens. Comp.		
Load Duration: 1.25	BCDL: 6.0 psf	FBC 8th Ed. 2023 Res. HVHZ	Max BC CSI: 0.364	A - B	1202 -3475		C - D	32		-40
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max Web CSI: 0.883	B - C	914 -2653					
	C&C Dist a: 3.00 ft	Rep Fac: Varies by Ld Case	Mfg Specified Camber:							
	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)	VIEW Ver: 23.02.01A.1204.18							
	GCpi: 0.18	Plate Type(s):								
	Wind Duration: 1.60	WAVE								

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x6 SP M-26;
Webs: 2x4 SP #2; W1 2x6 SP #2;

Special Loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 90 plf at 0.00 to 90 plf at 6.62
BC: From 10 plf at 0.46 to 10 plf at 6.62
BC: 1671 lb Conc. Load at 2.06, 4.06, 6.06

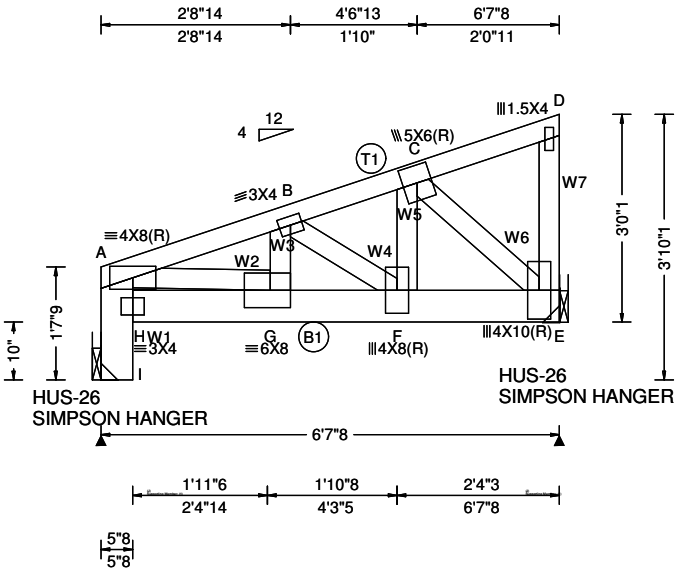
Hangers / Ties
(J) Hanger Support Required, by others

Wind
Wind loads and reactions based on MWFRS.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)
Chords Tens.Comp. Chords Tens. Comp.
H - G 214 -611 F - E 2174 -748
G - F 3123 -1082

Maximum Web Forces Per Ply (lbs)
Webs Tens.Comp. Webs Tens. Comp.
A - H 732 -1979 B - F 302 -803
A - G 3910 -1338 F - C 2608 -800
H - I 833 -2319 C - E 1009 -2935
G - B 661 -127 D - E 70 -85

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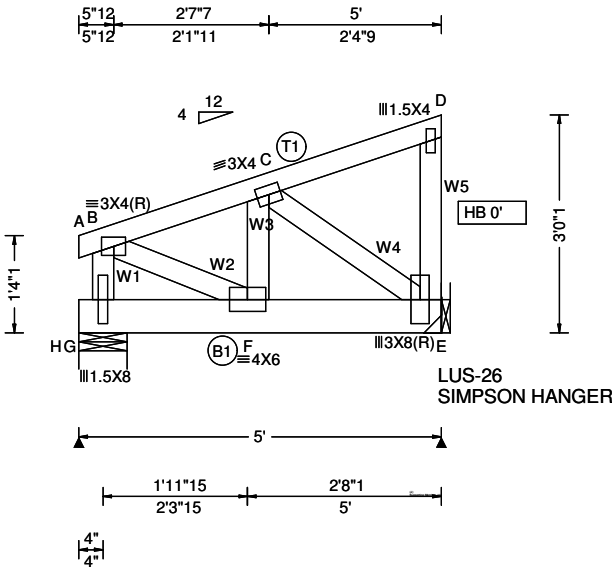


Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity			Non-Gravity			
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): 0.035 G 999 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.063 G 999 240	I	1890	-/-	-/-	-/-	/727	-/-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.037 D - -	E	2665	-/-	-/-	-/-	/950	-/-
Des Ld: 55.00	EXP: C Kzt: NA	Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	HORZ(TL): 0.066 D - -	Wind reactions based on MWFRS						
NCBCLL: 10.00	Mean Height: 26.15 ft		Creep Factor: 2.0	I	Brg Wid = -		Min Req = -			
Soffit: 2.00	TCDL: 6.0 psf		Max TC CSI: 0.309	E	Brg Wid = -		Min Req = -			
Load Duration: 1.25	BCDL: 6.0 psf		Max BC CSI: 0.816	Maximum Top Chord Forces Per Ply (lbs)						
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2		Max Web CSI: 0.736	Chords	Tens.Comp.	Chords		Tens. Comp.		
	C&C Dist a: 3.00 ft	Mfg Specified Camber:	A - B	1052 - 2859	C - D		32	- 40		
	Loc. from endwall: not in 4.50 ft		B - C	789 - 2157						
	GCpi: 0.18		VIEW Ver: 23.02.01A.1204.18							
	Wind Duration: 1.60									

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x6 SP #2;
Webs: 2x4 SP #2; W1 2x6 SP #2;

Special Loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 90 plf at 0.00 to 90 plf at 6.62
BC: From 10 plf at 0.46 to 10 plf at 6.62
BC: 1278 lb Conc. Load at 2.06
BC: 1310 lb Conc. Load at 4.06, 6.06

Wind
Wind loads and reactions based on MWFRS.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)																																																																																					
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 175 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 25.59 ft TCDL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.006 F 999 360 VERT(CR): 0.012 F 999 240 HORZ(LL): -0.002 D - - HORZ(TL): 0.003 D - - Creep Factor: 2.0 Max TC CSI: 0.122 Max BC CSI: 0.459 Max Web CSI: 0.199 Mfg Specified Camber: VIEW Ver: 23.02.01A.1204.18	<table><tr><th colspan="2">Gravity</th><th colspan="3">Non-Gravity</th></tr><tr><th>Loc</th><th>R+ / R-</th><th>/ Rh</th><th>/ Rw</th><th>/ U / RL</th></tr><tr><td>H</td><td>949</td><td>-/-</td><td>-/-</td><td>/358 -/-</td></tr><tr><td>E</td><td>867</td><td>-/-</td><td>-/-</td><td>/336 -/-</td></tr></table> Wind reactions based on MWFRS H Brg Wid = 8.0 Min Req = 1.5 (Truss) E Brg Wid = - Min Req = - Bearing H is a rigid surface. <table><tr><th colspan="5">Maximum Top Chord Forces Per Ply (lbs)</th></tr><tr><th>Chords</th><th>Tens.</th><th>Comp.</th><th>Chords</th><th>Tens. Comp.</th></tr><tr><td>A - B</td><td>5</td><td>-4</td><td>C - D</td><td>36 -44</td></tr><tr><td>B - C</td><td>324</td><td>-879</td><td></td><td></td></tr></table> <table><tr><th colspan="5">Maximum Bot Chord Forces Per Ply (lbs)</th></tr><tr><th>Chords</th><th>Tens.</th><th>Comp.</th><th>Chords</th><th>Tens. Comp.</th></tr><tr><td>H - G</td><td>0</td><td>0</td><td>F - E</td><td>730 -274</td></tr><tr><td>G - F</td><td>11</td><td>-5</td><td></td><td></td></tr></table> <table><tr><th colspan="5">Maximum Web Forces Per Ply (lbs)</th></tr><tr><th>Webs</th><th>Tens.</th><th>Comp.</th><th>Webs</th><th>Tens. Comp.</th></tr><tr><td>B - G</td><td>316</td><td>-756</td><td>C - E</td><td>339 -905</td></tr><tr><td>B - F</td><td>881</td><td>-310</td><td>D - E</td><td>74 -92</td></tr><tr><td>F - C</td><td>568</td><td>-83</td><td></td><td></td></tr></table>	Gravity		Non-Gravity			Loc	R+ / R-	/ Rh	/ Rw	/ U / RL	H	949	-/-	-/-	/358 -/-	E	867	-/-	-/-	/336 -/-	Maximum Top Chord Forces Per Ply (lbs)					Chords	Tens.	Comp.	Chords	Tens. Comp.	A - B	5	-4	C - D	36 -44	B - C	324	-879			Maximum Bot Chord Forces Per Ply (lbs)					Chords	Tens.	Comp.	Chords	Tens. Comp.	H - G	0	0	F - E	730 -274	G - F	11	-5			Maximum Web Forces Per Ply (lbs)					Webs	Tens.	Comp.	Webs	Tens. Comp.	B - G	316	-756	C - E	339 -905	B - F	881	-310	D - E	74 -92	F - C	568	-83		
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x6 SP #2;
Webs: 2x4 SP #2;

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From 90 plf at 0.00 to 90 plf at 5.00
BC: From 10 plf at 0.00 to 10 plf at 5.00
BC: 658 lb Conc. Load at 1.48, 3.48

Wind

Wind loads and reactions based on MWFRS.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
H - G	0	0	F - E	730	-274
G - F	11	-5			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - G	316	-756	C - E	339	-905
B - F	881	-310	D - E	74	-92
F - C	568	-83			

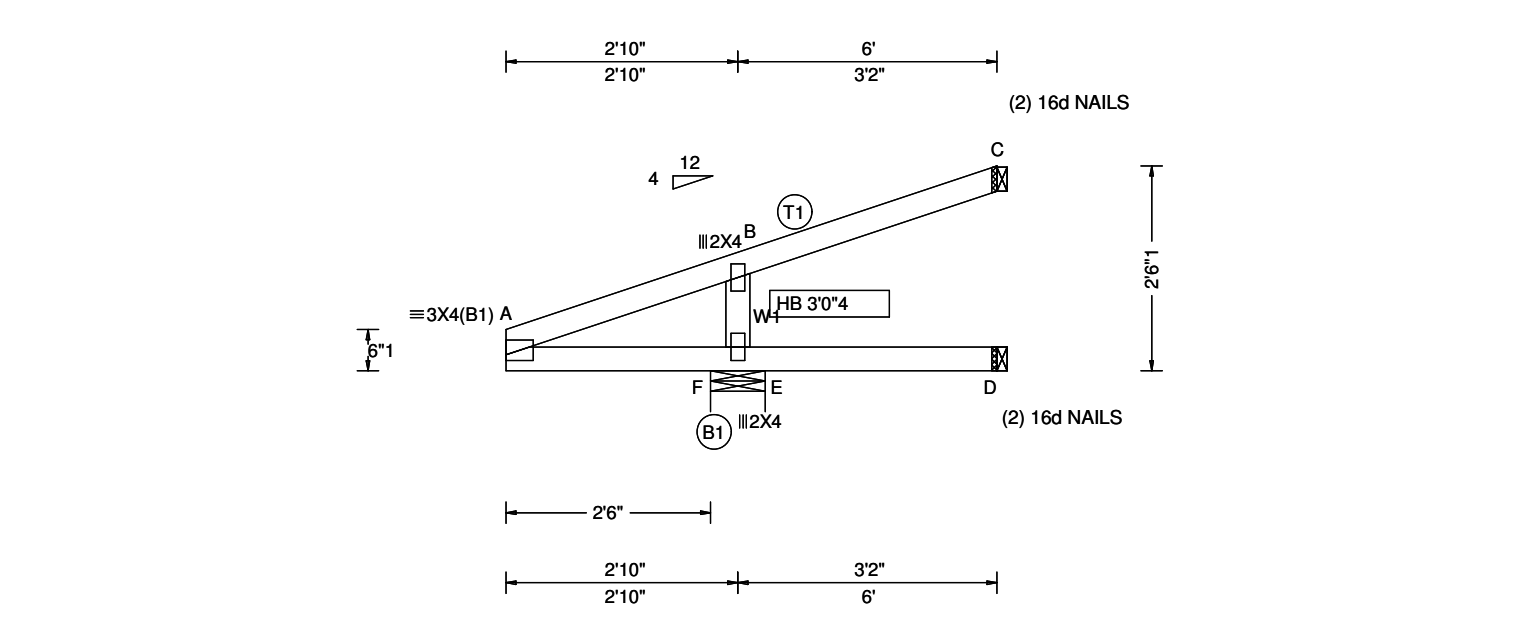
****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!

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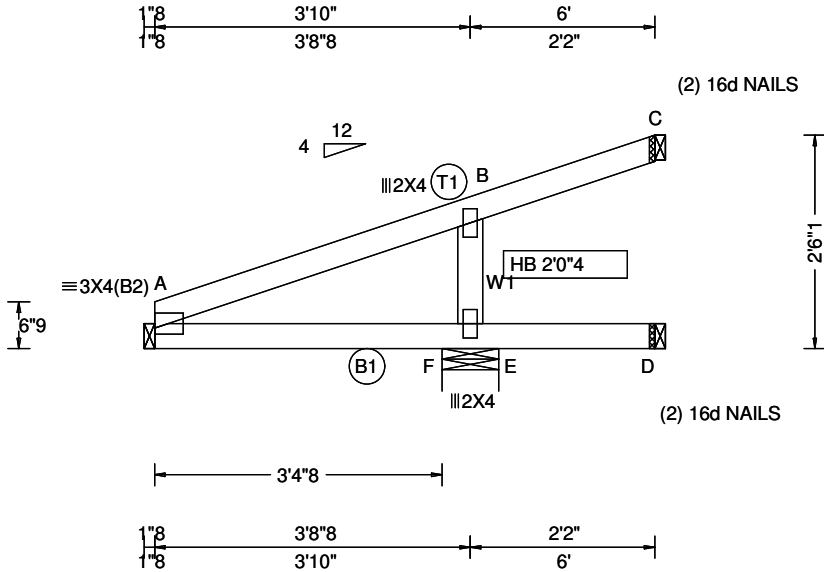
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)						
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity			
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): -0.154 A 231 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.282 A 126 240			F	655	/-	/-	/477	/794	/284
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.060 C - -			D	6	/-46	/-	/88	/49	/-
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.113 C - -			C	92	/-39	/-	/81	/164	/-
NCBCLL: 10.00		Mean Height: 15.00 ft					Creep Factor: 2.0			Wind reactions based on C&C						
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.607			F Brg Wid = 8.0 Min Req = 1.5 (Truss)						
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.441			D Brg Wid = 1.5 Min Req = -						
Spacing: 24.0 "		MWFRS Parallel Dist: 0 to h/2		Max Web CSI: 0.140			Mfg Specified Camber:			C Brg Wid = 1.5 Min Req = -						
		C&C Dist a: 3.00 ft								Bearing F is a rigid surface.						
		Loc. from endwall: not in 4.50 ft								Maximum Top Chord Forces Per Ply (lbs)						
		GCpi: 0.18								Chords Tens.Comp. Chords Tens. Comp.						
		Wind Duration: 1.60					VIEW Ver: 23.02.01A.1204.18									

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2;									
Wind Member design based on both MWFRS and C&C. Left cantilever is exposed to wind Wind loading based on both gable and hip roof types.									

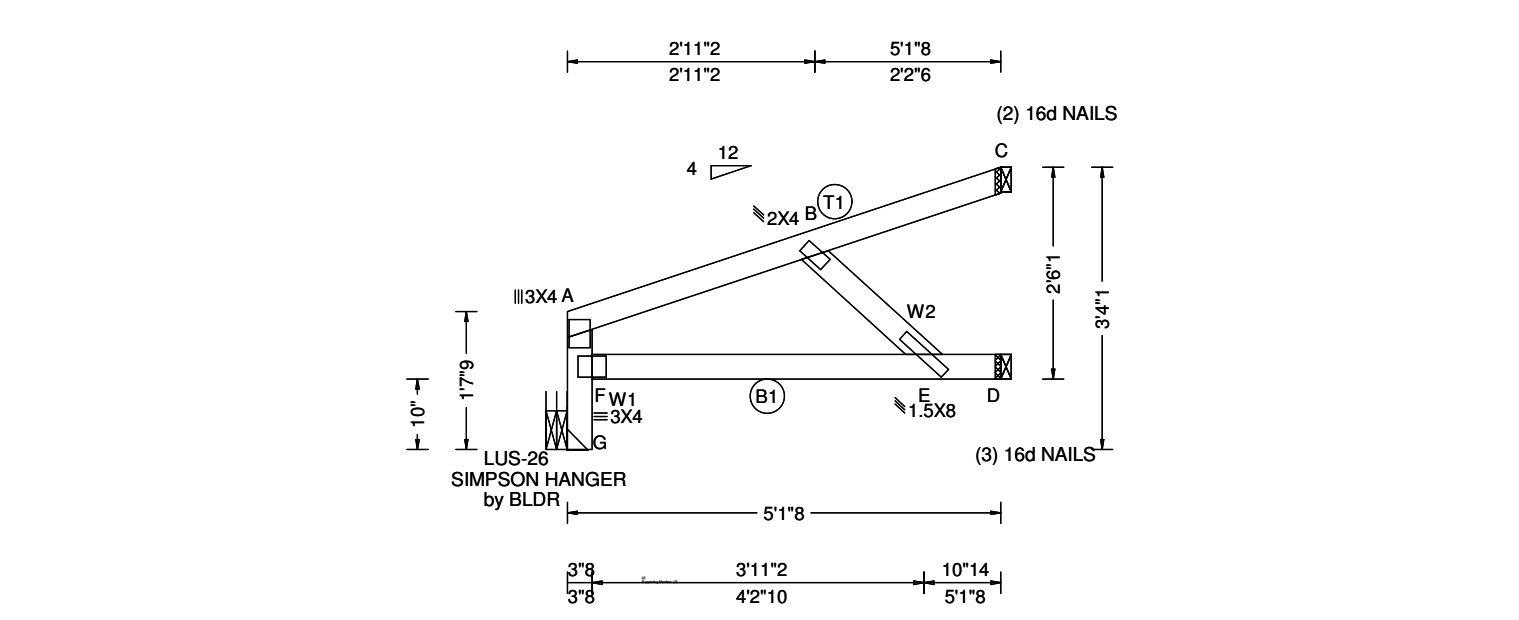
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)						
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity Non-Gravity						
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): -0.003 A 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.003 A 999 240			A	179	/-	/-	/103	/119	/311
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): -0.005 B - -			F	427	/-	/-	/289	/615	/-
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.006 B - -			D	16	/-	/-	/18	/-	/-
NCBCLL: 10.00		Mean Height: 25.78 ft					Creep Factor: 2.0			Wind reactions based on C&C						
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.321			A Brg Wid = 1.5 Min Req = -						
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.143			F Brg Wid = 8.0 Min Req = 1.5 (Truss)						
Spacing: 24.0 "		MWFRS Parallel Dist: h to 2h		Wind Duration: 1.60			Max Web CSI: 0.132			D Brg Wid = 1.5 Min Req = -						
		C&C Dist a: 3.00 ft					Mfg Specified Camber:			C Brg Wid = 1.5 Min Req = -						
		Loc. from endwall: not in 9.00 ft					VIEW Ver: 23.02.01A.1204.18			Bearing F is a rigid surface.						
		GCpi: 0.18								Maximum Top Chord Forces Per Ply (lbs)						

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2;	<table><tr><th>Chords</th><th>Tens.Comp.</th><th>Chords</th><th>Tens. Comp.</th></tr><tr><td>A - B</td><td>53 -228</td><td>B - C</td><td>21 -45</td></tr></table>	Chords	Tens.Comp.	Chords	Tens. Comp.	A - B	53 -228	B - C	21 -45				
Chords	Tens.Comp.	Chords	Tens. Comp.										
A - B	53 -228	B - C	21 -45										
Wind Member design based on both MWFRS and C&C. Wind loading based on both gable and hip roof types.	<table><tr><th colspan="4">Maximum Bot Chord Forces Per Ply (lbs)</th></tr><tr><th>Chords</th><th>Tens.Comp.</th><th>Chords</th><th>Tens. Comp.</th></tr><tr><td>A - E</td><td>8 -13</td><td>E - D</td><td>0 0</td></tr></table>	Maximum Bot Chord Forces Per Ply (lbs)				Chords	Tens.Comp.	Chords	Tens. Comp.	A - E	8 -13	E - D	0 0
Maximum Bot Chord Forces Per Ply (lbs)													
Chords	Tens.Comp.	Chords	Tens. Comp.										
A - E	8 -13	E - D	0 0										
Additional Notes Shim all supports to solid bearing.	<table><tr><th colspan="2">Maximum Web Forces Per Ply (lbs)</th></tr><tr><th>Webs</th><th>Tens.Comp.</th></tr><tr><td>B - E</td><td>613 -332</td></tr></table>	Maximum Web Forces Per Ply (lbs)		Webs	Tens.Comp.	B - E	613 -332						
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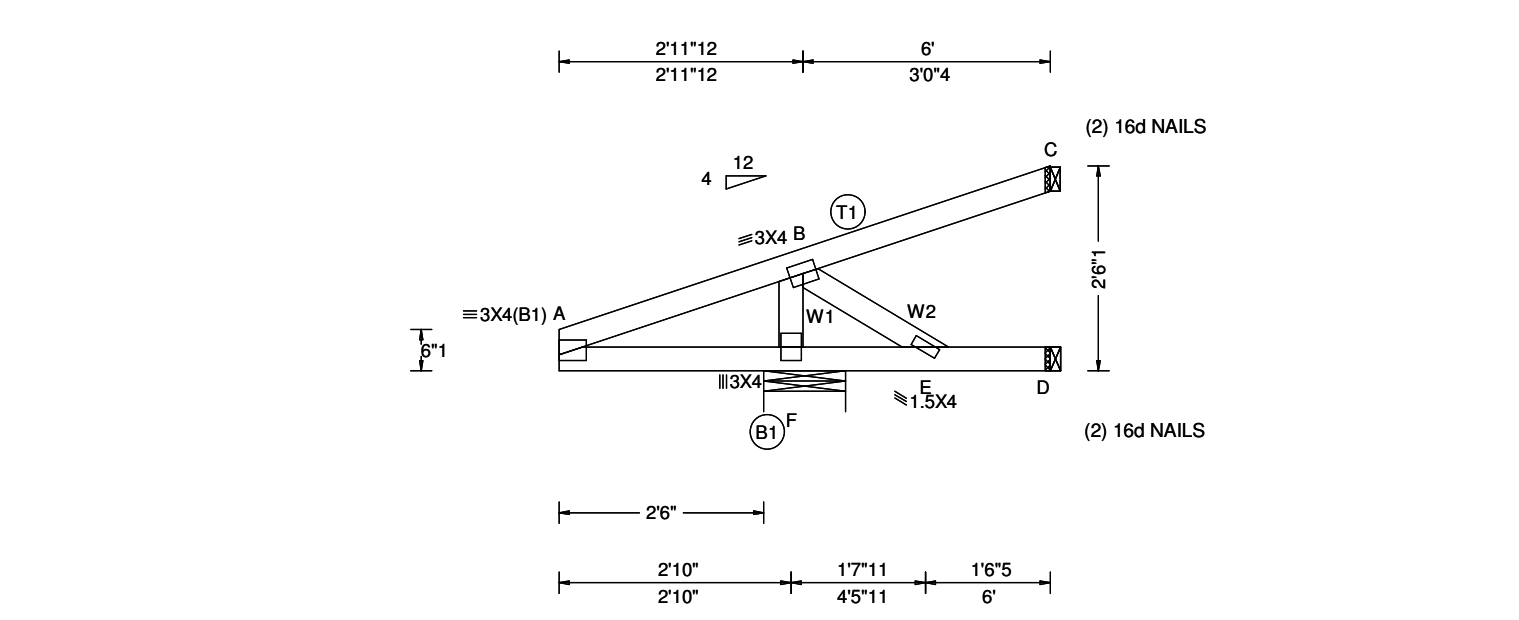
Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)							
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity		Non-Gravity					
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): -0.052 E 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.065 B 923 240			G	284	/-	/-	/166	/235	/279	
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): -0.216 C - -			D	152	/-	/-	/127	/346	/-	
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.231 C - -			C	122	/-	/-	/67	/135	/-	
NCBCLL: 10.00		Mean Height: 25.90 ft					Creep Factor: 2.0			Wind reactions based on C&C							
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.344			G Brg Wid = - Min Req = -							
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.474			D Brg Wid = 1.5 Min Req = -							
Spacing: 24.0 "		MWFRS Parallel Dist: 0 to h/2					Max Web CSI: 0.479			C Brg Wid = 1.5 Min Req = -							
		C&C Dist a: 3.00 ft					Mfg Specified Camber:			Maximum Top Chord Forces Per Ply (lbs)							
		Loc. from endwall: not in 4.50 ft								Chords		Tens.Comp.		Chords		Tens. Comp.	
		GCpi: 0.18															
		Wind Duration: 1.60					VIEW Ver: 23.02.01A.1204.18			A - B 250 -186 B - C 38 -52							

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Hangers / Ties
(J) Hanger Support Required, by others

Wind
Member design based on both MWFRS and C&C.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)																																																												
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 175 mph Enclosure: Part. Enc. Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.030 A 999 360 VERT(CL): 0.064 A 534 240 HORZ(LL): -0.011 B - - HORZ(TL): 0.023 B - - Creep Factor: 2.0 Max TC CSI: 0.359 Max BC CSI: 0.258 Max Web CSI: 0.135 Mfg Specified Camber: VIEW Ver: 23.02.01A.1204.18	<table><tr><th colspan="2">Gravity</th><th colspan="3">Non-Gravity</th></tr><tr><th>Loc</th><th>R+ / R-</th><th>/ Rh</th><th>/ Rw</th><th>/ U / RL</th></tr><tr><td>F</td><td>625</td><td>/-</td><td>/-</td><td>/563 /905 /334</td></tr><tr><td>C</td><td>111</td><td>/-</td><td>/-</td><td>/88 /205 /-</td></tr><tr><td>D</td><td>-</td><td>/-71</td><td>/-</td><td>/37 /62 /-</td></tr></table> Wind reactions based on C&C F Brg Wid = 12.0 Min Req = 1.5 (Truss) C Brg Wid = 1.5 Min Req = - D Brg Wid = 1.5 Min Req = - Bearing F is a rigid surface. <table><tr><th colspan="2">Maximum Top Chord Forces Per Ply (lbs)</th><th colspan="2">Chords</th><th>Tens. Comp.</th></tr><tr><td>A - B</td><td>354</td><td>-468</td><td>B - C</td><td>35 -89</td></tr></table> <table><tr><th colspan="2">Maximum Bot Chord Forces Per Ply (lbs)</th><th colspan="2">Chords</th><th>Tens. Comp.</th></tr><tr><td>A - F</td><td>473</td><td>-290</td><td>E - D</td><td>0 0</td></tr><tr><td>F - E</td><td>228</td><td>-293</td><td></td><td></td></tr></table> <table><tr><th colspan="2">Maximum Web Forces Per Ply (lbs)</th><th colspan="2">Webs</th><th>Tens. Comp.</th></tr><tr><td>F - B</td><td>765</td><td>-615</td><td>B - E</td><td>367 -286</td></tr></table>	Gravity		Non-Gravity			Loc	R+ / R-	/ Rh	/ Rw	/ U / RL	F	625	/-	/-	/563 /905 /334	C	111	/-	/-	/88 /205 /-	D	-	/-71	/-	/37 /62 /-	Maximum Top Chord Forces Per Ply (lbs)		Chords		Tens. Comp.	A - B	354	-468	B - C	35 -89	Maximum Bot Chord Forces Per Ply (lbs)		Chords		Tens. Comp.	A - F	473	-290	E - D	0 0	F - E	228	-293			Maximum Web Forces Per Ply (lbs)		Webs		Tens. Comp.	F - B	765	-615	B - E	367 -286
Gravity		Non-Gravity																																																														
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F - B	765	-615	B - E	367 -286																																																												

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Wind
Member design based on both MWFRS and C&C.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.
Left bottom chord exposed to wind.

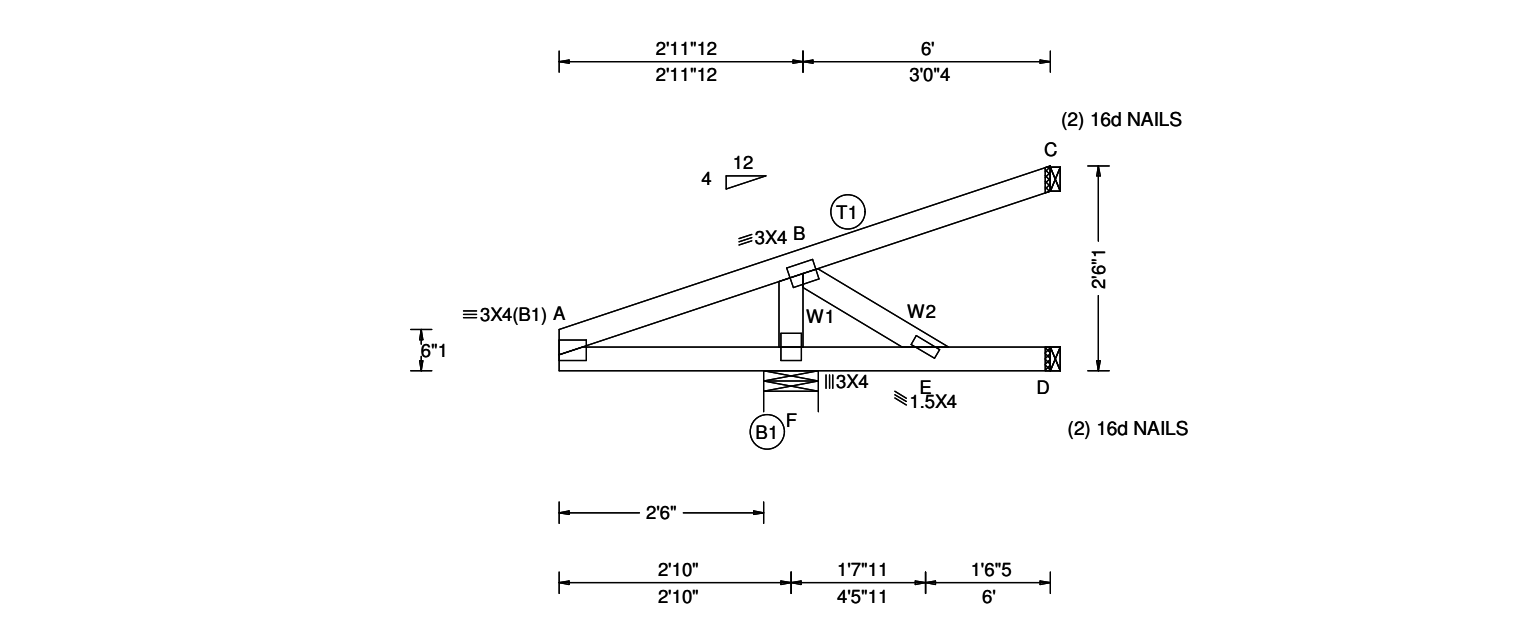
****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!

****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCEA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

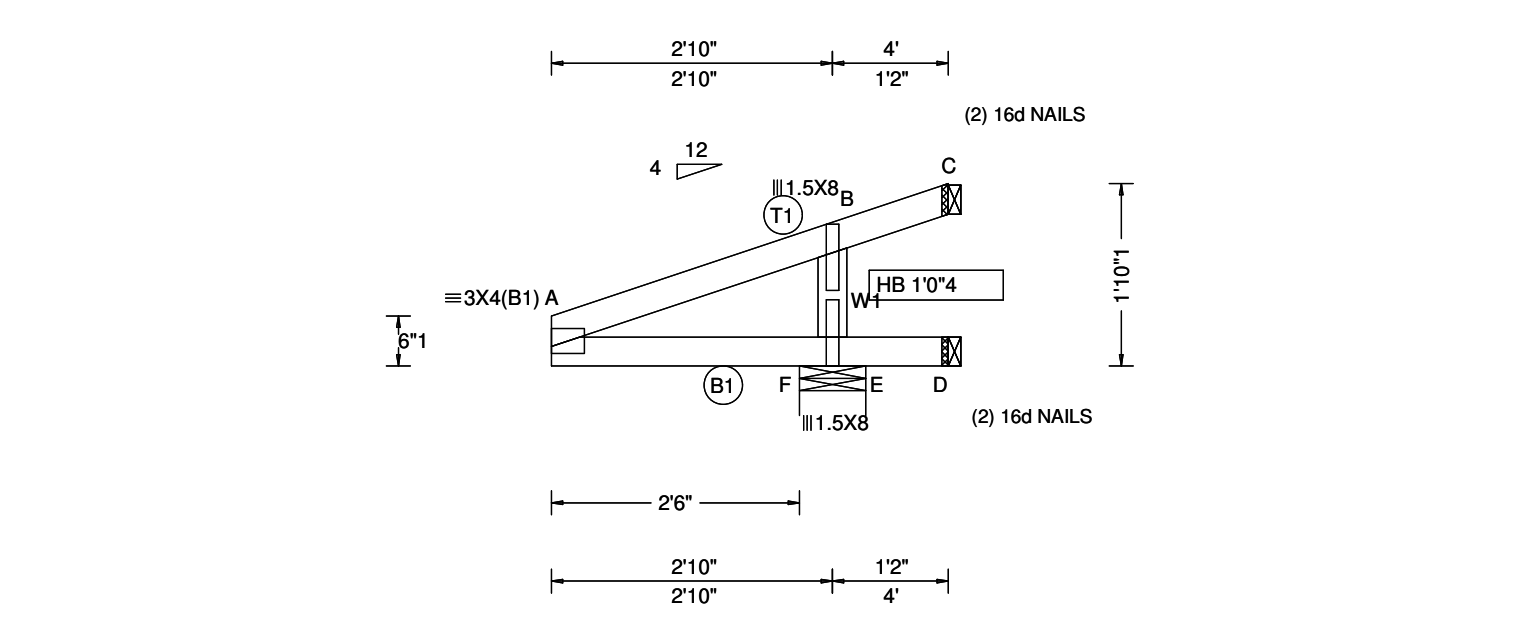
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCEA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity			Non-Gravity			
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): -0.052 A 654 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00	Enclosure: Part. Enc.	Lu: NA Cs: NA	VERT(CL): 0.064 A 530 240	F	625	/-	/-	/483	/784	/334
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.019 B - -	D	-	/-72	/-	/120	/96	/-
	EXP: C Kzt: NA		HORZ(TL): 0.023 B - -	C	112	/-	/-	/100	/209	/-
Des Ld: 55.00	Mean Height: 15.00 ft	Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Creep Factor: 2.0	Wind reactions based on C&C						
NCBCLL: 10.00	TCDL: 6.0 psf		Max TC CSI: 0.378	F	Brg Wid = 8.0	Min Req = 1.5 (Truss)				
Soffit: 2.00	BCDL: 6.0 psf		Max BC CSI: 0.280	D	Brg Wid = 1.5	Min Req = -				
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2		Max Web CSI: 0.135	C	Brg Wid = 1.5	Min Req = -				
Spacing: 24.0 "	C&C Dist a: 3.00 ft		Mfg Specified Camber:	Bearing F is a rigid surface.						
	Loc. from endwall: not in 4.50 ft		VIEW Ver: 23.02.01A.1204.18	Maximum Top Chord Forces Per Ply (lbs)						
	GCpi: 0.55			Chords		Tens.Comp.		Chords		Tens. Comp.
	Wind Duration: 1.60									

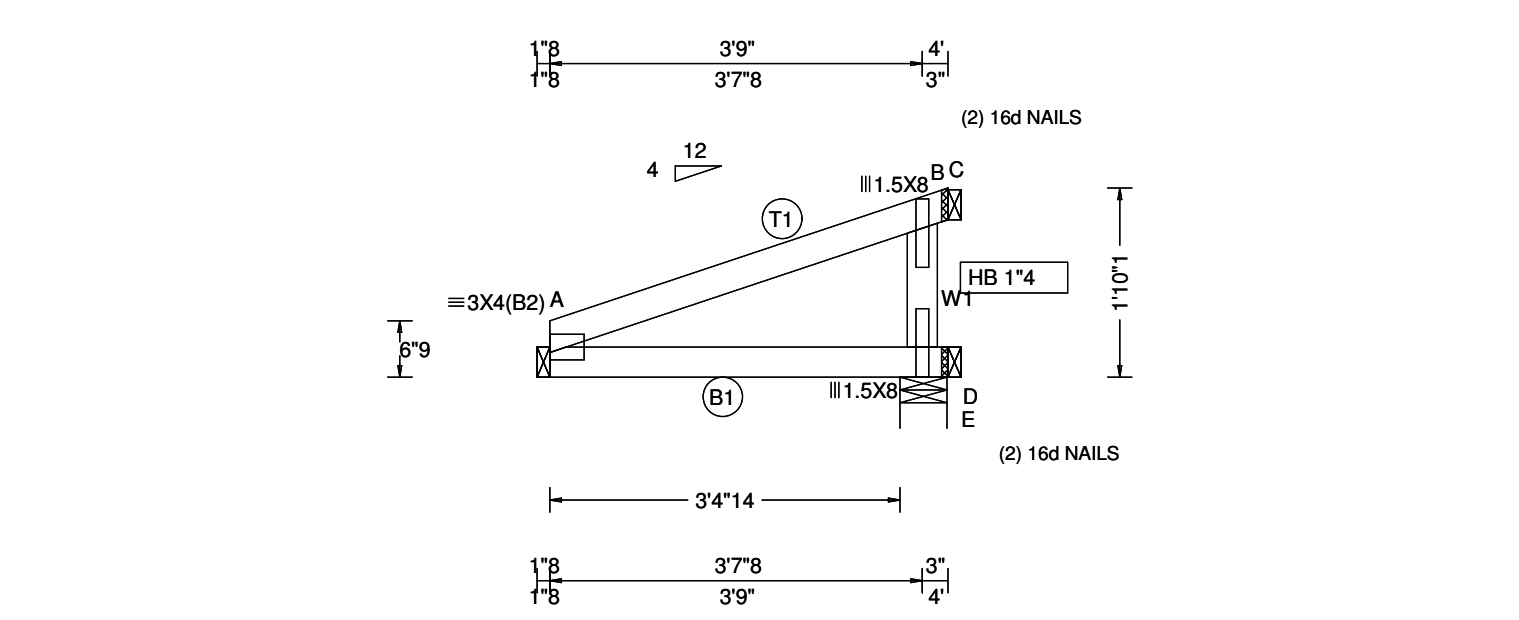
Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2;									
Wind Member design based on both MWFRS and C&C. Left cantilever is exposed to wind Wind loading based on both gable and hip roof types.									

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS
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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCEA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org



Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)						
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity			
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): -0.191 A 186 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.202 A 176 240			F	862	/-	/-	/685	/1466	/258
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.075 C - -			D	-	/-214	/-	/363	/221	/-
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.081 C - -			C	-	/-238	/-	/323	/179	/-
NCBCLL: 10.00		Mean Height: 15.00 ft					Creep Factor: 2.0			Wind reactions based on C&C						
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.648			F Brg Wid = 8.0 Min Req = 1.5 (Truss)						
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.547			D Brg Wid = 1.5 Min Req = -						
Spacing: 24.0 "		MWFRS Parallel Dist: 0 to h/2					Max Web CSI: 0.220			C Brg Wid = 1.5 Min Req = -						
		C&C Dist a: 3.00 ft		Mfg Specified Camber:			Bearing F is a rigid surface.									
		Loc. from endwall: Any		VIEW Ver: 23.02.01A.1204.18			Maximum Top Chord Forces Per Ply (lbs)									
		GCpi: 0.18					Chords Tens.Comp. Chords Tens. Comp.									
		Wind Duration: 1.60					Chords Tens.Comp. Chords Tens. Comp.									

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2;	
Wind Member design based on both MWFRS and C&C. Left cantilever is exposed to wind Wind loading based on both gable and hip roof types.	
Additional Notes Negative reaction(s) of -238# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.	

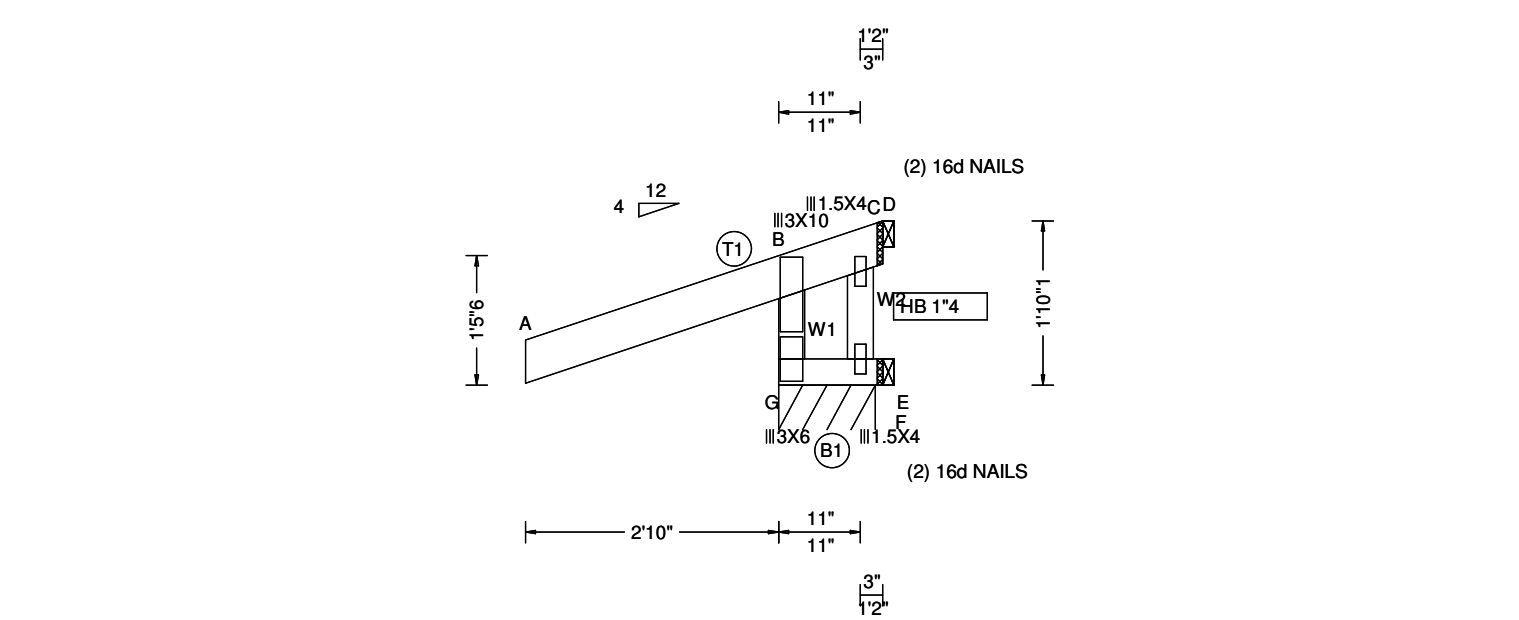


Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)								
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity					
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): NA			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL		
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): NA			A	191	/-	/-	/117	/190	/211		
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): -0.006 A - -			E	834	/-	/-	/580	/1178	/-		
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.006 A - -			D	-	/-287	/-	/122	/186	/-		
NCBCLL: 10.00		Mean Height: 25.44 ft					Creep Factor: 2.0			C			-	/-378	/-	/708	/234	/-
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.375			Wind reactions based on C&C								
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.153			A Brg Wid = 1.5 Min Req = -								
Spacing: 24.0 "		MWFRS Parallel Dist: h to 2h		Max Web CSI: 0.230			E Brg Wid = 5.5 Min Req = 1.5 (Truss)											
		C&C Dist a: 3.00 ft		Mfg Specified Camber:			D Brg Wid = 1.5 Min Req = -											
		Loc. from endwall: not in 9.00 ft		VIEW Ver: 23.02.01A.1204.18			C Brg Wid = 1.5 Min Req = -											
		GCpi: 0.18					Bearing E is a rigid surface.											
		Wind Duration: 1.60					Maximum Top Chord Forces Per Ply (lbs)											

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Wind
Member design based on both MWFRS and C&C.
Wind loading based on both gable and hip roof types.

Additional Notes
Negative reaction(s) of -378# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.
Shim all supports to solid bearing.

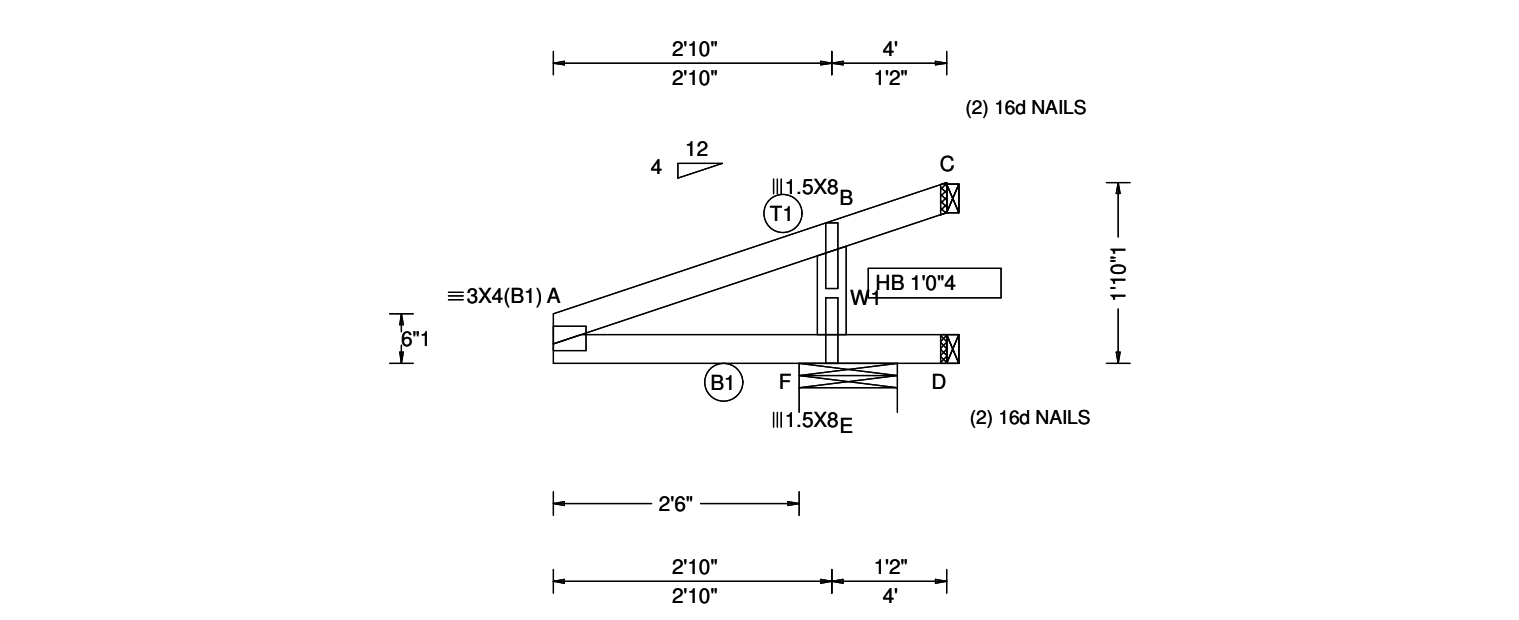


Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)								
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity					
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): -0.002 B 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL		
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): -0.002 B 999 240			G	618	/-	/-	/469	/1756	/-		
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): -0.001 B - -			E	-	/-4	/2	/1	/3	/8		
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.001 B - -			D	-	/-220	/2	/791	/198	/345		
NCBCLL: 10.00		Mean Height: 25.42 ft					Creep Factor: 2.0			Wind reactions based on C&C								
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.825			G Brg Wid = 13.0 Min Req = 1.5 (Truss)								
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.015			E Brg Wid = 1.5 Min Req = -								
Spacing: 24.0 "		MWFRS Parallel Dist: 0 to h/2		Max Web CSI: 0.359			Mfg Specified Camber:			D Brg Wid = 1.5 Min Req = -								
		C&C Dist a: 3.00 ft								Bearing G is a rigid surface.								
		Loc. from endwall: Any								Maximum Top Chord Forces Per Ply (lbs)								
		GCpi: 0.18								Chords Tens.Comp. Chords Tens. Comp.								
		Wind Duration: 1.60					VIEW Ver: 23.02.01A.1204.18											

Lumber
Top chord: 2x6 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

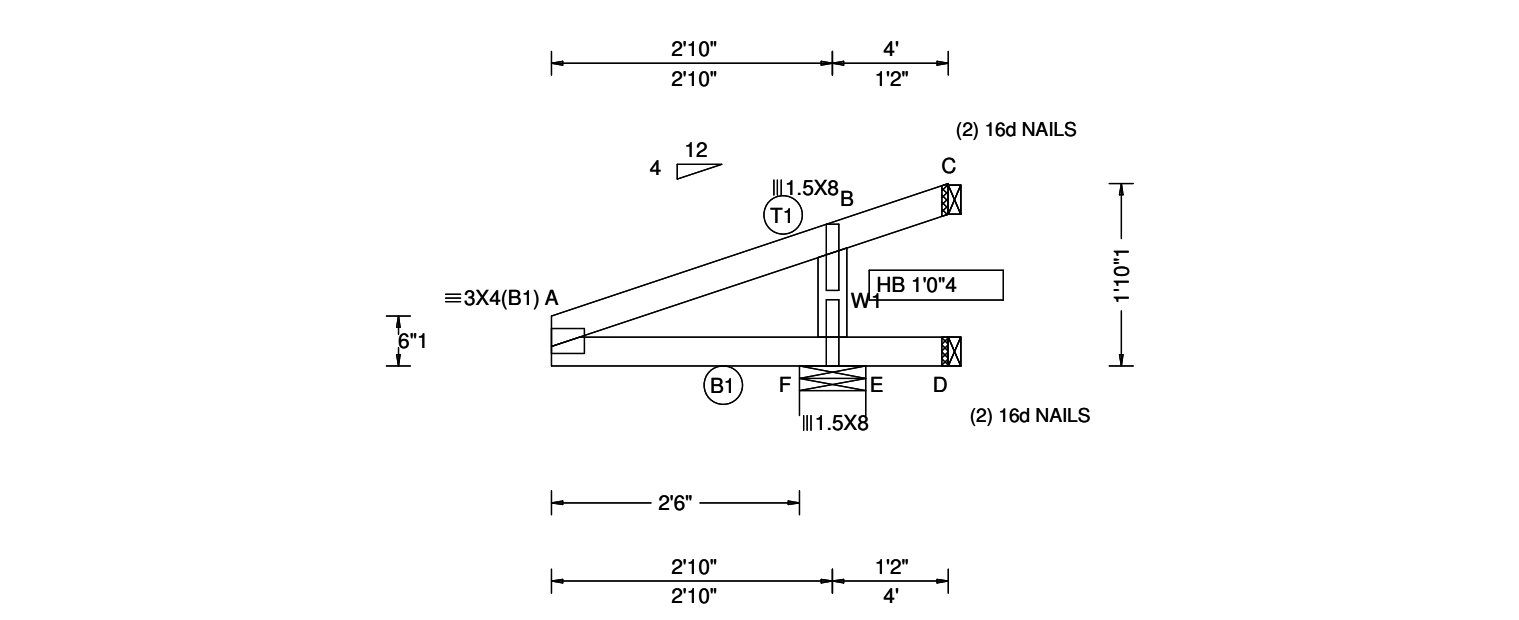
Wind
Member design based on both MWFRS and C&C.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
Negative reaction(s) of -220# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.



Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)						
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity			
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): -0.204 A 174 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00		Enclosure: Part. Enc.		Lu: NA Cs: NA			VERT(CL): 0.201 A 177 240			F	862	/-	/-	/782	/1588	/292
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): 0.081 C - -			D	-	/-213	/-	/349	/216	/-
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.085 C - -			C	-	/-234	/-	/337	/192	/-
NCBCLL: 10.00		Mean Height: 15.00 ft					Creep Factor: 2.0			Wind reactions based on C&C						
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.651			F Brg Wid = 12.0 Min Req = 1.5 (Truss)						
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.603			D Brg Wid = 1.5 Min Req = -						
Spacing: 24.0 "		MWFRS Parallel Dist: 0 to h/2		Max Web CSI: 0.228			Mfg Specified Camber:			C Brg Wid = 1.5 Min Req = -						
		C&C Dist a: 3.00 ft					VIEW Ver: 23.02.01A.1204.18			Bearing F is a rigid surface.						
		Loc. from endwall: Any								Maximum Top Chord Forces Per Ply (lbs)						
		GCpi: 0.55								Chords Tens.Comp. Chords Tens. Comp.						
		Wind Duration: 1.60														

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2;	Wind Member design based on both MWFRS and C&C. Left cantilever is exposed to wind Wind loading based on both gable and hip roof types. Left bottom chord exposed to wind.	Additional Notes Negative reaction(s) of -234# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.
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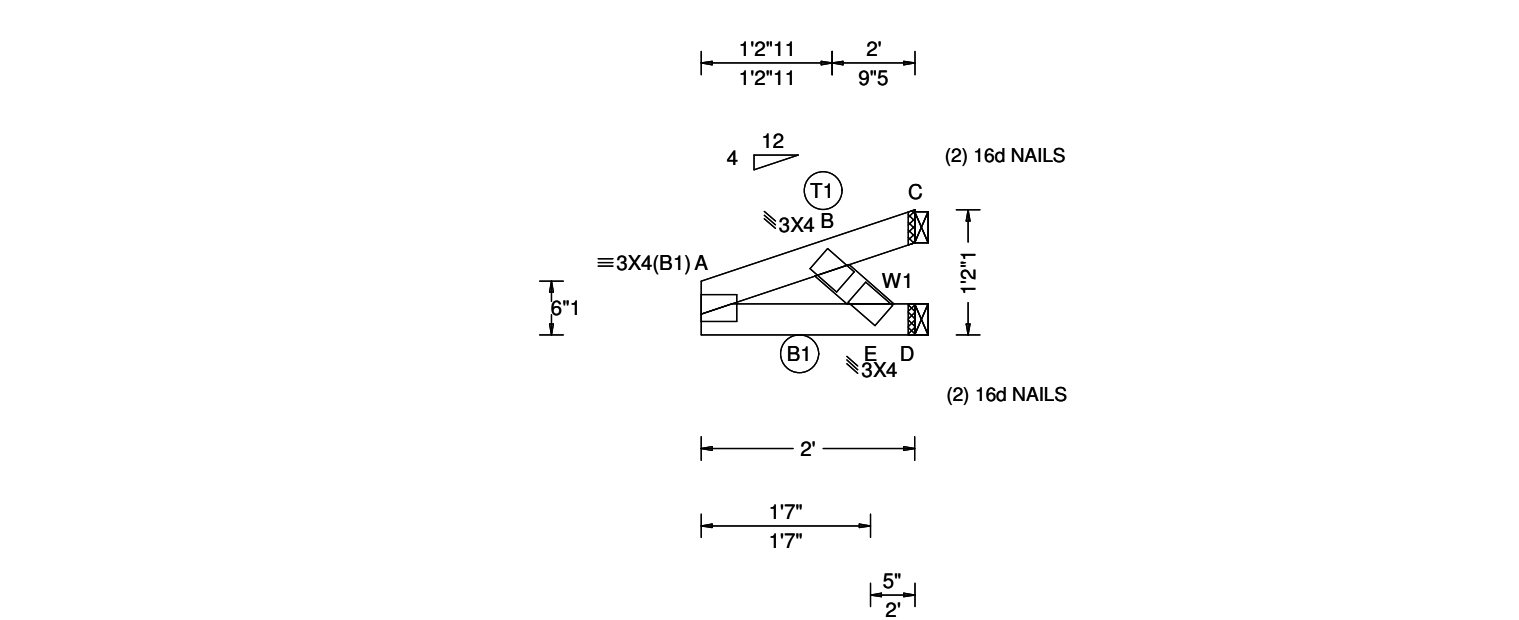


Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)							
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity			Non-Gravity				
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): -0.201 A 177 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL: 0.00	Enclosure: Part. Enc.	Lu: NA Cs: NA	VERT(CL): 0.202 A 176 240	F	862	/-	/-	/690	/1472	/292	
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.079 C - -	D	-	/-214	/-	/392	/250	/-	
	EXP: C Kzt: NA		HORZ(TL): 0.084 C - -	C	-	/-238	/-	/336	/191	/-	
Des Ld: 55.00	Mean Height: 15.00 ft		Creep Factor: 2.0	Wind reactions based on C&C							
NCBCLL: 10.00	TCDL: 6.0 psf	Building Code:	Max TC CSI: 0.650	F	Brg	Wid = 8.0	Min	Req = 1.5 (Truss)			
Soffit: 2.00	BCDL: 6.0 psf	FBC 8th Ed. 2023 Res. HVHZ	Max BC CSI: 0.587	D	Brg	Wid = 1.5	Min	Req = -			
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max Web CSI: 0.228	C	Brg	Wid = 1.5	Min	Req = -			
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Mfg Specified Camber:	Bearing F is a rigid surface.							
	Loc. from endwall: Any	FT/RT:20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)							
	GCpi: 0.55	Plate Type(s):		Chords		Tens.Comp.		Chords		Tens. Comp.	
	Wind Duration: 1.60	WAVE	VIEW Ver: 23.02.01A.1204.18								

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

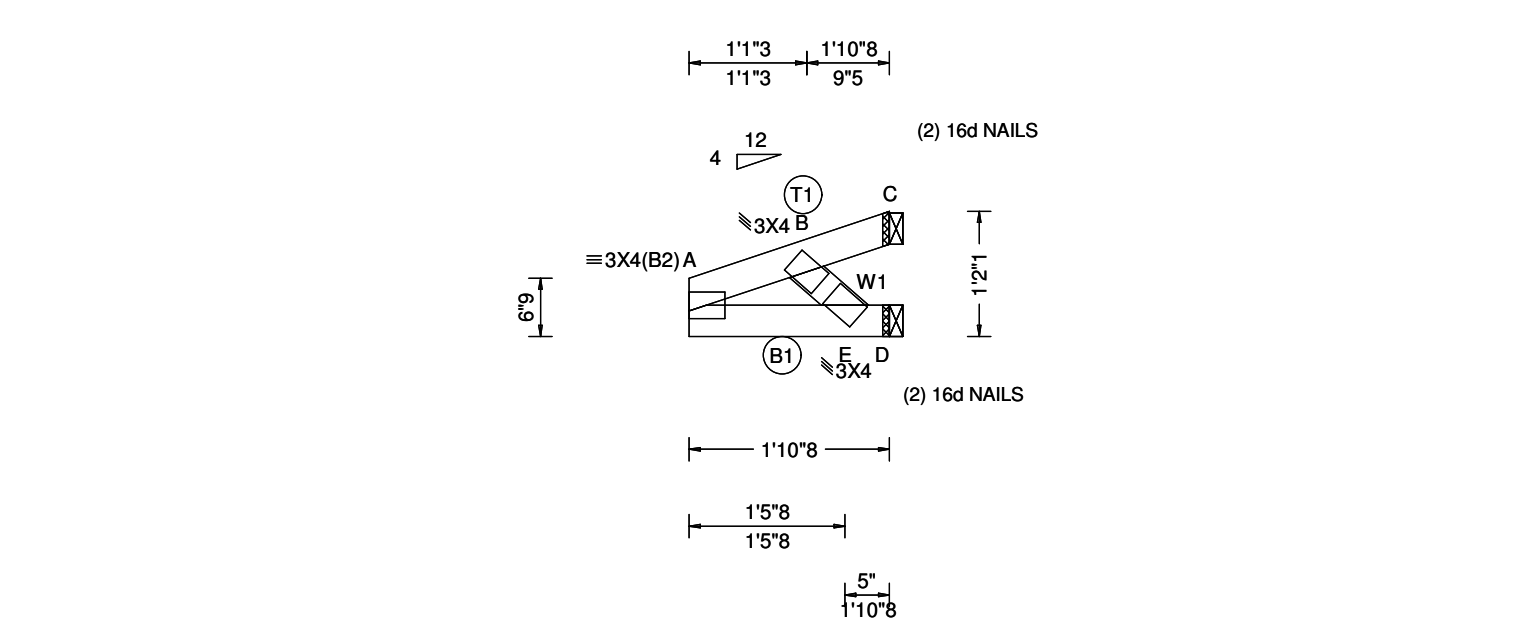
Wind
Member design based on both MWFRS and C&C.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

Additional Notes
Negative reaction(s) of -238# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)					
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 175 mph Enclosure: Part. Enc. Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): -0.007 A 999 360 VERT(CL): -0.007 A 999 240 HORZ(LL): -0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.084 Max BC CSI: 0.212 Max Web CSI: 0.065 Mfg Specified Camber: VIEW Ver: 23.02.01A.1204.18	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL					
	D 89 - / 253 /92 /207 /552 C 131 - / 253 /104 /249 /447								
	Wind reactions based on C&C D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = -								
	Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.								
	A - B 244 -452 B - C 281 -503								
Maximum Bot Chord Forces Per Ply (lbs)									

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2;	Wind Member design based on both MWFRS and C&C. Wind loading based on both gable and hip roof types.	Additional Notes The maximum horizontal reaction is 253# ***** BEARING ANALOG MODIFIED! *****
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)							
		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity			Non-Gravity				
		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): -0.005 A 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
		Enclosure: Part. Enc.		Lu: NA Cs: NA			VERT(CL): -0.006 A 999 240			D	84	/-	/222	/91	/197	/484	
		Risk Category: II		Snow Duration: NA			HORZ(LL): -0.001 B - -			C	122	/-	/222	/96	/236	/386	
		EXP: C Kzt: NA					HORZ(TL): 0.001 B - -			Wind reactions based on C&C							
Des Ld: 55.00		Mean Height: 15.00 ft		Building Code:			Creep Factor: 2.0			D Brg Wid = 1.5 Min Req = -							
NCBCLL: 10.00		TCDL: 6.0 psf		FBC 8th Ed. 2023 Res. HVHZ			Max TC CSI: 0.073			C Brg Wid = 1.5 Min Req = -							
Soffit: 2.00		BCDL: 6.0 psf		TPI Std: 2014			Max BC CSI: 0.194			Maximum Top Chord Forces Per Ply (lbs)							
Load Duration: 1.25		MWFRS Parallel Dist: 0 to h/2		Rep Fac: No			Max Web CSI: 0.060			Chords		Tens.Comp.		Chords		Tens. Comp.	
Spacing: 24.0 "		C&C Dist a: 3.00 ft		FT/RT:20(0)/10(0)			Mfg Specified Camber:			A - B		216 -396		B - C		249 -440	
		Loc. from endwall: Any		Plate Type(s):			VIEW Ver: 23.02.01A.1204.18			Maximum Bot Chord Forces Per Ply (lbs)							
		GCpi: 0.55		WAVE													
		Wind Duration: 1.60															

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Wind
Member design based on both MWFRS and C&C.
Wind loading based on both gable and hip roof types.

Additional Notes
The maximum horizontal reaction is 222#
***** BEARING ANALOG MODIFIED! *****

Maximum Reactions (lbs)

Gravity			Non-Gravity			
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
D	84	/-	/222	/91	/197	/484
C	122	/-	/222	/96	/236	/386

Wind reactions based on C&C

D Brg Wid = 1.5 Min Req = -

C Brg Wid = 1.5 Min Req = -

Maximum Top Chord Forces Per Ply (lbs)

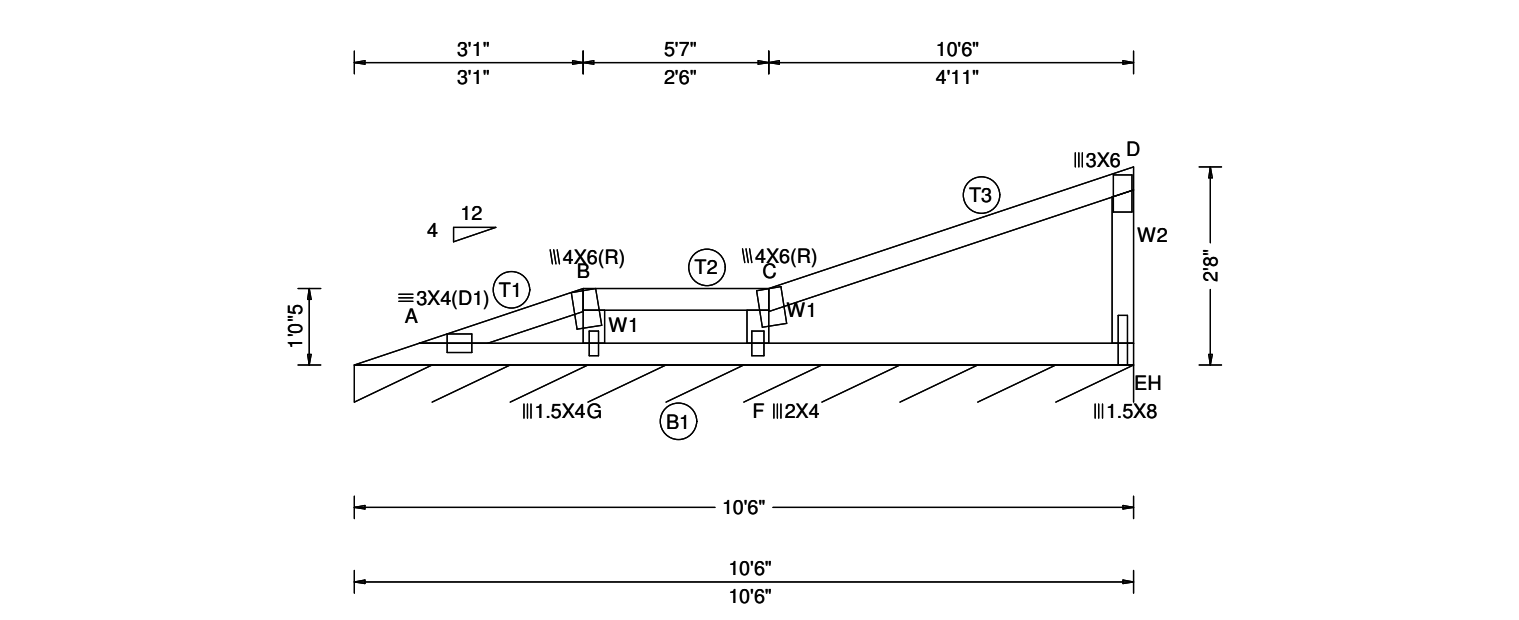
Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	216	-396	B - C	249	-440

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - E	392	-197	E - D	484	-222

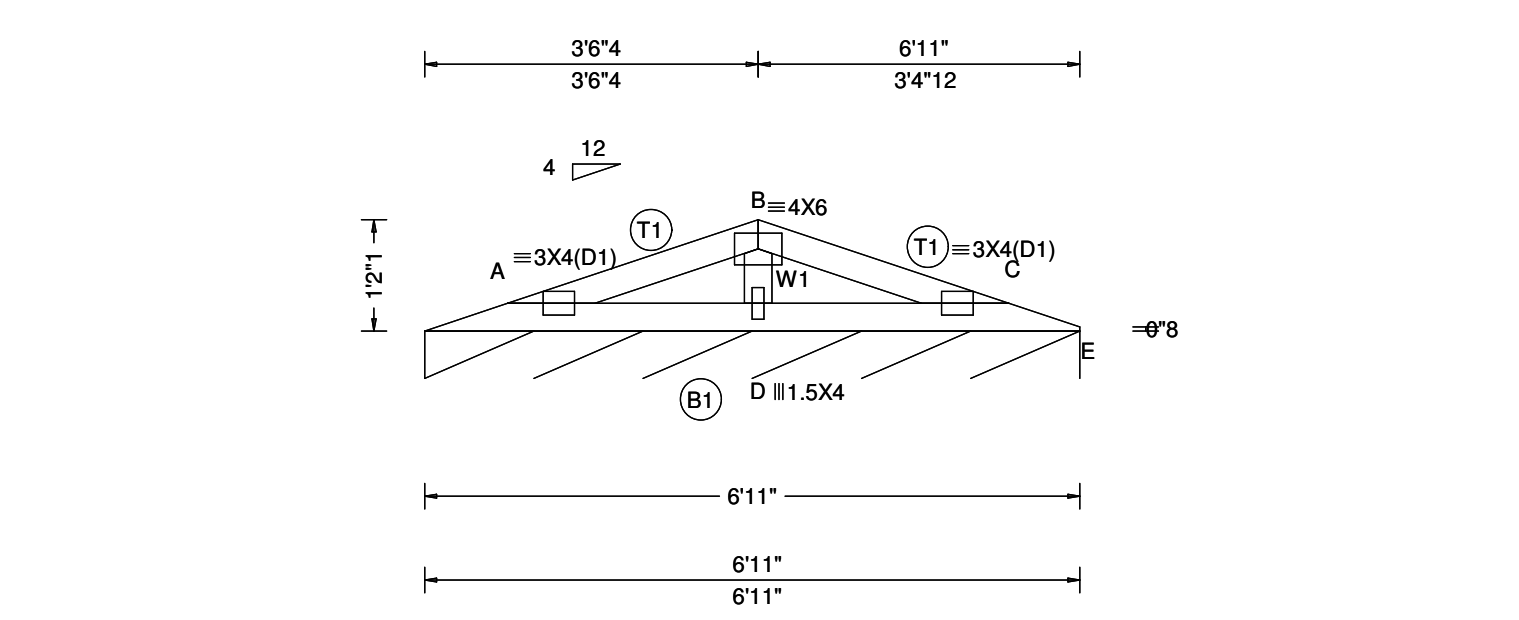
Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
B - E	293	-84



Loading Criteria (psf) TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 175 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 26.07 ft TCDL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.007 A 999 360 VERT(CL): 0.013 A 999 240 HORZ(LL): -0.005 D - - HORZ(TL): 0.005 D - - Creep Factor: 2.0 Max TC CSI: 0.822 Max BC CSI: 0.198 Max Web CSI: 0.197 Mfg Specified Camber: VIEW Ver: 23.02.01A.1204.18	▲ Maximum Reactions (lbs), or *PLF Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity H* 110 /- /- /51 /119 /31 Wind reactions based on C&C H Brg Wid = 126 Min Req = - Bearing A is a rigid surface. Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 49 -283 C - D 50 -101 B - C 45 -319 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - G 319 -43 F - E 292 -37 G - F 403 -45 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B - G 453 -212 D - E 350 -223 F - C 568 -339
Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2; Wind Member design based on both MWFRS and C&C. Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types. Additional Notes See DWG VAL180220723 for valley details.				

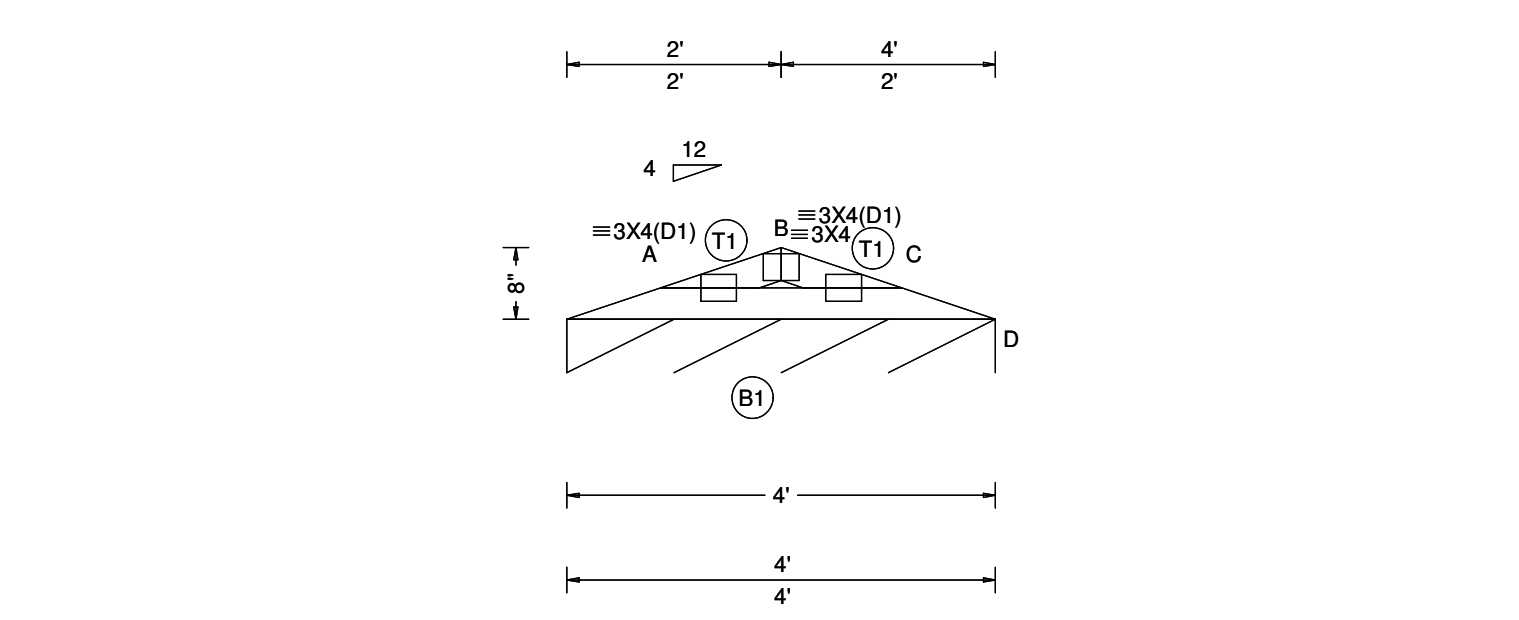
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs), or *PLF						
TCLL:	30.00	Wind Std:	ASCE 7-22	Pg: NA	Ct: NA	CAT: NA	PP Deflection in	loc L/defl	L/#	Gravity			Non-Gravity			
TCDL:	15.00	Speed:	175 mph	Pf: NA		Ce: NA	VERT(LL):	-0.015	A 999 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL):	0.015	A 999 240	E*	110	/-	/-	/59	/106	/6
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA		HORZ(LL):	0.004	C - -	Wind reactions based on C&C						
		EXP:	C Kzt: NA				HORZ(TL):	0.004	C - -	E Brg Wid = 83.0 Min Req = -						
Des Ld:	55.00	Mean Height:	28.19 ft				Creep Factor:	2.0		Bearing A is a rigid surface.						
NCBCLL:	10.00	TCDL:	6.0 psf	Building Code:			Max TC CSI:	0.168		Maximum Top Chord Forces Per Ply (lbs)						
Soffit:	2.00	BCDL:	6.0 psf	FBC 8th Ed. 2023 Res. HVHZ			Max BC CSI:	0.216		Chords	Tens.Comp.		Chords	Tens. Comp.		
Load Duration:	1.25	MWFRS Parallel Dist:	h/2 to h	TPI Std:	2014		Max Web CSI:	0.075		A - B	192 -169		B - C	191 -179		
Spacing:	24.0 "	C&C Dist a:	3.00 ft	Rep Fac:	Yes		Mfg Specified Camber:			Maximum Bot Chord Forces Per Ply (lbs)						
		Loc. from endwall:	not in 9.00 ft	FT/RT:	20(0)/10(0)					Chords	Tens.Comp.		Chords	Tens. Comp.		
		GCpi:	0.18	Plate Type(s):												
		Wind Duration:	1.60	WAVE			VIEW Ver:	23.02.01A.1204.18								

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2; Wind Member design based on both MWFRS and C&C. Wind loading based on both gable and hip roof types. Additional Notes See DWG VAL180220723 for valley details.	
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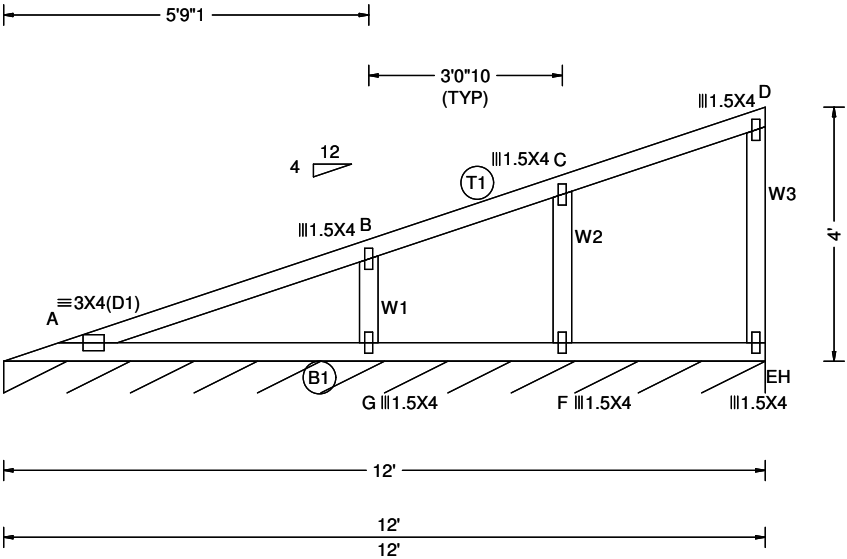


Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *PLF									
				Gravity			Non-Gravity						
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL							
TCLL:	30.00	Wind Std: ASCE 7-22	PP Deflection in loc L/defl L/#	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL			
TCDL:	15.00	Speed: 175 mph	VERT(LL): -0.010 C 999 360	D*	110	/-	/-	/47	/75	/4			
BCLL:	0.00	Enclosure: Closed	VERT(CL): 0.015 C 999 240	Wind reactions based on C&C									
BCDL:	10.00	Risk Category: II	HORZ(LL): 0.003 A - -	D Brg Wid = 48.0 Min Req = -									
Des Ld:	55.00	EXP: C Kzt: NA	HORZ(TL): 0.005 A - -	Bearing A is a rigid surface.									
NCBCLL:	10.00	Mean Height: 28.27 ft	Creep Factor: 2.0	Maximum Top Chord Forces Per Ply (lbs)									
Soffit:	2.00	TCDL: 6.0 psf	Max TC CSI: 0.171	Chords	Tens.Comp.	Chords	Tens. Comp.						
Load Duration: 1.25		BCDL: 6.0 psf	Max BC CSI: 0.157	A - B	455	-377	B - C	453	-377				
Spacing: 24.0 "		MWFRS Parallel Dist: h/2 to h	Max Web CSI: 0.000	Maximum Bot Chord Forces Per Ply (lbs)									
		C&C Dist a: 3.00 ft	Mfg Specified Camber:	Chords	Tens.Comp.								
		Loc. from endwall: not in 9.00 ft	VIEW Ver: 23.02.01A.1204.18	A - C	390	-423							
		GCpi: 0.18											
		Wind Duration: 1.60											
		WAVE											

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind
Member design based on both MWFRS and C&C.
Wind loading based on both gable and hip roof types.

Additional Notes
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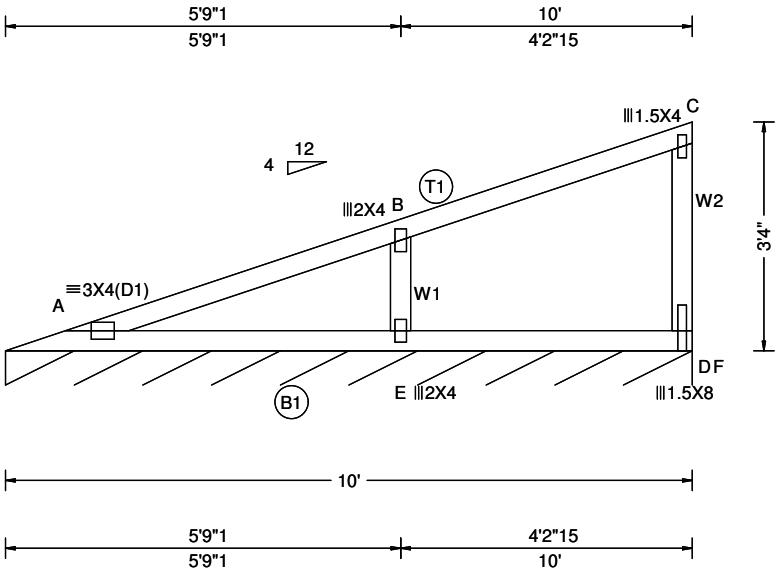


Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs), or *PLF						
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity Non-Gravity						
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.048 A 999 360			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.087 A 999 240			H* 110 /- /- /68 /80 /35						
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): -0.011 A - -			Wind reactions based on C&C						
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.015 A - -			H Brg Wid = 144 Min Req = -						
NCBCLL: 10.00		Mean Height: 26.56 ft					Creep Factor: 2.0			Bearing A is a rigid surface.						
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.483			Maximum Top Chord Forces Per Ply (lbs)						
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.331			Chords Tens.Comp. Chords Tens. Comp.						
Spacing: 24.0 "		MWFRS Parallel Dist: h/2 to h					Max Web CSI: 0.084			A - B 79 -330 C - D 38 -60						
		C&C Dist a: 3.00 ft					Mfg Specified Camber:			B - C 36 -177						
		Loc. from endwall: not in 9.00 ft					VIEW Ver: 23.02.01A.1204.18			Maximum Bot Chord Forces Per Ply (lbs)						
		GCpi: 0.18														
		Wind Duration: 1.60														

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2;	<table><tr><th>Chords</th><th>Tens.Comp.</th><th>Chords</th><th>Tens. Comp.</th></tr><tr><td>A - G</td><td>393 - 65</td><td>F - E</td><td>413 - 72</td></tr><tr><td>G - F</td><td>406 - 70</td><td></td><td></td></tr></table>	Chords	Tens.Comp.	Chords	Tens. Comp.	A - G	393 - 65	F - E	413 - 72	G - F	406 - 70						
Chords	Tens.Comp.	Chords	Tens. Comp.														
A - G	393 - 65	F - E	413 - 72														
G - F	406 - 70																
Wind Member design based on both MWFRS and C&C. Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.	<table><tr><th colspan="4">Maximum Web Forces Per Ply (lbs)</th></tr><tr><th>Webs</th><th>Tens.Comp.</th><th>Webs</th><th>Tens. Comp.</th></tr><tr><td>B - G</td><td>474 - 424</td><td>D - E</td><td>158 - 127</td></tr><tr><td>C - F</td><td>351 - 259</td><td></td><td></td></tr></table>	Maximum Web Forces Per Ply (lbs)				Webs	Tens.Comp.	Webs	Tens. Comp.	B - G	474 - 424	D - E	158 - 127	C - F	351 - 259		
Maximum Web Forces Per Ply (lbs)																	
Webs	Tens.Comp.	Webs	Tens. Comp.														
B - G	474 - 424	D - E	158 - 127														
C - F	351 - 259																

Additional Notes
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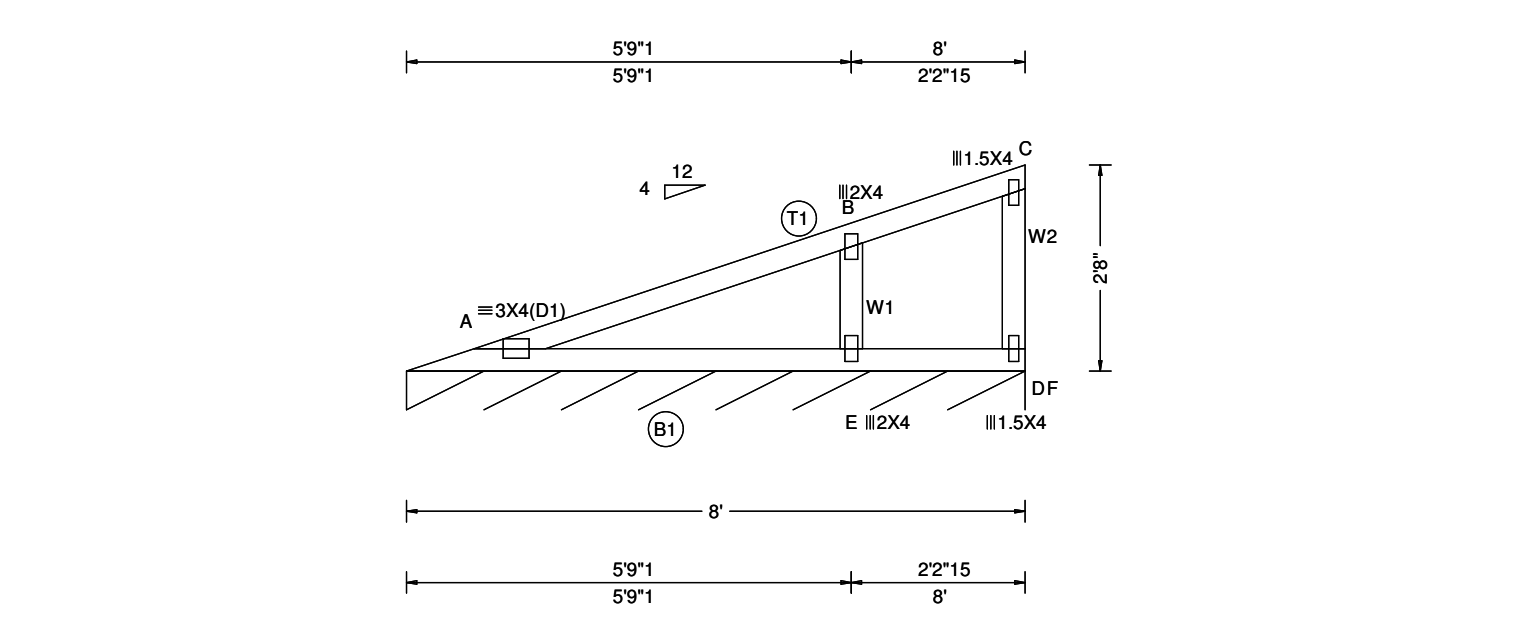


Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs), or *PLF									
TCLL: 30.00		Wind Std: ASCE 7-22		Pg: NA Ct: NA CAT: NA			PP Deflection in loc L/defl L/#			Gravity Non-Gravity									
TCDL: 15.00		Speed: 175 mph		Pf: NA Ce: NA			VERT(LL): 0.044 A 999 360			Loc		R+ / R-		/ Rh		/ Rw / U		/ RL	
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA			VERT(CL): 0.081 A 999 240			F*		110 /-		/-		/67		/90 /38	
BCDL: 10.00		Risk Category: II		Snow Duration: NA			HORZ(LL): -0.010 A - -			Wind reactions based on C&C									
Des Ld: 55.00		EXP: C Kzt: NA		Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE			HORZ(TL): 0.014 A - -			F Brg Wid = 120 Min Req = -									
NCBCLL: 10.00		Mean Height: 26.89 ft					Creep Factor: 2.0			Bearing A is a rigid surface.									
Soffit: 2.00		TCDL: 6.0 psf					Max TC CSI: 0.610			Maximum Top Chord Forces Per Ply (lbs)									
Load Duration: 1.25		BCDL: 6.0 psf					Max BC CSI: 0.302			Chords Tens.Comp. Chords Tens. Comp.									
Spacing: 24.0 "		MWFRS Parallel Dist: h/2 to h					Max Web CSI: 0.117			A - B 68 -286 B - C 41 -81									
		C&C Dist a: 3.00 ft					Mfg Specified Camber:			Maximum Bot Chord Forces Per Ply (lbs)									
		Loc. from endwall: not in 9.00 ft					VIEW Ver: 23.02.01A.1204.18			Chords Tens.Comp. Chords Tens. Comp.									
		GCpi: 0.18																	
		Wind Duration: 1.60																	

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

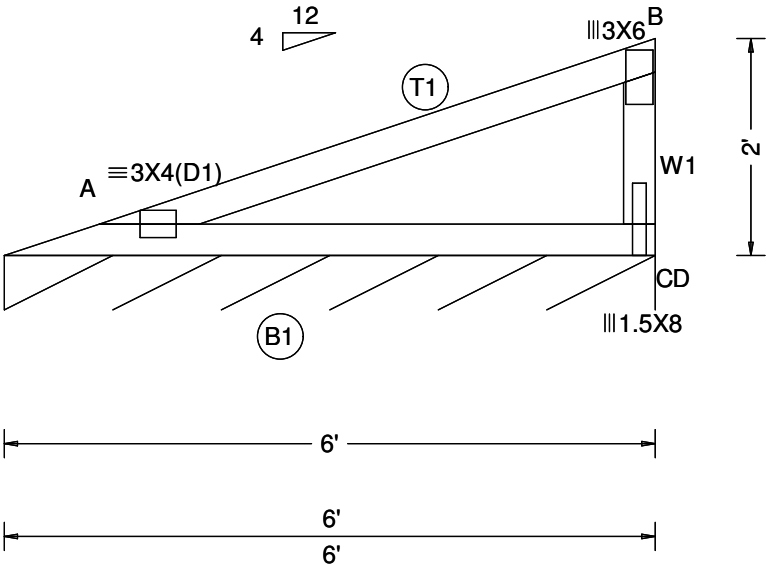
Wind
Member design based on both MWFRS and C&C.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
See DWG VAL180220723 for valley details.



Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs), or *PLF						
TCLL:	30.00	Wind Std:	ASCE 7-22	Pg: NA	Ct: NA	CAT: NA	PP Deflection in	loc L/defl	L/#	Gravity			Non-Gravity			
TCDL:	15.00	Speed:	175 mph	Pf: NA		Ce: NA	VERT(LL):	-0.048	A 999 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL):	0.085	A 999 240	F*	110	/-	/-	/66	/100	/41
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA		HORZ(LL):	-0.012	A - -	Wind reactions based on C&C						
		EXP:	C Kzt: NA				HORZ(TL):	0.015	A - -	F Brg Wid = 96.0 Min Req = -						
Des Ld:	55.00	Mean Height:	27.22 ft	Building Code:			Creep Factor:	2.0		Bearing A is a rigid surface.						
NCBCLL:	10.00	TCDL:	6.0 psf	FBC 8th Ed. 2023 Res.	HVHZ		Max TC CSI:	0.494		Maximum Top Chord Forces Per Ply (lbs)						
Soffit:	2.00	BCDL:	6.0 psf	TPI Std:	2014		Max BC CSI:	0.352		Chords	Tens.	Comp.	Chords	Tens.	Comp.	
Load Duration:	1.25	MWFRS Parallel Dist:	h/2 to h	Rep Fac:	Yes		Max Web CSI:	0.111		A - B	70	-227	B - C	10	-56	
Spacing:	24.0 "	C&C Dist a:	3.00 ft	FT/RT:	20(0)/10(0)		Mfg Specified Camber:			Maximum Bot Chord Forces Per Ply (lbs)						
		Loc. from endwall:	not in 9.00 ft	Plate Type(s):						Chords	Tens.	Comp.	Chords	Tens.	Comp.	
		GCpi:	0.18	WAVE												
		Wind Duration:	1.60				VIEW Ver:	23.02.01A.1204.18								

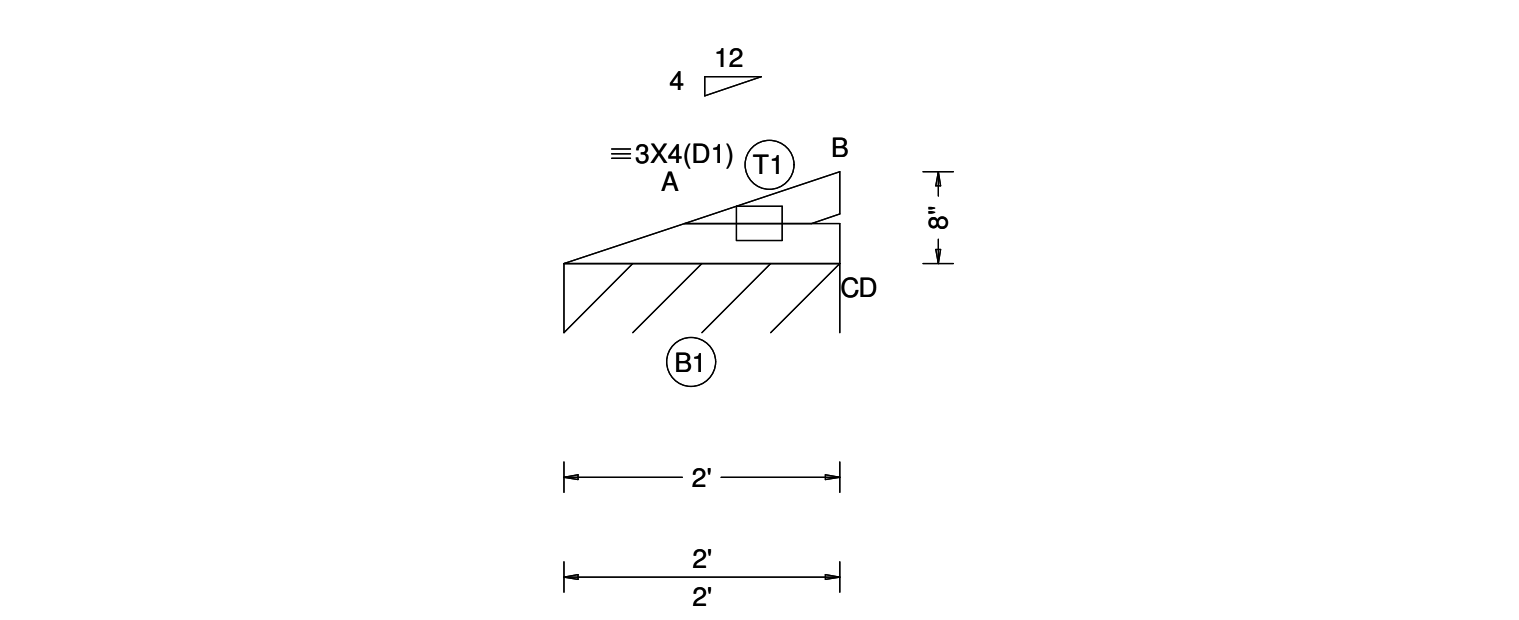
Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2; Wind Member design based on both MWFRS and C&C. Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types. Additional Notes See DWG VAL180220723 for valley details.	
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF						
TCLL: 30.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity			Non-Gravity			
	Speed: 175 mph	Pf: NA Ce: NA	VERT(LL): NA	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA	D* 110 /- /- /64 /109 /44						
	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.020 A - -	Wind reactions based on C&C						
	EXP: C Kzt: NA		HORZ(TL): 0.025 A - -	D Brg Wid = 72.0 Min Req = -						
	Mean Height: 25.77 ft		Creep Factor: 2.0	Bearing A is a rigid surface.						
	TCDL: 6.0 psf	Building Code:	Max TC CSI: 0.677	Maximum Top Chord Forces Per Ply (lbs)						
	BCDL: 6.0 psf	FBC 8th Ed. 2023 Res. HVHZ	Max BC CSI: 0.548	Chords Tens.Comp.						
	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max Web CSI: 0.181	A - B 25 -96						
	C&C Dist a: 3.00 ft	Rep Fac: Yes	Mfg Specified Camber:	Maximum Bot Chord Forces Per Ply (lbs)						
Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)	VIEW Ver: 23.02.01A.1204.18	Chords Tens.Comp.							
GCpi: 0.18	Plate Type(s):									
Wind Duration: 1.60	WAVE									

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #2;
Wind Member design based on both MWFRS and C&C. Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *≠PLF						
				Gravity			Non-Gravity			
	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL			
TCLL: 30.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	VERT(LL): -0.002 C 999 360	D* 110	/-	/-	/47	/87	/35
TCDL: 15.00	Speed: 175 mph	Pf: NA Ce: NA	VERT(CL): 0.002 C 999 240	Wind reactions based on C&C						
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	HORZ(LL): -0.001 C - -	D Brg Wid = 24.0 Min Req = -						
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(TL): 0.001 C - -	Bearing A is a rigid surface.						
Des Ld: 55.00	EXP: C Kzt: NA	Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Creep Factor: 2.0	Maximum Top Chord Forces Per Ply (lbs)						
NCBCLL: 10.00	Mean Height: 25.96 ft		Max TC CSI: 0.043	Chords Tens.Comp.						
Soffit: 2.00	TCDL: 6.0 psf		Max BC CSI: 0.064	A - B 0 -7						
Load Duration: 1.25	BCDL: 6.0 psf		Max Web CSI: 0.000	Maximum Bot Chord Forces Per Ply (lbs)						
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2		Mfg Specified Camber:	Chords Tens.Comp.						
	C&C Dist a: 3.00 ft		VIEW Ver: 23.02.01A.1204.18							
	Loc. from endwall: not in 9.00 ft									
	GCpi: 0.18									
	Wind Duration: 1.60									

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2;	
Wind Member design based on both MWFRS and C&C. Wind loading based on both gable and hip roof types.	
Additional Notes See DWG VAL180220723 for valley details.	

ICC-ES Evaluation Report

ESR-2552

Reissued March 2024

This report also contains:

- LABC Supplement

Subject to renewal March 2025

- FBC Supplement

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DIVISION: 06 00 00— WOOD, PLASTICS AND COMPOSITES Section: 06 05 23— Wood, Plastic, and Composite Fastenings	REPORT HOLDER: SIMPSON STRONG-TIE COMPANY INC. 	EVALUATION SUBJECT: SIMPSON STRONG-TIE® FACE-MOUNT HANGERS FOR SAWN LUMBER, STRUCTURAL COMPOSITE LUMBER (SCL), PREFABRICATED WOOD I-JOISTS AND GLULAM BEAMS (ENGINEERED WOOD PRODUCTS)	
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1.0 EVALUATION SCOPE

Compliance with the following codes:

- 2021, 2018, 2015, and 2012 [International Building Code® \(IBC\)](#)
- 2021, 2018, 2015, and 2012 [International Residential Code® \(IRC\)](#)

For evaluation for compliance with codes adopted by the [Los Angeles Department of Building and Safety \(LADBS\)](#), see [ESR-2552 LABC and LARC Supplement](#).

Property evaluated:

- Structural

2.0 USES

The Simpson Strong-Tie® face-mount hangers described in this report are used as wood framing connectors in wood construction in accordance with Section [2304.10.4](#) of the 2021 IBC, [2304.10.3](#) of the 2018 and 2015 IBC and Section [2304.9.3](#) of the 2012 IBC. The face-mount hangers may also be used in structures regulated under the IRC when an engineered design is submitted in accordance with Section [R301.1.3](#) of the IRC.

3.0 DESCRIPTION

3.1 General:

With the exception of the CBH, ACBH, SCBH and HSKP Hangers Series, the Simpson Strong-Tie face-mount hangers described in this report are U-shaped hangers that have prepunched holes for the installation of nails or Simpson Strong-Drive SDS screws, depending on the hanger model, into the face of the supporting wood header or beam or ledger. The CBH Hanger is used in pairs and is a die-formed steel plate with a welded stud and prepunched holes for the installation of Simpson Strong-Drive SDS screws. The ACBH hanger is used in pairs and is manufactured in one piece from aluminum with predrilled holes for the installation of Simpson Strong-Drive SDCF screws. The SCBH hanger is used in pairs and is manufactured in one piece from steel with predrilled holes for the installation of Simpson Strong-Drive SDCF screws. The HSKP hanger has a horizontal bearing plate to support the joist/beam and includes prepunched holes in its back plate for the installation of Simpson Strong-Drive SDCF screws into the face of the supporting wood Column.

3.1.1 LU Series Hangers: The LU series hangers are formed from No. 20 gage galvanized steel. See [Table 1](#) for hanger dimensions, required fasteners, and allowable loads; and [Figure 1](#) for a drawing of a typical LU series hanger.

3.1.2 LUCZ Series Hangers: The LUCZ hangers have concealed flanges to allow for installation near the end of a supporting member such as a ledger or header. The hangers are formed from No. 18 gage galvanized steel coated with a G185 zinc coating. See [Table 2](#) and [Figure 2](#) for hanger dimensions, required fastener schedule, allowable loads and a typical installation detail.

3.1.3 U Hangers: The U series hangers are formed from No. 16 gage galvanized steel. The hangers are face-nailed to the supporting wood header, and nailed to the supported solid sawn lumber, structural composite lumber or prefabricated wood I-joists with web stiffeners to accept the required size and number of joist nails shown in [Table 3](#). See [Table 3](#) for the U series hanger model numbers, hanger dimensions, required fasteners, and allowable loads. See [Figure 3](#) for a drawing of a typical U series joist hanger.

3.1.4 HU and HUC Series Hangers: The HU and HUC series hangers are formed from No. 14 gage galvanized steel, and are face-nailed to the supporting wood header/beam and nailed to the solid sawn lumber, supported structural composite lumber or prefabricated wood I-joists with web stiffeners. HU hangers having a seat width (W) equal to or greater than $2\frac{9}{16}$ inches (65 mm) are available with concealed flanges and are specified with the model designation HUC. See [Table 4](#) for hanger model numbers, hanger dimensions, required fasteners and allowable loads based on the nailing. The HU and HUC hangers have triangular and round holes in both the U-shaped portion supporting the wood joist and in the flanges attached to the supporting wood header/beam. The allowable loads are achieved by filling all holes (both the round and triangular) with size of nails shown in [Table 4](#). See [Figures 4A](#) and [4B](#) for drawings of typical HU and HUC hangers, and [Figure 4C](#) shows a typical installation of an HU hanger supporting a prefabricated wood I-joist with approved web stiffeners.

3.1.5 HUCQ Series Hangers: The HUCQ series hangers are formed from No. 14 gage galvanized steel and are used to support glulam beams or other approved wood members. Other approved wood members must be laterally supported at the ends in accordance with Section [2308.4.6](#) of the IBC and Section [R502.7](#) of the IRC. The hangers have prepunched holes for the installation of Simpson Strong-Drive SDS series wood screws (SDS). See [Table 5](#) for the HUCQ series model numbers, hanger dimensions, required fasteners and allowable loads. See [Figure 5](#) for a drawing of the HUCQ series hanger.

3.1.6 LUS Series Hangers: The LUS series hangers are formed from No. 18 gage galvanized steel. The hangers have prepunched holes for the installation of nails that are driven at a 45-degree angle through the joist and into the header, which is described as double shear nailing in the installation instructions. See [Table 6](#) for the hanger dimensions, required fasteners, and allowable loads; and [Figure 6](#) for a drawing of a typical LUS series hanger.

3.1.7 MUS Joist Hanger: The MUS series hangers are formed from No. 18 gage galvanized steel. The U-shaped portion of the hangers has prepunched holes for the installation of joist nails that are driven at an angle through the joist and into the header, which is described as double shear nailing in the installation instructions. See [Table 7](#) for the hanger dimensions, required fasteners, and allowable loads; [Figure 7](#) for a drawing of a typical MUS series hanger.

3.1.8 HUS Series Hangers: The HUS hangers having a seat width, W , of $3\frac{9}{16}$ inches (90 mm) are formed from No. 14 gage galvanized steel. The HUS1.81/10 hanger with a seat width, W , of $1\frac{13}{16}$ inches (46 mm) is formed from No. 16 gage galvanized steel. The hangers have prepunched holes for the installation of nails that are driven at a 45-degree angle through the joist and into the header, which is described as double shear nailing in the installation instructions. See [Table 8](#) for HUS series hanger models, hanger dimensions, required fasteners, and allowable loads. See [Figure 8](#) for a drawing of a typical HUS hanger.

3.1.9 HHUS Series Hangers: The HHUS series hangers are formed from No. 14 gage galvanized steel. The hangers have pre-punched holes for the installation of nails that are driven at a 45 degree angle through the joist and into the header, which is described as double shear nailing in the installation instructions. See [Table 9](#) for the hanger model numbers, hanger dimensions, required fasteners, and allowable loads. See [Figure 9](#) for a drawing of a typical HHUS hanger.

3.1.10 HGUS Series Hangers: The HGUS series hangers are formed from No. 12 gage galvanized steel and are used to support glulam beams or other approved wood members. Other approved wood members must be laterally supported at the ends in accordance with Section [2308.4.6](#) of the IBC and Section [R502.7](#) of the

IRC. The hangers have prepunched holes for the installation of nails that are driven at a 45-degree angle through the joist and into the header, which is described as double shear nailing in the installation instructions. See [Table 10](#) for the HGUS series hanger model numbers, hanger dimensions, required fasteners and allowable loads. See [Figure 10](#) for a drawing of a typical HGUS hanger.

3.1.11 HTU Series Hangers: The HTU hangers are designed to support trusses installed with full or partial heel heights and gaps between the truss and the supporting girders of up to, but not exceeding, $\frac{1}{2}$ inch (12.7 mm), as shown in [Tables 11A](#) and [11C](#), and $\frac{1}{8}$ inch (3.2 mm) as shown in [Table 11B](#). Minimum and maximum nailing options are given in [Tables 11A](#), [11B](#), and [11C](#) to address varying heel heights and support conditions. The HTU hangers are formed from No. 16 gage galvanized steel. See [Table 11A](#) and [Figures 11A](#) and [11B](#) for hanger dimensions, required fastener schedule, allowable loads and an installation detail for installations in which the gap between the truss and the supporting girders is less than or equal to $\frac{1}{2}$ inch (12.7 mm). See [Table 11B](#) and [Figures 11A](#) and [11B](#) for hanger dimensions, required fastener schedule, allowable loads and an installation detail for installations in which the gap between the truss and the supporting girders is less than or equal to $\frac{1}{8}$ inch (3.2 mm). See [Table 11C](#) and [Figures 11A](#) and [11C](#) for hanger dimensions, required fastener schedule, allowable loads and an installation detail for installations in which the minimum allowable number of nails is driven into the supporting girder, and the gap between the truss and supporting girder is less than or equal to $\frac{1}{2}$ inch (12.7 mm).

3.1.12 LGU, MGU, HGU and HHGU Series Hangers: The LGU and MGU series hangers are formed from No. 10 gage galvanized steel; HGU series hangers are formed from No. 7 gage galvanized steel; and HHGU series hangers are formed from No. 3 gage steel. The LGU, MGU, HGU and HHGU series hangers are used to support glulam beams or other approved wood members. Other approved wood members must be laterally supported at the ends in accordance with Section 2308.4.6 of the IBC and Section R502.7 of the IRC. The hangers have pre-punched holes for the installation of Simpson Strong-Drive SDS series wood screws (SDS). See [Table 12](#) for the LGU, MGU, HGU and HHGU series model numbers, hanger dimensions, required fasteners and allowable loads. See [Figure 12](#) for a drawing of the HHGU series hanger and a typical installation detail.

3.1.13 SUR/L Series Hangers: The SUR/L series hangers are formed from No. 16 gage galvanized steel. The SUR and SUL hangers are identical except they are skewed at 45 degrees to the right (SUR) and the left (SUL), respectively. See [Table 13](#) for hanger models, hanger dimensions, required fasteners, and allowable loads. See [Figure 13A](#) for a drawing of a typical SUL hanger and [Figure 13B](#) for a typical SUR hanger installation.

3.1.14 HSUR/L Series Hangers: The HSUR/L series hangers are formed from No. 14 gage galvanized steel. The HSUR and HSUL hangers are identical except they are skewed at 45 degrees to the right (HSUR) and the left (HSUL), respectively, and are designed to support prefabricated wood I-joists having approved web stiffeners, and structural composite lumber (SCL). See [Table 14](#) for the hanger model numbers, hanger dimensions, required fasteners, and allowable loads. See [Figure 14A](#) for a drawing of a typical HSUR hanger, and [Figure 14B](#) for a typical HSUR hanger installation.

3.1.15 IUS Series Hangers: The IUS series hangers are formed from No. 18 gage galvanized steel and are used exclusively to support prefabricated wood I-joists to a supporting wood member. See [Table 15A](#) for the IUS series hanger model numbers, hanger dimensions, and required fasteners; and [Table 15B](#) for allowable loads. See [Figures 15A](#), [15B](#), and [15C](#) for drawings of a typical IUS hanger and a typical IUS hanger installation.

3.1.16 MIU Series Joist Hangers: The MIU series hangers are formed from No. 16 gage galvanized steel and are used to support prefabricated wood I-joists or other approved wood members. Other approved wood members must be laterally supported at the ends in accordance with Section 2308.4.6 of the IBC and Section R502.7 of the IRC. See [Table 16](#) for the MIU series hanger model numbers, hanger dimensions, required fasteners, and allowable loads. See [Figure 16A](#) for a drawing of a typical MIU hanger and [Figure 16B](#) for a drawing of a typical MIU hanger installation.

3.1.17 DU, DHU and DHUTF Series Hangers: The DU, DHU and DHUTF series hangers are face-mount and top-flange hangers that are formed from either No. 14 gage or No. 12 gage galvanized steel. They are designed to transfer joist floor loads to a wood stud wall through two layers of $\frac{5}{8}$ -inch-thick (15.9 mm) gypsum board complying with [ASTM C1396](#), and are installed using a combination of nails and Simpson Strong-Drive SDS series wood screws (SDS), which are addressed in [ESR-2236](#). These hangers are installed after the two layers of $\frac{5}{8}$ " Type X gypsum board is installed and fastened using, at a minimum, the fastener schedule in

Item 14.1-5 in IBC Table 721.1(2). See [Table 17A](#) for hanger dimensions and allowable loads. See [Table 17B](#) for fastener schedule. See [Figure 17](#) for a drawing of the DU, DHU and DHUTF hangers and a typical installation.

3.1.18 CBH Series Hangers: The CBH series hangers are face-mounted and installed in inverted pairs. They are formed from No. 3 gage steel. They are used to support glulam beams or other approved wood members and are installed using Simpson Strong-Drive SDS series wood screws (SDS), which are addressed in [ESR-2236](#). See [Table 18A](#) & [18B](#) for hanger dimensions, fastener schedules and allowable loads. See [Figure 18](#) for a drawing of the CBH hangers and a typical installation.

3.1.19 ACBH and SCBH Series Hangers: The ACBH series hangers are aluminum and the SCBH series hangers are steel. The ACBH and SCBH series hangers are face-mounted and installed in inverted pairs. They are used to support glulam beams and are installed using Simpson Strong-Drive SDCF series wood screws, which are addressed in [ESR-3046](#). See [Table 19](#) for hanger dimensions, fastener schedules and allowable loads. See [Figure 19](#) for drawings of the ACBH and SCBH hangers and a typical installation.

3.1.20 HSKP Series Hangers: The HSKP series hangers are factory welded with a No. 3 gage steel back plate, 1/2" steel bearing plate and a No. 3 gage steel (for HSKP5.75x14.5 and HSKP5.75x17.0) or 3/8" steel (for HSKP5.75x19.5) knife plate. The HSKP is installed using Simpson Strong-Drive SDCF series wood screws, which are addressed in [ESR-3046](#). See [Table 20](#) for model numbers, hanger dimensions, fasteners schedules, allowable loads and installation instructions. See [Figure 20](#) for hanger drawings, typical installation and bearing area requirements for the carried members.

3.2 Materials:

3.2.1 Hanger Material: All hangers described in this report, with the exception of the CBH, ACBH, SCBH, HSKP, HTU, HGUS and HHGU series hangers, are manufactured from galvanized steel complying with [ASTM A653](#), SS designation, Grade 33, with a minimum yield strength, F_y , of 33,000 psi (227 MPa) and a minimum tensile strength, F_u , of 45,000 psi (310 MPa).

The HTU and HGUS series hangers are manufactured from galvanized steel complying with ASTM A653, SS designation, Grade 40, with a minimum yield strength, F_y , of 40,000 psi (276 MPa) and a minimum tensile strength, F_u , of 55,000 psi (379 MPa).

The HHGU and CBH series hangers are manufactured from nongalvanized steel complying with [ASTM A1011](#) SS, Grade 33, with a minimum yield strength, F_y , of 33,000 psi (227 MPa) and a minimum tensile strength, F_u , of 52,000 psi (358 MPa).

The stud of the CBH hangers is manufactured from nongalvanized steel complying with ASTM A108 Grade 1018 with a minimum yield strength, F_y , of 70,000 psi (483 MPa) and a minimum tensile strength, F_u , of 80,000 psi (552 MPa).

The ACBH series hangers are manufactured from ASTM B221 6061-T6 aluminum with a minimum yield strength, F_y , of 40,000 psi (276 MPa) and a minimum tensile strength, F_u , of 45,000 psi (310 MPa).

The SCBH series hangers are manufactured from nongalvanized steel complying with ASTM A572, Grade 50 with minimum yield strength, F_y , of 50,000 psi (345 MPa) and a minimum tensile strength, F_u , of 65,000 psi (448 MPa).

The HSKP series hanger is manufactured from three components: a back plate, a bearing plate and a knife plate. The HSKP back plate and knife plate are manufactured from nongalvanized steel complying with [ASTM A1011](#) SS, Grade 33, with a minimum yield strength, F_y , of 33,000 psi (227 MPa) and a minimum tensile strength, F_u , of 52,000 psi (358 MPa). The HSKP bearing plate is manufactured from nongalvanized steel complying with ASTM A572, Grade 50, with a minimum yield strength, F_y , of 50,000 psi (345 MPa) and a minimum tensile strength, F_u , of 65,000 psi (448 MPa).

Exception: The HSKP5.75x19.5 knife plate component is manufactured from nongalvanized steel complying with [ASTM A36](#) with a minimum yield strength, F_y , of 36,000 psi (248 MPa) and a minimum tensile strength, F_u , of 58,000 psi (400 MPa).

Base-metal thicknesses for the hangers in this report are as follows:

NOMINAL THICKNESS (gauge No. or inch)	MINIMUM BASE-METAL THICKNESS (inch)
1/2"	0.4900
3/8"	0.3650
No. 3	0.2285
No. 7	0.1715
No. 10	0.1275
No. 12	0.0975
No. 14	0.0685
No. 16	0.0555
No. 18	0.0444
No. 20	0.0335

For SI: 1 inch = 25.4 mm.

The thickness of the installed pairs of CBH, ACBH, and SCBH series hangers are shown in the applicable figures in this report.

With the exception of the CBH and SCBH hangers, the galvanized zinc coating for the steel hangers in Section 3.1 conforms to [ASTM A924](#) and ASTM A653 with a G90 designation. Some models of these hangers (designated with a model number ending with Z) are available with a G185 zinc coating specification in accordance with ASTM A653. Some models (designated with a model number ending with HDG) are available with a hot-dip galvanization, also known as “batch” galvanization, in accordance with [ASTM A123](#), with a minimum specified coating weight of 2.0 ounces of zinc per square foot of surface area (600 g/m²), total for both sides. With the exception of the LUCZ, model numbers in this report do not include the Z or HDG ending, but the information shown applies.

The CBH series hanger product is available with an electrogalvanized finish in accordance with ASTM B633 or with a hot-dip galvanized finish in accordance with ASTM A123 (designated with a model number ending with HDG). The SCBH series hanger product has an electrogalvanized finish in accordance with ASTM B633.

The lumber treater or holder of this report (Simpson Strong-Tie Company) should be contacted for recommendations on minimum corrosion resistance of steel connectors in contact with the specific proprietary preservative treated or fire retardant treated lumber.

3.2.2 Wood: Wood headers/beams and columns to which the connectors are fastened must be either sawn lumber, structural glued-laminated timber, or engineered lumber having a minimum specific gravity of 0.50 (minimum equivalent specific gravity of 0.50 for engineered lumber), and having a maximum moisture content of 19 percent (16 percent for engineered lumber) except as noted in Section 4.1. The thickness of the supporting wood member (header) must be equal to or greater than the length of the fasteners specified in the tables in this report, or as required by wood member design, whichever is greater.

Supported wood members that are prefabricated wood I-joists or SCL must be addressed in a current evaluation report, which will specify the allowable shear capacity and allowable reactions at supports for the proprietary engineered wood lumber. When required, web stiffeners for prefabricated wood I-joists must comply with specifications noted in the applicable evaluation report for the I-joists. The hangers described in this evaluation report may support I-joists having a current ICC-ES evaluation report. The evaluation report for the prefabricated wood I-joist must specify a minimum bearing length that is equal to or less than the hanger bearing length. When the hangers are used to support prefabricated wood I-joists, joist end reactions must not exceed the allowable joist end reaction per the I-joist manufacturer’s code report. For installation with engineered wood members, minimum allowable fastener spacing and end and edge distances, as specified in the applicable evaluation report for the engineered wood product, must be met.

3.2.3 Fasteners: Nails used for hangers described in this report must comply with [ASTM F1667](#) and have the following minimum fastener dimensions and bending yield strengths (F_{yb}):

FASTENERS	SHANK DIAMETER (inches)	NAIL LENGTH (inches)	F_{yb} (psi)
10d × 1½	0.148	1½	90,000
10d	0.148	3	90,000
16d × 2½	0.162	2½	90,000
16d	0.162	3½	90,000

For SI: 1 inch = 25.4 mm, 1 psi = 6.895 kPa.

Unless otherwise indicated, fasteners listed in this report as 10d and 16d are 10d common and 16d common nails, respectively.

Some of the hangers and concealed beam hangers (CBH) described in this report are attached to the wood members using Simpson Strong-Drive SDS series wood screws addressed in ESR-2236. Other concealed beam hangers (ACBH and SCBH) and HSKP hangers described in this report are attached to wood members using SDCF Strong-Drive wood screws addressed in [ESR-3046](#).

Fasteners used in contact with preservative treated or fire retardant treated lumber must comply with Section [2304.10.6](#) of the 2021 IBC, Section [2304.10.5](#) of the 2018 and 2015 IBC, Section [2304.9.5](#) of the 2012, and applicable sections of the IRC. The chemical treatment manufacturer or this report holder (Simpson Strong-Tie Company) should be contacted for recommendations on minimum corrosion resistance of fasteners and connection capacities of fasteners used with the specific proprietary preservative treated or fire retardant treated lumber.

4.0 DESIGN AND INSTALLATION

4.1 Design:

The tabulated allowable loads shown in this report are based on allowable stress design (ASD) and include the load duration factor, C_D , corresponding with the applicable loads in accordance with the NDS.

Tabulated allowable loads apply to products connected to wood used under dry conditions and where sustained temperatures are 100°F (37.8°C) or less. When products are installed to wood having a moisture content greater than 19 percent (16 percent for engineered wood products), or where wet service is expected, the allowable loads must be adjusted by the wet service factor, C_M , specified in the NDS. When connectors are installed in wood that will experience sustained exposure to temperatures exceeding 100°F (37.8°C), the allowable loads in this report must be adjusted by the temperature factor, C_t , specified in the NDS.

Connected wood members must be analyzed for load-carrying capacity at the connection in accordance with the NDS and the applicable evaluation report for engineered wood products.

4.2 Installation:

Installation of the connectors must be in accordance with this evaluation report and the manufacturer's published installation instructions. In the event of a conflict between this report and the manufacturer's published installation instructions, this report governs.

5.0 CONDITIONS OF USE:

The Simpson Strong-Tie face-mount hangers described in this report comply with, or are suitable alternatives to what is specified in, those codes listed in Section 1.0 of this report, subject to the following conditions:

- 5.1 The connectors must be manufactured, identified and installed in accordance with this report and the manufacturer's published installation instructions. A copy of the instructions must be available at the jobsite at all times during installation.
- 5.2 Calculations showing compliance with this report must be submitted to the code official. The calculations must be prepared by a registered design professional where required by the statutes of the jurisdiction in which the project is to be constructed.
- 5.3 Adjustment factors noted in Section 4.1 and the applicable codes must be considered, where applicable.
- 5.4 Connected wood members and fasteners must comply, respectively, with Sections 3.2.2 and 3.2.3 of this report.

- 5.5 Use of connectors with preservative or fire retardant treated lumber must be in accordance with Section 3.2.1 of this report. Use of fasteners with preservative treated or fire retardant treated lumber must be in accordance with Section 3.2.3 of this report.
- 5.6 The face mount hangers are manufactured under a quality control program with inspections by ICC-ES.

6.0 EVIDENCE SUBMITTED

- 6.1 Data in accordance with the [ICC-ES Acceptance Criteria for Joist Hangers and Similar Devices \(AC13\)](#), dated October 2018 (editorially revised December 2020).
- 6.2 Tests and calculations in accordance with the [ICC-ES Acceptance Criteria for Dowel-Type Threaded Fasteners used in Wood \(AC233\)](#), dated June 2023.

7.0 IDENTIFICATION

- 7.1 The products described in this report are identified with a die-stamp or an adhesive label indicating the name of the manufacturer (Simpson Strong-Tie) the model number, and the number of an index evaluation report ([ESR-2523](#)) that is used as an identifier for the products addressed in this report.
- 7.2 The report holder's contact information is the following:

SIMPSON STRONG-TIE COMPANY INC.
5956 WEST LAS POSITAS BOULEVARD
PLEASANTON, CALIFORNIA 94588
(800) 999-5099
www.strongtie.com
jellis@strongtie.com

DIVISION: 06 00 00—WOOD, PLASTICS AND COMPOSITES
Section: 06 05 23—Wood, Plastic, and Composite Fastenings

REPORT HOLDER:

SIMPSON STRONG-TIE COMPANY INC.

EVALUATION SUBJECT:

SIMPSON STRONG-TIE® FACE-MOUNT HANGERS FOR SAWN LUMBER, STRUCTURAL COMPOSITE LUMBER (SCL), PREFABRICATED WOOD I-JOISTS AND GLULAM BEAMS (ENGINEERED WOOD PRODUCTS)

1.0 REPORT PURPOSE AND SCOPE**Purpose:**

The purpose of this evaluation report supplement is to indicate that Simpson Strong-Tie® face-mount hangers for sawn lumber, structural composite lumber (SCL), prefabricated wood I-joists and glulam beams (engineered wood products), described in ICC-ES evaluation report [ESR-2552](#), have also been evaluated for compliance with the codes noted below as adopted by the Los Angeles Department of Building and Safety (LADBS).

Applicable code editions:

- 2023 *City of Los Angeles Building Code* (LABC)
- 2023 *City of Los Angeles Residential Code* (LARC)

2.0 CONCLUSIONS

The Simpson Strong-Tie® face mount hangers for sawn lumber, structural composite lumber (SCL), prefabricated wood I-joists and glulam beams (engineered wood products), described in Sections 2.0 through 7.0 of the evaluation report [ESR-2552](#), comply with the LABC Chapter 23, and the LARC, and are subjected to the conditions of use described in this supplement.

3.0 CONDITIONS OF USE

The Simpson Strong-Tie® face mount hangers for sawn lumber, structural composite lumber (SCL), prefabricated wood I-joists and glulam beams (engineered wood products), described in this evaluation report supplement, must comply with all of the following conditions:

- All applicable sections in the evaluation report [ESR-2552](#).
- The design, installation, conditions of use and identification are in accordance with the 2021 *International Building Code*® (2021 IBC) provisions noted in the evaluation report [ESR-2552](#).
- The design, installation and inspection are in accordance with additional requirements of LABC Chapters 16 and 17, as applicable.
- The supported end of a joist or beam must be within 1/4-inch from the supporting member.
- Solid blocking must be required for all joist hangers supporting roof joists having one end twisted more than one-half degree per foot of length relative to the other end, except as specifically noted in the evaluation report.
- Under the LARC, an engineered design in accordance with LARC Section R301.1.3 must be submitted.

This supplement expires concurrently with the evaluation report, reissued March 2024.

ICC-ES Evaluation Report

ESR-2552 FBC Supplement

Reissued March 2024

This report is subject to renewal March 2025.

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DIVISION: 06 00 00—WOOD, PLASTICS AND COMPOSITES
Section: 06 05 23—Wood, Plastic, and Composite Fastenings

REPORT HOLDER:

SIMPSON STRONG-TIE COMPANY INC.

EVALUATION SUBJECT:

SIMPSON STRONG-TIE® FACE-MOUNT HANGERS FOR SAWN LUMBER, STRUCTURAL COMPOSITE LUMBER (SCL), PREFABRICATED WOOD I-JOISTS AND GLULAM BEAMS (ENGINEERED WOOD PRODUCTS)

1.0 REPORT PURPOSE AND SCOPE

Purpose:

The purpose of this evaluation report supplement is to indicate that the Simpson Strong-Tie® face-mount hangers, described in ICC-ES evaluation report ESR-2552, have also been evaluated for compliance with the codes noted below.

Applicable code editions:

- 2023 *Florida Building Code—Building*
- 2023 *Florida Building Code—Residential*

2.0 CONCLUSIONS

The Simpson Strong-Tie® face-mount hangers, described in Sections 2.0 through 7.0 of ICC-ES evaluation report ESR-2552, comply with the *Florida Building Code—Building*, and the *Florida Building Code—Residential*, provided the design requirements are determined in accordance with the *Florida Building Code—Building* or the *Florida Building Code—Residential*, as applicable. The installation requirements noted in ICC-ES evaluation report ESR-2552 for the 2021 *International Building Code*® meet the requirements of the *Florida Building Code—Building* or the *Florida Building Code—Residential*, as applicable.

Use of the Simpson Strong-Tie® face-mount hangers has also been found to be in compliance with the High-Velocity Hurricane Zone provisions of the *Florida Building Code—Building*, and the *Florida Building Code—Residential* with the following condition:

- a. For connections subject to uplift, the connection must be designed for no less than 700 pounds (3114 N).

For products falling under Florida Rule 61G20-3, verification that the report holder's quality assurance program is audited by a quality assurance entity approved by the Florida Building Commission for the type of inspections being conducted is the responsibility of an approved validation entity (or the code official when the report holder does not possess an approval by the Commission).

This supplement expires concurrently with the evaluation report, reissued March 2024.

TABLE 10—ALLOWABLE LOADS FOR THE HGUS SERIES JOIST HANGERS

MODEL NO.	DIMENSIONS ¹ (Inches)			FASTENERS ² (Quantity-Type)		ALLOWABLE LOADS ^{3,5,6} (lbs)			
						Uplift ⁴	Download		
	W	H	B	Header	Joist ³	C _D = 1.6	C _D = 1.0	C _D = 1.15	C _D = 1.25
HGUS26	1 ⁵ / ₈	5 ³ / ₈	5	20-16d	8-16d	1,040	4,355	4,875	5,230
HGUS28	1 ⁵ / ₈	7 ¹ / ₈	5	36-16d	12-16d	1,650	7,275	7,275	7,275
HGUS210	1 ⁵ / ₈	9 ¹ / ₈	5	46-16d	16-16d	2,090	9,100	9,100	9,100
HGUS2.75/10	2 ³ / ₄	8 ⁷ / ₈	4	46-16d	16-16d	4,095	9,100	9,100	9,100
HGUS2.75/12	2 ³ / ₄	10 ⁷ / ₈	4	56-16d	20-16d	5,205	11,915	13,050	13,545
HGUS2.75/14	2 ³ / ₄	12 ⁷ / ₈	4	66-16d	22-16d	5,360	12,900	13,700	14,240
HGUS3.25/10	3 ¹ / ₄	8 ⁵ / ₈	4	46-16d	16-16d	4,095	9,100	9,100	9,100
HGUS3.25/12	3 ¹ / ₄	10 ⁵ / ₈	4	56-16d	20-16d	5,205	11,915	13,330	14,290
HGUS26-2	3 ⁷ / ₁₆	5 ⁷ / ₁₆	4	20-16d	8-16d	2,155	4,355	4,875	5,230
HGUS28-2	3 ⁷ / ₁₆	7 ³ / ₁₆	4	36-16d	12-16d	3,235	7,460	7,460	7,460
HGUS210-2	3 ⁷ / ₁₆	9 ³ / ₁₆	4	46-16d	16-16d	4,095	9,100	9,100	9,100
HGUS46	3 ⁵ / ₈	5 ⁵ / ₁₆	4	20-16d	8-16d	2,155	4,355	4,875	5,230
HGUS48	3 ⁵ / ₈	7 ¹ / ₁₆	4	36-16d	12-16d	3,235	7,460	7,460	7,460
HGUS410	3 ⁵ / ₈	9 ¹ / ₁₆	4	46-16d	16-16d	4,095	9,100	9,100	9,100
HGUS412	3 ⁵ / ₈	10 ⁷ / ₁₆	4	56-16d	20-16d	5,205	11,915	13,330	14,290
HGUS414	3 ⁵ / ₈	12 ⁷ / ₁₆	4	66-16d	22-16d	5,360	13,860	14,350	14,350
HGUS26-3	4 ¹⁵ / ₁₆	5 ⁷ / ₁₆	4	20-16d	8-16d	2,155	4,355	4,875	5,230
HGUS28-3	4 ¹⁵ / ₁₆	7 ³ / ₁₆	4	36-16d	12-16d	3,235	7,460	7,460	7,460
HGUS210-3	4 ¹⁵ / ₁₆	9 ³ / ₁₆	4	46-16d	16-16d	4,095	9,100	9,100	9,100
HGUS212-3	4 ¹⁵ / ₁₆	10 ³ / ₄	4	56-16d	20-16d	5,205	11,915	13,330	14,290
HGUS214-3	4 ¹⁵ / ₁₆	12 ³ / ₄	4	66-16d	22-16d	5,360	13,860	14,350	14,350
HGUS5.25/10	5 ¹ / ₄	9 ¹ / ₁₆	4	46-16d	16-16d	4,095	9,100	9,100	9,100
HGUS5.25/12	5 ¹ / ₄	10 ⁵ / ₈	4	56-16d	20-16d	5,205	11,915	13,330	14,290
HGUS5.5/8	5 ¹ / ₂	6 ¹⁵ / ₁₆	4	36-16d	12-16d	3,235	7,460	7,460	7,460
HGUS5.5/10	5 ¹ / ₂	8 ¹⁵ / ₁₆	4	46-16d	16-16d	4,095	9,100	9,100	9,100
HGUS5.5/12	5 ¹ / ₂	10 ¹ / ₂	4	56-16d	20-16d	5,205	11,915	13,330	14,290
HGUS5.5/14	5 ¹ / ₂	12 ¹ / ₂	4	66-16d	22-16d	5,515	9,695	9,695	9,695
HGUS5.62/10	5 ⁵ / ₈	8 ⁷ / ₈	4	46-16d	16-16d	4,095	9,100	9,100	9,100
HGUS5.62/12	5 ⁵ / ₈	10 ⁷ / ₁₆	4	56-16d	20-16d	5,205	11,915	13,330	14,290
HGUS5.62/14	55/8	12 ⁷ / ₁₆	4	66-16d	22-16d	5,515	9,695	9,695	9,695
HGUS26-4	6 ⁹ / ₁₆	5 ⁷ / ₁₆	4	20-16d	8-16d	2,155	4,355	4,875	5,230
HGUS28-4	6 ⁹ / ₁₆	7 ³ / ₁₆	4	36-16d	12-16d	3,235	7,460	7,460	7,460
HGUS210-4	6 ⁹ / ₁₆	9 ³ / ₁₆	4	46-16d	16-16d	4,095	9,100	9,100	9,100
HGUS212-4	6 ⁹ / ₁₆	10 ⁹ / ₁₆	4	56-16d	20-16d	5,205	11,915	13,330	14,290
HGUS214-4	6 ⁹ / ₁₆	12 ⁹ / ₁₆	4	66-16d	22-16d	5,360	13,735	14,360	14,360
HGUS6.88/10	6 ⁷ / ₈	8 ¹³ / ₁₆	4	46-16d	16-16d	4,095	9,100	9,100	9,100
HGUS6.88/12	6 ⁷ / ₈	10 ¹³ / ₁₆	4	56-16d	20-16d	5,205	11,915	13,330	14,290
HGUS6.88/14	6 ⁷ / ₈	12 ¹³ / ₁₆	4	66-16d	22-16d	5,515	9,695	9,695	9,695
HGUS7.25/8	7 ¹ / ₄	7 ¹ / ₄	4	36-16d	12-16d	3,235	7,460	7,460	7,460
HGUS7.25/10	7 ¹ / ₄	8 ⁵ / ₈	4	46-16d	16-16d	4,095	9,100	9,100	9,100
HGUS7.25/12	7 ¹ / ₄	10 ⁵ / ₈	4	56-16d	20-16d	5,205	11,915	13,330	14,290
HGUS7.25/14	7 ¹ / ₄	12 ⁵ / ₈	4	66-16d	22-16d	5,515	9,695	9,695	9,695
HGUS7.37/10	7 ³ / ₈	8 ⁹ / ₁₆	4	46-16d	16-16d	4,065	9,095	9,095	9,095
HGUS7.37/12	7 ³ / ₈	10 ⁹ / ₁₆	4	56-16d	20-16d	5,205	11,915	13,330	14,290
HGUS7.37/14	7 ³ / ₈	12 ⁹ / ₁₆	4	66-16d	22-16d	5,515	13,735	14,360	14,360

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N.

¹Refer to Figure 10 (this page) for definitions of hanger nomenclature (W, H, B).

²Refer to Section 3.2.3 of this report for nail sizes and required minimum physical properties.

³Tabulated allowable loads must be selected based on duration of load as permitted by the applicable building code.

⁴Joist nails must be driven at a 45 degree angle through the joist into the header/beam (double shear nailing) to achieve tabulated loads.

⁵Allowable uplift loads have been increased for wind or earthquake loading with no further increase allowed. The allowable uplift loads must be reduced when other load durations govern.

⁶HGUS series hangers provide torsional resistance, which is defined as the moment of not less than 75 pounds (334 N) times the depth of the joist at which the lateral movement of the top or bottom of the joist with respect to the vertical position of the joist is 0.125" (3.2 mm). The height, H, of the joist hanger must be at least 60 percent of the height of the joist unless additional lateral restraint is provided, as designed by others.

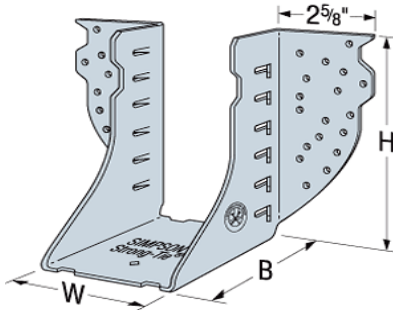


FIGURE 10—HGUS SERIES JOIST HANGER

TABLE 8—ALLOWABLE LOADS FOR THE HUS AND HUSC SERIES HANGERS

MODEL NO.	DIMENSIONS ¹ (inches)			COMMON NAILS ² (Quantity-Type)		ALLOWABLE LOADS ^{3,4,7} (lbf)			
	W	H	B	Header	Joist ⁵	Uplift ⁶	Download		
						C _D = 1.6	C _D = 1.0	C _D = 1.15	C _D = 1.25
HUS26	1 ⁵ / ₈	5 ³ / ₈	3	14-16d	6-16d	1,320	2,735	3095	3230
HUS28	1 ⁵ / ₈	7 ¹ / ₁₆	3	22-16d	8-16d	1,760	4095	4095	4095
HUS210	1 ⁵ / ₈	9 ¹ / ₁₆	3	30-16d	10-16d	2,635	5,450	5,795	5,830
HUS1.81/10	1 ¹³ / ₁₆	8 ⁷ / ₈	3	30-16d	10-16d	2,675	5,510	5,830	5,830
HUS46	3 ⁹ / ₁₆	4 ⁵ / ₁₆	2	4-16d	4-16d	1,165	1,090	1,225	1,320
HUS48	3 ⁹ / ₁₆	6 ¹⁵ / ₁₆	2	6-16d	6-16d	1,320	1,630	1,840	1,980
HUS410	3 ⁹ / ₁₆	8 ¹⁵ / ₁₆	2	8-16d	8-16d	3,265	2,175	2,455	2,640
HUS412	3 ⁹ / ₁₆	10 ³ / ₄	2	10-16d	10-16d	3,435	2,720	3,070	3,300
HUS26-2	3 ¹ / ₈	5 ³ / ₁₆	2	4-16d	4-16d	1,165	1,090	1,225	1,320
HUS28-2	3 ¹ / ₈	7 ³ / ₁₆	2	6-16d	6-16d	1,320	1,630	1,840	1,980
HUS210-2	3 ¹ / ₈	9 ³ / ₁₆	2	8-16d	8-16d	3,285	2,175	2,455	2,640
HUS212-2	3 ¹ / ₈	11	2	10-16d	10-16d	3,435	2,720	3,070	3,300

For SI: 1 inch = 25.4 mm, 1 pound = 4.45 N.

¹Refer to Figure 8 (this page) for definitions of hanger nomenclature (W, H, B).

²Refer to Section 3.2.3 of this report for nail sizes and required minimum physical properties.

³Tabulated allowable loads must be selected based on duration of load as permitted by the applicable building code.

⁴HUS series hangers provide torsional resistance, which is defined as a moment of not less than 75 pounds (334 N) times the depth of the joist at which the lateral movement of the top or bottom of the joist with respect to the vertical position of the joist is 0.125 inch (3.2 mm). The height, H, of the joist hanger must be at least 60 percent of the height of the joist unless additional lateral restraint is provided, as designed by others.

⁵Joist nails must be driven at a 45 degree angle through the joist into the header/beam (double shear nailing) to achieve the tabulated loads.

⁶Allowable uplift loads have been increased for wind or earthquake loading with no further increase is allowed. The allowable uplift loads must be reduced when other load durations govern.

⁷HUS series hangers with widths (W) equal to or greater than 3⁹/₁₆ inches (90 mm) are available with header flanges turned in (concealed) and are identified with the model designation HUSC#.

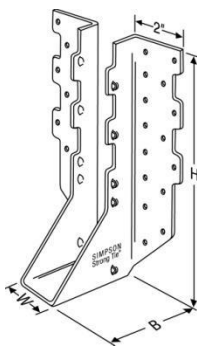


FIGURE 8—HUS SERIES HANGER (See Table 8)

TABLE 6 ALLOWABLE LOADS FOR THE LUS SERIES JOIST HANGERS

MODEL NO.	DIMENSIONS ¹ (inches)			COMMON NAILS ² (Quantity-Type)		ALLOWABLE LOADS ^{3,4} (lbf)			
	W	H	B	Header	Joist ⁵	Uplift ⁶	Download		
						C _D = 1.6	C _D = 1.0	C _D = 1.15	C _D = 1.25
LUS24	1 9/16	3 1/8	1 3/4	4-10d	2-10d	435	670	765	820
LUS26	1 9/16	4 3/4	1 3/4	4-10d	4-10d	1,165	865	990	1,060
LUS28	1 9/16	6 5/8	1 3/4	6-10d	4-10d	1,165	1,100	1,260	1,350
LUS210	1 9/16	7 13/16	1 3/4	8-10d	4-10d	1,165	1,335	1,530	1,640
LUS36	2 9/16	5 1/4	2	4-16d	4-16d	1,060	1,030	1,170	1,265
LUS310	2 9/16	7 9/32	2	6-16d	4-16d	1,070	1,315	1,490	1,610
LUS24-2	3 1/8	3 1/8	2	4-16d	2-16d	410	800	905	980
LUS26-2	3 1/8	4 15/16	2	4-16d	4-16d	1,060	1,030	1,170	1,265
LUS28-2	3 1/8	7	2	6-16d	4-16d	1,060	1,315	1,490	1,610
LUS210-2	3 1/8	8 15/16	2	8-16d	6-16d	1,445	1,830	2,075	2,245
LUS214-2	3 1/8	10 15/16	2	10-16d	6-16d	1,445	2,110	2,395	2,590
LUS44	3 9/16	3	2	4-16d	2-16d	410	800	905	980
LUS46	3 9/16	4 3/4	2	4-16d	4-16d	1,060	1,030	1,170	1,265
LUS48	3 9/16	6 3/4	2	6-16d	4-16d	1,060	1,315	1,490	1,610
LUS410	3 9/16	8 3/4	2	8-16d	6-16d	1,445	1,830	2,075	2,245
LUS414	3 9/16	10 3/4	2	10-16d	6-16d	1,445	2,110	2,395	2,590
LUS26-3	4 5/8	4 1/8	2	4-16d	4-16d	1,060	1,030	1,170	1,265
LUS28-3	4 5/8	6 1/4	2	6-16d	4-16d	1,060	1,315	1,490	1,610
LUS210-3	4 5/8	8 13/16	2	8-16d	6-16d	1,445	1,830	2,075	2,245

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N.

¹Refer to Figure 6 (this page) for definitions of hanger nomenclature (W, H, B).

²Refer to Section 3.2.3 of this report for nail sizes and required minimum physical properties.

³Tabulated allowable loads must be selected based on duration of load as permitted by the applicable building code.

⁴LUS Series hangers provide torsional resistance, which is defined as a moment of not less than 75 pounds (334 N) times the depth of the joist at which the lateral movement of the top or bottom of the joist with respect to the vertical position of the joist is 0.125 inch (3.2 mm). The height, H, of the joist hanger must be at least 60 percent of the height of the joist unless additional lateral restraint is provided, as designed by others.

⁵Joist nails must be driven at a 45 degree angle through the joist into the header/beam (double shear nailing) to achieve the tabulated loads.

⁶Allowable uplift loads have been increased for wind or earthquake loading with no further increase is allowed. The allowable uplift loads must be reduced when other load durations govern.

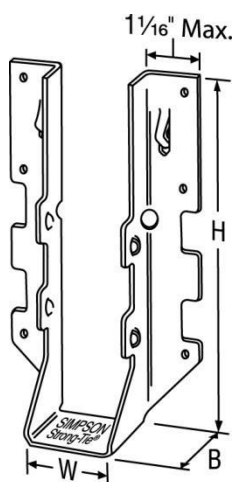


FIGURE 6—LUS SERIES HANGER
(See Table 6)

TABLE 7—NAILING SCHEDULES, DIMENSIONS AND ALLOWABLE LOADS FOR HTW, LTW AND MTW TWIST STRAP TIES⁵

STOCK NUMBER	STEEL GAGE	STRAP DIMENSIONS (in.)				FASTENER SCHEDULE		ALLOWABLE UPLIFT LOAD ^{1,2} C _D = 1.6 (lbs)	
		W	L	L1	L2	Qty ³	Type ⁴	Stud-to-rim joist installation	Truss-to-top – plate installation
HTW16	14	1 ¹ / ₄	16	5 ¹ / ₈	5 ¹ / ₈	16	10d x 1 ¹ / ₂	1,115	1,355
						16	10d common	1,300	1,355
HTW20	14	1 ¹ / ₄	20	7 ¹ / ₈	7 ¹ / ₈	24	10d x 1 ¹ / ₂	1,555	1,355
						20	10d common	1,355	1,355
HTW24	14	1 ¹ / ₄	24	9 ¹ / ₈	9 ¹ / ₈	24	10d x 1 ¹ / ₂	1,555	1,355
						20	10d common	1,355	1,355
HTW28	14	1 ¹ / ₄	28	11 ¹ / ₈	11 ¹ / ₈	24	10d x 1 ¹ / ₂	1,555	1,355
						20	10d common	1,355	1,355
HTW30	14	1 ¹ / ₄	30	7	17 ¹ / ₄	24	10d x 1 ¹ / ₂	1,555	1,355
						20	10d common	1,355	1,355
HTW30C	14	1 ¹ / ₄	30	12 ¹ / ₈	12 ¹ / ₈	24	10d x 1 ¹ / ₂	1,555	1,355
						20	10d common	1,355	1,355
LTW12	18	1 ¹ / ₄	12	4 ¹ / ₂	4 ¹ / ₂	12	10d x 1 ¹ / ₂	770	625
						12	10d common	770	625
LTW16	18	1 ¹ / ₄	16	6 ¹ / ₂	6 ¹ / ₂	12	10d x 1 ¹ / ₂	770	625
						12	10d common	770	625
LTW18	18	1 ¹ / ₄	18	7 ¹ / ₂	7 ¹ / ₂	12	10d x 1 ¹ / ₂	770	625
						12	10d common	770	625
LTW20	18	1 ¹ / ₄	20	8 ¹ / ₂	8 ¹ / ₂	12	10d x 1 ¹ / ₂	770	625
						12	10d common	770	625
MTW12	16	1 ¹ / ₄	12	4 ¹ / ₂	4 ¹ / ₂	14	10d x 1 ¹ / ₂	1,185	965
						14	10d common	1,185	965
MTW16	16	1 ¹ / ₄	16	6 ¹ / ₂	6 ¹ / ₂	14	10d x 1 ¹ / ₂	1,185	965
						14	10d common	1,185	965
MTW18	16	1 ¹ / ₄	18	7 ¹ / ₂	7 ¹ / ₂	14	10d x 1 ¹ / ₂	1,185	965
						14	10d common	1,185	965
MTW20	16	1 ¹ / ₄	20	8 ¹ / ₂	8 ¹ / ₂	14	10d x 1 ¹ / ₂	1,185	965
						14	10d common	1,185	965
MTW24C	16	1 ¹ / ₄	24	10 ⁷ / ₁₆	10 ⁷ / ₁₆	14	10d x 1 ¹ / ₂	1,185	965
						14	10d common	1,185	965
MTW28C	16	1 ¹ / ₄	28	12 ⁷ / ₁₆	12 ⁷ / ₁₆	14	10d x 1 ¹ / ₂	1,185	965
						14	10d common	1,185	965
MTW30C	16	1 ¹ / ₄	30	13 ⁷ / ₁₆	13 ⁷ / ₁₆	14	10d x 1 ¹ / ₂	1,185	965
						14	10d common	1,185	965
MTW30	16	1 ¹ / ₄	30	8 ⁵ / ₁₆	18 ⁹ / ₁₆	14	10d x 1 ¹ / ₂	1,185	965
						14	10d common	1,185	965

For SI: 1 inch = 25.4 mm, 1 lb = 4.45 N.

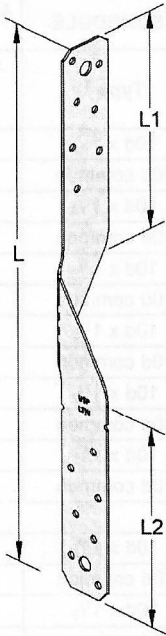
¹Allowable uplift loads have been adjusted for a load duration factor, C_D, of 1.60, corresponding to a ten-minute load duration (i.e., wind or earthquake loading), in accordance with the NDS. The allowable loads do not apply to loads of other durations, and are not permitted to be adjusted for other load durations. See Section 4.1 for additional design requirements.

²Allowable uplift loads shown are for installations in sawn lumber or structural composite lumber complying with Section 3.26.2.

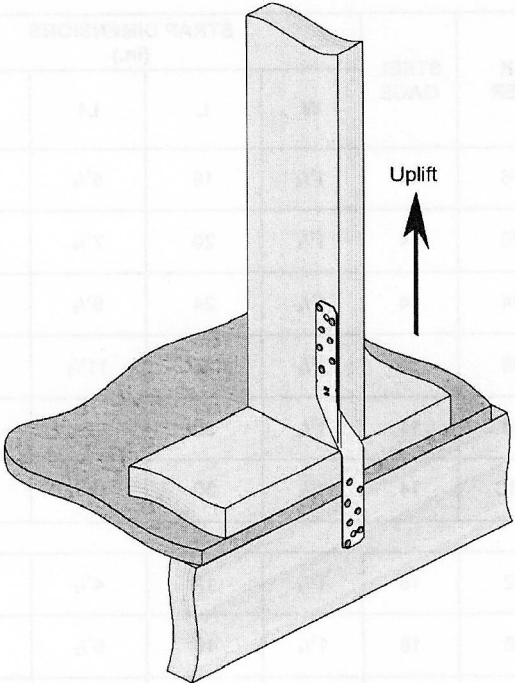
³Listed fastener quantity is the total required for the connection, with half the tabulated quantity at each end.

⁴See Section 3.26.3 for required nail dimensions and mechanical properties. HTW, LTW and MTW twist strap ties may be installed with either 10d x 1¹/₂ or 10d common nails.

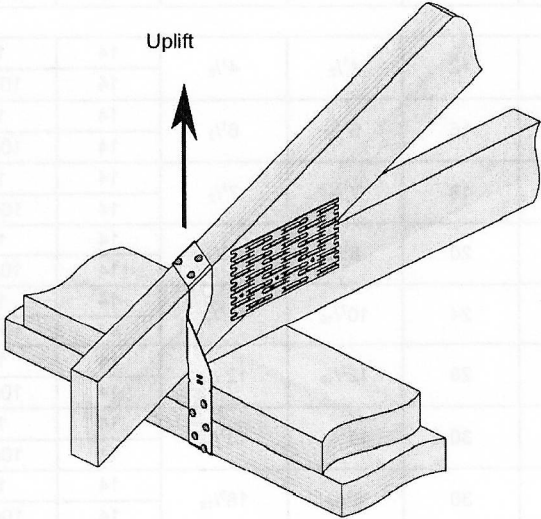
⁵Some illustrations for the twist straps show connection that could cause cross-grain tension or bending of the wood during loading if not reinforced sufficiently. In such cases, mechanical reinforcement should be considered.



LTW/MTW/HTW



Typical LTW/MTW/HTW
Stud-To-Rim Joist Installation



Typical LTW/MTW/HTW
Truss-To-Top Plate Installation

FIGURE 7—DIMENSIONS AND INSTALLATION OF LTW, MTW AND HTW TWIST STRAP TIES

TABLE 13—ALLOWABLE LOADS FOR THE SUR/SUL SERIES JOIST HANGERS

MODEL NO.	DIMENSIONS ¹ (inches)					FASTENERS (Quantity-Type)		ALLOWABLE LOADS ^{2,6,7} (lbs)			
	W	H	B	A ₁	A ₂	Header	Joist	Uplift ⁵ C _D =1.6	Download C _D =1.0 C _D =1.15 C _D =1.25		
SUR/L24	1 ⁹ / ₁₆	3 ¹ / ₁₆	2	1 ¹ / ₈	1 ¹ / ₄	4-16d	4-10d x 1 ¹ / ₂	395	575	650	705
SUR/L26	1 ⁹ / ₁₆	5	2	1 ¹ / ₈	1 ⁵ / ₁₆	6-16d	6-10d x 1 ¹ / ₂	675	865	980	1,055
SUR/L210	1 ⁹ / ₁₆	8	2	1 ¹ / ₈	1 ⁵ / ₁₆	10-16d	10-10d x 1 ¹ / ₂	1,445	1,440	1,635	1,760
SUR/L214	1 ⁹ / ₁₆	10	2	1 ¹ / ₈	1 ⁵ / ₁₆	12-16d	12-10d x 1 ¹ / ₂	1,890	1,730	1,960	2,115
SUR/L1.81/9	1 ¹³ / ₁₆	9	3	1 ⁵ / ₈	2 ⁵ / ₁₆	12-16d	2-10d x 1 ¹ / ₂	210 ^{3,4}	1,730	1,955	1,955
SUR/L1.81/11	1 ¹³ / ₁₆	11	3	1 ⁵ / ₈	2 ⁵ / ₁₆	16-16d	2-10d x 1 ¹ / ₂	210 ^{3,4}	2,300	2,560	2,560
SUR/L1.81/14	1 ¹³ / ₁₆	13 ³ / ₄	3	1 ⁵ / ₈	2 ⁵ / ₁₆	20-16d	2-10d x 1 ¹ / ₂	210 ^{3,4}	2,560	2,560	2,560
SUR/L2.06/9	2 ¹ / ₁₆	9 ¹ / ₁₆	3 ³ / ₁₆	1 ⁵ / ₈	2 ¹ / ₈	14-16d	2-10d x 1 ¹ / ₂	210 ³	2,015	2,285	2,465
SUR/L2.06/11	2 ¹ / ₁₆	11 ¹ / ₄	3 ³ / ₁₆	1 ⁵ / ₈	2 ¹ / ₈	16-16d	2-10d x 1 ¹ / ₂	210 ³	2,300	2,610	2,665
SUR/L2.06/14	2 ¹ / ₁₆	13 ⁵ / ₈	3 ³ / ₁₆	1 ⁵ / ₈	2 ¹ / ₈	18-16d	2-10d x 1 ¹ / ₂	210 ³	2,590	2,665	2,665
SUR/L2.1/9	2 ¹ / ₈	9 ¹ / ₁₆	3 ³ / ₁₆	1 ⁹ / ₁₆	2 ¹ / ₈	14-16d	2-10d x 1 ¹ / ₂	210 ³	2,015	2,285	2,465
SUR/L2.1/11	2 ¹ / ₈	11 ³ / ₁₆	3 ³ / ₁₆	1 ⁹ / ₁₆	2 ¹ / ₈	16-16d	2-10d x 1 ¹ / ₂	210 ³	2,300	2,610	2,665
SUR/L2.1/14	2 ¹ / ₈	13 ⁹ / ₁₆	3 ³ / ₁₆	1 ⁹ / ₁₆	2 ¹ / ₈	18-16d	2-10d x 1 ¹ / ₂	210 ³	2,590	2,665	2,665
SUR/L2.37/9	2 ³ / ₈	8 ¹⁵ / ₁₆	3 ³ / ₁₆	1 ⁵ / ₁₆	2 ¹ / ₈	14-16d	2-10d x 1 ¹ / ₂	210 ³	2,015	2,285	2,465
SUR/L2.37/11	2 ³ / ₈	11 ³ / ₁₆	3 ³ / ₁₆	1 ⁵ / ₁₆	2 ¹ / ₈	16-16d	2-10d x 1 ¹ / ₂	210 ³	2,300	2,610	2,665
SUR/L2.37/14	2 ³ / ₈	13 ⁷ / ₁₆	3 ³ / ₁₆	1 ⁵ / ₁₆	2 ¹ / ₈	18-16d	2-10d x 1 ¹ / ₂	210 ³	2,590	2,665	2,665
SUR/L2.56/9	2 ⁹ / ₁₆	8 ¹³ / ₁₆	3 ³ / ₁₆	1 ¹ / ₈	2 ¹ / ₈	14-16d	2-10d x 1 ¹ / ₂	210 ³	2,015	2,285	2,465
SUR/L2.56/11	2 ⁹ / ₁₆	11 ³ / ₁₆	3 ³ / ₁₆	1 ¹ / ₈	2 ¹ / ₈	16-16d	2-10d x 1 ¹ / ₂	210 ³	2,300	2,615	2,665
SUR/L2.56/14	2 ⁹ / ₁₆	13 ⁵ / ₁₆	3 ³ / ₁₆	1 ¹ / ₈	2 ¹ / ₈	18-16d	2-10d x 1 ¹ / ₂	210 ³	2,590	2,665	2,665
SUR/L26-2	3 ¹ / ₈	4 ¹⁵ / ₁₆	2 ⁵ / ₈	1 ¹ / ₂	2 ³ / ₈	8-16d	4-16d x 2 ¹ / ₂	725	1,150	1,305	1,325
SUR/L210-2	3 ¹ / ₈	8 ¹ / ₁₆	2 ⁵ / ₈	1 ⁷ / ₁₆	2 ³ / ₈	14-16d	6-16d x 2 ¹ / ₂	1,160	2,015	2,285	2,345
SUR/L214-2	3 ¹ / ₈	12 ¹ / ₁₆	2 ⁷ / ₁₆	1 ⁷ / ₁₆	2 ³ / ₈	18-16d	8-16d x 2 ¹ / ₂	1,505	2,345	2,345	2,345
SUR/L46	3 ⁹ / ₁₆	4 ³ / ₄	2 ⁵ / ₈	1	2 ³ / ₈	8-16d	4-16d	730	1,150	1,305	1,325
SUR/L410	3 ⁹ / ₁₆	8 ¹ / ₂	2 ⁵ / ₈	1	2 ³ / ₈	14-16d	6-16d	1,160	2,015	2,285	2,345
SUR/L414	3 ⁹ / ₁₆	12 ¹ / ₂	2 ⁵ / ₈	1	2 ³ / ₈	18-16d	8-16d	1,490	2,345	2,345	2,345

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N.

¹Refer to Figure 13a (this page) for definitions of hanger nomenclature (W, H, B, A₁, A₂).

²Tabulated allowable loads must be selected based on duration of load as permitted by the applicable building code.

³Optional triangle holes may be filled (requires web stiffeners for I-joist) for additional uplift resistance. When a total of six 10d x 1¹/₂-inch-long nails are installed into the joist for 9- and 11-inch models, an uplift value of 825 lbs may be used. When a total of eight 10d x 1¹/₂" nails are installed into the joist for 14-inch models, an uplift value of 1,190 lbs may be used.

⁴Uplift value based on use of solid sawn lumber or LVL joist member. When using an I-joist as the joist member, allowable uplift must be taken as 140 lbs.

⁵The uplift loads have been increased for wind or earthquake loading with no further increase allowed. The allowable uplift loads must be reduced when other load durations govern.

⁶SUR/L series hangers provide torsional resistance, which is defined as a moment of not less than 75 pounds (334 N) times the depth of the joist at which the lateral movement of the top or bottom of the joist with respect to its vertical position is 0.125 inch (3.2 mm). The height (H) of the hanger must be at least 60 percent of the height of the joist unless additional lateral restraint is provided, as designed by others.

⁷The 2-2x and a 4x SUR/L models are available with the A₂ flanges concealed and are specified with the model designation SUR/LC

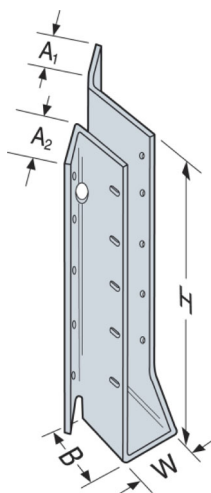


FIGURE 13A—SUL SERIES JOIST HANGER

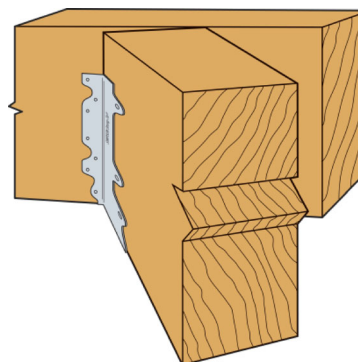


FIGURE 13B—TYPICAL SUR HANGER INSTALLATION

Valley Detail - ASCE 7-16: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

**** Attach each valley to every supporting truss with:**
 535# connection or with (1) Simpson H2.5A or
 equivalent connector for
 ASCE 7-16 180 mph. 30' Mean Height, Part. Enc.
 Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00
 Or
 ASCE 7-16 160 mph. 30' Mean Height, Part. Enc.
 Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut
 as shown.

Valleys short enough to be cut as solid triangular
 members from a single 2x6, or larger as required,
 shall be permitted in lieu of fabricating from
 separate 2x4 members.

All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical
 valley webs taller than 7'-9" apply 2x4 "T" reinforcement, 80% length of
 web, same species and grade or better, attached with 10d box
 (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous
 Lateral Restraint applied at mid-length of web is permitted with diagonal
 bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with:
 properly attached, rated sheathing applied prior to valley truss
 installation.

Or

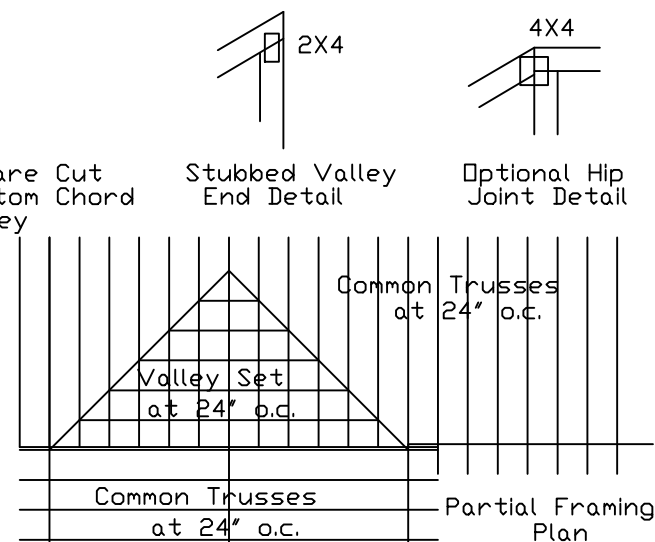
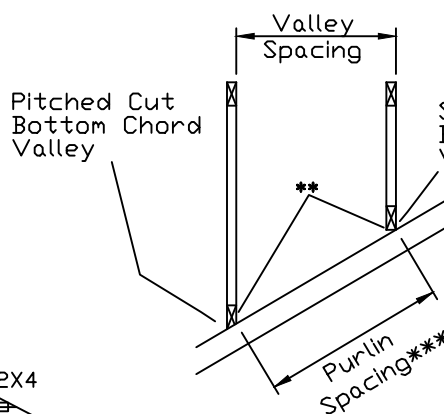
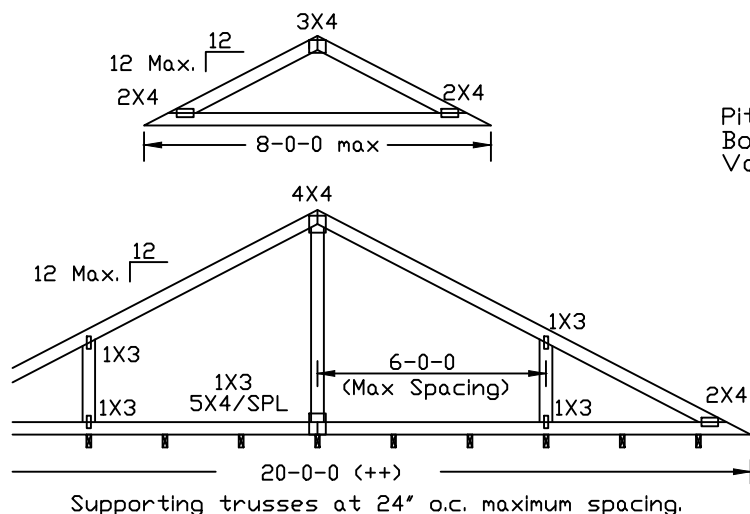
Purlins at 24" o.c. or as otherwise specified on engineer's sealed design

Or

By valley trusses used in lieu of purlin spacing as specified on
 Engineer's sealed design.

***** Note that the purlin spacing for bracing the top chord of the truss
 beneath the valley is measured along the slope of the top chord.**

**++ Larger spans may be built as long as the vertical height does
 not exceed 14'-0".**



*****WARNING*** READ AND FOLLOW ALL NOTES ON THIS DRAWING
 IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.**
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and
 follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety
 practices prior to performing these functions. Installers shall provide temporary bracing per BCSI.
 Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord
 shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs
 shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face
 of truss and position as shown above and on the Joint Details, unless noted otherwise.
 Refer to drawings 160A-Z for standard plate positions.
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from
 this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping,
 installation & bracing of trusses.
 A seal on this drawing or cover page listing this drawing, indicates acceptance of professional
 engineering responsibility solely for the design shown. The suitability and use of this drawing
 for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
 For more information see this Job's general notes page and these web sites:
 ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

TC LL	30	30	40PSF	REF	VALLEY DETAIL
TC DL	20	15	7PSF	DATE	01/26/2018
BC DL	10	10	10 PSF	DRWG	VAL180160118
BC LL	0	0	0PSF		
TOT. LD.	60	55	57PSF		
DUR.FAC.1.25/1.33	1.15	1.15			
SPACING	24.0"				

GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary lateral restraint and diagonal bracing. Follow the recommendations for handling, installing and temporary restraining and bracing of trusses. Refer to **BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses***** for more detailed information.

Truss Design Drawings may specify locations of permanent lateral restraint or reinforcement for individual truss members. Refer to the **BCSI-B3***** for more information. All other permanent bracing design is the responsibility of the building designer.

NOTAS GENERALES

Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de restricción lateral y arrioste diagonal temporales. Use las recomendaciones de manejo, instalación, restricción y arrioste temporal de los trusses. Vea el folleto **BCSI - Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arrioste de los Trusses de Madera Conectados con Placas de Metal***** para información más detallada.

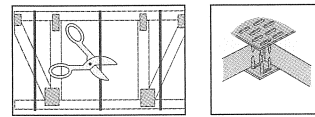
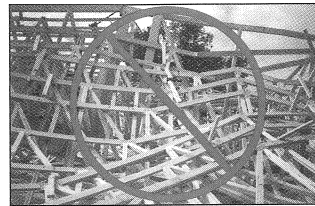
Los dibujos de diseño de los trusses pueden especificar las localizaciones de restricción lateral permanente o refuerzo en los miembros individuales del truss. Vea la hoja resumen **BCSI-B3***** para más información. El resto de los diseños de arriostres permanentes son la responsabilidad del diseñador del edificio.

WARNING! The consequences of improper handling, erecting, installing, restraining and bracing can result in a collapse of the structure, or worse, serious personal injury or death.

¡ADVERTENCIA! El resultado de un manejo, levantamiento, instalación, restricción y arrioste incorrecto puede ser la caída de la estructura o aún peor, heridos o muertos.

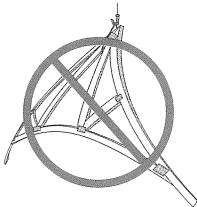
CAUTION! Banding and truss plates have sharp edges. Wear gloves when handling and safety glasses when cutting banding.

¡CAUTELA! Chapas de metal tienen bordes afilados. Lleve guantes y lentes protectores cuando corte las ataduras.

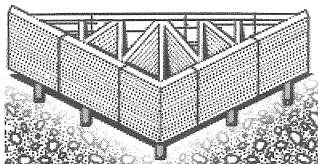


HANDLING — MANEJO

NOTICE Avoid lateral bending. Evite la flexión lateral.



NOTICE The contractor is responsible for properly receiving, unloading and storing the trusses at the jobsite. Unload trusses to smooth surface to prevent damage. El contratista tiene la responsabilidad de recibir, descargar y almacenar adecuadamente los trusses en la obra. Descargue los trusses en la tierra lisa para prevenir el daño.



Trusses may be unloaded directly on the ground at the time of delivery or stored temporarily in contact with the ground after delivery. If trusses are to be stored horizontally for more than one week, place blocking of sufficient height beneath the stack of trusses at 8' (2.4 m) to 10' (3 m) on-center (o.c.).

Los trusses pueden ser descargados directamente en el suelo en aquel momento de entrega o almacenados temporalmente en contacto con el suelo después de entrega. Si los trusses estarán guardados horizontalmente para más de una semana, ponga bloqueando de altura suficiente detrás de la pila de los trusses a 8 hasta 10 pies en centro (o.c.).

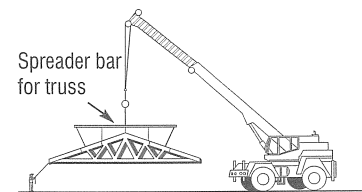
For trusses stored for more than one week, cover bundles to protect from the environment.

Para trusses guardados por más de una semana, cubra los paquetes para protegerlos del ambiente.

Refer to **BCSI***** for more detailed information pertaining to handling and jobsite storage of trusses.

Vea el folleto **BCSI***** para información más detallada sobre el manejo y almacenado de los trusses en área de trabajo.

CAUTION! Use special care in windy weather or near power lines and airports.



Use proper rigging and hoisting equipment.

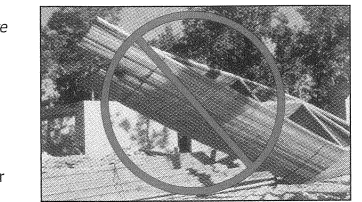
¡CAUTELA! Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.

Use equipo apropiado para levantar e improvisar.



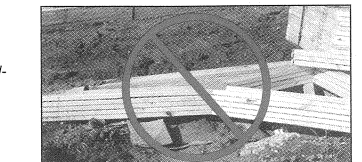
DO NOT store unbraced bundles upright.

NO almacene verticalmente los trusses sueltos.



DO NOT store on uneven ground.

NO almacene en tierra desigual.



HOISTING AND PLACEMENT OF TRUSS BUNDLES

RECOMENDACIONES PARA LEVANTAR PAQUETES DE TRUSSES

DON'T overload the crane.

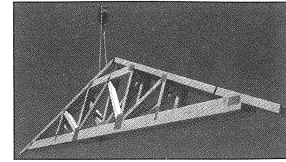
NO sobrecargue la grúa.

NEVER use banding to lift a bundle.

NUNCA use las ataduras para levantar un paquete.

A single lift point may be used for bundles of top chord pitch trusses up to 45' (13.7 m) and parallel chord trusses of 30' (9.1 m) or less. Use at least two lift points for bundles with trusses up to 60' (18.3 m). Use at least 3 lift points for bundles with trusses greater than 60' (18.3 m).

Puede usar un solo lugar de levantar para paquetes de trusses de la cuerda superior hasta 45' y trusses de cuerdas paralelas de 30' o menos. Puede usar dos puntos de levantar para paquetes más de 60 pies. Use por lo menos tres puntos de levantar para paquetes más de 60 pies.



WARNING! Do not over load supporting structure with truss bundle.

¡ADVERTENCIA! No sobrecargue la estructura apoyada con el paquete de trusses.

Place truss bundles in stable position.

Puse paquetes de trusses en una posición estable.

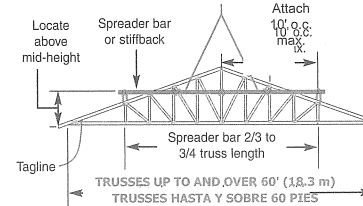
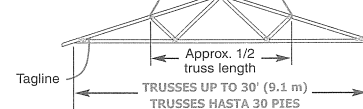
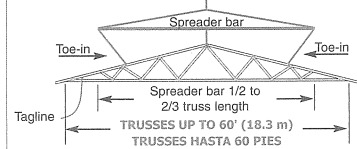
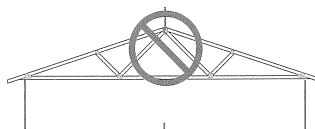
MECHANICAL HOISTING RECOMMENDATIONS FOR SINGLE TRUSSES
RECOMENDACIONES PARA LEVANTAR TRUSSES INDIVIDUALES

Hold each truss in position with the erection equipment until top chord temporary lateral restraint is installed and the truss is fastened to the bearing points.

Sostenga cada truss en posición con equipo de grúa hasta que la restricción lateral temporal de la cuerda superior esté instalada y el truss está asegurado en los soportes.

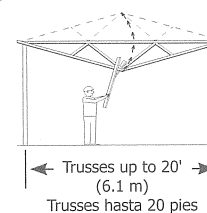
NOTICE Using a single pick-point at the peak can damage the truss.

El uso de un solo lugar en el pico para levantar puede hacer daño al truss.

INSTALLATION OF SINGLE TRUSSES BY HAND
RECOMENDACIONES DE LEVANTAMIENTO DE TRUSSES INDIVIDUALES POR LA MANO

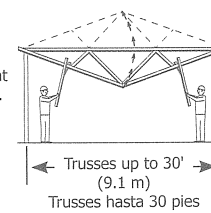
Trusses 20' (6.1 m) or less, support near peak.

Soporte cerca al pico los trusses de 20 pies o menos.



Trusses 30' (9.1 m) or less, support at quarter points.

Soporte de los cuartos de tramo los trusses de 30 pies o menos.

TEMPORARY RESTRAINT & BRACING
RESTRICCIÓN Y ARRIOSTRE TEMPORAL

NOTICE Refer to **BCSI-B2***** for more information.

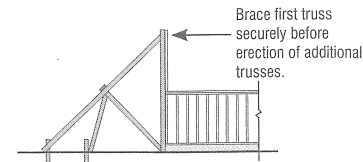
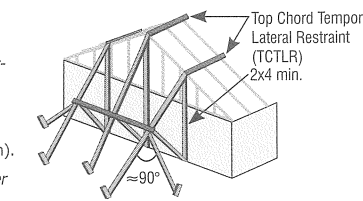
Vea el resumen **BCSI-B2***** para más información.

Locate ground braces for first truss directly in line with all rows of top chord temporary lateral restraint (see table in the next column).

Coloque los arriostres de tierra para el primer truss directamente en línea con cada una de las filas de restricción lateral temporal de la cuerda superior (vea la tabla en la próxima columna).



DO NOT walk on unbraced trusses. **NO** camine en trusses sueltos.



STEPS TO SETTING TRUSSES
LAS MEDIDAS DE LA INSTALACIÓN DE LOS TRUSSES

- 1) Install ground bracing. 2) Set first truss and attach securely to ground bracing. 3) Set next 4 trusses with short member temporary lateral restraint (see below). 4) Install top chord diagonal bracing (see below). 5) Install web member plane diagonal bracing to stabilize the first five trusses (see below). 6) Install bottom chord temporary lateral restraint and diagonal bracing (see below). 7) Repeat process on groups of four trusses until all trusses are set.

NOTICE Refer to BCSI-B2*** for more information.
Vea el resumen BCSI-B2*** para más información.

RESTRAINT/BRACING FOR ALL PLANES OF TRUSSES
RESTRICCIÓN/ARRIOSTRE PARA TODOS PLANOS DE TRUSSES

- This restraint & bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses (PCTs). See top of next column for temporary restraint and bracing of PCTs.
Este método de restricción y arriostre es para todo trusses excepto trusses de cuerdas paralelas (PCTs) 3x2 y 4x2. Vea la parte superior de la columna para la restricción y arriostre temporal de PCTs.

1) TOP CHORD — CUERDA SUPERIOR

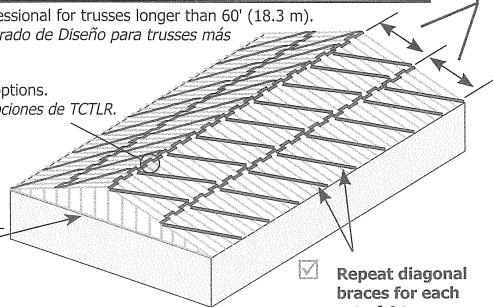
Table with 2 columns: Truss Span / Longitud de Tramo, Top Chord Temporary Lateral Restraint (TCLTR) Spacing / Espaciamiento del Arriostre Temporal de la Cuerda Superior. Rows show spacing requirements for spans up to 30', 30'-45', 45'-60', and 60'-80'.

*Consult a Registered Design Professional for trusses longer than 60' (18.3 m).
*Consulte a un Profesional Registrado de Diseño para trusses más de 60 pies.

- See BCSI-B2*** for TCLTR options.
Vea el BCSI-B2*** para las opciones de TCLTR.

NOTICE Refer to BCSI-B3*** for Gable End Frame restraint/bracing/ reinforcement information.

Para información sobre restricción/arriostre/refuerzo para Armazones Hastiales vea el resumen BCSI-B3***



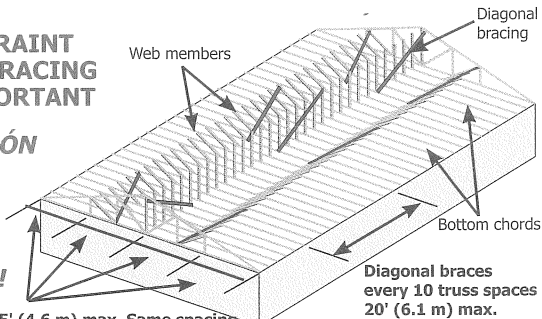
Note: Ground bracing not shown for clarity.

- Repeat diagonal braces for each set of 4 trusses.
Repita los arriostres diagonales para cada grupo de 4 trusses.

2) WEB MEMBER PLANE — PLANO DE LOS MIEMBROS SECUNDARIOS

NOTICE
LATERAL RESTRAINT & DIAGONAL BRACING ARE VERY IMPORTANT

LA RESTRICCIÓN LATERAL Y EL ARRIOSTRE DIAGONAL SON MUY IMPORTANTES!



10' (3 m) - 15' (4.6 m) max. Same spacing as bottom chord lateral restraint

Note: Some chord and web members not shown for clarity.

3) BOTTOM CHORD — CUERDA INFERIOR

Lateral Restraints - 2x4x12' or greater lapped over two trusses.

Bottom chords

10' (3 m) - 15' (4.6 m) max.

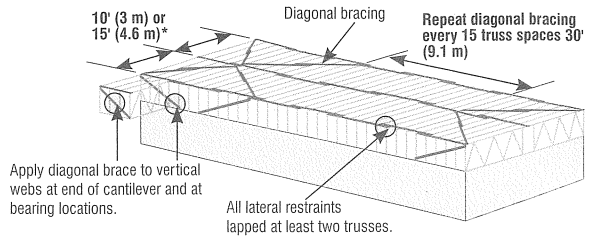
Diagonal braces every 10 truss spaces 20' (6.1 m) max.

Note: Some chord and web members not shown for clarity.

RESTRAINT & BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES
RESTRICCIÓN Y ARRIOSTRE PARA TRUSSES DE CUERDAS PARALELAS 3X2 Y 4X2

NOTICE Refer to BCSI-B7*** for more information.

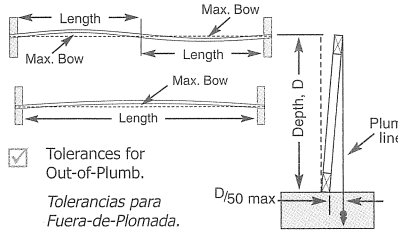
Vea el resumen BCSI-B7*** para más información.



*Top chord temporary lateral restraint spacing shall be 10' (3 m) o.c. max. for 3x2 chords and 15' (4.6 m) o.c. for 4x2 chords.

INSTALLING — INSTALACIÓN

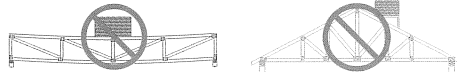
- Tolerances for Out-of-Plane.
Tolerancias para Fuera-de-Plano.



- Tolerances for Out-of-Plumb.
Tolerancias para Fuera-de-Plomada.

CONSTRUCTION LOADING
CARGA DE CONSTRUCCIÓN

- DO NOT proceed with construction until all lateral restraint and bracing is securely and properly in place.
NO proceda con la construcción hasta que todas las restricciones laterales y los arriostres estén colocados en forma apropiada y segura.
DO NOT exceed maximum stack heights. Refer to BCSI-B4*** for more information.
NO exceda las alturas máximas de montón. Vea el resumen BCSI-B4*** para más información.



- DO NOT overload small groups or single trusses.
NO sobrecargue pequeños grupos o trusses individuales.
NEVER stack materials near a peak or at mid-span.
NUNCA amontone los materiales cerca de un pico.
Place loads over as many trusses as possible.
Coloque las cargas sobre tantos trusses como sea posible.
Position loads over load bearing walls.
Coloque las cargas sobre las paredes soportantes.

ALTERATIONS — ALTERACIONES

NOTICE Refer to BCSI-B5***
Vea el resumen BCSI-B5***

- DO NOT cut, alter, or drill any structural member of a truss unless specifically permitted by the truss design drawing.
NO corte, altere o perforo ningún miembro estructural de un truss, a menos que esté específicamente permitido en el dibujo del diseño del truss.

NOTICE Trusses that have been overloaded during construction or altered without the Truss Manufacturer's prior approval may render the Truss Manufacturer's limited warranty null and void.
Trusses que se han sobrecargado durante la construcción o han sido alterados sin la autorización previa del Fabricante de Trusses, pueden hacer nulo y sin efecto la garantía limitada del Fabricante de Trusses.

***Contact the Component Manufacturer for more information or consult a Registered Design Professional for assistance.
To view a non-printing PDF of this document, visit www.sbcindustry.com/b1.

NOTE: The truss manufacturer and truss designer rely on the presumption that the contractor and crane operator (if applicable) are professionals with the capability to undertake the work they have agreed to do on any given project. If the contractor believes it needs assistance in some aspect of the construction project, it should seek assistance from a competent party. The methods and procedures outlined in this document are intended to ensure that the overall construction techniques employed will put the trusses into place SAFELY. These recommendations for handling, installing, restraining and bracing trusses are based upon the collective experience of leading personnel involved with truss design, manufacture and installation, but must, due to the nature of responsibilities involved, be presented only as a GUIDE for use by a qualified building designer or contractor. It is not intended that these recommendations be interpreted as superior to the building designer's design specification for handling, installing, restraining and bracing trusses and it does not preclude the use of other equivalent methods for restraining/bracing and providing stability for the walls, columns, floors, roofs and all the interrelated structural building components as determined by the contractor. Thus, SBCA and TPI expressly disclaim any responsibility for damages arising from the use, application, or reliance on the recommendations and information contained herein.



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