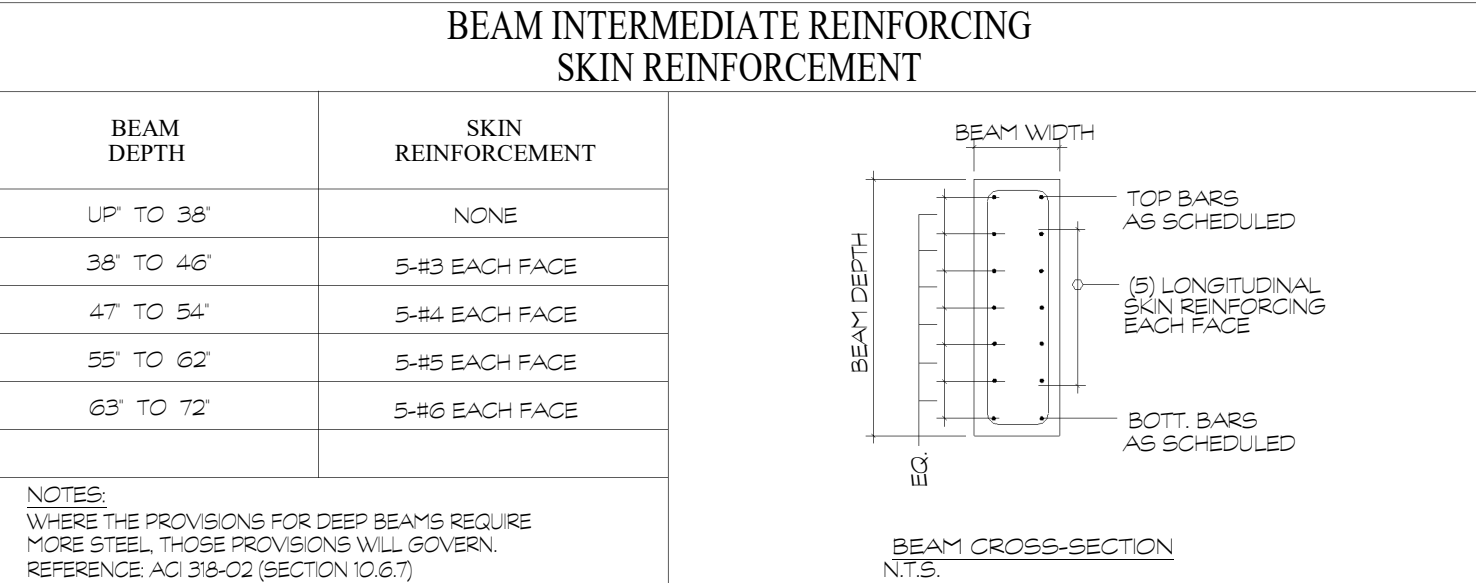


GENERAL STRUCTURAL NOTES:

- E
1. SITE AND SOIL PREPARATION:
GENERAL CONTRACTOR SHALL PERFORM THE SITE PREPARATION AND EXCAVATION WORK IN ACCORDANCE WITH THE RECOMMENDATIONS ON SOILS AND FOUNDATIONS INVESTIGATION PREPARED BY AN APPROVED TESTING LABORATORY PRIOR TO FOUNDATION WORK.
A COPY OF THE REPORT SHALL BE OBTAINED BY THE GENERAL CONTRACTOR FOR ADDITIONAL INFORMATION PERTAINING TO THE FOUNDATION.
2. FOUNDATION ALLOWABLE SOIL BEARING PRESSURE:
BASED ON SOIL BORINGS AND RECOMMENDATIONS MADE BY
-----, DATED -----, THE FOUNDATIONS HAVE BEEN DESIGNED FOR A SOIL BEARING CAPACITY OF ----- POUNDS PER SQUARE FOOT.
CONTRACTOR TO OBTAIN A DATA, SIGNED AND SILED COPY OF THE GEOTECHNICAL REPORT CONTRACTOR SHALL BE RESPONSIBLE FOR THE SOIL PREPARATION IN ACCORDANCE WITH THE RECOMMENDATION OF THE GEOTECHNICAL ENGINEER.
3. CONCRETE (SHOP DRAWINGS REQUIRED):
CAST-IN-PLACE CONCRETE TO ATTAIN THE FOLLOWING MINIMUM 28-DAY COMPRESSIVE STRENGTH:
- 3000 P.S.I. IN 28 DAYS FOOTINGS.
- 4000 P.S.I. IN 28 DAYS FOR ALL OTHER STRUCTURAL MEMBERS.
AGGREGATES TO BE CLEAN AND WELL GRADED, MAXIMUM SIZE 1" CONCRETE SLUMP: 3" MIN. TO 5" MAX. VERTICAL CONCRETE DROP NOT TO EXCEED 8'.
4. CONCRETE COVER:
TO BE AS FOLLOWS:

	BOTTOM	TOP	SIDES
FOOTINGS	3"	2"	3"
COLUMNS	1 1/2"	1 1/2"	1 1/2"
BEAMS	1 1/2"	1 1/2"	---
INTERIOR SLABS-ON-GRADE	1 1/2"	1 1/2"	---
EXTERIOR SLABS-ON-GRADE	1 1/2"	1 1/2"	---
5. REINFORCING STEEL (SHOP DRAWINGS REQUIRED):
TO BE NE HIGH STRENGTH BILLET STEEL, DEFORMED AS PER ASTM A-305, AND CONFORMING TO ASTM A-655, GRADE 60. LAP CONTINUOUS BARS 30-BAR DIAMETERS, UNLESS OTHERWISE NOTED IN PLANS.
HOOK DISCONTINUOUS ENDS OF ALL TOP BARS, REINFORCING STEEL TO BE DETAILED AND FABRICATED IN ACCORDANCE WITH MANUAL OF STANDARD PRACTICE OF DETAILING REINFORCING CONCRETE STRUCTURES, AND THE ACI BUILDING CODE 318, LATEST EDITION. SUBMIT SHOP DRAWINGS FOR APPROVAL BY ENGINEER BEFORE FABRICATION.
6. WELDING:
ALL WELDING TO BE DONE BY CERTIFIED WELDERS HOLDING CURRENT WELDING CERTIFICATES. ALL WELDING SHALL BE PERFORMED IN ACCORDANCE WITH A.W.S. 'STRUCTURAL WELDING CODE - STEEL' (ANSI/AWS D11), AN APPROVED TESTING LABORATORY, RETAINED BY THE OWNER, SHALL PERFORM VISUAL INSPECTION OF ALL FIELD FILLET WELDING AND ULTRASONIC INSPECTION OF ALL FIELD GROOVE WELDS. UPON COMPLETION OF SUCH INSPECTIONS THE LABORATORY SHALL ISSUE A WRITTEN SUMMARY LETTER, SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA STATING THAT ALL WELDS WITHIN AN INSPECTED AREA CONFORMED TO THE AWS REQUIREMENTS.
7. BUILDING CODES:
- THE FLORIDA BUILDING CODE 2023 (8th EDITION) AND ALL ITS REFERENCED STANDARDS ASSE 7-2022.
- ACI 318, LATEST EDITION.
- WIND LOAD CRITERIA AND CODE COMPLIANCE:
ALL STRUCTURAL ELEMENTS, EXPOSED TO WIND, HAVE BEEN DESIGNED PER THE GUIDELINES OF THE ASCE 7-22 CODE.
WIND SPEED: 175 MPH.
EXPOSURE = C
RISK CATEGORY = (I)
K1 = 0.85
INTERNAL PRESSURE COEFFICIENT +/- 0.18
8. SHOP DRAWINGS:
NO SHOP DRAWING SHALL BE SUBMITTED FOR ARCHITECT/ENGINEERS REVIEW UNTIL AFTER THEY HAVE BEEN REVIEWED AND NOTED FOR CONSTRUCTION METHOD, DIMENSIONING, AND OTHER TRACE REQUIREMENTS BY THE CONTRACTOR, AND STAMPED WITH THE CONTRACTORS APPROVAL SEAL. ENGINEER ASSUMES NO THE LATEST ASCE SPECIFICATIONS. ALL STEEL EXPOSED TO THE WEATHER SHALL BE GALVANIZED.
RESPONSIBILITY FOR DIMENSIONS, QUANTITIES, ERRORS OR OMISSIONS, AS A RESULT OF CHECKING AND REVIEWING ANY SHOP DRAWINGS, ANY ERRORS OR OMISSIONS MUST BE MADE GOOD BY CONTRACTOR, IRRESPECTIVE OF RECEIPT, CHECKING OR REVIEW OF DRAWINGS BY ENGINEER, AND EVEN THOUGH WORK IS DONE IN ACCORDANCE WITH SUCH SHOP DRAWINGS.
9. METAL STUDS:
SHALL CONFORM TO THE AMERICAN IRON AND STEEL INSTITUTE (LATEST EDITION) AS FOR FABRICATION, ERECTION AND DETAILING.
STEEL STUDS TO BE HOT DIPPED GALVANIZED, WITH G-90 GALVANIZING, AS PROVIDED BY DALE INC.
PROVIDE Fy = 33 KSI MINIMUM YIELD STRESS FOR 16, 20, 22, AND 25 GAUGES. PROVIDE Fy = 50 KSI MINIMUM YIELD STRESS FOR 16, 14, AND 12 GAUGES. SUBMIT MILL REPORTS AND SHOP DRAWINGS FOR APPROVAL BEFORE FABRICATION.
10. SLAB ON FILL:
FILL AND BACKFILL TO BE COMPACTED TO 95% OF ITS MODIFIED PROCTOR DENSITY (ASTM D-1557). THICKNESS OF COMPACTION LAYERS (LOOSE MATERIAL) SHALL NOT EXCEED 12". BACKFILL MATERIAL SHALL BE APPROVED BY GEOTECHNICAL ENGINEER. 3/16 INCH WIDE X (SLAB THICKNESS/4) DEEP SAW CUT OR TOOLED CONTRACTION JOINTS SHALL BE INSTALLED WITHIN 8 HOURS FOLLOWING PLACEMENT OF CONCRETE. JOINTS SHALL INTERSECT AT COLUMN LINES. LENGTH OF INDIVIDUAL SLAB PANELS CONTAINED WITHIN THE CONTRACTION JOINTS SHALL NOT EXCEED 30 X SLAB THICKNESS IN ANY DIRECTION. THE RATIO OF LONGER TO SHORTER DIMENSION SHALL NOT EXCEED 2. AT ALL FLOOR LOCATIONS WITH ARCHITECTURAL FINISHES (TILE, ETC.).
LOCATION OF CONTRACTION JOINTS SHALL FOLLOW THAT OF JOINTS IN FINISHES. CONTRACTOR TO SUBMIT A SHOP DRAWING INDICATING PATTERN OF CONTRACTION JOINTS FOR APPROVAL PRIOR TO CONSTRUCTION. PROVIDE 15-MIL, NON-POLYETHYLENE VAPOR BARRIER BELOW ALL SLABS ON FILL (LAP SHEETS MINIMUM OF 6" AND SEAL WITH MANUFACTURERS SPECIFIED TAPE, TAPE ALL PENETRATIONS). REFER TO ARCHITECTURAL DRAWINGS FOR REQUIRED CHEMICAL TREATMENT OF UNDERLYING SOILS. ALL SLAB REINFORCEMENT SHALL BE SUPPORTED ON CHAIRS OR CONCRETE SLAB FINISH TOLERANCES MEASURED AS PER ACI 117 (SECTION 4) SHALL NOT EXCEED 1/4"/10 FT. AREAS OF SLAB NOT MEETING THE REQUIRED TOLERANCES SHALL BE REMOVED AND REPLACED (BETWEEN NEAREST CONTRACTION OR CONSTRUCTION JOINTS) AT NO ADDITIONAL COST TO THE OWNER (PATCHING OR APPLICATION OF LEVELING MATERIALS SHALL NOT BE ALLOWED).
11. CONSTRUCTION JOINTS (SHOP DRAWINGS REQUIRED):
SLABS-ON-GRADE (IF APPLICABLE)
LOCATION OF A CONSTRUCTION JOINT SHALL COINCIDE WITH THAT OF A CONTRACTION JOINT. PROVIDE A CONTINUOUS 1 1/2" DEEP X 1/2" DEPTH OF SLAB CONTINUOUS SHEAR KEY AT MID-DEPTH. PROVIDE RECESS ALONG TOP OF JOINT SIMILAR TO CONTRACTION JOINT. UNLESS NOTED OTHERWISE ON PLAN, DISCONTINUE SLAB REINFORCEMENT THROUGH THE JOINT.
PROVIDE (THAT CONT. PARALLEL TO JOINT WITHIN NEW SLAB (DISCONTINUE AT TRANSVERSE CONTRACTION JOINTS).
BEAMS (NON POST-TENSIONED): PROVIDE BULKHEAD AT MID-SPAN WITH 3 1/2" DEEP X 2/3 DEPTH OF BEAM X 2/3 WIDTH OF BEAM POCKET, CENTERED ON BEAM'S CROSS-SECTION. LENGTH OF EXPOSED REINFORCEMENT SHALL BE SUFFICIENT TO PROVIDE A FULL TENSION LAP SPlice.
STRUCTURAL SLABS (NON POST-TENSIONED): PROVIDE BULKHEAD AT MID-SPAN WITH A CONTINUOUS 1 1/2" DEEP X 2/3 DEPTH OF SLAB SHEAR KEY AT MID-DEPTH. LENGTH OF EXPOSED REINFORCEMENT SHALL BE SUFFICIENT TO PROVIDE A FULL TENSION LAP SPlice.
COLUMNS: DISCONTINUE CONCRETE AT UNDERSIDE OF BEAM, SLAB, SHEAR-HEAD, ETC. AND CONTINUE ABOVE MONOLITHICALLY CAST BEAM, SLAB, SHEAR-HEAD, ETC. UPON CONTRACTORS PRIOR WRITTEN REQUEST AND ENGINEERS PRIOR WRITTEN APPROVAL. COLUMN-BEAM CONNECTIONS AT ISOLATED LOCATIONS MAY BE REINFORCED TO ALLOW FOR INSTALLATION OF COLUMN PRIOR TO PLACEMENT OF BEAM CONCRETE. THE CONTRACTOR SHALL BE BILLED ON AN HOURLY RATE BASIS FOR THE RESULTING ADDITIONAL DESIGN/DRAFTING SERVICES PROVIDED BY THE ENGINEER. COST OF SUCH SERVICES AS WELL AS THAT OF ADDITIONAL REINFORCEMENT SHALL BE COVERED SOLELY BY THE CONTRACTOR.
PRIOR TO COMMENCEMENT OF WORK, CONTRACTOR SHALL SUBMIT A SHOP DRAWING INDICATING LOCATION OF ALL INTENDED BULKHEADS TO THE ENGINEER FOR APPROVAL. THE ENGINEER RESERVES A RIGHT TO REFUSE ALLOWING A COLD JOINT AT A SPECIFIC LOCATION.
- A

12. MASONRY:
A. ALL CONCRETE BLOCK TO BE GRADE N-2, CONFORMING TO ASTM C-90, WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 1,900 PSI, AND A PRISM STRENGTH OF 1,500 PSI (MINIMUM). MORTAR SHALL BE TYPE M, WITH A MINIMUM STRENGTH OF 2,500 PSI (USE PORTLAND TYPE CEMENT). TEST MASONRY UNITS AND MORTAR BY TESTING LAB AT A RATE OF EVERY 1,000 SQUARE FEET OF WALLS ERECTED.
B. MASONRY WALLS SHALL BE REINFORCED HORIZONTALLY WITH 8 GAUGE DEFORMED GALVANIZED STEEL, SPACED AT 16" C/C VERT. EXTEND HORIZONTAL REINFORCING 4" INTO ADJACENT COLUMNS. PROVIDE TRUSS TYPE FOR NON-REINFORCED MASONRY AND LADDER TYPE FOR REINFORCED MASONRY.
C. FOR VERTICAL REINFORCEMENT, SEE SCHEDULE AND LAP 48 BAR DIAMETERS MINIMUM. PROVIDE FULL BED OF MORTAR FOR REINFORCED MASONRY.
D. AT ALL INTERIOR NON-BEARING BLOCK WALLS, MUST PROVIDE A TIE-BEAM 8X12 WITH FOUR #5 CONT. AND #3 TIES AT 48" C/C.
E. PROVIDE CLEANOUTS WHEN GROUTING BLOCK CELLS, AND CLEAN OUT BLOCK CELLS OF ALL MORTAR DROPPINGS. MAXIMUM VERTICAL DROP FOR GROUTING IS 4'-0".
F. AT THE END OF EACH DAY COVER ALL MASONRY WALLS.
13. WELDED WIRE FABRIC:
SHALL CONFORM TO ASTM A 165 AND BE FREE OF SCALE, RUST OR OIL. SPlice SHEETS ONE-AND-A-HALF SPACE LENGTHS (MINIMUM).
14. SHORING AND RE-SHORING:
CONTRACTOR WILL PROVIDE SHORING AND RE-SHORING DRAWINGS, ALONGS WITH RE-SHORING SCHEDULE FOR ALL ELEVATED STRUCTURAL COMPONENTS, INCLUDING BUT NOT LIMITED TO BEAMS, SLABS AND PRECAST JOIST. DRAWINGS TO BE SIGNED AND SEALED BY A FLORIDA REGISTERED PROFESSIONAL STRUCTURAL ENGINEER FOR SUBMITTAL TO THE ARCHITECT AND STRUCTURAL ENGINEER AND THE THRESHOLD INSPECTOR. NO ELEVATED WORK MAY PROCEED WITHOUT THIS SUBMITTAL.
CONTRACTOR TO ALSO RETAIN SAME ENGINEER TO PROVIDE FIELD INSPECTIONS, ALONG WITH REPORTS, THAT ALL OF THE SHORING AND RE-SHORING IS COMPLETED AS PER HIS DRAWINGS AND SPECIFICATIONS, FOR EACH LEVEL OF WORK.
SHORING SCHEDULE TO PROVIDE TIME THAT SHORES MAY BE REMOVED IN ORDER TO REMOVE FORMWORK, AND SPECIFIC SEQUENCE OF THE REMOVAL OF THE SHORING AS IT PROGRESSES THRU THE HEIGHT OF THE BUILDING, WITH SUFFICIENT DETAIL AS TO THE NUMBER OF DAYS TO COMPLETE REMOVAL. ALSO, IF WOOD AND METAL SHORES ARE MIXED, THE SHORING PLANS TO NOT SPECIFY PERCENTAGES, BUT RATHER EXACT NUMBERS OF WOOD POST SHORES, METAL POST SHORES, OR METAL FRAMES. SHORING DRAWINGS TO ALSO CONTAIN DETAIL OF CONNECTION OF HEADERS AT THE TOP OF THE SHORES. NOTE: IF THESE SHORING AND RE-SHORING CONFLICT WITH ANY DIRECTIVES CONTAINED IN THE JOB SPECIFICATION, THEN THE GOVERNING GUIDELINES WILL BE THESE.
15. EXPANSION ANCHORS:
THE CONTRACTOR SHALL USE ONE OF THE FOLLOWING: POWER-STUD BY 'POWERS/RAWL', KWIK BOLT II BY 'HILTI', TRUBOLT WEDGE BY 'RAMSET/REDHEAD' OR AN ENGINEER APPROVED SUBSTITUTE. INSTALLATION OF ANCHORS SHALL FOLLOW MANUFACTURERS SPECIFICATIONS.
16. EPOXY ANCHORS:
THE CONTRACTOR SHALL USE A TWO-COMPONENT EPOXY BASED PRODUCT SUPPLIED AS A SIDE-BY-SIDE CARTRIDGE SYSTEM AND DISPENSED THROUGH A STATIC MIXING NOZZLE. THE CONTRACTOR MAY USE ONE OF THE FOLLOWING: POWER-FAST BY 'POWERS/RAWL', HIT RE 500 BY 'HILTI', EPOCH BY 'RAMSET/REDHEAD', ALLIED GOLD A-1000 BY 'ALLIED FASTENER AND TOOL', ET BY 'SIMPSON' OR AN ENGINEER APPROVED SUBSTITUTE. INSTALLATION OF DOWELS SHALL FOLLOW MANUFACTURERS SPECIFICATIONS. ALL HOLES SHOULD BE BRUSHED WITH A NYLON BRUSH AND BLOWN WITH COMPRESSED AIR. ALL HOLES MUST BE OBSERVED BY THE PROJECT INSPECTOR PRIOR TO INSTALLATION OF EPOXY.
17. SCREW ANCHORS:
THE CONTRACTOR SHALL USE ONE OF THE FOLLOWING: TAPPER BY 'POWERS/RAWL', KWIK-CON BY 'HILTI', TARGON BY 'RAMSET/REDHEAD' OR AN ENGINEER APPROVED SUBSTITUTE. INSTALLATION OF ANCHORS SHALL FOLLOW MANUFACTURERS SPECIFICATIONS.
18. GENERAL:
IF ANY ERRORS OR OMISSIONS APPEAR IN THE DRAWINGS, SPECIFICATIONS, OR OTHER DOCUMENTS, THE CONTRACTOR MUST NOTIFY THE ARCHITECT AND ENGINEER, IN WRITING, OF THE SAME PRIOR TO PROCEEDING WITH THE WORK. IN QUESTION. IN THE EVENT THAT THE CONTRACTOR FAILS TO GIVE NOTICE, OR PROVIDE SUFFICIENT TIME FOR A RESPONSE, CONTRACTOR IS RESPONSIBLE FOR THE RESULTS OF SUCH ERRORS OR OMISSIONS, AND FOR ALL COSTS FOR RECTIFYING SAME AND FOR ANY DELAYS OR ANY OTHER COSTS INCURRED BY SAME. THE CONTRACTOR SHALL USE THE STRUCTURAL DRAWINGS TOGETHER WITH THE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS TO LOCATE DEPRESSIONS, SLABS, SLOPES, DRAINS, OUTLETS, RECESSES, OPENINGS, REGISTS, BOLT SETTINGS, SLEEVES, DIMENSIONS, ETC.. POTENTIAL CONFLICTS SHALL BE TRANSMITTED TO THE ARCHITECT AND ENGINEER BEFORE PROCEEDING WITH THE WORK. CONTRACTOR TO PROVIDE ADEQUATE TIME FOR RESPONSE FROM ARCHITECT/ENGINEER.
19. WOOD MEMBERS:
A) - WOOD:
1- NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION, NATIONAL FOREST PRODUCTS ASSOCIATIONS WITH THE EDITION 2014 FBC ALL STRUCTURAL WOOD MEMBERS (UNLESS OTHERWISE NOTED ON PLANS) SHALL BE SOUTHERN PINE NO. 1 WITH THE MINIMUM 1000 PSI ALLOWABLE BENDING STRESS TREATED, VISUALLY GRADE WITH STAMP FROM AN ACCREDITED AGENCY AND SHY MAX. MOISTURE CONTENT 19% SURFACED DRY OR SHYPS MAX. MOISTURE CONTENT 19% SURFACED DRY OR SURFACED GREEN.
2- ANY VISUALLY TWISTED OR BOWED LUMBER, IN ANY DIRECTIONS, SHALL NOT BE ACCEPTED. NO STRUCTURAL MEMBER SHALL BE CUT NOTCHED OR OTHERWISE REDUCED IN SIZE OR STRENGTH WITHOUT PRIOR APPROVAL IN WRITING FROM THE STRUCTURAL ENGINEER.



LINTEL SCHEDULE					
WIDTH	HEIGHT	CLEAR SPAN IN FEET	REINFORCING		REMARKS
			TOP	BOTTOM	
7' 5/8"	7' 5/8"	UP TO 7'-0"	2 #5	2 #5	#2 TIES @ 12" c/c
7' 5/8"	12"	7'-0" TO 9'-0"	2 #5	2 #5	#2 TIES @ 12" c/c
7' 5/8"	12"	9'-0" TO 12'-0"	2 #5	2 #5	#2 TIES @ 12" c/c

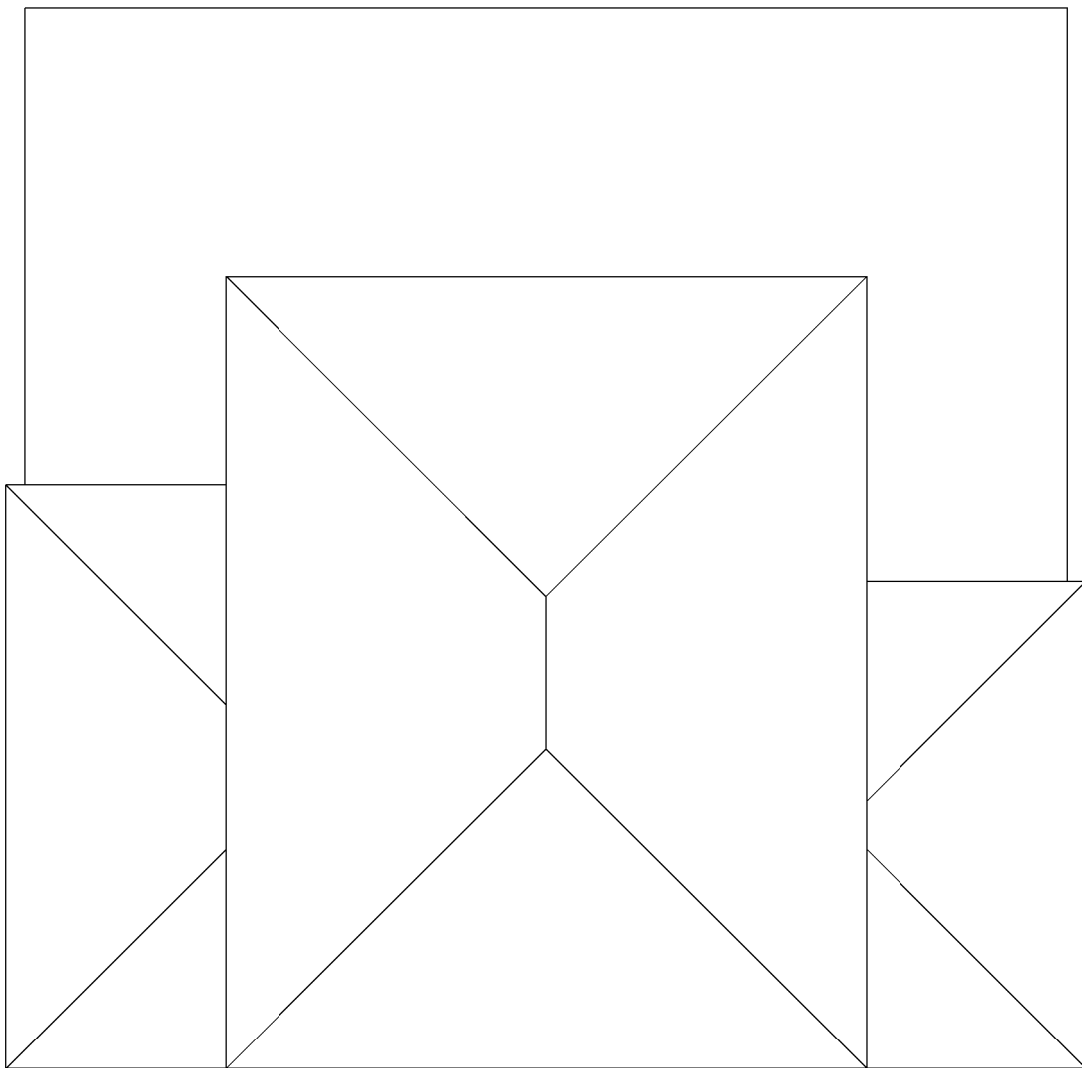
COLUMN BAR NOTES:	
1- ALL COLUMNS SHALL HAVE #3 TIES, EXCEPT FOR COLUMNS WITH #1 VERTICAL REINFORCING WHICH SHALL HAVE #4 TIES. (TYPICAL UNLESS OTHERWISE NOTED.)	
2- TIE SPACING TO BE AS FOLLOWS (U.O.N. IN SCHEDULE).	
VERTICAL BARS:	SPACING:
#5.....	10" in.
#6.....	12" in.
#7.....	14" in.
#8.....	16" in.
#9.....	18" in.
#10.....	18" in.
#11.....	20" in.
IN NO CASE SHALL THE TIE SPACING EXCEED THE LEAST DIMENSION OF THE COLUMN	
TENSION LAP SPlice SCHEDULE FOR CONG. VERTICAL REINF.	
VERTICAL BAR SIZE	SPlice LENGTH
#4	1'-10"
#5	2'-4"
#6	2'-0"
#7	4'-1"
#8	4'-6"
#9	5'-3"
#10	5'-11"
#11	6'-6"

WIND LOAD PRESSURE FOR PANELS, WINDOWS, DOORS AND STOREFRONTS (POUNDS PER SQUARE FOOT) (ALLOWABLE STRESS DESIGN LOADS)					
AREA	ZONE 4		ZONE 5		
	POSITIVE	NEGATIVE	POSITIVE	NEGATIVE	
10 ft. to 25 ft.					
26 ft. to 50 ft.					
51 ft. to 100 ft.					
101 ft. to 250 ft.					
251 ft. to 500 ft.					
501 ft. AND UP					
NOTES: 1- ZONE 5 IS DEFINED AS ANY DOOR OR WINDOW WITHIN (' + ') FROM ANY CORNER OF THE BUILDING. ALL OTHER LOCATIONS CAN BE DEFINED AS ZONE 4.					

NOTE: FOR DESIGN, THE ABOVE WIND LOAD PRESSURES ARE TO BE USED WITH THE LOAD COMBINATIONS OF THE ASCE 7-16 BUILDING CODE. Kz = 0.85.

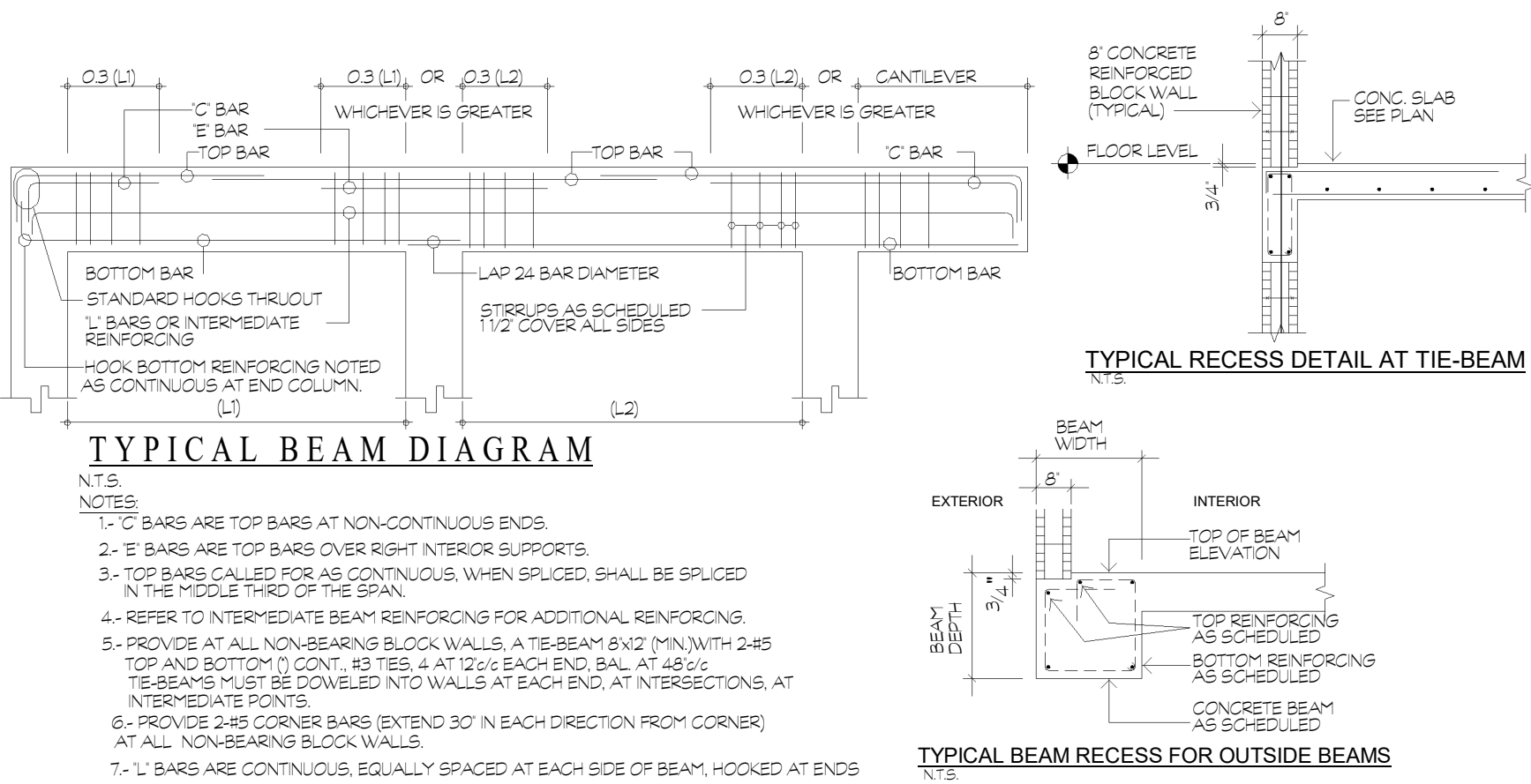
WIND LOAD PRESSURES FOR ROOFING DESIGN (ALLOWABLE STRESS DESIGN LOADS)	
ZONE	PRESSURE
1	
2	
3	

NET WIND LOAD PRESSURES FOR TRUSS DESIGN (ALLOWABLE STRESS DESIGN LOADS)		
ZONE	PRESSURE	
	JACK TRUSSES	ALL OTHER TRUSSES
1		
2		
3		



UPLIFT DIAGRAM

PREFAB WOOD TRUSSES & GIRDERS CONNECTORS SCHEDULE					
CONNECTION TYPE	CONNECTOR BY "SIMPSON" UNLESS OTHERWISE NOTED	CONNECTOR ALLOWABLE LOAD VALUES			FLORIDA APPROVAL
		UPLIFT	LATERAL LOAD PARALLEL TO WALL	LATERAL LOAD PERPENDICULAR TO WALL	
A					
B					
C					



TIE BEAM SCHEDULE							
MARK	TOP OF BEAM ELEV.	SIZE (IN.)	REINFORCEMENT				REMARKS
			B	T	C	E	
RTB-1	+10'-0"	8x12					
HRTB-1	+15'-0"	8x16					
HRTB-2	+15'-1"	8x12					

BLOCK WALL SCHEDULE			
MARK	THICKNESS	REINFORCEMENT	
		VERTICAL	HORIZONTAL
BW-1	8"	#5 @ 16" c/c	NO. 9 GAGE DUR-O-WALL LADDER TYPE @ 16"
		FILL REINFORCED CELLS WITH CONCRETE.	

CONCRETE COLUMN SCHEDULE			
MARK	SIZE (IN.)	REINFORCEMENT	
		VERTICAL	TIES
1	8x24		

WALL FOOTING SCHEDULE			
MARK	SIZE (IN.)	REINFORCEMENT	
WF-24	24x12	3-#5 BOTTOM CONTINUOUS #5@12" c/c BOTTOM TRANSV.	
WF-42	24x14	4-#5 BOTTOM CONTINUOUS #5@12" c/c BOTTOM TRANSV.	

PROJECT LEAD & CIVIL ENGINEERING:

301 EAST ATLANTIC BLVD.
POMPANO BEACH, FL 33060
PH: (954) 788-3400

Florida Engineering Business License: CA7928
Florida Surveyor and Mapper Business License: L86860
Florida Landscape Architecture Business License: LC20090457

LANDSCAPE ARCHITECT:

801 INTERNATIONAL PARKWAY
LAKE MARY, FL 32746
407.871.9807
MKSKSTUDIOS.COM

ARCHITECT:

WANNEMACHER, JENSEN ARCHITECTS
815 NORTHWEST 57 AVE. SUITE 219
MIAMI, FLORIDA 33126
727-492-6568
WJARO.COM A40002277

MECHANICAL, PLUMBING, & ELECTRICAL ENGINEERING:

5301 WATERFORD DISTRICT DR.
SUITE 750
954-421-1944
SGMENGINEERING.COM CA-00006208

REVISIONS		
NO.	DESCRIPTION	DATE

**PRELIMINARY PLAN
NOT FOR CONSTRUCTION**
THESE PLANS ARE NOT FULLY PERMITTED
AND ARE SUBJECT TO REVISIONS MADE
DURING THE PERMITTING PROCESS.
THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE USE OF THESE
PLANS PRIOR TO OBTAINING PERMITS FROM
ALL AGENCIES HAVING JURISDICTION OVER
THE PROJECT WILL FALL SOLELY UPON THE
USER.

ISSUE DATE:

DESIGNED BY:

DRAWN BY:

CHECKED BY:

ENGINEER OF RECORD: ADA M. ALABASA P.E. No 45130	
CLIENT	
VILLAGE OF PINECREST PINECREST PARKS & RECREATION	
PROJECT	
GARY MATZNER PARK	
SHEET TITLE	
GENERAL STRUCTURAL NOTES AND SCHEDULES	
SHEET NUMBER	
S-300	
KEITH PROJECT NUMBER	
13861.00	

4930 S.W. 74TH Court
Miami, Florida, 33155
Phone: (305)666-0711
Fax: (305)666-5259

30% CONSTRUCTION DOCUMENTS (08-07-2024)

Plotted by: dleigo On: ---

Drawing name: C:\2347 Gary Matzner Park\06 PRODUCTION\01 BIM\CAL LIN\SG24\0822 - Client Title Block.dwg

STATUS: PRELIMINARY