G E N E R A L   S T R U C T U R A L   N O T E S

FOUNDATION:

1. SOIL STATEMENT: VISUAL INSPECTION REVEALS SAND AND GRAVEL WITH AN ESTIMATED ALLOWABLE MINIMUM BEARING CAPACITY OF 2,000 P.S.F. IF OTHER CONDITIONS OR MATERIALS BE ENCOUNTERED, THE ENGINEER SHALL BE NOTIFIED IN WRITING BEFORE PROCEEDING WITH THE WORK. GEOTECHNICAL ENGINEER WILL PROVIDE A SIGNED AND SEAL SOIL COMPACTION CERTIFICATION LETTER TO THE BUILDING OFFICIAL AFTER THE COMPACTION HAS BEEN FINALIZED TO ATTEST THE SOIL CONDITIONS AND THE ESTIMATED SOIL BEARING CAPACITY.
2. ALL FOOTING EXCAVATION AND COMPACTION SHALL BE INSPECTED AND APPROVED BY THE GEOTECHNICAL ENGINEER FOR THE BEARING CAPACITY INDICATED ABOVE PRIOR TO CONCRETE PLACEMENT.
3. FOOTINGS SHALL BE FOUNDED ON WELL COMPACTED VIRGIN SOIL OR ON ENGINEERED FILL AT THE ELEVATIONS SHOWN.
4. KEEP FOUNDATION EXCAVATIONS FREE OF WATER AT ALL TIMES. REPLACE SOFT OR WEAKENED SOIL WITH STRUCTURAL FILL.
5. ELEVATIONS SHOWN AT FOOTINGS ARE TOP OF FOOTING ELEVATION (T.O.F.).
6. ALL EXTERIOR FOOTINGS TO BEAR MINIMUM 1'-4" BELOW ADJACENT GRADE. UNLESS OTHERWISE NOTED. ADJUST BOTTOM OF FOOTING AS REQUIRED.
7. FOR CONTINUOUS WALL FOOTINGS, SPLICES IN REINFORCING BARS SHALL NOT BE LESS THAN (48) BAR DIAMETERS AND REINFORCEMENT SHALL BE CONTINUOUS AROUND ALL CORNERS AND CHANGES IN DIRECTION. CONTINUITY SHALL BE PROVIDED AT CORNERS OR CHANGES IN DIRECTION BY BENDING THE LONGITUDINAL STEEL AROUND THE CORNER (48) BAR DIAMETERS OR BY ADDING MATCHING REINFORCING STEEL, WHICH SHALL EXTEND (48) BAR DIAMETERS FROM EACH CORNER OR CHANGE IN DIRECTION. WHEN THREE OR MORE BARS ARE REQUIRED, THE BARS SHALL BE HELD IN PLACE AND ALIGNMENT BY TRANSVERSE BARS SPACED NOT MORE THAN (4) FEET APART.

SOIL COMPACTION:

1. COMPACTED SOIL IS TO PROVIDE A SATISFACTORY SUBGRADE FOR THE LOAD BEARING CONCRETE SLAB ON GRADE. BACKFILL MATERIAL SHALL BE GRAVELLY SOILS, WELL-GRADED SANDS, AND SAND-GRAVEL MIXTURES RELATIVELY FREE OF PLASTIC FINES.
2. FILL AND BACKFILL SHALL BE COMPACTED TO A MINIMUM OF 98% AASHO T-180C DENSITY AND SHALL CONFORM TO ABOVE REFERENCED GEOTECHNICAL ENGINEER'S REQUIREMENTS.
3. MINIMUM SUBGRADE MODULUS, k, SHALL ACHIEVE 200 LB/IN² PER IN (OR pci) AFTER COMPACTION.

WOOD MEMBERS:

1. ALL STRUCTURAL WOOD MEMBERS TO BE SAWN LUMBER. 'DIMENSIONS LUMBER TO BE "SOUTHERN PINE" WITH A MINIMUM COMMERCIAL GRADE OF 'NO. 2'. DESIGN BENDING STRESS OF FB= 1200 PSI
2. ALL WOOD MEMBERS TO BE FREE OF ALL IMPERFECTIONS AS. SPLITS, CHECKS, OR EXCESSIVE KNOTS. UNSATISFACTORY MATERIALS TO BE REPLACE AT NO COST TO THE OWNER.
3. ALL MEMBERS IN CONTACT WITH CONCRETE OR EXPOSED TO THE ELEMENTS, TO BE PRESSURE TREATED.

CONCRETE CONSTRUCTION NOTES:

1. CAST-IN-PLACE CONCRETE SHALL ATTAIN THE FOLLOWING MINIMUM 28-DAY COMPRESSIVE STRENGTHS (f'c):

   A) FOOTINGS AND GRADE BEAMS__________3000 PSI
   B) SLAB-ON-GRADE__________3000 PSI
   C) COLUMNS AND ELEVATED BEAMS__________3000 PSI
   D) WALLS__________3000 PSI
   E) CONCRETE SLAB ON METAL DECK__________3000 PSI
   F) ALL OTHER CONCRETE__________3000 PSI

2. CONCRETE DENSITY SHALL BE NORMAL WEIGHT UNLESS SPECIFICALLY OTHERWISE NOTED.
3. MINIMUM CONCRETE COVER FOR REINFORCING STEEL SHALL BE AS INDICATED. IN NO CASE SHALL REINFORCEMENT COVER BE LESS THAN THE REQUIREMENTS OF ACI 301.

	BOTTOM	TOP	SIDES
A) FOOTING	3"	2"	2"
B) BEAMS	1-1/2"	1-1/2"	1-1/2"
C) COLUMNS	—	—	1/2"
D) SLABS	1"	3/4"	1"
E) WALLS	—	—	1 1/2"

4. CONCRETE REINFORCING STEEL MARKED STANDARD HOOK SHALL HAVE A 90 DEGREE HOOK WITH A MINIMUM EXTENSION BEYOND THE BEND OF 12 BAR DIAMETERS IN LENGTH, UNLESS OTHERWISE NOTED. STIRRUP, TIE AND 180-DEGREE HOOKS SHALL CONFORM TO THE REQUIREMENTS OF ACI 318.
5. PROVIDE ½" THICK JOINT FILLER MATERIAL WHERE SLABS ON GRADE ABUT VERTICAL SURFACES.
6. REINFORCING STEEL SHALL BE SPREAD AT SLEEVES, TIEBACKS, RECESSES AND OTHER EMBEDDED ITEMS UNLESS OTHERWISE INDICATED. REINFORCEMENT SHALL NOT BE CUT TO FACILITATE PLACEMENT OF EMBEDDED ITEMS.
7. ALL EXPOSED CONCRETE EDGES SHALL BE CHAMFERED ¾" OR AS INDICATED.
8. ALUMINUM SHALL NOT BE PLACED IN DIRECT CONTACT WITH CONCRETE UNLESS EFFECTIVELY COATED OR COVERED TO PREVENT ALUMINUM-CONCRETE REACTION AND ELECTROLYTIC ACTION BETWEEN ALUMINUM AND STEEL.
9. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ESTIMATING AND SUPPLYING ANY ADDITIONAL CONCRETE REQUIRED TO ACHIEVE SPECIFIED FLOOR FLATNESS CRITERIA.
10. A MIN. OF 5 CONCRETE SPECIMENS SHALL BE TAKEN FROM EACH 50 CU. YD. OR PORTION THEREOF SPECIMENS SHALL BE TESTED ACCORDING TO A.S.T.M. C-39, ONE AT 3, ONE AT 7, AND 3 AT 28 DAYS.

WELDING:

1. ALL WELDING SHOULD BE PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS SET FORTH BY A.W.S. BY CERTIFIED WELDERS.
2. CONTRACTORS TO USE E-70 SERIES LOW HYDROGEN ELECTRODES.

REINFORCING STEEL:

1. CONCRETE REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A615/A615M, GRADE 60.
2. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185. PROVIDE SHEET-TYPE WELDED WIRE FABRIC. SHEET LAPS SHALL BE TIED AND LAPPED ONE FULL MESH SPACING.
3. CONCRETE REINFORCING STEEL SHALL BE CONTINUOUS UNLESS OTHERWISE INDICATED. CONTINUOUS REINFORCING STEEL SHALL BE LAPPED IN ACCORDANCE WITH THE REQUIREMENTS OF ACI 318.
4. FABRICATION AND DETAILING ACCORDING TO A.C.I.-315.
5. ALL ACCESSORIES TO HAVE UPTURNED LEGS AND BE PLASTIC DIPPED AFTER FABRICATION.

CONCRETE SLAB ON GRADE:

1. ISOLATION JOINTS MUST BE USED AT JUNCTIONS WITH WALLS AND COLUMNS, USE 1/2" THICK PREMOLDED JOINTS FULL DEPTH OF SLAB.
2. PLACE CONTROL JOINTS AT CENTERLINE OF COLUMNS. PROVIDE INTERMEDIATE JOINTS IF COLUMN SPACING IS GREATER THAN 20 FT. IN ANY CASE JOINT SPACING SHALL NOT BE GREATER THAN 15 FT. IN SIDE WALKS TOOLED JOINTS SHALL BE SPACED AS THE WIDTH OF THE SLAB.
3. CONTROL JOINT DEPTH SHALL BE 1" TOOLED FOR SLAB THICKNESS 4" TO 5". JOINTS SHALL BE SAWED AT 15' INTERVALS WITHIN 24 HOURS AFTER CONCRETING.
4. CONSTRUCTION JOINT MUST BE PLACED IN THE SLAB WHERE BUILDING EXPANSION JOINTS ARE SHOWN.
5. WHEN CONRETING AND OPERATING ARE CONCLUDED FOR THE DAY, CONSTRUCTION JOINTS SHALL BE FORMED WITH BURKE KEYED KOLD METAL JOINT FORM OR APPROVED EQUAL.
6. WATERPROOF MEMBRANES, OVERLAPPED 6" AT JOINTS, WITH A PERMEANCE OF LESS THAN 0.3% PERMS IN ACCORDANCE WITH A.S.T.M. E-98 SHALL BE PROVIDED UNDER INTERIOR SLAB. SHERE NO VAPOR BARRIER IS USED, THE SUBGRADE MUST BE DAMPENED WITH WATER IN ADVANCE OF CONCRETING TO FREE WATER STANDING ON THE SUBGRADE NOR ANY MUDDY OR SOFT SPOT IS PERMITTED.
7. ANY STRUCTURAL MEMBER PENETRATING SLAB ON FILL IS TO BE ½" PRE-MOLDED JOINT FILLER COMPLYING WITH ASTM D-1752, TYPE I.
8. PREMATURE FINISHING IS NOT ALLOWED. IMMEDIATE FOLLOWING FLOATING TROWELING WITH STEEL TROWELS SHOULD BE COMMENCED IF REQUIRED. BROOMING SHALL BE AFTER THE STEEL TROWELING OPERATION.
9. SLAB FINISH TYPE: INSIDE BUILDING - STEEL TROWELED; OUTSIDE SLAB - BROOMED, UNLESS OTHERWISE NOTED.

CONCRETE CYLINDER AND SLUMPS TESTS

1. **AT LEAST ONE SET OF CYLINDERS SHALL BE PROVIDED FOR STRENGTH AND SLUMP TESTS PER POUR OR FOR EACH 50 CUBIC YARDS OF CONCRETE, WHICHEVER IS LESS. AT LEAST TWO SETS OF TESTS ARE RECOMMENDED FOR COLUMN POURS. FOR EACH POUR THE ENGINEER SHALL BE PROVIDED WITH ONE (1) 3-DAY TEST, ONE (1) 7-DAY TEST, ONE (1) 28-DAY TEST, ONE (1) SPARE, AND ONE (1) SLUMP TEST.**

DESIGN CRITERIA:

1. STRUCTURAL DESIGN IS IN ACCORDANCE WITH THE FOLLOWING CODES AND SPECIFICATIONS:
   A). THE FLORIDA BUILDING CODE (FBC), 2020,7TH EDITION, SECTIONS OF "HIGH VELOCITY HURRICANE ZONE".
   B). ACI 318-14, BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE.
   C). ASCE 7-16, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES.
   D). TMS 402-2016, BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES.
2. DESIGN LOAD CRITERIA:

   LIVE LOADS:
   NON-ACCESSIBLE ROOF__________30 PSF

   DEAD LOADS:
   ROOF__________25 PSF

   WIND LOAD__________(PER ASCE 7-16):
   ULTIMATE DESIGN WIND SPEED: Vult = 175 MPH
   NOMINAL DESIGN WIND SPEED: Vasd = 135 MPH
   IMPORTANCE FACTOR: I=1.00
   EXPOSURE: C
   INTERNAL PRESSURE COEFFICIENTS: GCpi +/- 0.18

STRUCTURAL STEEL:

1. **STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST AISC STEEL CONSTRUCTION MANUAL AND SHALL CONFORM TO LATEST ASTM A36 SPECIFICATIONS. BOLTS TO CONFORM TO ASTM A325. ANCHOR BOLTS SHALL CONFORM TO ASTM A307. ALL WELDING SHALL BE PERFORMED BY CERTIFIED WELDERS (AMERICAN WELDING SOCIETY).**
2. **ALL CONNECTIONS SHALL DEVELOP THE STRENGTH OF THE PART BEING CONNECTED. SHOP CONNECTIONS SHALL BE WELDED. FIELD CONNECTIONS SHALL BE BOLTED UNLESS OTHERWISE NOTED ON THE PLAN.**
3. **PROVIDE ONE SHOP COAT OF RUST-INHIBITIVE PAINT (MINIMUM 3 MIL DRY FILM). TOUCH UP ALL ABRASIONS AFTER ERECTION. APPLY FIELD FINISH PAINT COATS AS SPECIFIED, FOR INTERIOR STEEL COLUMNS, AND MISC. FABRICATIONS.**
4. **SUBMIT SHOP DRAWINGS FOR APPROVAL PRIOR TO FABRICATION OR ERECTION OF STRUCTURAL STEEL.**
5. **ALL STRUCTURAL STEEL, MISCELLANEOUS PLATES, EMBED ITEMS, BOLTS, NUTS, WASHERS, ANGLES, BRACKETS, NAILS, SCREWS AND CONNECTORS USED IN NON-CONDITIONED SPACES AND EXTERIOR AREAS EXPOSED TO THE WEATHER SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION NO EXCEPTIONS WILL BE PERMITTED (GALVANIZED SPRAY NOT DEEMED EQUIVALENT).**

4. **THE STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE FOLLOWING:**

   A. W HOT-ROLLED SHAPES..........ASTM A992, Fy = 50 Ksi
   B. SQUARE AND RECTANGULAR HSS..........ASTM A500, GRADE B, Fy = 46 Ksi
   C. PLATES, ANGLES..........ASTM A36/ A 36M
   D. HOT-ROLLED CARBON STEEL SHEETS AND STRIPS..........ASTM A 245
   E. HIGH STRENGTH BOLTS..........ASTM A325
   F. HOOKED ANCHOR RODS..........ASTM F1554, GRADE 36
   G. HEADED ANCHOR RODS..........ASTM F1554, GRADE 36
   H. THREADED ANCHOR RODS..........ASTM F1554, GRADE 36
   I. WELDING ELECTRODES..........AWS D1.1/D1.1M, E70XX
   J. WELDED WIRE MESH..........ASTM A185

5. **FIELD CUTTING OF STRUCTURAL STEEL MEMBERS BY ANY TRADE SHALL NOT BE PERMITTED. BOLT HOLES SHALL NOT BE CUT OR ENLARGED BY FLAME CUTTING IN THE FIELD.**
6. **ALL STEEL TO HAVE A SHOP COAT OF RUST INHIBITIVE PAINT.**
7. **DELETE PAINT ON ALL STEEL TO RECEIVE SPRAYED ON FIREPROOFING OR CONCRETE ENCASEMENT.**

ASTM INTERNATIONAL.

1. STANDARD SPECIFICATION FOR GENERAL REQUIREMENTS FOR ROLLED STEEL PLATES, SHAPES, SHEET PILING AND BARS FOR STRUCTURAL USE, ASTM A6.
2. STANDARD SPECIFICATIONS FOR HIGH-STRENGTH BOLTS FOR STRUCTURAL STEEL JOINTS, ASTM A325.
3. STANDARD SPECIFICATION FOR HEAT-TREATED STEEL STRUCTURAL BOLTS, ALLOY STEEL, HEAT TREATED 150 KSI MINIMUM TENSILE STRENGTH, ASTM A490.
4. STANDARD SPECIFICATION FOR SHEET STEEL, CARBON, METALLIC, AND NONMETALLIC COATED FOR COLDFORMED STEEL FRAMING MEMBERS, ASTM A1003- A1003M.

MASONRY WALLS:

1. DESIGN MASONRY ASSEMBLAGE STRENGTH, F'M = 1500 PSI. NET AREA COMPRESSIVE STRENGTH OF CONCRETE MASONRY UNITS SHALL BE A MINIMUM OF 1900 PSI.
2. CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90 AND BE MANUFACTURED WITH LIGHTWEIGHT AGGREGATE, PLACE IN RUNNING BONDS.
3. MORTAR SHALL CONFORM TO ASTM C 270 TYPE "M" (2500 PSI). GROUT FOR FILLED CELL SHALL CONFORM TO ASTM C476 AND SHALL NOT CONTAIN ADMIXTURES. GROUT SHALL ATTAIN A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 3000 PSI, SLUMP 9 IN.
4. REINFORCEMENT SHALL BE DEFORMED BARS CONFORMING TO ASTM A615 / A615M, GRADE 60 AND SHALL HAVE FABRICATION TOLERANCES IN ACCORDANCE WITH ACI 315. SHOP-FABRICATE REINFORCING BARS WHICH ARE INDICATED TO BE BENT OR HOOKED.
5. PROVIDE 9 GAGE HORIZONTAL REINFORCEMENT AT 16" SPACING LADDER TYPE FOR NON-LOAD BEARING WALL, TRUSS TYPE FOR LOAD BEARING WALL.
6. ALUMINUM CONDUITS, PIPES, AND ACCESSORIES SHALL NOT BE EMBEDDED IN MASONRY GROUT, OR MORTAR, UNLESS EFFECTIVELY COATED OR COVERED TO PREVENT ALUMINUM-CEMENT CHEMICAL REACTION OR ELECTROLYTIC REACTION BETWEEN ALUMINUM AND STEEL.
7. PROVIDE VERTICAL REBARS ACCORDING TO PLAN OR WALL SCHEDULE. SEE "TYPICAL WALL END AND CORNER DETAILS" FOR SPECIAL REINFORCING DETAIL.
8. VERTICAL REINFORCEMENT MUST HAVE A MINIMUM 48 BAR DIAMETER LAP SPLICE. UNLESS OTHERWISE NOTED, CENTER WALL REINFORCEMENT IN BLOCK CELLS. USE NONMETALLIC BAR POSITIONERS.
9. PROVIDE DOWEL REINFORCEMENT IN FOUNDATION OF SAME SIZE AND SPACING AS VERTICAL WALL REINFORCEMENT.
10. BRACE TOP OF INTERIOR, NON-LOAD BEARING MASONRY WALLS TERMINATING AT THE UNDERSIDE OF FLOOR OR ROOF STRUCTURE AGAINST OUT-OF-PLANE MOVEMENT IN ACCORDANCE WITH THE "TYPICAL NON-BEARING CMU WALL BRACING" DETAIL.
11. PROVIDE CONCRETE LINTEL AND WINDOW SILL ACCORDING TO SCHEDULES IN TYPICAL OPENING DETAIL.
12. BEARING MASONRY SHOWN IN PLAN SHALL BE ERECTED PRIOR TO THE STRUCTURE ABOVE HAS BEEN POURED.
13. NON-BEARING MASONRY SHOWN IN PLAN SHALL BE ERECTED AFTER THE STRUCTURE ABOVE HAS BEEN POURED.
14. PROVIDE 9 GAGE LADDER TYPE HORIZONTAL REINFORCEMENT AT 16" O/C (FOR REINFORCED WALLS).
15. WHENEVER ANCHORS BOLTS ARE TO BE SET IN MASONRY. TWO CELLS AT THE SETTING LOCATION SHALL BE FILLED WITH CONCRETE.

SAFETY OSHA AND LABOR LAWS

1. THE ARCHITECT OF RECORD DOES NOT POSSES NOR PRESUMES TO POSSES ANY KNOWLEDGE OR EXPERTISE IN MATTERS TO JOB SITE EMPLOYEE SAFETY, OSHA OR LABOR LAW REQUIREMENTS FOR A CONSTRUCTION PROJECT.SAFETY AND COMPLINCE WITH OSHA AND LABOR LAWS ARE THE ABSOLUTE RESPONSIBILITY OF THE GENERAL CONTRACTOR AND HIS CONSULTANTS TO ADDRESS THESE MATTERS. THE ARCHITECT OF RECORD SPECIALIZES IN STRUCTURAL DESIGN ONLY. THE BOARD OF PROFESSIONAL REGULATION FORBIDS HIM FROM ASSUMING RESPONSIBILITY OUTSIDE HIS AREA OF EXPERTISE.

SHORING, RESHORING AND TEMPORARY BRACING

1. THE GENERAL CONTRACTOR (GC) IS RESPONSIBLE FOR HIRING A FLORIDA PROFESSIONAL ENGINEER SPECIALIZED IN THE FIELD OF FORM WORK AND SHORING DESIGN.
2. SHORING AND RESHORING DESIGN MUST INCLUDE CONSIDERAION OF THE RATE AND METHOD OF PLACING CONCRETE.
3. GC SHALL SUBMIT SIGNED AND SEALED SHORING AND RESHORING DESIGN DRAWINGS FOR REVIEW AND APPROVAL. SHORING INSTALLATION MUST BE INSPECTED BY THE SHORING DESIGN ENGINEER PRIOR TO PLACEMENT OF CONCRETE.

HAMBRO STRUCTURAL SYSTEM

1. "HAMBRO" SYSTEM CONSIST OF COMPOSITE FLOOR SYSTEM (3 ½" CONC. SLAB W/16" STEEL JOIST AT 4'-0" O/C. MAX.
2. PROVIDE ONE SHOP COAT OF RUST INHIBITING PAINT (MIN. 3 MIL. DRY FILM THICKNESS) TOUCH UPS REQUIRED AFTER ERECTION.
3. CONTRACTOR TO COORDINATE LOCATION OF ALL EQUIPMENT, AND PROVIDE DOUBLE JOIST SUCH WHERE DEEMED NECESSARY.
4. SUBMIT SIGNED & SEALED SHOP DRAWINGS AND CALCULATIONS FOR APPROVAL INDICATING LOCATION, SIZES, SPACING FOR ALL ELEMENTS OF THE SYSTEM.

SHOP DRAWING SUBMITTALS:

SUBMIT ONE PRINT OF ALL SHOP DRAWINGS LISTED BELOW. IF SIGNED AND SEALED SHOP DRAWINGS ARE REQUIRED, THEN SUBMIT TWO ADDITIONAL SIGNED AND SEALED PRINTS FOR APPROVAL.

SHOP DRAWINGS:

a. ROOF TRUSSES
b. "HAMBRO" COMPOSITE FLOOR SYSTEM.
c. METAL STAIR AND RAILING.

STRUCTURAL DRAWINGS INDEX

S-1.0	GENERAL STRUCTURAL NOTES.
S-1.1	TYPICAL DETRAIL.
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S-4.0	ROOF FRAMING PLAN.
S-5.0	SECTIONS.
S-6.0	SECTIONS.

STRUCTURAL LEGEND

SYMBOL	DESCRIPTION
AFF	ABOVE FINISHED FLOOR
AHR	ANCHOR
APPROX	APPROXIMATELY
ARCH	ARCHITECTURAL
BOC	BOTTOM OF CONCRETE
BOS	BOTTOM OF STEEL
BRG	BEARING
CJ	SLAB CONSTRUCTION JOINT
℄	CENTER LINE
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONN	CONNECTION
CONC	CONCRETE
CONT	CONTINUOUS
DN	DOWN
EA	EACH
EF	EACH FACE
EJ	EXPANSION JOINT
ELEV	ELEVATION
EOS	EDGE OF SLAB
EQ	EQUAL
EXP	EXPOSED
FFE	FINISHED FLOOR ELEVATION
FOB	FACE OF BRICK
FTG	FOOTING
GA	GAGE
HORIZ	HORIZONTAL
HS	HIGH STRENGTH
JBE	JOIST BEARING ELEVATION
LG	LONG
LF	LINEAR FEET
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
MAX	MAXIMUM
MFR	MANUFACTURER
MIN	MINIMUM
NIC	NOT IN CONTRACT
NO.	NUMBER
OC	ON CENTER
OPP	OPPOSITE
PJF	PREMOLDED JOINT FILLER
ℙ	PLATE
PLF	POUNDS PER LINEAR FOOT
PSF	POUNDS PER SQUARE FOOT
REINF	REINFORCEMENT
SCHED	SCHEDULE
SF	SQUARE FEET
SIM	SIMILAR
SJ	SLAB CONTROL JOINT
STD	STANDARD
SS	SURFACE SPALL
T&B	TOP AND BOTTOM
TOC	TOP OF CONCRETE
TOF	TOP OF FOOTING
TOM	TOP OF MASONRY
TOS	TOP OF STEEL
TE	THICKENED SLAB
TYP	TYPICAL
UON	UNLESS OTHERWISE NOTED
VERT	VERTICAL
WP	WORKING POINT
WWF	WELDED WIRE FABRIC
DN (arrow)	SLOPE DIRECTION
/ / / / / /	DEPRESSED SLAB
FSx (arrow)	CONCRETE SLAB TYPE (SEE SCHEDULE)
[+X'-X"]	TO FINISHED FIRST FLOOR INDICATES ELEVATION REFERENCED
1 (box)	KEYED CONSTRUCTION NOTE
1 (circle)	COLUMN REFERENCE LINE (CENTER LINE OF COLUMN)
[+X'-X"]	SPOT ELEVATION
(hatch)	STOOP SLAB DELAMINATED AREAS
(cross-hatch)	STOOP SLAB SPALLED AREAS

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FL. P.E. No 90741

△	DATE	REVISION-RECORD

DRAWING CONTENT: GENERAL NOTE

TO THE BEST OF THE ARCHITECT'S OR ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES AND THE APPLICABLE FIRE-SAFETY STANDARDS AS DETERMINED BY THE LOCAL AUTHORITY.

PROJECT: 2-Story Addition.
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