STRUCTURAL CALCULATIONS FOR

FIDANZA'S FAMILY HOUSE

11041 SW 77th Ave.,
Miami, FL 33156

Submitted to:

Village of Pinecrest

Building Department

08-25-2023

Seal:

Alaym Aguilar

Digitally signed by Alaym Aguilar
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INDEX

Subject	Page No.
Design Loads (ASCE 7-16)	1
Wind Loads Design (ASCE 7-16)	2
Concrete Slabs Design (ACI 318-14)	12
Concrete Beams Design (ACI 318-14)	438
Columns Design (ACI 318-14/AISC)	527
Masonry Walls Design (TMS 402/602-16)	559
Footings Design (ACI 318-14)	567

ENGINEER OF RECORD ASSUMES RESPONSIBILITY FOR BOTH MANUAL AND COMPUTER-GENERATED CALCULATIONS INCLUDED IN THIS BOOK (PAGES 1 THRU 603).

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DESIGN LOADS

(ASCE 7-16):

	SDL (psf)	**LL** (psf)
Floors	25	40
Roof	25	30
Balcony	25	60
Terrace	25	100

Wind Loads: (ASCE 7-16):

Velocity = 175 mph
Exposure = C
Building Risk Category = II
Importance Factor = 1.0
Internal Pressure Coeff. = +/- 0.18
Directionality Factor, Kd = 0.85

Note: This structure has been designed as "enclosed" and requires impact resistant glass or shutters to comply with Florida Building Code sections R4403.16 & R4410.

Ph: (305) 710-2987 Email: 3aengdrafting@gmail.com

WIND LOADS DESIGN

(ASCE 7-16)

Ph: (305) 710-2987 Email: 3aengdrafting@gmail.com

MecaWind v2403

Software Developer: Meca Enterprises Inc., www.meca.biz, Copyright © 2020

Calculations Prepared by:
Date: Mar 18, 2022

Location: 11041 SW 77th Ave, Miami, FL 33156

Basic Wind Parameters

Wind Load Standard	= ASCE 7-16	Exposure Category	= C
Wind Design Speed	= 175.0 mph	Risk Category	= II
Structure Type	= Building	Building Type	= Enclosed

General Wind Settings

Incl_LF	= Include ASD Load Factor of 0.6 in Pressures	= True
DynType	= Dynamic Type of Structure	= Rigid
Zg	= Altitude (Ground Elevation) above Sea Level	= 0.000 ft
Bdist	= Base Elevation of Structure	= 0.000 ft
SDB	= Simple Diaphragm Building	= True
MWFRSType	= MWFRS Method Selected	= Ch 27 Pt 2

Topographic Factor per Fig 26.8-1

Topo	= Topographic Feature	= None
Kzt	= Topographic Factor	= 1.000

Building Inputs

RoofType:	Building Roof Type	= Flat	RfHt :	Roof Height	= 28.000 ft
W :	Building Width	= 112.000 ft	L :	Building Length	= 177.000 ft
Par :	Is there a Parapet	= True	PHt :	Height of Parapet	= 1.200 ft
Por :	Parapet Porosity	= Solid	P_GCPi :	Parapet Int Press	= +/-0.00

Exposure Constants per Table 26.11-1:

Alpha:	Table 26.11-1 Const	= 9.500	Zg:	Table 26.11-1 Const	= 900.000 ft
At:	Table 26.11-1 Const	= 0.105	Bt:	Table 26.11-1 Const	= 1.000
Am:	Table 26.11-1 Const	= 0.154	Bm:	Table 26.11-1 Const	= 0.650
C:	Table 26.11-1 Const	= 0.200	Eps:	Table 26.11-1 Const	= 0.200

Overhang Inputs:

Std	= Overhangs on all sides are the same	= True
OHType	= Type of Roof Wall Intersections	= Overhang
OH	= Overhang of Roof Beyond Wall	= 3.000 ft

Gust Factor Calculation:

Gust Factor Category I Rigid Structures - Simplified Method

G1	= For Rigid Structures (Nat. Freq.>1 Hz) use 0.85	= 0.85

Gust Factor Category II Rigid Structures - Complete Analysis

Zm	= Max(0.6 * Ht, Zmin)	= 16.800 ft
Izm	= Cc * (33 / Zm) ^ 0.167	= 0.224
Lzm	= L * (Zm / 33) ^ Eps	= 436.847
Q	= (1 / (1 + 0.63 * ((B + Ht) / Lzm)^0.63))^0.5	= 0.875
G2	= 0.925*((1+0.7*Izm*3.4*Q)/(1+0.7*3.4*Izm))	= 0.860

Gust Factor Used in Analysis

G	= Lessor Of G1 Or G2	= 0.850

Main Wind Force Resisting System (MWFRS) Calculations per Ch 27 Part 2:

h	= Mean Roof Height above grade	= 28.000 ft
Kh	= 15 ft [4.572 m]< Z <Zg -->(2.01*(Z/zg)^(2/Alpha) {Table 26.10-1}	= 0.968
Kzt	= Topographic Factor is 1 since no Topographic feature specified	= 1.000
Kd	= Wind Directionality Factor per Table 26.6-1	= 0.85
GCPi	= Ref Table 26.13-1 for Enclosed Building	= +/-0.18
Theta	= Slope of Roof	= 0.0 Deg
LF	= Load Factor based upon ASD Design	= 0.60

Wind Pressures per Ch 27 Part 2 - Normal to Ridge

All wind pressures include a load factor of 0.6

L	= Building Length Parallel To the Direction Of Wind	= 112.000 ft
B	= Building Width Normal To the Wind Direction	= 177.000 ft
L/B	= Ratio Of L / B	= 0.633
LHD	= Least Horizontal Dimension: Min(B, L)	= 112.000 ft
a1	= Min(0.1 * LHD, 0.4 * h)	= 11.200 ft
a	= Max(a1, 0.04 * LHD, 3 ft [0.9 m])	= 11.200 ft
0.5*h	= Parameter used to define zone width: 0.5*h	= 14.000 ft
Ph	= Net pressure at the top Of the wall per Table 27.5-1	= 43.66 psf

```
Po        = Net pressure at the base Of the wall per Table 27.5-1          = 41.50 psf
Ps        = Side Wall Press: Table 27.5-1: 0.2<= L/B <= 1, use 0.54*Ph     = -23.58 psf
Pl        = Leeward Wall Press: Table 27.5-1 : 0.2<= L/B <= 1, use 0.38*Ph  = -16.59 psf
Pwh       = Windward Wall Pressure at the Top  per Table 27.5-1: Ph + Pl   = 27.07 psf
Pwo       = Windward Wall Pressure at the Base per Table 27.5-1: Po + Pl   = 24.91 psf

Povh3     = Overhang Bottom with Roof Slope < 10 Deg, Load Case 1: 0.75*Pz3 = 30.79 psf
Povh3     = Overhang Bottom with Roof Slope < 10 Deg, Load Case 2: 0.75*Pz3 = 0.00 psf
```

Pressure on Parapet per Sec 27.5.2 - Normal to Ridge:

```
Pressure accounts for Windward & Leeward on both Windward & Leeward Building Surfaces
hp        = Height to top of parapet                                       = 29.200 ft
Pp_table  = Pressure from Table 27.6-1 at hp And L/B = 1                    = 73.53 psf
Pp        = Net Pressure on Parapet:  2.25 * Pp_table * LF                  = 99.27 psf

Lambda    = Exposure Adjustment Factor for Roofs per Table 27.5-2           = 1.000
```

Roof Pressures per Table 27.5-2 - Normal to Ridge

Zone Pressures Normal to Ridge

Zone	Load Case 1 psf	Load Case 2 psf
1	0.00	0.00
2	0.00	0.00
3	-41.05	0.00
4	-36.58	0.00
5	-30.02	0.00

A pressure of zero '0' indicates that the load case is not applicable for that zone.

Wind Pressures per Ch 27 Part 2 - Parallel to Ridge

All wind pressures include a load factor of 0.6

```
L         = Building Length Parallel To the Direction Of Wind              = 177.000 ft
B         = Building Width Normal To the Wind Direction                    = 112.000 ft
L/B       = Ratio Of L / B                                                 = 1.580
LHD       = Least Horizontal Dimension:  Min(B, L)                         = 112.000 ft
a1        = Min(0.1 * LHD,  0.4 * h)                                       = 11.200 ft
a         = Max(a1, 0.04 * LHD, 3 ft [0.9 m])                              = 11.200 ft
0.5*h     = Parameter used to define zone width:  0.5*h                    = 14.000 ft

Ph        = Net pressure at the top Of the wall per Table 27.5-1           = 40.49 psf
Po        = Net pressure at the base Of the wall per Table 27.5-1          = 38.26 psf
Ps        = Side Wall Press: Table 27.5-1: 1 < L/B < 2, interpolate        = -24.22 psf
Pl        = Leeward Wall Press: Table 27.5-1 : 1 < L/B < 2, interpolate    = -12.80 psf
Pwh       = Windward Wall Pressure at the Top  per Table 27.5-1: Ph + Pl   = 27.69 psf
Pwo       = Windward Wall Pressure at the Base per Table 27.5-1: Po + Pl   = 25.46 psf

Povh3     = Overhang Bottom with Roof Slope < 10 Deg, Load Case 1: 0.75*Pz3 = 30.79 psf
Povh3     = Overhang Bottom with Roof Slope < 10 Deg, Load Case 2: 0.75*Pz3 = 0.00 psf
```

Pressure on Parapet per Sec 27.5.2 - Parallel to Ridge:

```
Pressure accounts for Windward & Leeward on both Windward & Leeward Building Surfaces
hp        = Height to top of parapet                                       = 29.200 ft
Pp_table  = Pressure from Table 27.6-1 at hp And L/B = 1                    = 73.53 psf
Pp        = Net Pressure on Parapet:  2.25 * Pp_table * LF                  = 99.27 psf

Lambda    = Exposure Adjustment Factor for Roofs per Table 27.5-2           = 1.000
```

Roof Pressures per Table 27.5-2 - Parallel to Ridge

Zone Pressures Parallel to Ridge

Zone	Load Case 1 psf	Load Case 2 psf
1	0.00	0.00
2	0.00	0.00
3	-41.05	0.00
4	-36.58	0.00
5	-30.02	0.00

A pressure of zero '0' indicates that the load case is not applicable for that zone.

Components and Cladding (C&C) Zone Summary per Ch 30 Pt 1:

h/W	= Ratio of mean roof height to building width	= 0.250
h/L	= Ratio of mean roof height to building length	= 0.158
h	= Mean Roof Height above grade	= 28.000 ft
Kh	= 15 ft [4.572 m]< Z <Zg -->(2.01*(Z/zg)^(2/Alpha) {Table 26.10-1}	= 0.968
Kzt	= Topographic Factor is 1 since no Topographic feature specified	= 1.000
Kd	= Wind Directionality Factor per Table 26.6-1	= 0.85
GCPi	= Ref Table 26.13-1 for Enclosed Building	= +/-0.18
LF	= Load Factor based upon ASD Design	= 0.60
qh	= (0.00256 * Kh * Kzt * Kd * Ke * V^2) * LF	= 38.71 psf
LHD	= Least Horizontal Dimension: Min(B, L)	= 112.000 ft
a1	= Min(0.1 * LHD, 0.4 * h)	= 11.200 ft
a	= Max(a1, 0.04 * LHD, 3 ft [0.9 m])	= 11.200 ft
h/B	= Ratio of mean roof height to least hor dim: h / B	= 0.250
0.2*h	= Parameter used to define Zone 3	= 5.600 ft
0.6*h	= Parameter used to define Zones 1 and 2	= 16.800 ft

Wind Pressure Summary for C&C Zones based Upon Areas Ch 30 Pt 1 (Table 1 of 2)
All wind pressures include a load factor of 0.6

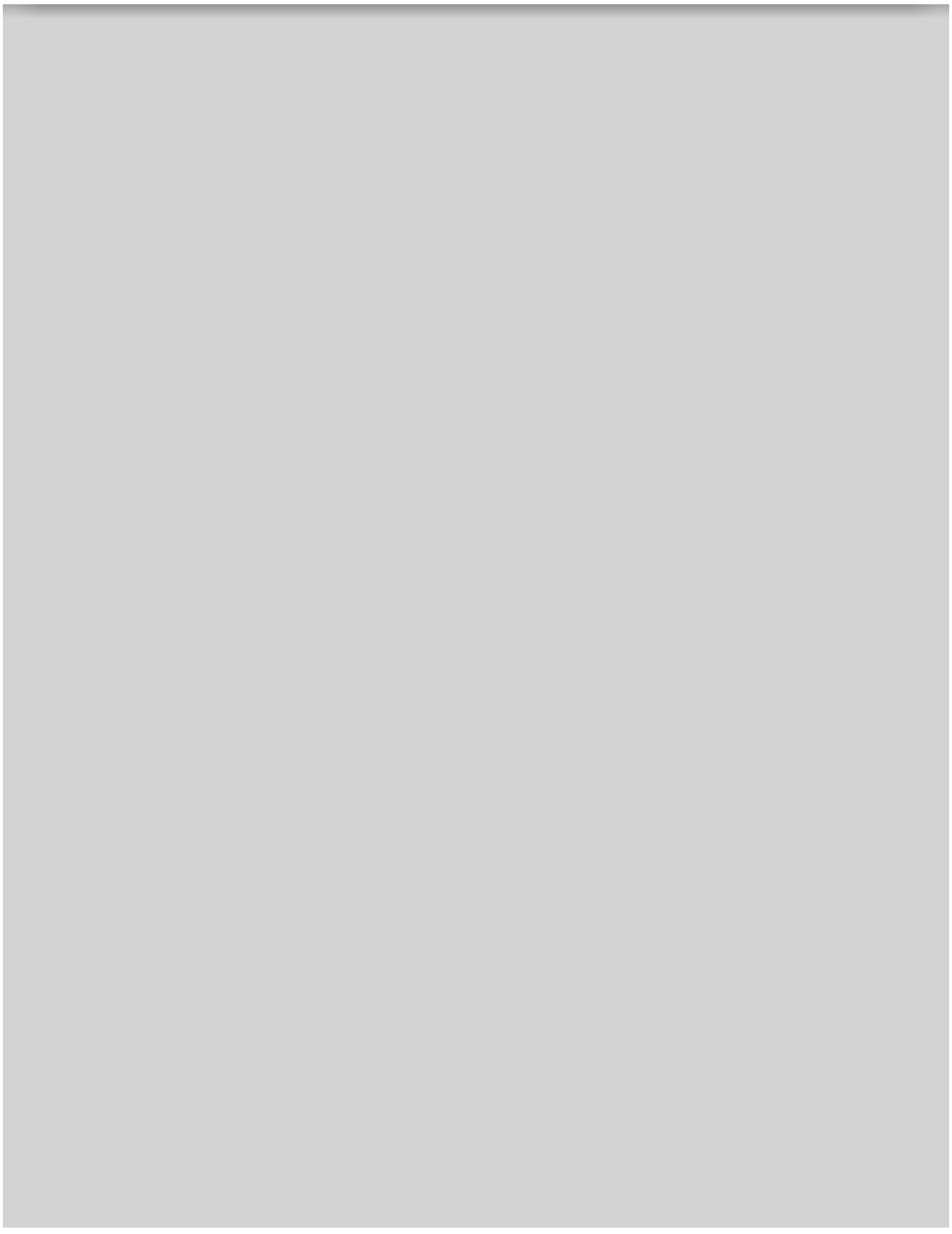
Zone	Figure	A <= 10.00 sq ft psf	A = 20.00 sq ft psf	A = 50.00 sq ft psf	A = 100.00 sq ft psf
1	30.3-2A	18.58 -72.77	17.41 -67.97	15.87 -61.62	14.71 -56.82
1'	30.3-2A	18.58 -41.80	17.41 -41.80	15.87 -41.80	14.71 -41.80
1_OH	30.3-2A	9.60 -65.80	9.60 -64.64	9.60 -63.10	9.60 -61.93
1'_OH	30.3-2A	9.60 -65.80	9.60 -64.64	9.60 -63.10	9.60 -61.93
2	30.3-2A	18.58 -96.00	17.41 -89.82	15.87 -81.66	14.71 -75.49
2_OH	30.3-2A	9.60 -89.03	9.60 -80.80	9.60 -69.92	9.60 -61.69
3	30.3-2A	18.58 -130.83	17.41 -118.49	15.87 -102.17	14.71 -89.82
3_OH	30.3-2A	9.60 -123.87	9.60 -109.46	9.60 -90.42	9.60 -76.02
4	30.3-1	41.80 -45.29	39.95 -43.44	37.51 -40.99	35.65 -39.14
4_P (P1/P4)	30.3-2A	35.15 -38.66	33.28 -36.79	30.81 -34.32	28.94 -32.45
4_P (P3/P2)	30.3-2A	35.15 -89.82	33.28 -83.59	30.81 -75.36	28.94 -69.13
4_P (P1-P2)/(P3-P4)	30.3-2A	124.96 73.81	116.87 70.07	106.17 65.13	98.07 61.40
5	30.3-1	41.80 -55.74	39.95 -52.04	37.51 -47.14	35.65 -43.44
5_P (P1/P4)	30.3-2A	35.15 -49.20	33.28 -45.47	30.81 -40.53	28.94 -36.79
5_P (P3/P2)	30.3-2A	35.15 -124.96	33.28 -112.51	30.81 -96.05	28.94 -83.59
5_P (P1-P2)/(P3-P4)	30.3-2A	160.11 84.35	145.79 78.75	126.85 71.34	112.53 65.73

Wind Pressure Summary for C&C Zones based Upon Areas Ch 30 Pt 1 (Table 2 of 2)
All wind pressures include a load factor of 0.6

Zone	Figure	A = 200.00 sq ft psf	A = 500.00 sq ft psf	A > 1000.00 sq ft psf
1	30.3-2A	14.71 -52.02	14.71 -45.68	14.71 -45.68
1'	30.3-2A	14.71 -35.98	14.71 -28.28	14.71 -22.45
1_OH	30.3-2A	9.60 -51.93	9.60 -38.71	9.60 -38.71
1'_OH	30.3-2A	9.60 -51.93	9.60 -38.71	9.60 -38.71
2	30.3-2A	14.71 -69.32	14.71 -61.16	14.71 -61.16
2_OH	30.3-2A	9.60 -53.46	9.60 -42.58	9.60 -42.58
3	30.3-2A	14.71 -77.48	14.71 -61.16	14.71 -61.16
3_OH	30.3-2A	9.60 -61.62	9.60 -42.58	9.60 -42.58
4	30.3-1	33.80 -37.29	31.35 -34.84	31.35 -34.84
4_P (P1/P4)	30.3-2A	27.07 -30.59	24.60 -28.12	24.60 -28.12
4_P (P3/P2)	30.3-2A	27.07 -62.90	24.60 -54.67	24.60 -54.67
4_P (P1-P2)/(P3-P4)	30.3-2A	89.98 57.66	79.27 52.72	79.27 52.72
5	30.3-1	33.80 -39.73	31.35 -34.84	31.35 -34.84
5_P (P1/P4)	30.3-2A	27.07 -33.06	24.60 -28.12	24.60 -28.12
5_P (P3/P2)	30.3-2A	27.07 -71.14	24.60 -54.67	24.60 -54.67
5_P (P1-P2)/(P3-P4)	30.3-2A	98.21 60.13	79.27 52.72	79.27 52.72

* A is effective wind area for C&C: Span Length * Effective Width
* Effective width need not be less than 1/3 of the span length
* Maximum and minimum values of pressure shown.
* + Pressures acting toward surface, - Pressures acting away from surface
* _OH represents an Overhang in the zone specified
* Overhang pressures calculated per Para 30.9
* Per Para 30.2.2 the Minimum Pressure for C&C is 9.60 psf [0.460 kPa] {Includes LF}

_P represents a zone on the parapet
Parapet C&C Surface (P1/P4) - Maximum Positive and Negative Pressures on Exterior Parapet Surfaces
Parapet C&C Surface (P3/P2) - Maximum Positive and Negative Pressures on Interior Parapet Surfaces
Parapet Net Pressure (P1-P2) - Net Pressure Acting on Upwind Parapet (Exterior + Interior)
Parapet Net Pressure (P3-P4) - Net Pressure Acting on Downwind Parapet (Exterior + Interior)
* Interpolation can be used for values of A that are between those values shown.



MecaWind v2403

Software Developer: Meca Enterprises Inc., www.meca.biz, Copyright © 2020

Calculations Prepared by:
Date: Oct 07, 2022

Location: 11041 SW 77th Ave., Miami, FL 33156

Description:
Gym

File Location:
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Wind Pressures 11041 SW 77th Ave (Gym).wnd

Basic Wind Parameters

Wind Load Standard	= ASCE 7-16	Exposure Category	= C
Wind Design Speed	= 175.0 mph	Risk Category	= II
Structure Type	= Building	Building Type	= Enclosed

General Wind Settings

Incl_LF	= Include ASD Load Factor of 0.6 in Pressures	= True
DynType	= Dynamic Type of Structure	= Rigid
Zg	= Altitude (Ground Elevation) above Sea Level	= 0.000 ft
Bdist	= Base Elevation of Structure	= 0.000 ft
SDB	= Simple Diaphragm Building	= True
MWFRSType	= MWFRS Method Selected	= Ch 27 Pt 2

Topographic Factor per Fig 26.8-1

Topo	= Topographic Feature	= None
Kzt	= Topographic Factor	= 1.000

Building Inputs

RoofType:	Building Roof Type	= Flat	RfHt :	Roof Height	= 15.000 ft
W :	Building Width	= 19.500 ft	L :	Building Length	= 29.000 ft
Par :	Is there a Parapet	= False			

Exposure Constants per Table 26.11-1:

Alpha:	Table 26.11-1 Const	= 9.500	Zg:	Table 26.11-1 Const	= 900.000 ft
At:	Table 26.11-1 Const	= 0.105	Bt:	Table 26.11-1 Const	= 1.000
Am:	Table 26.11-1 Const	= 0.154	Bm:	Table 26.11-1 Const	= 0.650
C:	Table 26.11-1 Const	= 0.200	Eps:	Table 26.11-1 Const	= 0.200

Overhang Inputs:

Std	= Overhangs on all sides are the same	= True
OHType	= Type of Roof Wall Intersections	= Overhang
OH	= Overhang of Roof Beyond Wall	= 3.670 ft

Gust Factor Calculation:

Gust Factor Category I Rigid Structures - Simplified Method

G1	= For Rigid Structures (Nat. Freq.>1 Hz) use 0.85	= 0.85

Gust Factor Category II Rigid Structures - Complete Analysis

Zm	= Max(0.6 * Ht, Zmin)	= 15.000 ft
Izm	= Cc * (33 / Zm) ^ 0.167	= 0.228
Lzm	= L * (Zm / 33) ^ Eps	= 427.057
Q	= (1 / (1 + 0.63 * ((B + Ht) / Lzm)^0.63))^0.5	= 0.941
G2	= 0.925*((1+0.7*Izm*3.4*Q)/(1+0.7*3.4*Izm))	= 0.894

Gust Factor Used in Analysis

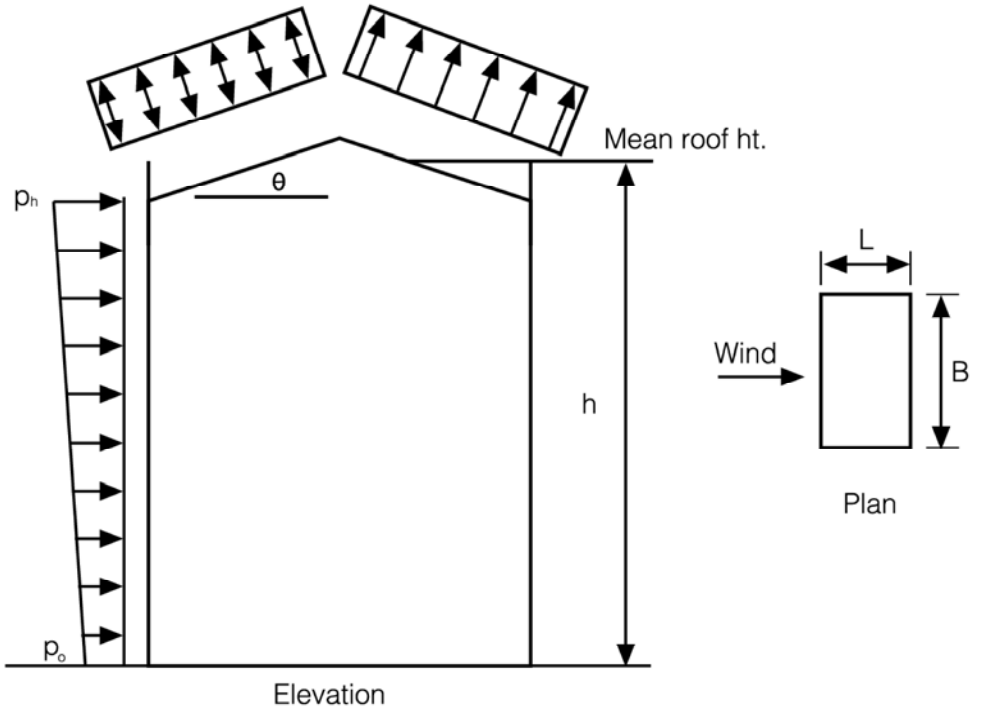
G	= Lessor Of G1 Or G2	= 0.850

Main Wind Force Resisting System (MWFRS) Calculations per Ch 27 Part 2:

h	= Mean Roof Height above grade	= 15.000 ft
Kh	= 15 ft [4.572 m]< Z <Zg -->(2.01*(Z/zg)^(2/Alpha) {Table 26.10-1}	= 0.849
Kzt	= Topographic Factor is 1 since no Topographic feature specified	= 1.000
Kd	= Wind Directionality Factor per Table 26.6-1	= 0.85
GCPi	= Ref Table 26.13-1 for Enclosed Building	= +/-0.18
Theta	= Slope of Roof	= 0.0 Deg
LF	= Load Factor based upon ASD Design	= 0.60

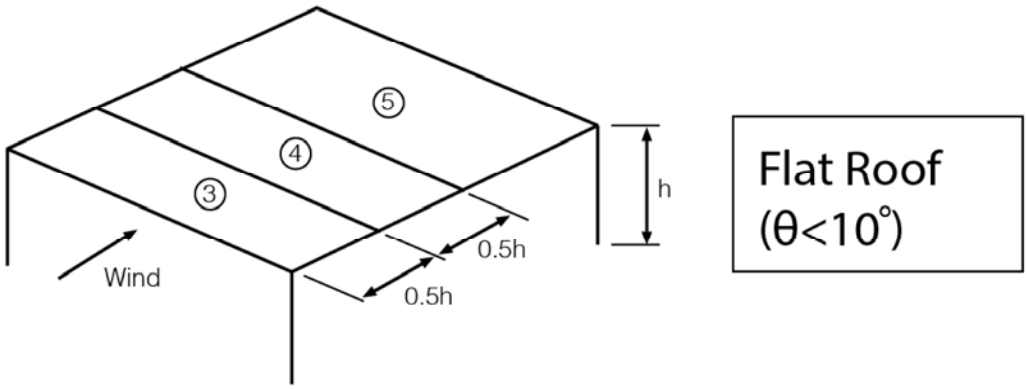
Wind Pressures per Ch 27 Part 2 - Normal to Ridge

All wind pressures include a load factor of 0.6



L	= Building Length Parallel To the Direction Of Wind	= 19.500 ft
B	= Building Width Normal To the Wind Direction	= 29.000 ft
L/B	= Ratio Of L / B	= 0.672
LHD	= Least Horizontal Dimension: Min(B, L)	= 19.500 ft
a1	= Min(0.1 * LHD, 0.4 * h)	= 1.950 ft
a	= Max(a1, 0.04 * LHD, 3 ft [0.9 m])	= 3.000 ft
0.5*h	= Parameter used to define zone width: 0.5*h	= 7.500 ft
Ph	= Net pressure at the top Of the wall per Table 27.5-1	= 38.70 psf
Po	= Net pressure at the base Of the wall per Table 27.5-1	= 38.70 psf
Ps	= Side Wall Press: Table 27.5-1: 0.2<= L/B <= 1, use 0.54*Ph	= -20.90 psf
Pl	= Leeward Wall Press: Table 27.5-1 : 0.2<= L/B <= 1, use 0.38*Ph	= -14.71 psf
Pwh	= Windward Wall Pressure at the Top per Table 27.5-1: Ph + Pl	= 23.99 psf
Pwo	= Windward Wall Pressure at the Base per Table 27.5-1: Po + Pl	= 23.99 psf
Povh3	= Overhang Bottom with Roof Slope < 10 Deg, Load Case 1: 0.75*Pz3	= 27.03 psf
Povh3	= Overhang Bottom with Roof Slope < 10 Deg, Load Case 2: 0.75*Pz3	= 0.00 psf
Lambda	= Exposure Adjustment Factor for Roofs per Table 27.5-2	= 1.000

Roof Pressures per Table 27.5-2 - Normal to Ridge



Zone Pressures Normal to Ridge

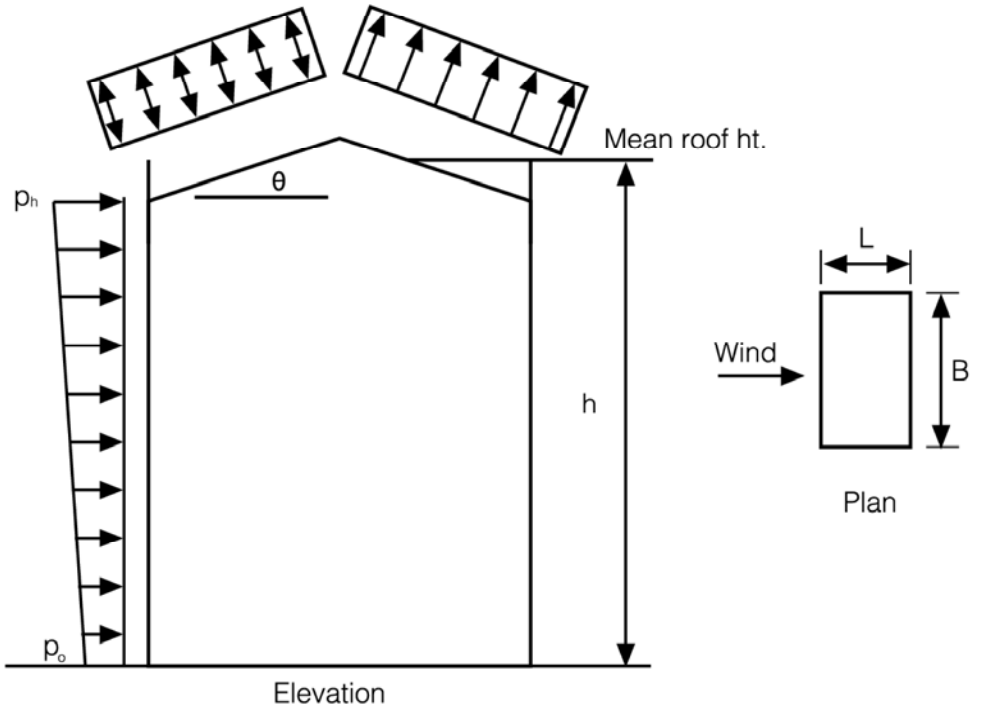
Zone	Load Case 1 psf	Load Case 2 psf

1	0.00	0.00
2	0.00	0.00
3	-36.05	0.00
4	-32.18	0.00
5	-26.37	0.00

A pressure of zero '0' indicates that the load case is not applicable for that zone.

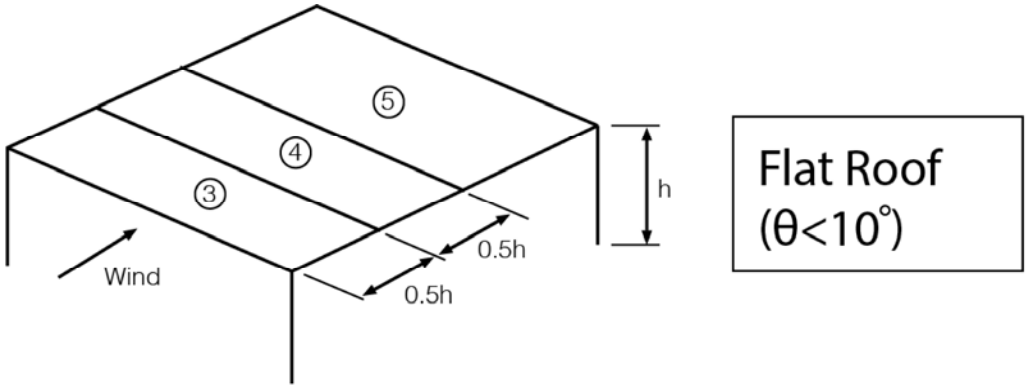
Wind Pressures per Ch 27 Part 2 - Parallel to Ridge

All wind pressures include a load factor of 0.6



L	= Building Length Parallel To the Direction Of Wind	= 29.000 ft
B	= Building Width Normal To the Wind Direction	= 19.500 ft
L/B	= Ratio Of L / B	= 1.487
LHD	= Least Horizontal Dimension: Min(B, L)	= 19.500 ft
a1	= Min(0.1 * LHD, 0.4 * h)	= 1.950 ft
a	= Max(a1, 0.04 * LHD, 3 ft [0.9 m])	= 3.000 ft
0.5*h	= Parameter used to define zone width: 0.5*h	= 7.500 ft
Ph	= Net pressure at the top Of the wall per Table 27.5-1	= 36.25 psf
Po	= Net pressure at the base Of the wall per Table 27.5-1	= 36.25 psf
Ps	= Side Wall Press: Table 27.5-1: 1 < L/B < 2, interpolate	= -21.34 psf
Pl	= Leeward Wall Press: Table 27.5-1 : 1 < L/B < 2, interpolate	= -11.83 psf
Pwh	= Windward Wall Pressure at the Top per Table 27.5-1: Ph + Pl	= 24.42 psf
Pwo	= Windward Wall Pressure at the Base per Table 27.5-1: Po + Pl	= 24.42 psf
Povh3	= Overhang Bottom with Roof Slope < 10 Deg, Load Case 1: 0.75*Pz3	= 27.03 psf
Povh3	= Overhang Bottom with Roof Slope < 10 Deg, Load Case 2: 0.75*Pz3	= 0.00 psf
Lambda	= Exposure Adjustment Factor for Roofs per Table 27.5-2	= 1.000

Roof Pressures per Table 27.5-2 - Parallel to Ridge

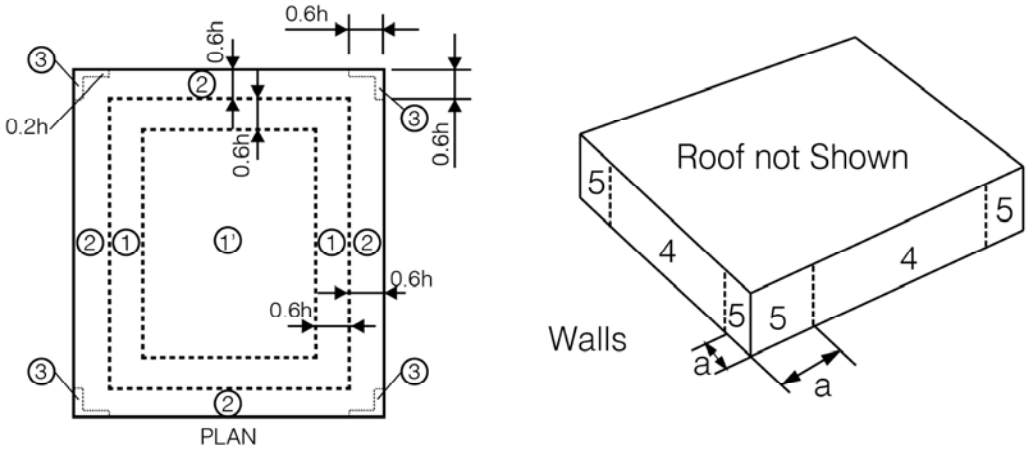


Zone Pressures Parallel to Ridge

Zone	Load Case 1 psf	Load Case 2 psf
1	0.00	0.00
2	0.00	0.00
3	-36.05	0.00
4	-32.18	0.00
5	-26.37	0.00

A pressure of zero '0' indicates that the load case is not applicable for that zone.

Components and Cladding (C&C) Zone Summary per Ch 30 Pt 1:



h/W	= Ratio of mean roof height to building width	= 0.769	
h/L	= Ratio of mean roof height to building length	= 0.517	
h	= Mean Roof Height above grade	= 15.000 ft	
Kh	= 15 ft [4.572 m]< Z <Zg -->(2.01*(Z/zg)^(2/Alpha) {Table 26.10-1}	= 0.849	
Kzt	= Topographic Factor is 1 since no Topographic feature specified	= 1.000	
Kd	= Wind Directionality Factor per Table 26.6-1	= 0.85	
GCPi	= Ref Table 26.13-1 for Enclosed Building	= +/-0.18	
LF	= Load Factor based upon ASD Design	= 0.60	
qh	= (0.00256 * Kh * Kzt * Kd * Ke * V^2) * LF	= 33.94 psf	
LHD	= Least Horizontal Dimension: Min(B, L)	= 19.500 ft	
a1	= Min(0.1 * LHD, 0.4 * h)	= 1.950 ft	
a	= Max(a1, 0.04 * LHD, 3 ft [0.9 m])	= 3.000 ft	
h/B	= Ratio of mean roof height to least hor dim: h / B	= 0.769	
0.2*h	= Parameter used to define Zone 3	= 3.000 ft	
0.6*h	= Parameter used to define Zones 1 and 2	= 9.000 ft	

Wind Pressure Summary for C&C Zones based Upon Areas Ch 30 Pt 1 (Table 1 of 2)
All wind pressures include a load factor of 0.6

Zone	Figure	A <= 10.00 sq ft psf	A = 20.00 sq ft psf	A = 50.00 sq ft psf	A = 100.00 sq ft psf

1	30.3-2A	16.29	-63.81	15.27	-59.60	13.92	-54.04	12.90	-49.83
1'	30.3-2A	16.29	-36.66	15.27	-36.66	13.92	-36.66	12.90	-36.66
1_OH	30.3-2A	9.60	-57.70	9.60	-56.68	9.60	-55.33	9.60	-54.31
1'_OH	30.3-2A	9.60	-57.70	9.60	-56.68	9.60	-55.33	9.60	-54.31
2	30.3-2A	16.29	-84.18	15.27	-78.76	13.92	-71.61	12.90	-66.20
2_OH	30.3-2A	9.60	-78.07	9.60	-70.85	9.60	-61.31	9.60	-54.09
3	30.3-2A	16.29	-114.72	15.27	-103.90	13.92	-89.59	12.90	-78.76
3_OH	30.3-2A	9.60	-108.61	9.60	-95.98	9.60	-79.29	9.60	-66.66
4	30.3-1	36.66	-39.71	35.03	-38.09	32.89	-35.94	31.26	-34.32
5	30.3-1	36.66	-48.88	35.03	-45.63	32.89	-41.34	31.26	-38.09

Wind Pressure Summary for C&C Zones based Upon Areas Ch 30 Pt 1 (Table 2 of 2)
All wind pressures include a load factor of 0.6

Zone	Figure	A = 200.00 sq ft psf	A = 500.00 sq ft psf	A > 1000.00 sq ft psf
1	30.3-2A	12.90 -45.62	12.90 -40.05	12.90 -40.05
1'	30.3-2A	12.90 -31.55	12.90 -24.79	12.90 -19.69
1_OH	30.3-2A	9.60 -45.54	9.60 -33.94	9.60 -33.94
1'_OH	30.3-2A	9.60 -45.54	9.60 -33.94	9.60 -33.94
2	30.3-2A	12.90 -60.78	12.90 -53.63	12.90 -53.63
2_OH	30.3-2A	9.60 -46.88	9.60 -37.34	9.60 -37.34
3	30.3-2A	12.90 -67.94	12.90 -53.63	12.90 -53.63
3_OH	30.3-2A	9.60 -54.03	9.60 -37.34	9.60 -37.34
4	30.3-1	29.64 -32.69	27.49 -30.55	27.49 -30.55
5	30.3-1	29.64 -34.84	27.49 -30.55	27.49 -30.55

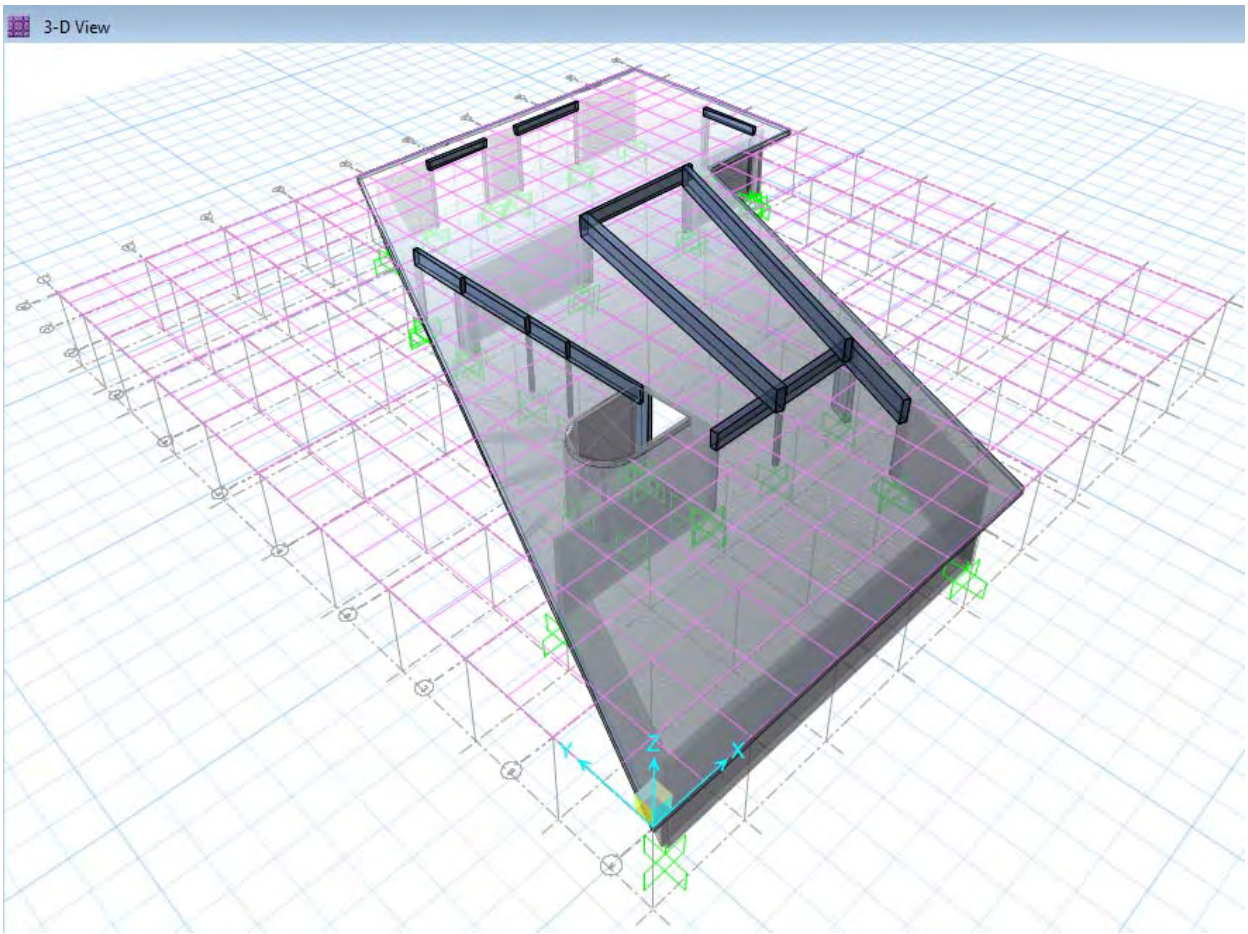
* A is effective wind area for C&C: Span Length * Effective Width
* Effective width need not be less than 1/3 of the span length
* Maximum and minimum values of pressure shown.
* + Pressures acting toward surface, - Pressures acting away from surface
* _OH represents an Overhang in the zone specified
* Overhang pressures calculated per Para 30.9
* Per Para 30.2.2 the Minimum Pressure for C&C is 9.60 psf [0.460 kPa] {Includes LF}
* Interpolation can be used for values of A that are between those values shown.

CONCRETE SLABS DESIGN

(ACI 318-14)

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SAFE®



SAFE Analysis & Design Report

Model Name: ROOF.fdb

7 October 2022

Contents

Model Definition 4
1. Model geometry 5
1.1. Connectivity 5
2. Model properties 15
2.1. Material properties 15
2.2. Section properties 16
2.3. Support properties 17
3. Model assignments 18
3.1. Slab assignments 18
3.2. Beam assignments 18
3.3. Column assignments 19
3.4. Wall assignments 19
3.5. Support assignments 20
4. Model loading 21
4.1. Load patterns 21
4.2. Load cases 22
4.3. Load combinations 22
Analysis Results 26
5. Analysis results 28
5.1. Support results 28
5.2. Structure results 41
Design 55
6. Design summary 56
6.1. Preferences 56
6.2. Overwrites 56
6.3. Slab design 59
6.4. Punching check/design 150

List of Figures

Figure 1: Finite element model 4
Figure 2: Deformed shape 27
Figure 3: Finite element model 55

List of Tables

Table 1: Concrete Slab Design Summary 02 - Span Definition Data 5
Table 2: Material Properties 03 - Concrete 15
Table 3: Material Properties 04 - Rebar 15
Table 4: Material Properties 05 - Tendon 15
Table 5: Material Properties 02 - Steel 15
Table 6: Material Properties 06 - Other 15
Table 7: Slab Properties 02 - Solid Slabs, Part 1 of 2 16
Table 7: Slab Properties 02 - Solid Slabs, Part 2 of 2 16
Table 8: Beam Properties 02 - Rectangular Beam 16
Table 9: Beam Properties 06 - Design Data 16
Table 10: Tendon Properties 16
Table 11: Column Properties 02 - Rectangular 17
Table 12: Wall Properties 17
Table 13: Soil Properties 17

Table 14: Spring Properties - Point 17
Table 15: Spring Properties - Line 17
Table 16: Slab Property Assignments 18
Table 17: Slab Edge Releases 18
Table 18: Beam Property Assignments 18
Table 19: Column Property Assignments 19
Table 20: Wall Property Assignments 19
Table 21: Point Restraint Assignments 20
Table 22: Load Patterns 21
Table 23: Load Assignments - Surface Loads 21
Table 24: Load Assignments - Line Objects - Distributed Loads, Part 1 of 2 21
Table 24: Load Assignments - Line Objects - Distributed Loads, Part 2 of 2 21
Table 25: Load Cases 02 - Static 22
Table 26: Load Cases 06 - Loads Applied 22
Table 27: Load Combinations 22
Table 28: Nodal Reactions 28
Table 29: Integrated Wall Forces 35
Table 30: Sum Of Reactions, Part 1 of 2 41
Table 30: Sum Of Reactions, Part 2 of 2 42
Table 31: Nodal Displacements - Summary, Part 1 of 2 42
Table 31: Nodal Displacements - Summary, Part 2 of 2 48
Table 32: Design Preferences 01 - Resistance Factors 56
Table 33: Design Preferences 02 - Rebar Cover - Slabs 56
Table 34: Design Preferences 03 - Rebar Cover - Beams 56
Table 35: Design Preferences 04 - Prestress Data 56
Table 36: Slab Design Overwrites 01 - Strip Based 56
Table 37: Slab Design Overwrites 02 - Finite Element Based, Part 1 of 2 57
Table 37: Slab Design Overwrites 02 - Finite Element Based, Part 2 of 2 57
Table 38: Beam Design Overwrites 58
Table 39: Punching Shear Design Overwrites 01 - General 58
Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3 59
Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3 87
Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3 115
Table 42: Concrete Slab Design 02 - Punching Shear Data, Part 1 of 3 150
Table 42: Concrete Slab Design 02 - Punching Shear Data, Part 2 of 3 150
Table 42: Concrete Slab Design 02 - Punching Shear Data, Part 3 of 3 151

Model Definition

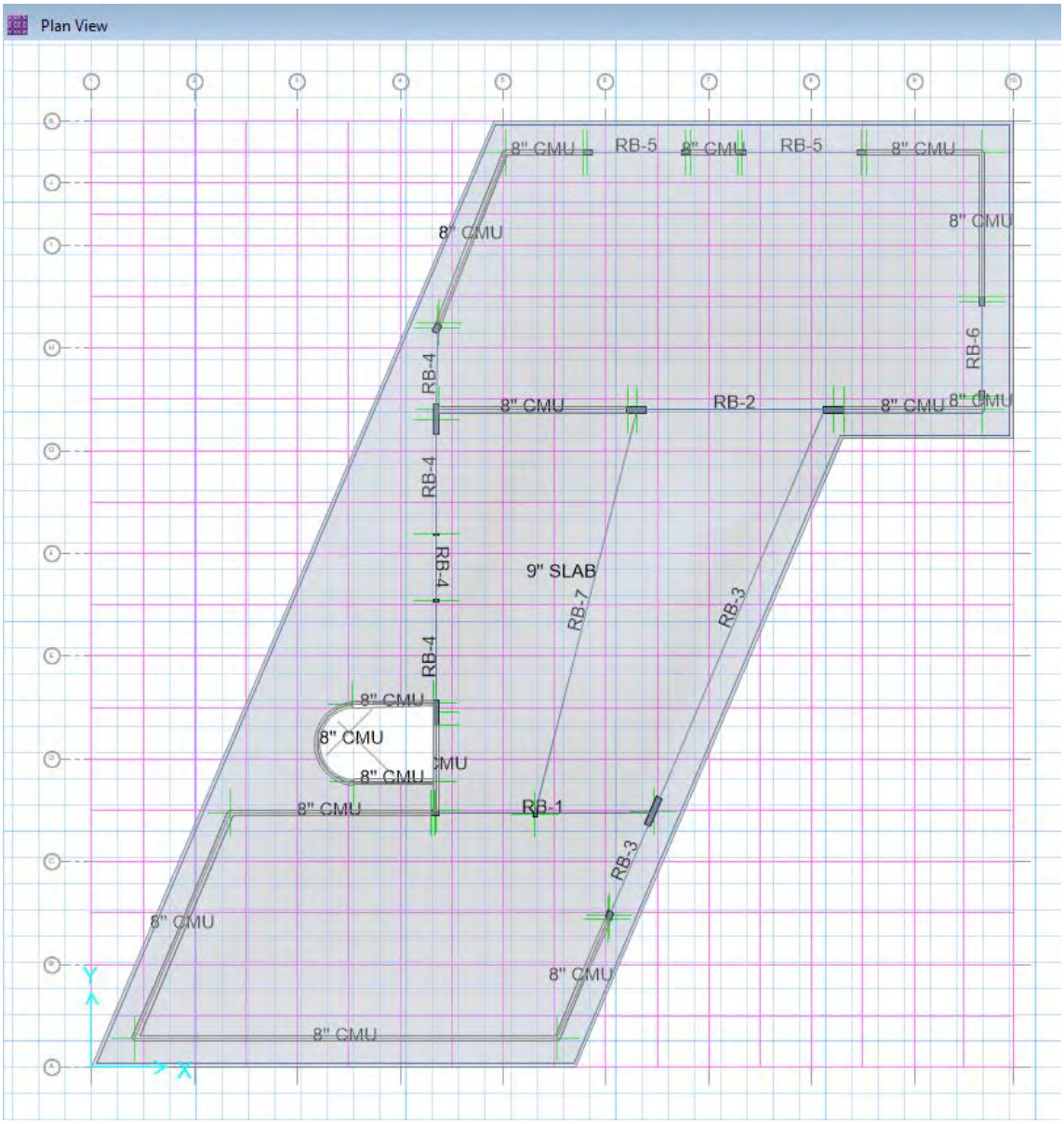


Figure 1: Finite element model

1. Model geometry

This section provides model geometry information, including items such as joint coordinates, joint restraints, and element connectivity.

1.1. Connectivity

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SA1	Span 1	12.0000	0.0000	0.0000	0.0000	12.0000	0.0000
SA1	Span 2	12.0000	12.0000	12.0000	0.0000	24.0000	0.0000
SA1	Span 3	12.0000	24.0000	24.0000	0.0000	36.0000	0.0000
SA1	Span 4	12.0000	36.0000	36.0000	0.0000	48.0000	0.0000
SA1	Span 5	12.0000	48.0000	48.0000	0.0000	60.0000	0.0000
SA1	Span 6	12.0000	60.0000	60.0000	0.0000	72.0000	0.0000
SA1	Span 7	12.0000	72.0000	72.0000	0.0000	84.0000	0.0000
SA1	Span 8	12.0000	84.0000	84.0000	0.0000	96.0000	0.0000
SA1	Span 9	11.5000	96.0000	96.0000	0.0000	107.5000	0.0000
SA2	Cantilever Start	2.4861	0.0000	2.5468	6.0000	5.0330	6.0000
SA2	Span 1	6.9670	2.4861	5.0330	6.0000	12.0000	6.0000
SA2	Span 2	12.0000	9.4532	12.0000	6.0000	24.0000	6.0000
SA2	Span 3	12.0000	21.4532	24.0000	6.0000	36.0000	6.0000
SA2	Span 4	12.0000	33.4532	36.0000	6.0000	48.0000	6.0000
SA2	Span 5	6.2834	45.4532	48.0000	6.0000	54.2834	6.0000
SA2	Span 6	5.7166	51.7365	54.2834	6.0000	60.0000	6.0000
SA2	Span 7	12.0000	57.4532	60.0000	6.0000	72.0000	6.0000
SA2	Span 8	12.0000	69.4532	72.0000	6.0000	84.0000	6.0000
SA2	Span 9	12.0000	81.4532	84.0000	6.0000	96.0000	6.0000
SA2	Span 10	11.5000	93.4532	96.0000	6.0000	107.5000	6.0000
SA3	Span 1	12.0000	0.0000	0.0000	12.0000	12.0000	12.0000
SA3	Span 2	12.0000	12.0000	12.0000	12.0000	24.0000	12.0000
SA3	Span 3	12.0000	24.0000	24.0000	12.0000	36.0000	12.0000
SA3	Span 4	12.0000	36.0000	36.0000	12.0000	48.0000	12.0000
SA3	Span 5	12.0000	48.0000	48.0000	12.0000	60.0000	12.0000
SA3	Span 6	12.0000	60.0000	60.0000	12.0000	72.0000	12.0000
SA3	Span 7	12.0000	72.0000	72.0000	12.0000	84.0000	12.0000
SA3	Span 8	12.0000	84.0000	84.0000	12.0000	96.0000	12.0000
SA3	Span 9	11.5000	96.0000	96.0000	12.0000	107.5000	12.0000
SA4	Span 1	12.0000	0.0000	0.0000	18.0000	12.0000	18.0000
SA4	Span 2	12.0000	12.0000	12.0000	18.0000	24.0000	18.0000
SA4	Span 3	12.0000	24.0000	24.0000	18.0000	36.0000	18.0000
SA4	Span 4	12.0000	36.0000	36.0000	18.0000	48.0000	18.0000
SA4	Span 5	12.0000	48.0000	48.0000	18.0000	60.0000	18.0000
SA4	Span 6	0.1927	60.0000	60.0000	18.0000	60.1927	18.0000
SA4	Span 7	0.1946	60.1927	60.1927	18.0000	60.3873	18.0000
SA4	Span 8	0.1161	60.3873	60.3873	18.0000	60.5034	18.0000
SA4	Span 9	11.4966	60.5034	60.5034	18.0000	72.0000	18.0000
SA4	Span 10	12.0000	72.0000	72.0000	18.0000	84.0000	18.0000
SA4	Span 11	12.0000	84.0000	84.0000	18.0000	96.0000	18.0000
SA4	Span 12	11.5000	96.0000	96.0000	18.0000	107.5000	18.0000
SA5	Span 1	12.0000	0.0000	0.0000	24.0000	12.0000	24.0000
SA5	Span 2	12.0000	12.0000	12.0000	24.0000	24.0000	24.0000

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SA5	Span 3	12.0000	24.0000	24.0000	24.0000	36.0000	24.0000
SA5	Span 4	12.0000	36.0000	36.0000	24.0000	48.0000	24.0000
SA5	Span 5	12.0000	48.0000	48.0000	24.0000	60.0000	24.0000
SA5	Span 6	3.0503	60.0000	60.0000	24.0000	63.0503	24.0000
SA5	Span 7	8.9497	63.0503	63.0503	24.0000	72.0000	24.0000
SA5	Span 8	12.0000	72.0000	72.0000	24.0000	84.0000	24.0000
SA5	Span 9	12.0000	84.0000	84.0000	24.0000	96.0000	24.0000
SA5	Span 10	11.5000	96.0000	96.0000	24.0000	107.5000	24.0000
SA6	Span 1	12.0000	0.0000	0.0000	30.0000	12.0000	30.0000
SA6	Span 2	4.2108	12.0000	12.0000	30.0000	16.2108	30.0000
SA6	Span 3	7.7892	16.2108	16.2108	30.0000	24.0000	30.0000
SA6	Span 4	12.0000	24.0000	24.0000	30.0000	36.0000	30.0000
SA6	Span 5	3.5282	36.0000	36.0000	30.0000	39.5282	30.0000
SA6	Span 6	0.4981	39.5282	39.5282	30.0000	40.0262	30.0000
SA6	Span 7	0.1647	40.0262	40.0262	30.0000	40.1910	30.0000
SA6	Span 8	7.8090	40.1910	40.1910	30.0000	48.0000	30.0000
SA6	Span 9	3.7560	48.0000	48.0000	30.0000	51.7560	30.0000
SA6	Span 10	0.1220	51.7560	51.7560	30.0000	51.8780	30.0000
SA6	Span 11	8.1220	51.8780	51.8780	30.0000	60.0000	30.0000
SA6	Span 12	5.5475	60.0000	60.0000	30.0000	65.5475	30.0000
SA6	Span 13	0.0496	65.5475	65.5475	30.0000	65.5971	30.0000
SA6	Span 14	6.4029	65.5971	65.5971	30.0000	72.0000	30.0000
SA6	Span 15	12.0000	72.0000	72.0000	30.0000	84.0000	30.0000
SA6	Span 16	12.0000	84.0000	84.0000	30.0000	96.0000	30.0000
SA6	Span 17	11.5000	96.0000	96.0000	30.0000	107.5000	30.0000
SA7	Span 1	12.0000	0.0000	0.0000	36.0000	12.0000	36.0000
SA7	Span 2	12.0000	12.0000	12.0000	36.0000	24.0000	36.0000
SA7	Span 3	6.5162	24.0000	24.0000	36.0000	30.5162	36.0000
SA7	Span 4	5.4838	30.5162	30.5162	36.0000	36.0000	36.0000
SA7	Span 5	3.8576	36.0000	36.0000	36.0000	39.8576	36.0000
SA7	Span 6	8.1424	39.8576	39.8576	36.0000	48.0000	36.0000
SA7	Span 7	5.3809	48.0000	48.0000	36.0000	53.3809	36.0000
SA7	Span 8	6.6191	53.3809	53.3809	36.0000	60.0000	36.0000
SA7	Span 9	8.1445	60.0000	60.0000	36.0000	68.1445	36.0000
SA7	Span 10	3.8555	68.1445	68.1445	36.0000	72.0000	36.0000
SA7	Span 11	12.0000	72.0000	72.0000	36.0000	84.0000	36.0000
SA7	Span 12	12.0000	84.0000	84.0000	36.0000	96.0000	36.0000
SA7	Span 13	11.5000	96.0000	96.0000	36.0000	107.5000	36.0000
SA8	Span 1	12.0000	0.0000	0.0000	42.0000	12.0000	42.0000
SA8	Span 2	12.0000	12.0000	12.0000	42.0000	24.0000	42.0000
SA8	Span 3	6.3591	24.0000	24.0000	42.0000	30.3591	42.0000
SA8	Span 4	5.6409	30.3591	30.3591	42.0000	36.0000	42.0000
SA8	Span 5	3.8576	36.0000	36.0000	42.0000	39.8576	42.0000
SA8	Span 6	0.3341	39.8576	39.8576	42.0000	40.1918	42.0000
SA8	Span 7	7.8082	40.1918	40.1918	42.0000	48.0000	42.0000
SA8	Span 8	6.8839	48.0000	48.0000	42.0000	54.8839	42.0000
SA8	Span 9	5.1161	54.8839	54.8839	42.0000	60.0000	42.0000
SA8	Span 10	10.6919	60.0000	60.0000	42.0000	70.6919	42.0000
SA8	Span 11	1.3081	70.6919	70.6919	42.0000	72.0000	42.0000
SA8	Span 12	12.0000	72.0000	72.0000	42.0000	84.0000	42.0000
SA8	Span 13	12.0000	84.0000	84.0000	42.0000	96.0000	42.0000
SA8	Span 14	11.5000	96.0000	96.0000	42.0000	107.5000	42.0000
SA9	Span 1	12.0000	0.0000	0.0000	48.0000	12.0000	48.0000
SA9	Span 2	12.0000	12.0000	12.0000	48.0000	24.0000	48.0000

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SA9	Span 3	12.0000	24.0000	24.0000	48.0000	36.0000	48.0000
SA9	Span 4	4.1929	36.0000	36.0000	48.0000	40.1929	48.0000
SA9	Span 5	7.8071	40.1929	40.1929	48.0000	48.0000	48.0000
SA9	Span 6	8.3868	48.0000	48.0000	48.0000	56.3868	48.0000
SA9	Span 7	3.6132	56.3868	56.3868	48.0000	60.0000	48.0000
SA9	Span 8	12.0000	60.0000	60.0000	48.0000	72.0000	48.0000
SA9	Span 9	1.2392	72.0000	72.0000	48.0000	73.2392	48.0000
SA9	Span 10	10.7608	73.2392	73.2392	48.0000	84.0000	48.0000
SA9	Span 11	12.0000	84.0000	84.0000	48.0000	96.0000	48.0000
SA9	Span 12	11.5000	96.0000	96.0000	48.0000	107.5000	48.0000
SB1	Span 1	12.0000	0.0000	0.0000	0.0000	0.0000	12.0000
SB1	Span 2	12.0000	12.0000	0.0000	12.0000	0.0000	24.0000
SB1	Span 3	12.0000	24.0000	0.0000	24.0000	0.0000	36.0000
SB1	Span 4	12.0000	36.0000	0.0000	36.0000	0.0000	48.0000
SB1	Span 5	12.0000	48.0000	0.0000	48.0000	0.0000	60.0000
SB1	Span 6	12.0000	60.0000	0.0000	60.0000	0.0000	72.0000
SB1	Span 7	12.0000	72.0000	0.0000	72.0000	0.0000	84.0000
SB1	Span 8	12.0000	84.0000	0.0000	84.0000	0.0000	96.0000
SB1	Span 9	7.2500	96.0000	0.0000	96.0000	0.0000	103.2500
SB1	Span 10	7.2500	103.2500	0.0000	103.2500	0.0000	110.5000
SB2	Span 1	3.3464	0.0000	6.0000	0.0000	6.0000	3.3464
SB2	Span 2	8.6536	3.3464	6.0000	3.3464	6.0000	12.0000
SB2	Span 3	12.0000	12.0000	6.0000	12.0000	6.0000	24.0000
SB2	Span 4	12.0000	24.0000	6.0000	24.0000	6.0000	36.0000
SB2	Span 5	12.0000	36.0000	6.0000	36.0000	6.0000	48.0000
SB2	Span 6	12.0000	48.0000	6.0000	48.0000	6.0000	60.0000
SB2	Span 7	12.0000	60.0000	6.0000	60.0000	6.0000	72.0000
SB2	Span 8	12.0000	72.0000	6.0000	72.0000	6.0000	84.0000
SB2	Span 9	12.0000	84.0000	6.0000	84.0000	6.0000	96.0000
SB2	Span 10	7.2500	96.0000	6.0000	96.0000	6.0000	103.2500
SB2	Span 11	7.2500	103.2500	6.0000	103.2500	6.0000	110.5000
SB3	Span 1	12.0000	0.0000	12.0000	0.0000	12.0000	12.0000
SB3	Span 2	12.0000	12.0000	12.0000	12.0000	12.0000	24.0000
SB3	Span 3	12.0000	24.0000	12.0000	24.0000	12.0000	36.0000
SB3	Span 4	12.0000	36.0000	12.0000	36.0000	12.0000	48.0000
SB3	Span 5	12.0000	48.0000	12.0000	48.0000	12.0000	60.0000
SB3	Span 6	12.0000	60.0000	12.0000	60.0000	12.0000	72.0000
SB3	Span 7	12.0000	72.0000	12.0000	72.0000	12.0000	84.0000
SB3	Span 8	12.0000	84.0000	12.0000	84.0000	12.0000	96.0000
SB3	Span 9	7.2500	96.0000	12.0000	96.0000	12.0000	103.2500
SB3	Span 10	7.2500	103.2500	12.0000	103.2500	12.0000	110.5000
SB4	Span 1	12.0000	0.0000	18.0000	0.0000	18.0000	12.0000
SB4	Span 2	12.0000	12.0000	18.0000	12.0000	18.0000	24.0000
SB4	Span 3	5.6798	24.0000	18.0000	24.0000	18.0000	29.6798
SB4	Span 4	6.3202	29.6798	18.0000	29.6798	18.0000	36.0000
SB4	Span 5	12.0000	36.0000	18.0000	36.0000	18.0000	48.0000
SB4	Span 6	12.0000	48.0000	18.0000	48.0000	18.0000	60.0000
SB4	Span 7	12.0000	60.0000	18.0000	60.0000	18.0000	72.0000
SB4	Span 8	12.0000	72.0000	18.0000	72.0000	18.0000	84.0000
SB4	Span 9	12.0000	84.0000	18.0000	84.0000	18.0000	96.0000
SB4	Span 10	7.2500	96.0000	18.0000	96.0000	18.0000	103.2500
SB4	Span 11	7.2500	103.2500	18.0000	103.2500	18.0000	110.5000
SB5	Span 1	12.0000	0.0000	24.0000	0.0000	24.0000	12.0000
SB5	Span 2	12.0000	12.0000	24.0000	12.0000	24.0000	24.0000

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SB5	Span 3	12.0000	24.0000	24.0000	24.0000	24.0000	36.0000
SB5	Span 4	12.0000	36.0000	24.0000	36.0000	24.0000	48.0000
SB5	Span 5	12.0000	48.0000	24.0000	48.0000	24.0000	60.0000
SB5	Span 6	12.0000	60.0000	24.0000	60.0000	24.0000	72.0000
SB5	Span 7	12.0000	72.0000	24.0000	72.0000	24.0000	84.0000
SB5	Span 8	12.0000	84.0000	24.0000	84.0000	24.0000	96.0000
SB5	Span 9	7.2500	96.0000	24.0000	96.0000	24.0000	103.2500
SB5	Span 10	7.2500	103.2500	24.0000	103.2500	24.0000	110.5000
SB6	Span 1	12.0000	0.0000	30.0000	0.0000	30.0000	12.0000
SB6	Span 2	12.0000	12.0000	30.0000	12.0000	30.0000	24.0000
SB6	Span 3	9.3380	24.0000	30.0000	24.0000	30.0000	33.3380
SB6	Span 4	2.6620	33.3380	30.0000	33.3380	30.0000	36.0000
SB6	Span 5	6.3367	36.0000	30.0000	36.0000	30.0000	42.3367
SB6	Span 6	5.6633	42.3367	30.0000	42.3367	30.0000	48.0000
SB6	Span 7	12.0000	48.0000	30.0000	48.0000	30.0000	60.0000
SB6	Span 8	12.0000	60.0000	30.0000	60.0000	30.0000	72.0000
SB6	Span 9	12.0000	72.0000	30.0000	72.0000	30.0000	84.0000
SB6	Span 10	12.0000	84.0000	30.0000	84.0000	30.0000	96.0000
SB6	Span 11	7.2500	96.0000	30.0000	96.0000	30.0000	103.2500
SB6	Span 12	7.2500	103.2500	30.0000	103.2500	30.0000	110.5000
SB7	Span 1	12.0000	0.0000	36.0000	0.0000	36.0000	12.0000
SB7	Span 2	12.0000	12.0000	36.0000	12.0000	36.0000	24.0000
SB7	Span 3	12.0000	24.0000	36.0000	24.0000	36.0000	36.0000
SB7	Span 4	12.0000	36.0000	36.0000	36.0000	36.0000	48.0000
SB7	Span 5	12.0000	48.0000	36.0000	48.0000	36.0000	60.0000
SB7	Span 6	12.0000	60.0000	36.0000	60.0000	36.0000	72.0000
SB7	Span 7	12.0000	72.0000	36.0000	72.0000	36.0000	84.0000
SB7	Span 8	12.0000	84.0000	36.0000	84.0000	36.0000	96.0000
SB7	Span 9	7.2500	96.0000	36.0000	96.0000	36.0000	103.2500
SB7	Span 10	7.2500	103.2500	36.0000	103.2500	36.0000	110.5000
SB8	Span 1	12.0000	0.0000	42.0000	0.0000	42.0000	12.0000
SB8	Span 2	12.0000	12.0000	42.0000	12.0000	42.0000	24.0000
SB8	Span 3	5.6798	24.0000	42.0000	24.0000	42.0000	29.6798
SB8	Span 4	0.3333	29.6798	42.0000	29.6798	42.0000	30.0131
SB8	Span 5	3.3152	30.0131	42.0000	30.0131	42.0000	33.3284
SB8	Span 6	2.6716	33.3284	42.0000	33.3284	42.0000	36.0000
SB8	Span 7	4.0131	36.0000	42.0000	36.0000	42.0000	40.0131
SB8	Span 8	1.4179	40.0131	42.0000	40.0131	42.0000	41.4310
SB8	Span 9	1.0714	41.4310	42.0000	41.4310	42.0000	42.5025
SB8	Span 10	5.4975	42.5025	42.0000	42.5025	42.0000	48.0000
SB8	Span 11	6.4730	48.0000	42.0000	48.0000	42.0000	54.4730
SB8	Span 12	5.5270	54.4730	42.0000	54.4730	42.0000	60.0000
SB8	Span 13	2.2224	60.0000	42.0000	60.0000	42.0000	62.2224
SB8	Span 14	9.7776	62.2224	42.0000	62.2224	42.0000	72.0000
SB8	Span 15	3.6515	72.0000	42.0000	72.0000	42.0000	75.6515
SB8	Span 16	1.1120	75.6515	42.0000	75.6515	42.0000	76.7635
SB8	Span 17	7.2365	76.7635	42.0000	76.7635	42.0000	84.0000
SB8	Span 18	2.3523	84.0000	42.0000	84.0000	42.0000	86.3523
SB8	Span 19	0.6477	86.3523	42.0000	86.3523	42.0000	87.0000
SB8	Span 20	9.0000	87.0000	42.0000	87.0000	42.0000	96.0000
SB8	Span 21	7.2500	96.0000	42.0000	96.0000	42.0000	103.2500
SB8	Span 22	7.2500	103.2500	42.0000	103.2500	42.0000	110.5000
SB9	Span 1	12.0000	0.0000	48.0000	0.0000	48.0000	12.0000
SB9	Span 2	12.0000	12.0000	48.0000	12.0000	48.0000	24.0000

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SB9	Span 3	5.6798	24.0000	48.0000	24.0000	48.0000	29.6798
SB9	Span 4	6.3202	29.6798	48.0000	29.6798	48.0000	36.0000
SB9	Span 5	12.0000	36.0000	48.0000	36.0000	48.0000	48.0000
SB9	Span 6	12.0000	48.0000	48.0000	48.0000	48.0000	60.0000
SB9	Span 7	12.0000	60.0000	48.0000	60.0000	48.0000	72.0000
SB9	Span 8	12.0000	72.0000	48.0000	72.0000	48.0000	84.0000
SB9	Span 9	12.0000	84.0000	48.0000	84.0000	48.0000	96.0000
SB9	Span 10	7.2500	96.0000	48.0000	96.0000	48.0000	103.2500
SB9	Span 11	3.5964	103.2500	48.0000	103.2500	48.0000	106.8464
SB9	Span 12	3.6536	106.8464	48.0000	106.8464	48.0000	110.5000
SA10	Span 1	12.0000	0.0000	0.0000	54.0000	12.0000	54.0000
SA10	Span 2	12.0000	12.0000	12.0000	54.0000	24.0000	54.0000
SA10	Span 3	12.0000	24.0000	24.0000	54.0000	36.0000	54.0000
SA10	Span 4	4.1941	36.0000	36.0000	54.0000	40.1941	54.0000
SA10	Span 5	7.8059	40.1941	40.1941	54.0000	48.0000	54.0000
SA10	Span 6	9.8898	48.0000	48.0000	54.0000	57.8898	54.0000
SA10	Span 7	2.1102	57.8898	57.8898	54.0000	60.0000	54.0000
SA10	Span 8	12.0000	60.0000	60.0000	54.0000	72.0000	54.0000
SA10	Span 9	3.7866	72.0000	72.0000	54.0000	75.7866	54.0000
SA10	Span 10	8.2134	75.7866	75.7866	54.0000	84.0000	54.0000
SA10	Span 11	12.0000	84.0000	84.0000	54.0000	96.0000	54.0000
SA10	Span 12	11.5000	96.0000	96.0000	54.0000	107.5000	54.0000
SA11	Span 1	12.0000	0.0000	0.0000	60.0000	12.0000	60.0000
SA11	Span 2	12.0000	12.0000	12.0000	60.0000	24.0000	60.0000
SA11	Span 3	12.0000	24.0000	24.0000	60.0000	36.0000	60.0000
SA11	Span 4	4.1910	36.0000	36.0000	60.0000	40.1910	60.0000
SA11	Span 5	7.8090	40.1910	40.1910	60.0000	48.0000	60.0000
SA11	Span 6	11.3927	48.0000	48.0000	60.0000	59.3927	60.0000
SA11	Span 7	0.6073	59.3927	59.3927	60.0000	60.0000	60.0000
SA11	Span 8	12.0000	60.0000	60.0000	60.0000	72.0000	60.0000
SA11	Span 9	6.3339	72.0000	72.0000	60.0000	78.3339	60.0000
SA11	Span 10	5.6661	78.3339	78.3339	60.0000	84.0000	60.0000
SA11	Span 11	12.0000	84.0000	84.0000	60.0000	96.0000	60.0000
SA11	Span 12	11.5000	96.0000	96.0000	60.0000	107.5000	60.0000
SA12	Span 1	12.0000	0.0000	0.0000	66.0000	12.0000	66.0000
SA12	Span 2	12.0000	12.0000	12.0000	66.0000	24.0000	66.0000
SA12	Span 3	12.0000	24.0000	24.0000	66.0000	36.0000	66.0000
SA12	Span 4	4.1910	36.0000	36.0000	66.0000	40.1910	66.0000
SA12	Span 5	7.8090	40.1910	40.1910	66.0000	48.0000	66.0000
SA12	Span 6	12.0000	48.0000	48.0000	66.0000	60.0000	66.0000
SA12	Span 7	0.8957	60.0000	60.0000	66.0000	60.8957	66.0000
SA12	Span 8	11.1043	60.8957	60.8957	66.0000	72.0000	66.0000
SA12	Span 9	8.8813	72.0000	72.0000	66.0000	80.8813	66.0000
SA12	Span 10	3.1187	80.8813	80.8813	66.0000	84.0000	66.0000
SA12	Span 11	12.0000	84.0000	84.0000	66.0000	96.0000	66.0000
SA12	Span 12	11.5000	96.0000	96.0000	66.0000	107.5000	66.0000
SA13	Span 1	12.0000	0.0000	0.0000	72.0000	12.0000	72.0000
SA13	Span 2	12.0000	12.0000	12.0000	72.0000	24.0000	72.0000
SA13	Span 3	12.0000	24.0000	24.0000	72.0000	36.0000	72.0000
SA13	Span 4	4.1910	36.0000	36.0000	72.0000	40.1910	72.0000
SA13	Span 5	7.8090	40.1910	40.1910	72.0000	48.0000	72.0000
SA13	Span 6	12.0000	48.0000	48.0000	72.0000	60.0000	72.0000
SA13	Span 7	2.3986	60.0000	60.0000	72.0000	62.3986	72.0000
SA13	Span 8	9.6014	62.3986	62.3986	72.0000	72.0000	72.0000

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SA13	Span 9	11.4287	72.0000	72.0000	72.0000	83.4287	72.0000
SA13	Span 10	0.5713	83.4287	83.4287	72.0000	84.0000	72.0000
SA13	Span 11	12.0000	84.0000	84.0000	72.0000	96.0000	72.0000
SA13	Span 12	11.5000	96.0000	96.0000	72.0000	107.5000	72.0000
SA14	Span 1	12.0000	0.0000	0.0000	78.0000	12.0000	78.0000
SA14	Span 2	12.0000	12.0000	12.0000	78.0000	24.0000	78.0000
SA14	Span 3	12.0000	24.0000	24.0000	78.0000	36.0000	78.0000
SA14	Span 4	4.1910	36.0000	36.0000	78.0000	40.1910	78.0000
SA14	Span 5	0.0258	40.1910	40.1910	78.0000	40.2168	78.0000
SA14	Span 6	0.3075	40.2168	40.2168	78.0000	40.5243	78.0000
SA14	Span 7	7.4757	40.5243	40.5243	78.0000	48.0000	78.0000
SA14	Span 8	12.0000	48.0000	48.0000	78.0000	60.0000	78.0000
SA14	Span 9	2.5243	60.0000	60.0000	78.0000	62.5243	78.0000
SA14	Span 10	0.9901	62.5243	62.5243	78.0000	63.5144	78.0000
SA14	Span 11	0.0752	63.5144	63.5144	78.0000	63.5896	78.0000
SA14	Span 12	8.4104	63.5896	63.5896	78.0000	72.0000	78.0000
SA14	Span 13	12.0000	72.0000	72.0000	78.0000	84.0000	78.0000
SA14	Span 14	1.4510	84.0000	84.0000	78.0000	85.4510	78.0000
SA14	Span 15	1.1471	85.4510	85.4510	78.0000	86.5981	78.0000
SA14	Span 16	1.2627	86.5981	86.5981	78.0000	87.8607	78.0000
SA14	Span 17	8.1393	87.8607	87.8607	78.0000	96.0000	78.0000
SA14	Span 18	7.8592	96.0000	96.0000	78.0000	103.8592	78.0000
SA14	Span 19	3.6408	103.8592	103.8592	78.0000	107.5000	78.0000
SA15	Span 1	12.0000	0.0000	0.0000	84.0000	12.0000	84.0000
SA15	Span 2	12.0000	12.0000	12.0000	84.0000	24.0000	84.0000
SA15	Span 3	12.0000	24.0000	24.0000	84.0000	36.0000	84.0000
SA15	Span 4	4.2827	36.0000	36.0000	84.0000	40.2827	84.0000
SA15	Span 5	0.0258	40.2827	40.2827	84.0000	40.3085	84.0000
SA15	Span 6	0.1915	40.3085	40.3085	84.0000	40.5000	84.0000
SA15	Span 7	7.5000	40.5000	40.5000	84.0000	48.0000	84.0000
SA15	Span 8	12.0000	48.0000	48.0000	84.0000	60.0000	84.0000
SA15	Span 9	12.0000	60.0000	60.0000	84.0000	72.0000	84.0000
SA15	Span 10	12.0000	72.0000	72.0000	84.0000	84.0000	84.0000
SA15	Span 11	12.0000	84.0000	84.0000	84.0000	96.0000	84.0000
SA15	Span 12	7.8592	96.0000	96.0000	84.0000	103.8592	84.0000
SA15	Span 13	3.6408	103.8592	103.8592	84.0000	107.5000	84.0000
SA16	Span 1	12.0000	0.0000	0.0000	90.0000	12.0000	90.0000
SA16	Span 2	12.0000	12.0000	12.0000	90.0000	24.0000	90.0000
SA16	Span 3	12.0000	24.0000	24.0000	90.0000	36.0000	90.0000
SA16	Span 4	4.5000	36.0000	36.0000	90.0000	40.5000	90.0000
SA16	Span 5	7.5000	40.5000	40.5000	90.0000	48.0000	90.0000
SA16	Span 6	12.0000	48.0000	48.0000	90.0000	60.0000	90.0000
SA16	Span 7	12.0000	60.0000	60.0000	90.0000	72.0000	90.0000
SA16	Span 8	12.0000	72.0000	72.0000	90.0000	84.0000	90.0000
SA16	Span 9	12.0000	84.0000	84.0000	90.0000	96.0000	90.0000
SA16	Span 10	7.8592	96.0000	96.0000	90.0000	103.8592	90.0000
SA16	Span 11	3.6408	103.8592	103.8592	90.0000	107.5000	90.0000
SA17	Span 1	12.0000	0.0000	0.0000	96.0000	12.0000	96.0000
SA17	Span 2	12.0000	12.0000	12.0000	96.0000	24.0000	96.0000
SA17	Span 3	12.0000	24.0000	24.0000	96.0000	36.0000	96.0000
SA17	Span 4	12.0000	36.0000	36.0000	96.0000	48.0000	96.0000
SA17	Span 5	12.0000	48.0000	48.0000	96.0000	60.0000	96.0000
SA17	Span 6	12.0000	60.0000	60.0000	96.0000	72.0000	96.0000
SA17	Span 7	12.0000	72.0000	72.0000	96.0000	84.0000	96.0000

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SA17	Span 8	12.0000	84.0000	84.0000	96.0000	96.0000	96.0000
SA17	Span 9	11.5000	96.0000	96.0000	96.0000	107.5000	96.0000
SA18	Span 1	12.0000	0.0000	0.0000	99.6250	12.0000	99.6250
SA18	Span 2	12.0000	12.0000	12.0000	99.6250	24.0000	99.6250
SA18	Span 3	12.0000	24.0000	24.0000	99.6250	36.0000	99.6250
SA18	Span 4	12.0000	36.0000	36.0000	99.6250	48.0000	99.6250
SA18	Span 5	12.0000	48.0000	48.0000	99.6250	60.0000	99.6250
SA18	Span 6	12.0000	60.0000	60.0000	99.6250	72.0000	99.6250
SA18	Span 7	12.0000	72.0000	72.0000	99.6250	84.0000	99.6250
SA18	Span 8	12.0000	84.0000	84.0000	99.6250	96.0000	99.6250
SA18	Span 9	11.5000	96.0000	96.0000	99.6250	107.5000	99.6250
SA19	Span 1	12.0000	0.0000	0.0000	103.2500	12.0000	103.2500
SA19	Span 2	12.0000	12.0000	12.0000	103.2500	24.0000	103.2500
SA19	Span 3	12.0000	24.0000	24.0000	103.2500	36.0000	103.2500
SA19	Span 4	12.0000	36.0000	36.0000	103.2500	48.0000	103.2500
SA19	Span 5	12.0000	48.0000	48.0000	103.2500	60.0000	103.2500
SA19	Span 6	12.0000	60.0000	60.0000	103.2500	72.0000	103.2500
SA19	Span 7	12.0000	72.0000	72.0000	103.2500	84.0000	103.2500
SA19	Span 8	12.0000	84.0000	84.0000	103.2500	96.0000	103.2500
SA19	Span 9	11.5000	96.0000	96.0000	103.2500	107.5000	103.2500
SA20	Span 1	12.0000	0.0000	0.0000	106.8750	12.0000	106.8750
SA20	Span 2	12.0000	12.0000	12.0000	106.8750	24.0000	106.8750
SA20	Span 3	12.0000	24.0000	24.0000	106.8750	36.0000	106.8750
SA20	Span 4	12.0000	36.0000	36.0000	106.8750	48.0000	106.8750
SA20	Span 5	0.2650	48.0000	48.0000	106.8750	48.2650	106.8750
SA20	Span 6	9.0965	48.2650	48.2650	106.8750	57.3615	106.8750
SA20	Span 7	0.4981	57.3615	57.3615	106.8750	57.8596	106.8750
SA20	Span 8	2.1404	57.8596	57.8596	106.8750	60.0000	106.8750
SA20	Span 9	9.3557	60.0000	60.0000	106.8750	69.3557	106.8750
SA20	Span 10	0.4981	69.3557	69.3557	106.8750	69.8538	106.8750
SA20	Span 11	2.1462	69.8538	69.8538	106.8750	72.0000	106.8750
SA20	Span 12	3.3615	72.0000	72.0000	106.8750	75.3615	106.8750
SA20	Span 13	0.4981	75.3615	75.3615	106.8750	75.8596	106.8750
SA20	Span 14	8.1404	75.8596	75.8596	106.8750	84.0000	106.8750
SA20	Span 15	5.8557	84.0000	84.0000	106.8750	89.8557	106.8750
SA20	Span 16	0.4981	89.8557	89.8557	106.8750	90.3538	106.8750
SA20	Span 17	5.6462	90.3538	90.3538	106.8750	96.0000	106.8750
SA20	Span 18	7.8577	96.0000	96.0000	106.8750	103.8577	106.8750
SA20	Span 19	3.6423	103.8577	103.8577	106.8750	107.5000	106.8750
SA21	Span 1	12.0000	0.0000	0.0000	110.5000	12.0000	110.5000
SA21	Span 2	12.0000	12.0000	12.0000	110.5000	24.0000	110.5000
SA21	Span 3	12.0000	24.0000	24.0000	110.5000	36.0000	110.5000
SA21	Span 4	12.0000	36.0000	36.0000	110.5000	48.0000	110.5000
SA21	Span 5	12.0000	48.0000	48.0000	110.5000	60.0000	110.5000
SA21	Span 6	12.0000	60.0000	60.0000	110.5000	72.0000	110.5000
SA21	Span 7	12.0000	72.0000	72.0000	110.5000	84.0000	110.5000
SA21	Span 8	12.0000	84.0000	84.0000	110.5000	96.0000	110.5000
SA21	Span 9	11.5000	96.0000	96.0000	110.5000	107.5000	110.5000
SB10	Span 1	3.3464	0.0000	54.0000	0.0000	54.0000	3.3464
SB10	Span 2	8.6536	3.3464	54.0000	3.3464	54.0000	12.0000
SB10	Span 3	12.0000	12.0000	54.0000	12.0000	54.0000	24.0000
SB10	Span 4	5.5131	24.0000	54.0000	24.0000	54.0000	29.5131
SB10	Span 5	0.1667	29.5131	54.0000	29.5131	54.0000	29.6798
SB10	Span 6	6.3202	29.6798	54.0000	29.6798	54.0000	36.0000

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SB10	Span 7	2.4714	36.0000	54.0000	36.0000	54.0000	38.4714
SB10	Span 8	9.5286	38.4714	54.0000	38.4714	54.0000	48.0000
SB10	Span 9	12.0000	48.0000	54.0000	48.0000	54.0000	60.0000
SB10	Span 10	12.0000	60.0000	54.0000	60.0000	54.0000	72.0000
SB10	Span 11	12.0000	72.0000	54.0000	72.0000	54.0000	84.0000
SB10	Span 12	12.0000	84.0000	54.0000	84.0000	54.0000	96.0000
SB10	Span 13	7.2500	96.0000	54.0000	96.0000	54.0000	103.2500
SB10	Span 14	7.2500	103.2500	54.0000	103.2500	54.0000	110.5000
SB11	Span 1	12.0000	0.0000	60.0000	0.0000	60.0000	12.0000
SB11	Span 2	5.2681	12.0000	60.0000	12.0000	60.0000	17.2681
SB11	Span 3	0.4585	17.2681	60.0000	17.2681	60.0000	17.7265
SB11	Span 4	6.2735	17.7265	60.0000	17.7265	60.0000	24.0000
SB11	Span 5	5.6798	24.0000	60.0000	24.0000	60.0000	29.6798
SB11	Span 6	6.3202	29.6798	60.0000	29.6798	60.0000	36.0000
SB11	Span 7	12.0000	36.0000	60.0000	36.0000	60.0000	48.0000
SB11	Span 8	12.0000	48.0000	60.0000	48.0000	60.0000	60.0000
SB11	Span 9	2.4244	60.0000	60.0000	60.0000	60.0000	62.4244
SB11	Span 10	9.5756	62.4244	60.0000	62.4244	60.0000	72.0000
SB11	Span 11	4.7635	72.0000	60.0000	72.0000	60.0000	76.7635
SB11	Span 12	7.2365	76.7635	60.0000	76.7635	60.0000	84.0000
SB11	Span 13	12.0000	84.0000	60.0000	84.0000	60.0000	96.0000
SB11	Span 14	7.2500	96.0000	60.0000	96.0000	60.0000	103.2500
SB11	Span 15	3.5964	103.2500	60.0000	103.2500	60.0000	106.8464
SB11	Span 16	3.6536	106.8464	60.0000	106.8464	60.0000	110.5000
SB12	Span 1	12.0000	0.0000	66.0000	0.0000	66.0000	12.0000
SB12	Span 2	12.0000	12.0000	66.0000	12.0000	66.0000	24.0000
SB12	Span 3	5.6798	24.0000	66.0000	24.0000	66.0000	29.6798
SB12	Span 4	0.2033	29.6798	66.0000	29.6798	66.0000	29.8831
SB12	Span 5	1.0658	29.8831	66.0000	29.8831	66.0000	30.9489
SB12	Span 6	5.0511	30.9489	66.0000	30.9489	66.0000	36.0000
SB12	Span 7	12.0000	36.0000	66.0000	36.0000	66.0000	48.0000
SB12	Span 8	12.0000	48.0000	66.0000	48.0000	66.0000	60.0000
SB12	Span 9	12.0000	60.0000	66.0000	60.0000	66.0000	72.0000
SB12	Span 10	4.7545	72.0000	66.0000	72.0000	66.0000	76.7545
SB12	Span 11	0.0227	76.7545	66.0000	76.7545	66.0000	76.7772
SB12	Span 12	7.2228	76.7772	66.0000	76.7772	66.0000	84.0000
SB12	Span 13	12.0000	84.0000	66.0000	84.0000	66.0000	96.0000
SB12	Span 14	7.2500	96.0000	66.0000	96.0000	66.0000	103.2500
SB12	Span 15	3.5964	103.2500	66.0000	103.2500	66.0000	106.8464
SB12	Span 16	3.6536	106.8464	66.0000	106.8464	66.0000	110.5000
SB13	Span 1	12.0000	0.0000	72.0000	0.0000	72.0000	12.0000
SB13	Span 2	12.0000	12.0000	72.0000	12.0000	72.0000	24.0000
SB13	Span 3	12.0000	24.0000	72.0000	24.0000	72.0000	36.0000
SB13	Span 4	9.0812	36.0000	72.0000	36.0000	72.0000	45.0812
SB13	Span 5	2.9188	45.0812	72.0000	45.0812	72.0000	48.0000
SB13	Span 6	12.0000	48.0000	72.0000	48.0000	72.0000	60.0000
SB13	Span 7	12.0000	60.0000	72.0000	60.0000	72.0000	72.0000
SB13	Span 8	4.7721	72.0000	72.0000	72.0000	72.0000	76.7721
SB13	Span 9	7.2279	76.7721	72.0000	76.7721	72.0000	84.0000
SB13	Span 10	12.0000	84.0000	72.0000	84.0000	72.0000	96.0000
SB13	Span 11	7.2500	96.0000	72.0000	96.0000	72.0000	103.2500
SB13	Span 12	3.5964	103.2500	72.0000	103.2500	72.0000	106.8464
SB13	Span 13	3.6536	106.8464	72.0000	106.8464	72.0000	110.5000
SB14	Span 1	12.0000	0.0000	78.0000	0.0000	78.0000	12.0000

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SB14	Span 2	12.0000	12.0000	78.0000	12.0000	78.0000	24.0000
SB14	Span 3	12.0000	24.0000	78.0000	24.0000	78.0000	36.0000
SB14	Span 4	12.0000	36.0000	78.0000	36.0000	78.0000	48.0000
SB14	Span 5	11.2135	48.0000	78.0000	48.0000	78.0000	59.2135
SB14	Span 6	0.7865	59.2135	78.0000	59.2135	78.0000	60.0000
SB14	Span 7	12.0000	60.0000	78.0000	60.0000	78.0000	72.0000
SB14	Span 8	4.7685	72.0000	78.0000	72.0000	78.0000	76.7685
SB14	Span 9	7.2315	76.7685	78.0000	76.7685	78.0000	84.0000
SB14	Span 10	12.0000	84.0000	78.0000	84.0000	78.0000	96.0000
SB14	Span 11	7.2500	96.0000	78.0000	96.0000	78.0000	103.2500
SB14	Span 12	3.5964	103.2500	78.0000	103.2500	78.0000	106.8464
SB14	Span 13	3.6536	106.8464	78.0000	106.8464	78.0000	110.5000
SB15	Span 1	12.0000	0.0000	84.0000	0.0000	84.0000	12.0000
SB15	Span 2	12.0000	12.0000	84.0000	12.0000	84.0000	24.0000
SB15	Span 3	12.0000	24.0000	84.0000	24.0000	84.0000	36.0000
SB15	Span 4	12.0000	36.0000	84.0000	36.0000	84.0000	48.0000
SB15	Span 5	12.0000	48.0000	84.0000	48.0000	84.0000	60.0000
SB15	Span 6	12.0000	60.0000	84.0000	60.0000	84.0000	72.0000
SB15	Span 7	1.3457	72.0000	84.0000	72.0000	84.0000	73.3457
SB15	Span 8	3.4175	73.3457	84.0000	73.3457	84.0000	76.7633
SB15	Span 9	7.2367	76.7633	84.0000	76.7633	84.0000	84.0000
SB15	Span 10	12.0000	84.0000	84.0000	84.0000	84.0000	96.0000
SB15	Span 11	7.2500	96.0000	84.0000	96.0000	84.0000	103.2500
SB15	Span 12	3.5964	103.2500	84.0000	103.2500	84.0000	106.8464
SB15	Span 13	3.6536	106.8464	84.0000	106.8464	84.0000	110.5000
SB16	Span 1	12.0000	0.0000	90.0000	0.0000	90.0000	12.0000
SB16	Span 2	12.0000	12.0000	90.0000	12.0000	90.0000	24.0000
SB16	Span 3	12.0000	24.0000	90.0000	24.0000	90.0000	36.0000
SB16	Span 4	12.0000	36.0000	90.0000	36.0000	90.0000	48.0000
SB16	Span 5	12.0000	48.0000	90.0000	48.0000	90.0000	60.0000
SB16	Span 6	12.0000	60.0000	90.0000	60.0000	90.0000	72.0000
SB16	Span 7	4.7631	72.0000	90.0000	72.0000	90.0000	76.7631
SB16	Span 8	7.2369	76.7631	90.0000	76.7631	90.0000	84.0000
SB16	Span 9	12.0000	84.0000	90.0000	84.0000	90.0000	96.0000
SB16	Span 10	7.2500	96.0000	90.0000	96.0000	90.0000	103.2500
SB16	Span 11	3.5964	103.2500	90.0000	103.2500	90.0000	106.8464
SB16	Span 12	3.6536	106.8464	90.0000	106.8464	90.0000	110.5000
SB17	Span 1	12.0000	0.0000	96.0000	0.0000	96.0000	12.0000
SB17	Span 2	12.0000	12.0000	96.0000	12.0000	96.0000	24.0000
SB17	Span 3	12.0000	24.0000	96.0000	24.0000	96.0000	36.0000
SB17	Span 4	12.0000	36.0000	96.0000	36.0000	96.0000	48.0000
SB17	Span 5	12.0000	48.0000	96.0000	48.0000	96.0000	60.0000
SB17	Span 6	12.0000	60.0000	96.0000	60.0000	96.0000	72.0000
SB17	Span 7	12.0000	72.0000	96.0000	72.0000	96.0000	84.0000
SB17	Span 8	12.0000	84.0000	96.0000	84.0000	96.0000	96.0000
SB17	Span 9	7.2500	96.0000	96.0000	96.0000	96.0000	103.2500
SB17	Span 10	7.2500	103.2500	96.0000	103.2500	96.0000	110.5000
SB18	Span 1	12.0000	0.0000	101.7500	0.0000	101.7500	12.0000
SB18	Span 2	12.0000	12.0000	101.7500	12.0000	101.7500	24.0000
SB18	Span 3	12.0000	24.0000	101.7500	24.0000	101.7500	36.0000
SB18	Span 4	12.0000	36.0000	101.7500	36.0000	101.7500	48.0000
SB18	Span 5	12.0000	48.0000	101.7500	48.0000	101.7500	60.0000
SB18	Span 6	12.0000	60.0000	101.7500	60.0000	101.7500	72.0000
SB18	Span 7	4.7631	72.0000	101.7500	72.0000	101.7500	76.7631

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SB18	Span 8	1.2500	76.7631	101.7500	76.7631	101.7500	78.0131
SB18	Span 9	0.4186	78.0131	101.7500	78.0131	101.7500	78.4317
SB18	Span 10	5.5683	78.4317	101.7500	78.4317	101.7500	84.0000
SB18	Span 11	5.4279	84.0000	101.7500	84.0000	101.7500	89.4279
SB18	Span 12	0.4981	89.4279	101.7500	89.4279	101.7500	89.9259
SB18	Span 13	6.0741	89.9259	101.7500	89.9259	101.7500	96.0000
SB18	Span 14	7.2500	96.0000	101.7500	96.0000	101.7500	103.2500
SB18	Span 15	3.5964	103.2500	101.7500	103.2500	101.7500	106.8464
SB18	Span 16	3.6536	106.8464	101.7500	106.8464	101.7500	110.5000
SB19	Span 1	12.0000	0.0000	107.5000	0.0000	107.5000	12.0000
SB19	Span 2	12.0000	12.0000	107.5000	12.0000	107.5000	24.0000
SB19	Span 3	12.0000	24.0000	107.5000	24.0000	107.5000	36.0000
SB19	Span 4	12.0000	36.0000	107.5000	36.0000	107.5000	48.0000
SB19	Span 5	12.0000	48.0000	107.5000	48.0000	107.5000	60.0000
SB19	Span 6	12.0000	60.0000	107.5000	60.0000	107.5000	72.0000
SB19	Span 7	12.0000	72.0000	107.5000	72.0000	107.5000	84.0000
SB19	Span 8	12.0000	84.0000	107.5000	84.0000	107.5000	96.0000
SB19	Span 9	7.2500	96.0000	107.5000	96.0000	107.5000	103.2500
SB19	Span 10	7.2500	103.2500	107.5000	103.2500	107.5000	110.5000

2. Model properties

This section provides model properties, including items such as material properties, section properties, and support properties.

2.1. Material properties

Table 2: Material Properties 03 - Concrete

Table 2: Material Properties 03 - Concrete

Material	E	U	A	UnitWt	Fc	LtWtConc	UserModRup
	kip/in2		1/F	lb/ft3	kip/in2		
4000Psi	3604.997	0.200000	5.5000E-06	1.5000E+02	4.000	No	No
5000Psi	4030.509	0.200000	5.5000E-06	1.5000E+02	5.000	No	No

Table 3: Material Properties 04 - Rebar

Table 3: Material Properties 04 - Rebar

Material	E	UnitWt	Fy	Fu
	kip/in2	lb/ft3	kip/in2	kip/in2
A615Gr60	29000.000	4.9000E+02	60.000	90.000

Table 4: Material Properties 05 - Tendon

Table 4: Material Properties 05 - Tendon

Material	E	UnitWt	Fy	Fu
	kip/in2	lb/ft3	kip/in2	kip/in2
A416Gr270	28500.000	4.9000E+02	245.100	270.000

Table 5: Material Properties 02 - Steel

Table 5: Material Properties 02 - Steel

Material	E	U	A	UnitWt	Fy	Fu
	kip/in2		1/F	lb/ft3	kip/in2	kip/in2
A500GrB46	29000.000	0.300000	6.5000E-06	4.9000E+02	46.000	58.000

Table 6: Material Properties 06 - Other

Table 6: Material Properties 06 - Other

Material	E	U	A	UnitWt
	kip/in2		1/F	lb/ft3
CMU	1500.000	0.200000	4.5000E-06	1.3000E+02

2.2. Section properties

Table 7: Slab Properties 02 - Solid Slabs

Table 7: Slab Properties 02 - Solid Slabs

Slab	Type	MatProp	Thickness	Ortho
			in	
9" SLAB	Slab	5000Psi	9.0000	No

Table 8: Beam Properties 02 - Rectangular Beam

Table 8: Beam Properties 02 - Rectangular Beam

Beam	MatProp	Depth	WidthTop	WidthBot
		in	in	in
RB-1	5000Psi	30.0000	12.0000	12.0000
RB-2	5000Psi	30.0000	8.0000	8.0000
RB-3	5000Psi	42.0000	12.0000	12.0000
RB-4	5000Psi	30.0000	8.0000	8.0000
RB-5	5000Psi	18.0000	8.0000	8.0000
RB-6	5000Psi	18.0000	8.0000	8.0000
RB-7	5000Psi	42.0000	16.0000	16.0000

Table 9: Beam Properties 06 - Design Data

Table 9: Beam Properties 06 - Design Data

Beam	MatRebarL	MatRebarS	FlngWOpt	CoverTop	CoverBot	NoDesign
				in	in	
RB-1	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
RB-2	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
RB-3	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
RB-4	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
RB-5	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
RB-6	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
RB-7	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No

Table 10: Tendon Properties

Table 10: Tendon Properties

TendonProp	MatProp	StrandArea
		in2
TENDON1	A416Gr270	0.15

Table 11: Column Properties 02 - Rectangular

Table 11: Column Properties 02 - Rectangular

Column	MatProp	SecDim2	SecDim3	AutoRigid	AutoDrop	IncludeCap
		in	in			
HSS-1	A500GrB46	6.0000	6.0000	No	No	No
HSS-2	A500GrB46	6.0000	3.0000	No	No	No
TC-1	5000Psi	8.0000	12.0000	No	No	No
TC-2	5000Psi	8.0000	28.0000	No	No	No
TC-3	5000Psi	8.0000	34.0000	No	No	No
TC-4	5000Psi	8.0000	42.0000	No	No	No

Table 12: Wall Properties

Table 12: Wall Properties

Wall	MatProp	Thickness	AutoRigid	OutOfPlane
		in		
8" CMU	CMU	8.0000	No	No

2.3. Support properties

Table 13: Soil Properties

Table 13: Soil Properties

Soil	Subgrade	NonlinOpt
	lb/ft3	
SOIL1	1.7280E+05	Compression Only

Table 14: Spring Properties - Point

Table 14: Spring Properties - Point

Spring	Ux	Uy	Uz	Rx	Ry	Rz	NonlinOpt
	kip/in	kip/in	kip/in	kip-in/rad	kip-in/rad	kip-in/rad	
PSPR1	0.0000	0.0000	0.0010	0.000	0.000	0.000	None (Linear)

Table 15: Spring Properties - Line

Table 15: Spring Properties - Line

Spring	VertStiff	RotStiff	NonlinOpt
	kip/in/in	kip/rad	
LSPR1	0.00100	0.0010	None (Linear)

3. Model assignments

This section provides model assignments, including assignments to slabs, beams, and joints.

3.1. Slab assignments

Table 16: Slab Property Assignments

Table 16: Slab Property Assignments

Area	SlabProp	OpeningType
8	9" SLAB	None
23	None	Unloaded

Table 17: Slab Edge Releases

Table 17: Slab Edge Releases

Area	Edge	Shear	Moment
8	1	No	Yes
8	2	No	Yes
8	3	No	Yes
8	4	No	Yes
8	5	No	Yes
8	6	No	Yes

3.2. Beam assignments

Table 18: Beam Property Assignments

Table 18: Beam Property Assignments

Line	BeamProp
25	RB-1
26	RB-2
27	RB-3
28	RB-3
33	RB-4
34	RB-4
35	RB-4
36	RB-4
37	RB-5
38	RB-5
39	RB-6
40	RB-7

3.3. Column assignments

Table 19: Column Property Assignments

Table 19: Column Property Assignments

Line	ColProp
2	TC-1
3	TC-1
4	TC-1
5	TC-1
6	TC-1
7	TC-1
10	TC-2
14	TC-2
16	TC-4
17	TC-3
18	TC-1
21	TC-1
23	TC-4
24	TC-1
30	HSS-1
31	HSS-2
32	HSS-2

3.4. Wall assignments

Table 20: Wall Property Assignments

Table 20: Wall Property Assignments

Area	WallProp
1	8" CMU
2	8" CMU
3	8" CMU
4	8" CMU
5	8" CMU
6	8" CMU
7	8" CMU
9	8" CMU
10	8" CMU
11	8" CMU
12	8" CMU
13	8" CMU
14	8" CMU
15	8" CMU
16	8" CMU
18	8" CMU

3.5. Support assignments

Table 21: Point Restraint Assignments

Table 21: Point Restraint Assignments

Point	Ux	Uy	Uz	Rx	Ry	Rz
3	Yes	Yes	Yes	Yes	Yes	Yes
5	Yes	Yes	Yes	Yes	Yes	Yes
7	Yes	Yes	Yes	Yes	Yes	Yes
9	Yes	Yes	Yes	Yes	Yes	Yes
11	Yes	Yes	Yes	Yes	Yes	Yes
13	Yes	Yes	Yes	Yes	Yes	Yes
19	Yes	Yes	Yes	Yes	Yes	Yes
27	Yes	Yes	Yes	Yes	Yes	Yes
31	Yes	Yes	Yes	Yes	Yes	Yes
33	Yes	Yes	Yes	Yes	Yes	Yes
35	Yes	Yes	Yes	Yes	Yes	Yes
41	Yes	Yes	Yes	Yes	Yes	Yes
45	Yes	Yes	Yes	Yes	Yes	Yes
47	Yes	Yes	Yes	Yes	Yes	Yes
49	Yes	Yes	Yes	Yes	Yes	Yes
50	Yes	Yes	Yes	Yes	Yes	Yes
53	Yes	Yes	Yes	Yes	Yes	Yes
55	Yes	Yes	Yes	Yes	Yes	Yes
56	Yes	Yes	Yes	Yes	Yes	Yes
59	Yes	Yes	Yes	Yes	Yes	Yes
60	Yes	Yes	Yes	Yes	Yes	Yes
63	Yes	Yes	Yes	Yes	Yes	Yes
65	Yes	Yes	Yes	Yes	Yes	Yes
66	Yes	Yes	Yes	Yes	Yes	Yes
69	Yes	Yes	Yes	Yes	Yes	Yes
75	Yes	Yes	Yes	Yes	Yes	Yes
76	Yes	Yes	Yes	Yes	Yes	Yes
79	Yes	Yes	Yes	Yes	Yes	Yes
80	Yes	Yes	Yes	Yes	Yes	Yes
83	Yes	Yes	Yes	Yes	Yes	Yes
84	Yes	Yes	Yes	Yes	Yes	Yes
87	Yes	Yes	Yes	Yes	Yes	Yes
89	Yes	Yes	Yes	Yes	Yes	Yes
91	Yes	Yes	Yes	Yes	Yes	Yes
93	Yes	Yes	Yes	Yes	Yes	Yes
94	Yes	Yes	Yes	Yes	Yes	Yes
97	Yes	Yes	Yes	Yes	Yes	Yes
101	Yes	Yes	Yes	Yes	Yes	Yes
108	Yes	Yes	Yes	Yes	Yes	Yes
110	Yes	Yes	Yes	Yes	Yes	Yes
112	Yes	Yes	Yes	Yes	Yes	Yes

4. Model loading

This section provides model loading information, including load patterns, load cases, and load combinations.

4.1. Load patterns

Table 22: Load Patterns

Table 22: Load Patterns

LoadPat	Type	SelfWtMult
DEAD	DEAD	1.000000
LIVE	LIVE	0.000000
SDL	SUPER DEAD	0.000000

Table 23: Load Assignments - Surface Loads

Table 23: Load Assignments - Surface Loads

Area	LoadPat	Dir	UnifLoad	A	B	C
			lb/ft2	lb/ft3	lb/ft3	lb/ft2
8	SDL	Gravity	25.00	0.0000E+00	0.0000E+00	0.00
8	LIVE	Gravity	30.00	0.0000E+00	0.0000E+00	0.00

Table 24: Load Assignments - Line Objects - Distributed Loads, Part 1 of 2

Table 24: Load Assignments - Line Objects - Distributed Loads, Part 1 of 2

Line	LoadPat	Type	Dir	RelDistA	RelDistB	AbsDistA	AbsDistB
						ft	ft
1	DEAD	Force	Gravity	0.0000	1.0000	0.0000	59.9998
8	DEAD	Force	Gravity	0.0000	1.0000	0.0000	36.3333
9	DEAD	Force	Gravity	0.0000	1.0000	0.0000	19.7443
11	DEAD	Force	Gravity	0.0000	1.0000	0.0000	79.7570
12	DEAD	Force	Gravity	0.0000	1.0000	0.0000	55.6780
13	DEAD	Force	Gravity	0.0000	1.0000	0.0000	119.2281

Table 24: Load Assignments - Line Objects - Distributed Loads, Part 2 of 2

Table 24: Load Assignments - Line Objects - Distributed Loads, Part 2 of 2

Line	LoadPat	FOverLA	FOverLB
		kip/ft	kip/ft
1	DEAD	0.121	0.121
8	DEAD	0.121	0.121
9	DEAD	0.121	0.121
11	DEAD	0.121	0.121
12	DEAD	0.121	0.121
13	DEAD	0.121	0.121

4.2. Load cases

Table 25: Load Cases 02 - Static

Table 25: Load Cases 02 - Static

LoadCase	InitialCond	AType	RelForceTol	CreepCoeff	ShrinkStrn
DEAD	Zero	Linear			
LIVE	Zero	Linear			
SDL	Zero	Linear			
CASE-1	Zero	Cracked	0.000010		
CASE-2	Zero	Cracked	0.000010		
CASE-3	Zero	Long Term Cracked	0.000010	2.000000	0.000500
CASE-4	Zero	Long Term Cracked	0.000010	1.000000	0.000500
CASE-5	Zero	Long Term Cracked	0.000010	2.000000	0.000500

Table 26: Load Cases 06 - Loads Applied

Table 26: Load Cases 06 - Loads Applied

LoadCase	LoadPat	SF
DEAD	DEAD	1.000000
LIVE	LIVE	1.000000
SDL	SDL	1.000000
CASE-1	DEAD	1.000000
CASE-1	SDL	1.000000
CASE-1	LIVE	1.000000
CASE-2	DEAD	1.000000
CASE-2	SDL	1.000000
CASE-2	LIVE	0.250000
CASE-3	DEAD	1.000000
CASE-3	SDL	1.000000
CASE-3	LIVE	0.250000
CASE-4	DEAD	1.000000
CASE-4	SDL	0.300000
CASE-5	DEAD	1.000000
CASE-5	SDL	1.000000

4.3. Load combinations

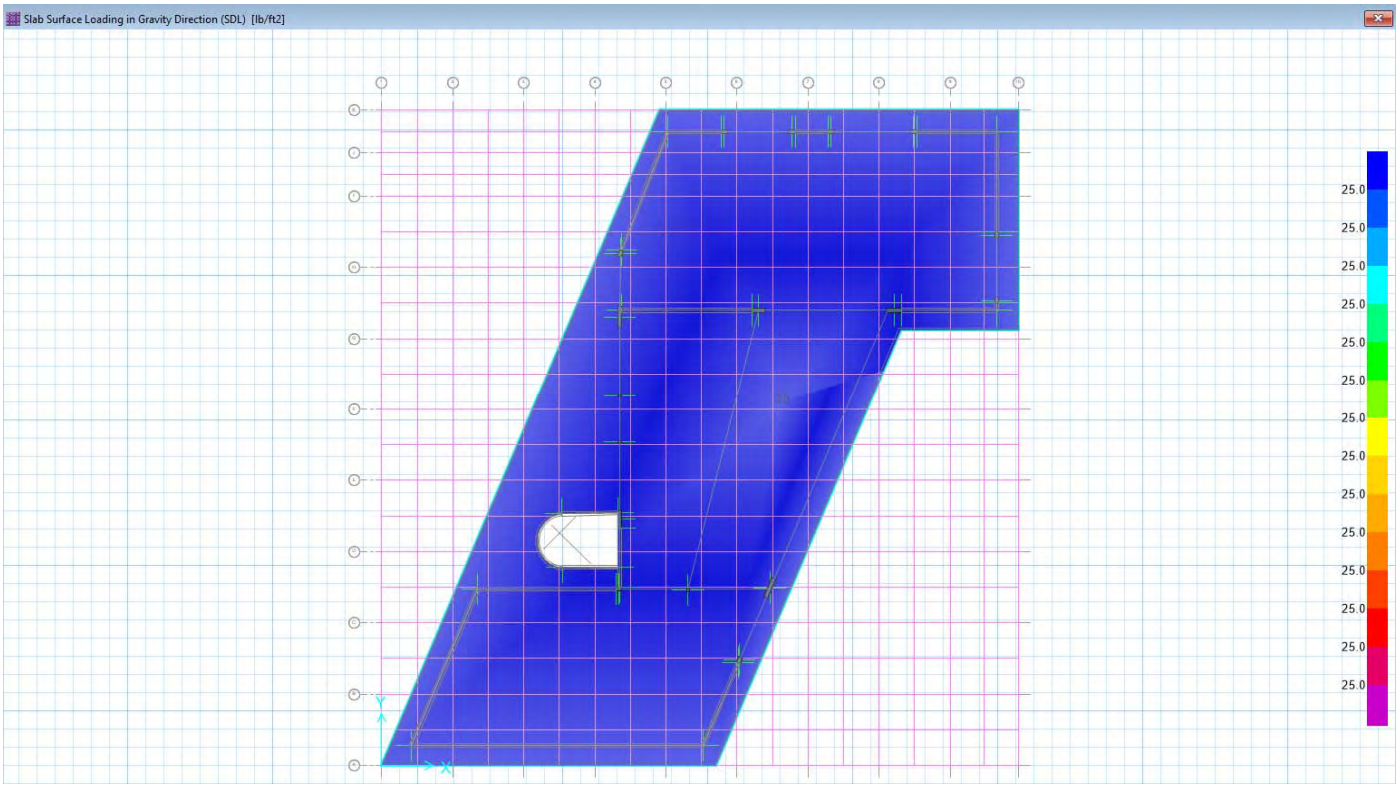
Table 27: Load Combinations

Table 27: Load Combinations

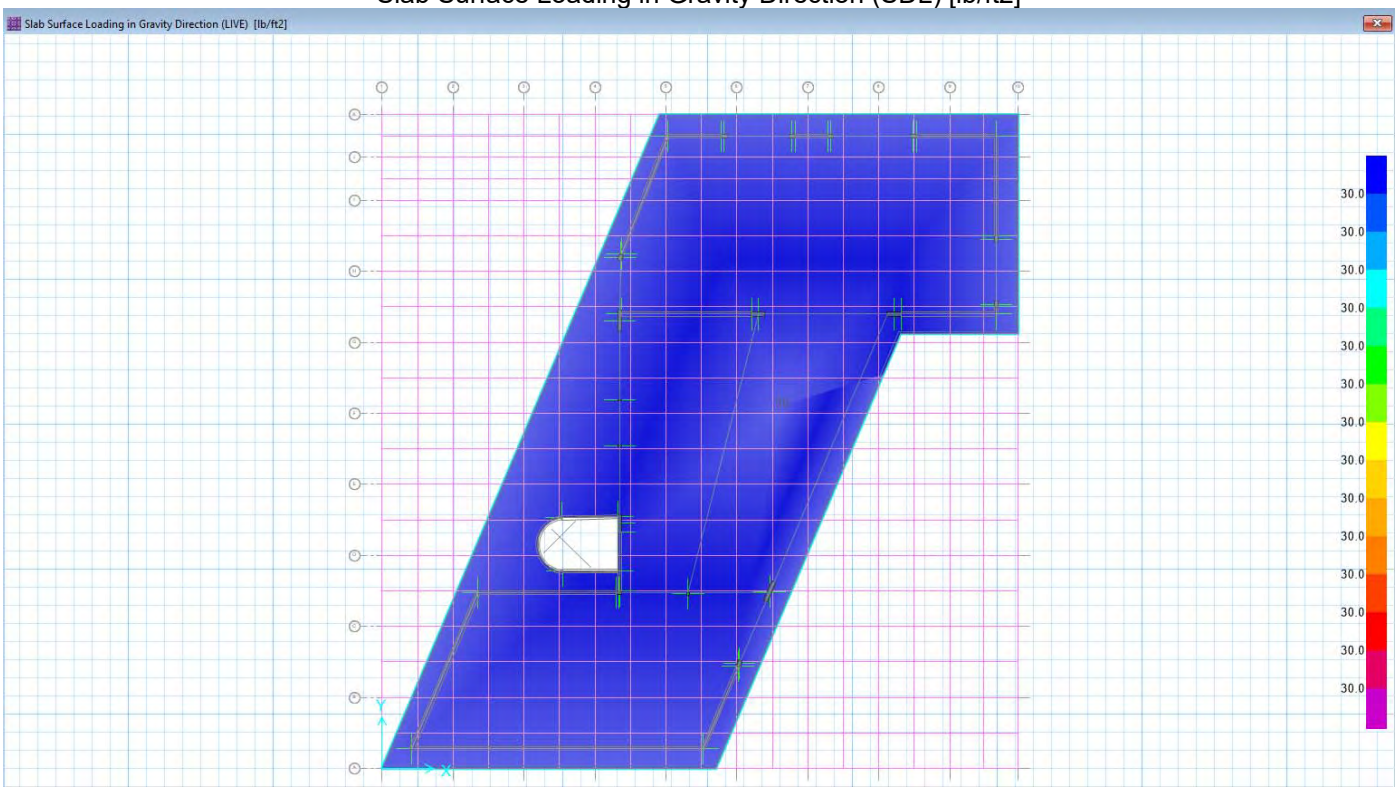
Combo	Load	SF	Type	DSStrength	DSServInit	DSServNorm	DSServLong
LONG TERM (ACI L/240 LIMIT)	CASE-3	1.000000	Linear Add	No	No	No	Yes

Table 27: Load Combinations

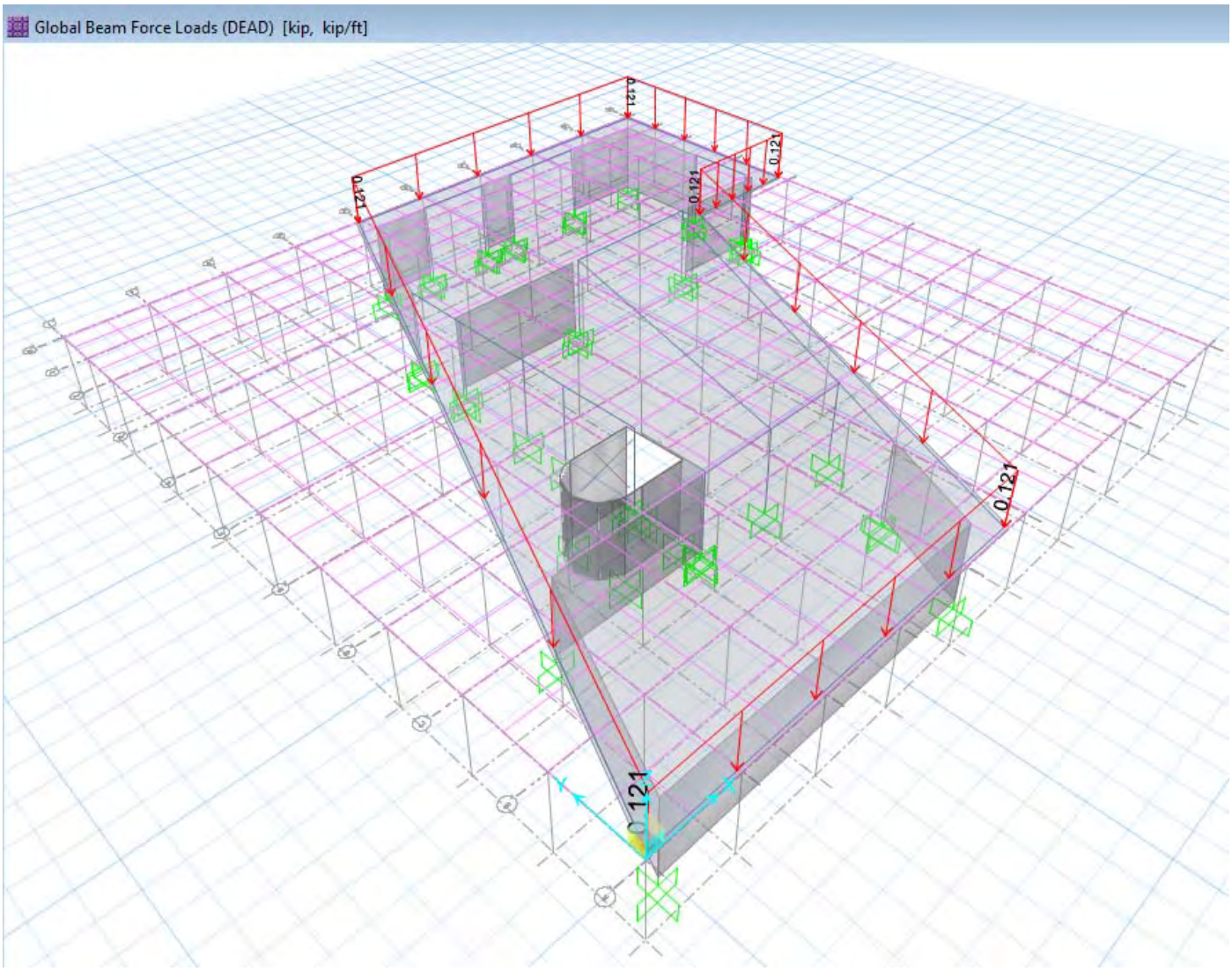
Combo	Load	SF	Type	DSStrength	DSServInit	DSServNorm	DSServLong
LONG TERM (ACI L/240 LIMIT)	CASE-1	1.000000					
LONG TERM (ACI L/240 LIMIT)	CASE-2	-1.000000					
LONG TERM (ACI L/240 LIMIT)	CASE-4	-1.000000					
LONG TERM (ACI LIVE L/360 LIMIT)	CASE-3	1.000000	Linear Add	No	No	No	Yes
LONG TERM (ACI LIVE L/360 LIMIT)	CASE-1	1.000000					
LONG TERM (ACI LIVE L/360 LIMIT)	CASE-2	-1.000000					
LONG TERM (ACI LIVE L/360 LIMIT)	CASE-5	-1.000000					
LONG TERM-ALL	CASE-3	1.000000	Linear Add	No	No	No	Yes
LONG TERM-ALL	CASE-1	1.000000					
LONG TERM-ALL	CASE-2	-1.000000					
UDCONN1	DEAD	1.000000	Linear Add	No	No	Yes	No
UDCONN1	SDL	1.000000					
UDCONN2	DEAD	1.000000	Linear Add	No	No	Yes	No
UDCONN2	LIVE	1.000000					
UDCONN2	SDL	1.000000					
UDCONU1	DEAD	1.400000	Linear Add	Yes	No	No	No
UDCONU1	SDL	1.400000					
UDCONU2	DEAD	1.200000	Linear Add	Yes	No	No	No
UDCONU2	LIVE	1.600000					
UDCONU2	SDL	1.200000					



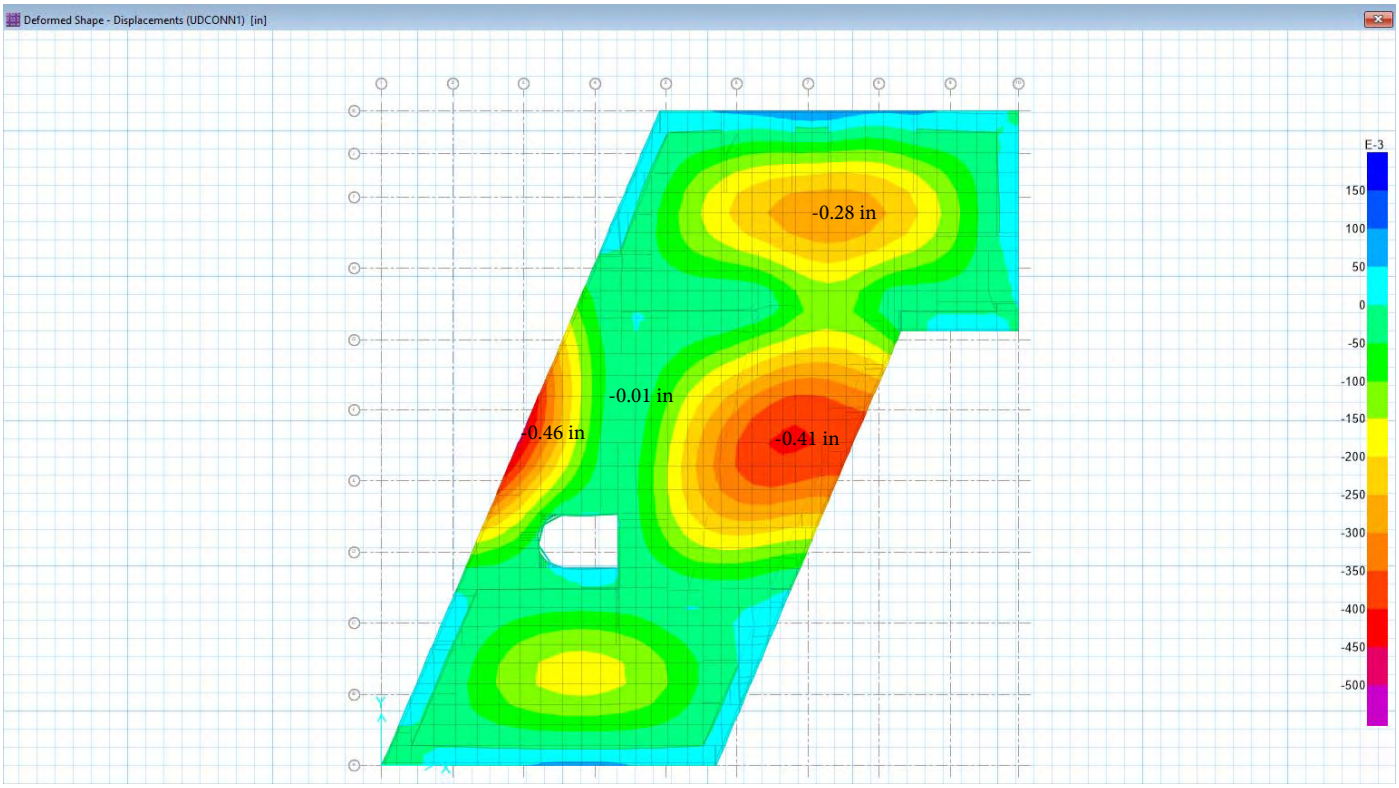
Slab Surface Loading in Gravity Direction (SDL) [lb/ft2]



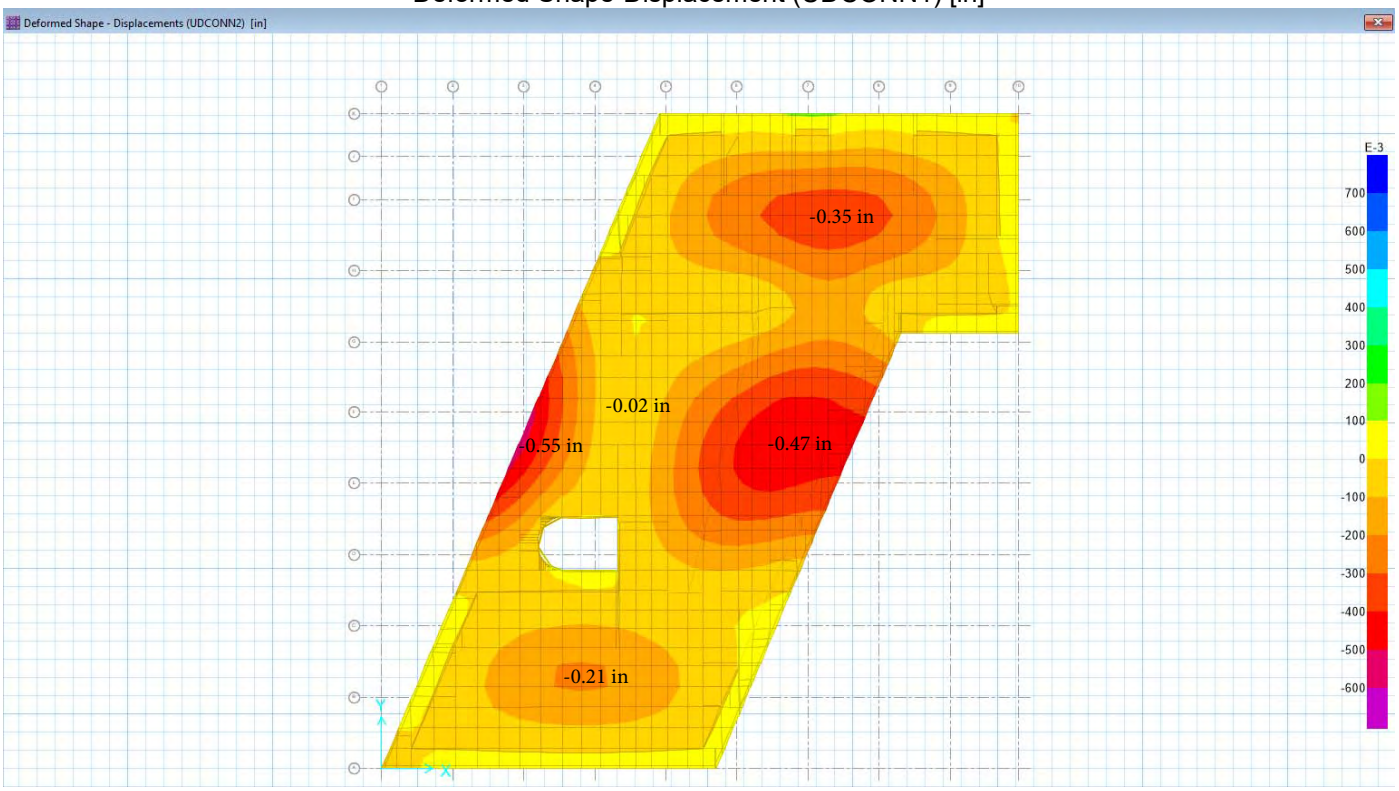
Slab Surface Loading in Gravity Direction (LIVE) [lb/ft2]


Global Beam Force Loads (DEAD) [kip, kip/ft]
0.121
0.121
0.121
0.121
0.121
0.121

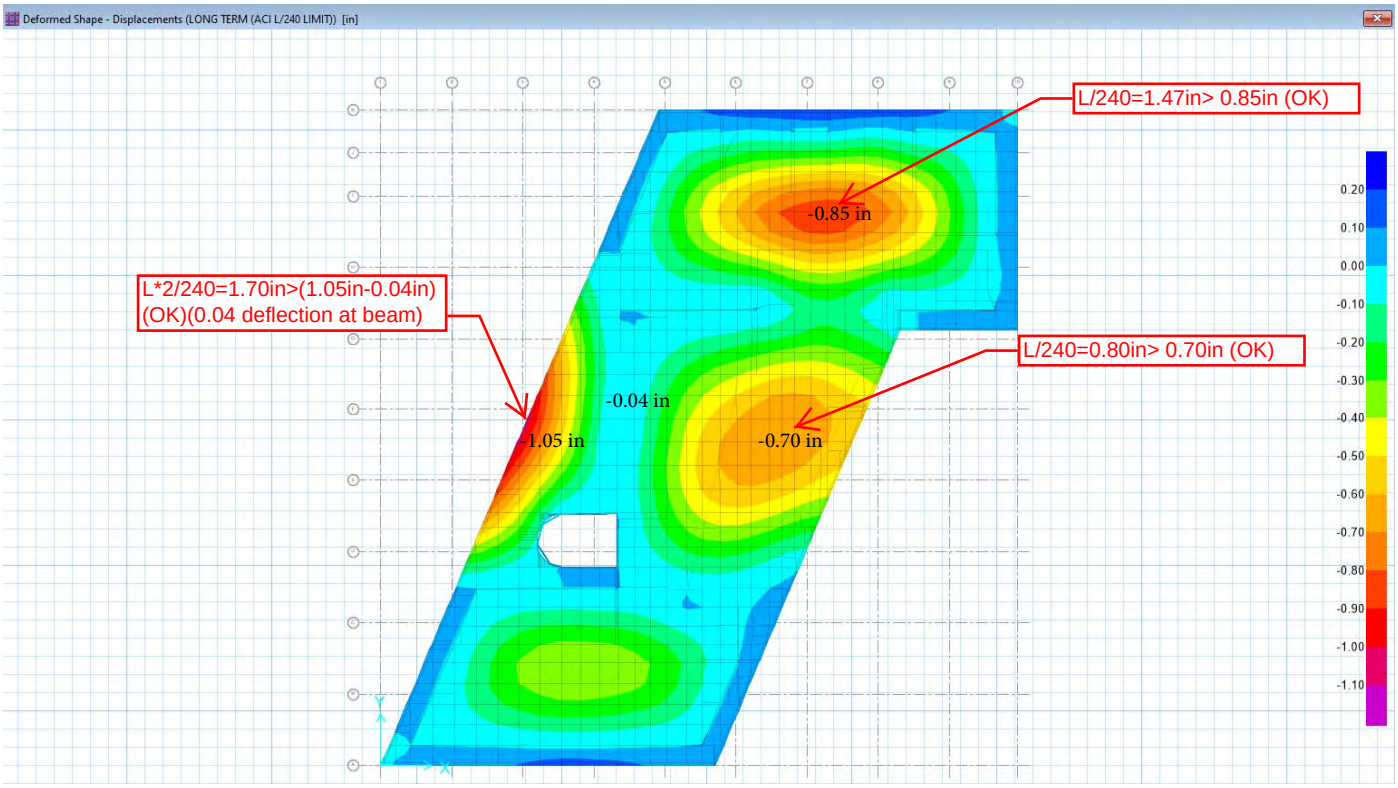
Analysis Results



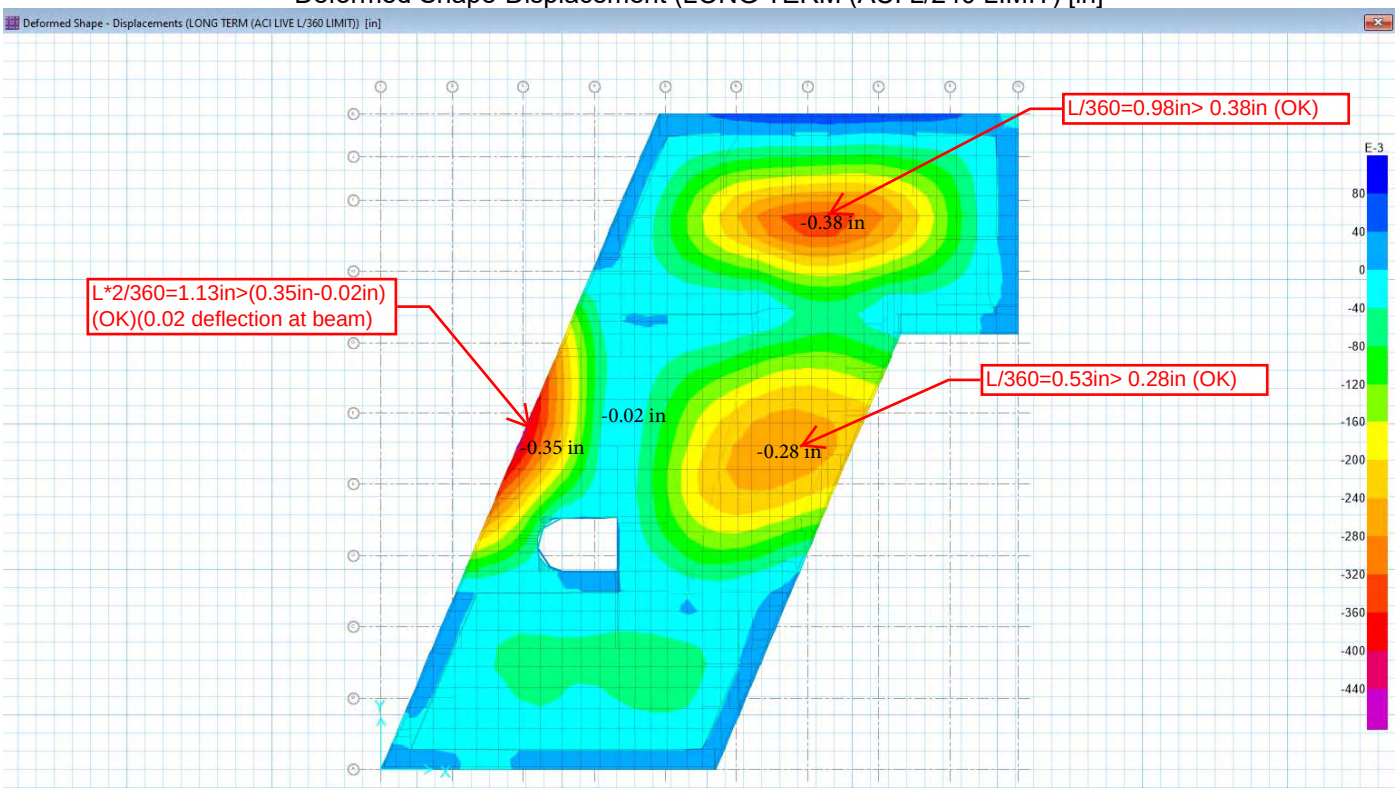
Deformed Shape-Displacement (UDCONN1) [in]



Deformed Shape-Displacement (UDCONN2) [in]



Deformed Shape-Displacement (LONG TERM (ACI L/240 LIMIT) [in]



Deformed Shape-Displacement (LONG TERM (ACI LIVE L/360 LIMIT) [in]

Figure 2: Deformed shape

5. Analysis results

5.1. Support results

This section provides support results, including items such as column, support, and spring reactions, .

Table 28: Nodal Reactions

Table 28: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
3	CASE-1	0.339	-1.746	17.857	6.9650	1.3337	3.920E-004
3	CASE-1	0.339	-1.746	17.857	6.9650	1.3337	3.920E-004
5	CASE-1	-0.317	-2.557	17.905	10.1906	-1.2583	0.0036
5	CASE-1	-0.317	-2.557	17.905	10.1906	-1.2583	0.0036
7	CASE-1	0.544	-2.643	22.410	10.5313	2.1472	0.0021
7	CASE-1	0.544	-2.643	22.410	10.5313	2.1472	0.0021
9	CASE-1	-0.563	-1.872	20.765	7.4528	-2.2318	0.0042
9	CASE-1	-0.563	-1.872	20.765	7.4528	-2.2318	0.0042
11	CASE-1	-0.161	0.227	7.024	-0.9223	-0.6235	0.0063
11	CASE-1	-0.161	0.227	7.024	-0.9223	-0.6235	0.0063
13	CASE-1	-1.106	-0.170	16.029	0.6490	-4.3948	0.0042
13	CASE-1	-1.106	-0.170	16.029	0.6490	-4.3948	0.0042
19	CASE-1	-19.868	-4.320	96.984	17.2293	-74.3856	0.0468
19	CASE-1	-19.868	-4.320	96.984	17.2293	-74.3856	0.0468
27	CASE-1	12.778	-6.497	107.558	25.9156	48.1982	0.0083
27	CASE-1	12.778	-6.497	107.558	25.9156	48.1982	0.0083
31	CASE-1	-1.263	-2.867	40.573	12.4451	-5.0061	0.0041
31	CASE-1	-1.263	-2.867	40.573	12.4451	-5.0061	0.0041
33	CASE-1	3.021	5.350	30.991	-17.9016	12.0405	0.0013
33	CASE-1	3.021	5.350	30.991	-17.9016	12.0405	0.0013
35	CASE-1	0.198	-0.456	12.078	1.8466	0.7882	-0.0048
35	CASE-1	0.198	-0.456	12.078	1.8466	0.7882	-0.0048
41	CASE-1	-0.561	-0.139	0.072	0.6485	-2.2604	-0.0016
41	CASE-1	-0.561	-0.139	0.072	0.6485	-2.2604	-0.0016
45	CASE-1	18.096	48.275	120.860	-164.0173	60.1763	-0.0189
45	CASE-1	18.096	48.275	120.860	-164.0173	60.1763	-0.0189
47	CASE-1	0.147	-0.146	8.677	0.6245	0.5799	0.0011
47	CASE-1	0.147	-0.146	8.677	0.6245	0.5799	0.0011
49	CASE-1	0.280	0.715	4.341	0.0837	-0.0309	-2.301E-004
49	CASE-1	0.280	0.715	4.341	0.0837	-0.0309	-2.301E-004
50	CASE-1	1.968	-3.604	15.216	1.6860	0.8156	-4.087E-007
50	CASE-1	1.968	-3.604	15.216	1.6860	0.8156	-4.087E-007
53	CASE-1	0.981	-1.424E-003	3.672	0.0033	0.3822	5.220E-004
53	CASE-1	0.981	-1.424E-003	3.672	0.0033	0.3822	5.220E-004
55	CASE-1	1.108	-1.285E-003	7.522	0.0057	0.0740	-8.960E-004
55	CASE-1	1.108	-1.285E-003	7.522	0.0057	0.0740	-8.960E-004
56	CASE-1	-0.603	-1.324E-003	6.837	0.0057	0.2156	9.020E-004
56	CASE-1	-0.603	-1.324E-003	6.837	0.0057	0.2156	9.020E-004
59	CASE-1	-0.054	-1.425E-003	5.514	0.0042	-0.6084	-5.849E-004
59	CASE-1	-0.054	-1.425E-003	5.514	0.0042	-0.6084	-5.849E-004
60	CASE-1	-2.070	-1.939	10.495	0.6283	-0.6792	1.065E-005
60	CASE-1	-2.070	-1.939	10.495	0.6283	-0.6792	1.065E-005
63	CASE-1	-1.357E-003	1.287	8.445	0.0374	-0.0039	5.316E-004
63	CASE-1	-1.357E-003	1.287	8.445	0.0374	-0.0039	5.316E-004

Table 28: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
65	CASE-1	2.370E-004	-0.152	0.301	-0.0623	-9.230E-005	6.418E-006
65	CASE-1	2.370E-004	-0.152	0.301	-0.0623	-9.230E-005	6.418E-006
66	CASE-1	-1.283	0.321	5.114	-0.0150	-0.4253	-1.720E-005
66	CASE-1	-1.283	0.321	5.114	-0.0150	-0.4253	-1.720E-005
69	CASE-1	1.203	-6.470E-004	6.133	0.0013	-0.0227	-2.287E-004
69	CASE-1	1.203	-6.470E-004	6.133	0.0013	-0.0227	-2.287E-004
75	CASE-1	1.148	1.141E-004	4.117	1.141E-004	-0.0115	-1.708E-005
75	CASE-1	1.148	1.141E-004	4.117	1.141E-004	-0.0115	-1.708E-005
76	CASE-1	-1.096	-1.210E-003	6.874	0.0034	0.3334	6.311E-004
76	CASE-1	-1.096	-1.210E-003	6.874	0.0034	0.3334	6.311E-004
79	CASE-1	6.812E-004	-1.602	6.349	-0.2620	0.0028	3.699E-004
79	CASE-1	6.812E-004	-1.602	6.349	-0.2620	0.0028	3.699E-004
80	CASE-1	-7.964E-005	-0.832	1.499	1.0900	0.0010	-1.582E-004
80	CASE-1	-7.964E-005	-0.832	1.499	1.0900	0.0010	-1.582E-004
83	CASE-1	-1.579	-3.208E-004	6.566	0.0013	-0.0300	2.299E-004
83	CASE-1	-1.579	-3.208E-004	6.566	0.0013	-0.0300	2.299E-004
84	CASE-1	1.560	-1.336	17.289	-0.1592	0.3233	5.176E-007
84	CASE-1	1.560	-1.336	17.289	-0.1592	0.3233	5.176E-007
87	CASE-1	1.139	0.538	7.774	0.0818	-0.0490	-5.947E-006
87	CASE-1	1.139	0.538	7.774	0.0818	-0.0490	-5.947E-006
89	CASE-1	-1.677	0.534	7.830	-0.5068	-0.3797	7.445E-006
89	CASE-1	-1.677	0.534	7.830	-0.5068	-0.3797	7.445E-006
91	CASE-1	-1.260	-2.968	10.521	0.8742	-0.3732	-2.355E-004
91	CASE-1	-1.260	-2.968	10.521	0.8742	-0.3732	-2.355E-004
93	CASE-1	-0.966	-0.017	4.182	-0.0573	3.1994	-2.394E-004
93	CASE-1	-0.966	-0.017	4.182	-0.0573	3.1994	-2.394E-004
94	CASE-1	-1.376	1.380	8.205	-1.3308	-3.4366	1.083E-004
94	CASE-1	-1.376	1.380	8.205	-1.3308	-3.4366	1.083E-004
97	CASE-1	-1.448	0.152	-0.357	-0.1730	-2.1420	-2.604E-005
97	CASE-1	-1.448	0.152	-0.357	-0.1730	-2.1420	-2.604E-005
101	CASE-1	2.257	-2.223E-003	-2.500	9.854E-004	2.0007	-1.355E-004
101	CASE-1	2.257	-2.223E-003	-2.500	9.854E-004	2.0007	-1.355E-004
108	CASE-1	0.270	3.393	92.121	-13.4731	1.0815	-0.0040
108	CASE-1	0.270	3.393	92.121	-13.4731	1.0815	-0.0040
110	CASE-1	0.291	-0.017	57.599	0.0726	1.1715	-1.570E-005
110	CASE-1	0.291	-0.017	57.599	0.0726	1.1715	-1.570E-005
112	CASE-1	-0.105	1.975E-003	48.752	-0.0025	-0.4106	2.087E-004
112	CASE-1	-0.105	1.975E-003	48.752	-0.0025	-0.4106	2.087E-004
3	CASE-2	0.293	-1.317	15.528	5.2565	1.1547	-1.150E-004
3	CASE-2	0.293	-1.317	15.528	5.2565	1.1547	-1.150E-004
5	CASE-2	-0.269	-1.944	15.788	7.7544	-1.0690	0.0024
5	CASE-2	-0.269	-1.944	15.788	7.7544	-1.0690	0.0024
7	CASE-2	0.431	-2.017	19.984	8.0412	1.6991	0.0019
7	CASE-2	0.431	-2.017	19.984	8.0412	1.6991	0.0019
9	CASE-2	-0.449	-1.428	18.159	5.6904	-1.7783	0.0034
9	CASE-2	-0.449	-1.428	18.159	5.6904	-1.7783	0.0034
11	CASE-2	-0.108	0.194	7.098	-0.7786	-0.4161	0.0048
11	CASE-2	-0.108	0.194	7.098	-0.7786	-0.4161	0.0048
13	CASE-2	-0.817	-0.153	13.494	0.5933	-3.2463	0.0034
13	CASE-2	-0.817	-0.153	13.494	0.5933	-3.2463	0.0034
19	CASE-2	-17.149	-3.921	87.192	15.6400	-64.2749	0.0409
19	CASE-2	-17.149	-3.921	87.192	15.6400	-64.2749	0.0409
27	CASE-2	11.351	-5.574	96.510	22.2352	42.7837	0.0061
27	CASE-2	11.351	-5.574	96.510	22.2352	42.7837	0.0061

Table 28: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
31	CASE-2	-1.146	-3.174	39.169	13.1438	-4.5458	5.069E-004
31	CASE-2	-1.146	-3.174	39.169	13.1438	-4.5458	5.069E-004
33	CASE-2	2.524	4.692	27.386	-15.7687	10.0559	0.0073
33	CASE-2	2.524	4.692	27.386	-15.7687	10.0559	0.0073
35	CASE-2	0.201	-0.304	11.550	1.2372	0.8000	-0.0049
35	CASE-2	0.201	-0.304	11.550	1.2372	0.8000	-0.0049
41	CASE-2	-0.427	-0.134	-0.501	0.6099	-1.7193	-9.599E-004
41	CASE-2	-0.427	-0.134	-0.501	0.6099	-1.7193	-9.599E-004
45	CASE-2	15.790	41.756	111.078	-142.1063	52.6924	-0.0223
45	CASE-2	15.790	41.756	111.078	-142.1063	52.6924	-0.0223
47	CASE-2	0.119	-0.105	7.538	0.4574	0.4665	6.751E-004
47	CASE-2	0.119	-0.105	7.538	0.4574	0.4665	6.751E-004
49	CASE-2	0.252	0.645	3.910	0.0349	-0.0124	-1.676E-004
49	CASE-2	0.252	0.645	3.910	0.0349	-0.0124	-1.676E-004
50	CASE-2	1.604	-2.941	13.689	1.2855	0.5835	-5.112E-007
50	CASE-2	1.604	-2.941	13.689	1.2855	0.5835	-5.112E-007
53	CASE-2	0.619	-1.068E-003	3.644	0.0025	0.3323	3.923E-004
53	CASE-2	0.619	-1.068E-003	3.644	0.0025	0.3323	3.923E-004
55	CASE-2	1.054	-9.771E-004	6.883	0.0043	0.1023	-6.830E-004
55	CASE-2	1.054	-9.771E-004	6.883	0.0043	0.1023	-6.830E-004
56	CASE-2	-0.511	-1.011E-003	6.139	0.0044	0.2248	6.881E-004
56	CASE-2	-0.511	-1.011E-003	6.139	0.0044	0.2248	6.881E-004
59	CASE-2	0.013	-1.088E-003	5.028	0.0032	-0.5352	-4.469E-004
59	CASE-2	0.013	-1.088E-003	5.028	0.0032	-0.5352	-4.469E-004
60	CASE-2	-1.772	-1.720	10.022	0.4958	-0.5266	9.687E-006
60	CASE-2	-1.772	-1.720	10.022	0.4958	-0.5266	9.687E-006
63	CASE-2	-1.042E-003	1.056	7.069	0.0478	-0.0030	4.030E-004
63	CASE-2	-1.042E-003	1.056	7.069	0.0478	-0.0030	4.030E-004
65	CASE-2	2.880E-004	-0.170	0.361	-0.0586	-7.386E-005	7.130E-006
65	CASE-2	2.880E-004	-0.170	0.361	-0.0586	-7.386E-005	7.130E-006
66	CASE-2	-1.143	0.349	5.106	-0.0207	-0.3130	-1.610E-005
66	CASE-2	-1.143	0.349	5.106	-0.0207	-0.3130	-1.610E-005
69	CASE-2	0.985	-5.921E-004	5.295	0.0011	-0.0515	-1.951E-004
69	CASE-2	0.985	-5.921E-004	5.295	0.0011	-0.0515	-1.951E-004
75	CASE-2	0.961	1.155E-004	3.432	9.948E-005	-0.0851	-1.412E-005
75	CASE-2	0.961	1.155E-004	3.432	9.948E-005	-0.0851	-1.412E-005
76	CASE-2	-0.968	-1.112E-003	6.144	0.0032	0.3180	5.986E-004
76	CASE-2	-0.968	-1.112E-003	6.144	0.0032	0.3180	5.986E-004
79	CASE-2	5.419E-004	-1.593	5.830	-0.1670	0.0023	3.112E-004
79	CASE-2	5.419E-004	-1.593	5.830	-0.1670	0.0023	3.112E-004
80	CASE-2	-8.668E-005	-0.737	1.616	0.9648	8.123E-004	-1.277E-004
80	CASE-2	-8.668E-005	-0.737	1.616	0.9648	8.123E-004	-1.277E-004
83	CASE-2	-1.398	-2.165E-004	5.834	9.671E-004	0.0072	1.695E-004
83	CASE-2	-1.398	-2.165E-004	5.834	9.671E-004	0.0072	1.695E-004
84	CASE-2	1.290	-1.112	15.675	-0.1528	0.1259	1.567E-006
84	CASE-2	1.290	-1.112	15.675	-0.1528	0.1259	1.567E-006
87	CASE-2	1.056	0.494	7.373	0.0780	-0.0475	-5.642E-006
87	CASE-2	1.056	0.494	7.373	0.0780	-0.0475	-5.642E-006
89	CASE-2	-1.449	0.528	7.453	-0.4555	-0.3047	6.231E-006
89	CASE-2	-1.449	0.528	7.453	-0.4555	-0.3047	6.231E-006
91	CASE-2	-1.082	-2.549	8.946	0.7366	-0.3143	-1.830E-004
91	CASE-2	-1.082	-2.549	8.946	0.7366	-0.3143	-1.830E-004
93	CASE-2	-0.891	-0.015	3.896	-0.0459	2.5641	-1.937E-004
93	CASE-2	-0.891	-0.015	3.896	-0.0459	2.5641	-1.937E-004

Table 28: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
94	CASE-2	-1.153	1.034	7.421	-0.9971	-2.8844	8.253E-005
94	CASE-2	-1.153	1.034	7.421	-0.9971	-2.8844	8.253E-005
97	CASE-2	-1.195	0.136	0.053	-0.1404	-1.8128	-4.540E-006
97	CASE-2	-1.195	0.136	0.053	-0.1404	-1.8128	-4.540E-006
101	CASE-2	1.908	-1.904E-003	-1.846	9.058E-004	1.7320	-1.126E-004
101	CASE-2	1.908	-1.904E-003	-1.846	9.058E-004	1.7320	-1.126E-004
108	CASE-2	0.198	2.802	82.936	-11.1268	0.7918	-0.0031
108	CASE-2	0.198	2.802	82.936	-11.1268	0.7918	-0.0031
110	CASE-2	0.263	-0.014	50.341	0.0616	1.0572	-1.044E-005
110	CASE-2	0.263	-0.014	50.341	0.0616	1.0572	-1.044E-005
112	CASE-2	-0.042	1.545E-003	42.447	-0.0016	-0.1624	1.600E-004
112	CASE-2	-0.042	1.545E-003	42.447	-0.0016	-0.1624	1.600E-004
3	CASE-3	0.773	-3.522	19.133	14.0147	3.0328	-0.0029
3	CASE-3	0.773	-3.522	19.133	14.0147	3.0328	-0.0029
5	CASE-3	-0.823	-4.731	18.436	18.8106	-3.2597	0.0266
5	CASE-3	-0.823	-4.731	18.436	18.8106	-3.2597	0.0266
7	CASE-3	1.177	-5.041	22.732	20.0442	4.6501	-0.0116
7	CASE-3	1.177	-5.041	22.732	20.0442	4.6501	-0.0116
9	CASE-3	-1.133	-3.916	22.284	15.5655	-4.4618	0.0086
9	CASE-3	-1.133	-3.916	22.284	15.5655	-4.4618	0.0086
11	CASE-3	-0.247	0.578	10.116	-2.3417	-0.9570	0.0074
11	CASE-3	-0.247	0.578	10.116	-2.3417	-0.9570	0.0074
13	CASE-3	-2.386	-0.438	16.181	1.6776	-9.4847	-2.339E-004
13	CASE-3	-2.386	-0.438	16.181	1.6776	-9.4847	-2.339E-004
19	CASE-3	-24.046	-6.256	90.911	24.9067	-89.4353	0.1044
19	CASE-3	-24.046	-6.256	90.911	24.9067	-89.4353	0.1044
27	CASE-3	13.920	-10.089	98.492	40.1915	52.0847	0.0033
27	CASE-3	13.920	-10.089	98.492	40.1915	52.0847	0.0033
31	CASE-3	-2.238	-2.405	43.328	10.9389	-8.8763	0.0156
31	CASE-3	-2.238	-2.405	43.328	10.9389	-8.8763	0.0156
33	CASE-3	3.809	3.977	25.314	-11.7858	15.1438	-0.0803
33	CASE-3	3.809	3.977	25.314	-11.7858	15.1438	-0.0803
35	CASE-3	0.385	-0.839	13.483	3.3831	1.5152	-0.0176
35	CASE-3	0.385	-0.839	13.483	3.3831	1.5152	-0.0176
41	CASE-3	-1.117	-0.095	3.431	0.5429	-4.5023	0.0070
41	CASE-3	-1.117	-0.095	3.431	0.5429	-4.5023	0.0070
45	CASE-3	18.536	53.528	104.461	-174.3175	57.4702	-0.1494
45	CASE-3	18.536	53.528	104.461	-174.3175	57.4702	-0.1494
47	CASE-3	0.446	-0.301	9.483	1.2320	1.7691	0.0015
47	CASE-3	0.446	-0.301	9.483	1.2320	1.7691	0.0015
49	CASE-3	0.286	0.729	4.158	0.0943	-0.0325	-5.702E-004
49	CASE-3	0.286	0.729	4.158	0.0943	-0.0325	-5.702E-004
50	CASE-3	1.805	-3.239	12.045	1.9726	0.9581	1.072E-008
50	CASE-3	1.805	-3.239	12.045	1.9726	0.9581	1.072E-008
53	CASE-3	0.762	-2.958E-003	3.867	0.0067	0.2993	0.0011
53	CASE-3	0.762	-2.958E-003	3.867	0.0067	0.2993	0.0011
55	CASE-3	0.501	-2.447E-003	5.164	0.0110	0.0312	-0.0017
55	CASE-3	0.501	-2.447E-003	5.164	0.0110	0.0312	-0.0017
56	CASE-3	-0.147	-2.625E-003	4.822	0.0112	0.2773	0.0018
56	CASE-3	-0.147	-2.625E-003	4.822	0.0112	0.2773	0.0018
59	CASE-3	0.016	-3.137E-003	5.432	0.0094	-0.9267	-0.0013
59	CASE-3	0.016	-3.137E-003	5.432	0.0094	-0.9267	-0.0013
60	CASE-3	-1.465	-0.890	7.255	0.1906	-0.6675	4.312E-006
60	CASE-3	-1.465	-0.890	7.255	0.1906	-0.6675	4.312E-006

Table 28: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
63	CASE-3	-2.445E-003	1.225	7.959	0.2348	-0.0079	0.0011
63	CASE-3	-2.445E-003	1.225	7.959	0.2348	-0.0079	0.0011
65	CASE-3	2.396E-004	0.227	-0.783	-0.1348	5.029E-006	6.998E-005
65	CASE-3	2.396E-004	0.227	-0.783	-0.1348	5.029E-006	6.998E-005
66	CASE-3	-0.369	0.199	3.462	-0.0429	-0.0985	-4.464E-005
66	CASE-3	-0.369	0.199	3.462	-0.0429	-0.0985	-4.464E-005
69	CASE-3	1.402	-7.023E-004	5.869	5.964E-004	0.1624	-9.231E-005
69	CASE-3	1.402	-7.023E-004	5.869	5.964E-004	0.1624	-9.231E-005
75	CASE-3	0.440	1.237E-004	2.587	-8.188E-005	-0.4241	1.987E-005
75	CASE-3	0.440	1.237E-004	2.587	-8.188E-005	-0.4241	1.987E-005
76	CASE-3	-0.967	-2.065E-003	6.230	0.0054	0.4880	0.0010
76	CASE-3	-0.967	-2.065E-003	6.230	0.0054	0.4880	0.0010
79	CASE-3	1.267E-003	-1.517	7.146	-0.8599	0.0046	5.957E-004
79	CASE-3	1.267E-003	-1.517	7.146	-0.8599	0.0046	5.957E-004
80	CASE-3	-2.667E-004	-1.925	0.163	2.0455	0.0016	-2.412E-004
80	CASE-3	-2.667E-004	-1.925	0.163	2.0455	0.0016	-2.412E-004
83	CASE-3	-1.406	-6.581E-004	6.526	0.0029	0.3086	5.071E-004
83	CASE-3	-1.406	-6.581E-004	6.526	0.0029	0.3086	5.071E-004
84	CASE-3	1.455	-1.152	16.153	-0.1822	0.2842	5.466E-007
84	CASE-3	1.455	-1.152	16.153	-0.1822	0.2842	5.466E-007
87	CASE-3	0.602	0.254	7.148	0.2667	-0.5087	-5.307E-006
87	CASE-3	0.602	0.254	7.148	0.2667	-0.5087	-5.307E-006
89	CASE-3	-1.835	0.391	5.254	-1.0578	-0.4578	-6.637E-007
89	CASE-3	-1.835	0.391	5.254	-1.0578	-0.4578	-6.637E-007
91	CASE-3	-1.452	-3.418	12.140	0.9349	-0.4024	-6.263E-004
91	CASE-3	-1.452	-3.418	12.140	0.9349	-0.4024	-6.263E-004
93	CASE-3	0.150	3.328E-003	3.816	-0.1035	5.7637	-5.135E-004
93	CASE-3	0.150	3.328E-003	3.816	-0.1035	5.7637	-5.135E-004
94	CASE-3	2.356	5.192	5.210	-6.4279	1.0573	8.022E-005
94	CASE-3	2.356	5.192	5.210	-6.4279	1.0573	8.022E-005
97	CASE-3	-1.322	0.152	-1.304	-0.1710	-1.6259	7.298E-006
97	CASE-3	-1.322	0.152	-1.304	-0.1710	-1.6259	7.298E-006
101	CASE-3	2.235	-2.204E-003	-2.897	5.586E-004	1.6573	-1.466E-004
101	CASE-3	2.235	-2.204E-003	-2.897	5.586E-004	1.6573	-1.466E-004
108	CASE-3	0.601	6.403	84.253	-25.4307	2.3826	-0.0143
108	CASE-3	0.601	6.403	84.253	-25.4307	2.3826	-0.0143
110	CASE-3	0.220	-0.041	48.101	0.1709	0.8933	1.041E-005
110	CASE-3	0.220	-0.041	48.101	0.1709	0.8933	1.041E-005
112	CASE-3	-0.636	0.024	42.270	-0.0903	-2.5241	3.801E-004
112	CASE-3	-0.636	0.024	42.270	-0.0903	-2.5241	3.801E-004
3	CASE-4	0.501	-2.116	15.140	8.4295	1.9699	-0.0015
3	CASE-4	0.501	-2.116	15.140	8.4295	1.9699	-0.0015
5	CASE-4	-0.476	-3.113	15.204	12.3940	-1.8859	0.0077
5	CASE-4	-0.476	-3.113	15.204	12.3940	-1.8859	0.0077
7	CASE-4	0.751	-3.207	18.445	12.7685	2.9671	-0.0014
7	CASE-4	0.751	-3.207	18.445	12.7685	2.9671	-0.0014
9	CASE-4	-0.703	-2.386	17.743	9.4933	-2.7734	0.0043
9	CASE-4	-0.703	-2.386	17.743	9.4933	-2.7734	0.0043
11	CASE-4	-0.119	0.350	8.058	-1.4117	-0.4548	0.0044
11	CASE-4	-0.119	0.350	8.058	-1.4117	-0.4548	0.0044
13	CASE-4	-1.488	-0.270	12.495	1.0409	-5.9142	0.0024
13	CASE-4	-1.488	-0.270	12.495	1.0409	-5.9142	0.0024
19	CASE-4	-18.804	-5.763	78.813	22.9583	-70.1425	0.0743
19	CASE-4	-18.804	-5.763	78.813	22.9583	-70.1425	0.0743

Table 28: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
27	CASE-4	10.302	-8.553	82.734	34.0881	38.6072	1.360E-004
27	CASE-4	10.302	-8.553	82.734	34.0881	38.6072	1.360E-004
31	CASE-4	-1.301	-2.904	39.103	12.4440	-5.1567	-0.0019
31	CASE-4	-1.301	-2.904	39.103	12.4440	-5.1567	-0.0019
33	CASE-4	3.047	4.416	24.970	-14.0054	12.1242	-0.0586
33	CASE-4	3.047	4.416	24.970	-14.0054	12.1242	-0.0586
35	CASE-4	0.260	-0.494	10.746	2.0006	1.0249	-0.0107
35	CASE-4	0.260	-0.494	10.746	2.0006	1.0249	-0.0107
41	CASE-4	-0.690	-0.148	2.118	0.7141	-2.7860	0.0034
41	CASE-4	-0.690	-0.148	2.118	0.7141	-2.7860	0.0034
45	CASE-4	16.109	45.467	94.562	-149.8590	51.0079	-0.1022
45	CASE-4	16.109	45.467	94.562	-149.8590	51.0079	-0.1022
47	CASE-4	0.237	-0.223	8.032	0.9227	0.9400	6.545E-004
47	CASE-4	0.237	-0.223	8.032	0.9227	0.9400	6.545E-004
49	CASE-4	0.216	0.551	3.388	0.0670	-0.0237	-3.297E-004
49	CASE-4	0.216	0.551	3.388	0.0670	-0.0237	-3.297E-004
50	CASE-4	1.343	-2.731	11.584	1.5069	0.4995	8.203E-007
50	CASE-4	1.343	-2.731	11.584	1.5069	0.4995	8.203E-007
53	CASE-4	0.467	-1.753E-003	3.461	0.0040	0.2823	6.333E-004
53	CASE-4	0.467	-1.753E-003	3.461	0.0040	0.2823	6.333E-004
55	CASE-4	0.518	-1.598E-003	4.961	0.0071	-0.0071	-0.0011
55	CASE-4	0.518	-1.598E-003	4.961	0.0071	-0.0071	-0.0011
56	CASE-4	-0.260	-1.648E-003	4.643	0.0071	0.1840	0.0011
56	CASE-4	-0.260	-1.648E-003	4.643	0.0071	0.1840	0.0011
59	CASE-4	0.027	-1.879E-003	4.625	0.0056	-0.6869	-7.761E-004
59	CASE-4	0.027	-1.879E-003	4.625	0.0056	-0.6869	-7.761E-004
60	CASE-4	-1.273	-1.109	7.834	0.3297	-0.4288	3.136E-006
60	CASE-4	-1.273	-1.109	7.834	0.3297	-0.4288	3.136E-006
63	CASE-4	-1.576E-003	0.981	6.238	0.1040	-0.0050	6.825E-004
63	CASE-4	-1.576E-003	0.981	6.238	0.1040	-0.0050	6.825E-004
65	CASE-4	2.944E-004	-0.046	-0.051	-0.1219	-6.640E-005	2.747E-005
65	CASE-4	2.944E-004	-0.046	-0.051	-0.1219	-6.640E-005	2.747E-005
66	CASE-4	-0.338	0.317	3.824	0.0102	0.1721	-2.634E-005
66	CASE-4	-0.338	0.317	3.824	0.0102	0.1721	-2.634E-005
69	CASE-4	1.077	-8.475E-004	5.075	0.0014	-0.0487	-2.417E-004
69	CASE-4	1.077	-8.475E-004	5.075	0.0014	-0.0487	-2.417E-004
75	CASE-4	0.416	1.096E-004	2.397	7.405E-005	-0.2552	-9.303E-006
75	CASE-4	0.416	1.096E-004	2.397	7.405E-005	-0.2552	-9.303E-006
76	CASE-4	-0.955	-1.713E-003	5.486	0.0046	0.2639	8.544E-004
76	CASE-4	-0.955	-1.713E-003	5.486	0.0046	0.2639	8.544E-004
79	CASE-4	9.837E-004	-1.480	6.518	-0.5051	0.0036	4.783E-004
79	CASE-4	9.837E-004	-1.480	6.518	-0.5051	0.0036	4.783E-004
80	CASE-4	-1.803E-004	-1.177	0.962	1.3804	0.0012	-1.852E-004
80	CASE-4	-1.803E-004	-1.177	0.962	1.3804	0.0012	-1.852E-004
83	CASE-4	-1.216	-3.367E-004	5.138	0.0014	0.1644	2.474E-004
83	CASE-4	-1.216	-3.367E-004	5.138	0.0014	0.1644	2.474E-004
84	CASE-4	1.030	-0.939	14.346	-0.2302	-0.1233	2.411E-006
84	CASE-4	1.030	-0.939	14.346	-0.2302	-0.1233	2.411E-006
87	CASE-4	0.404	0.226	6.727	0.2383	-0.6321	-7.215E-006
87	CASE-4	0.404	0.226	6.727	0.2383	-0.6321	-7.215E-006
89	CASE-4	-1.553	0.113	5.270	-0.5711	-0.3735	3.428E-006
89	CASE-4	-1.553	0.113	5.270	-0.5711	-0.3735	3.428E-006
91	CASE-4	-1.193	-2.809	9.977	0.7813	-0.3347	-3.503E-004
91	CASE-4	-1.193	-2.809	9.977	0.7813	-0.3347	-3.503E-004

Table 28: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
93	CASE-4	-0.390	-6.415E-003	3.112	-0.0633	3.5224	-3.265E-004
93	CASE-4	-0.390	-6.415E-003	3.112	-0.0633	3.5224	-3.265E-004
94	CASE-4	1.788	3.444	5.306	-4.2359	0.9825	6.802E-005
94	CASE-4	1.788	3.444	5.306	-4.2359	0.9825	6.802E-005
97	CASE-4	-0.677	0.089	-0.266	-0.0077	-0.8488	-5.887E-006
97	CASE-4	-0.677	0.089	-0.266	-0.0077	-0.8488	-5.887E-006
101	CASE-4	1.906	-1.903E-003	-2.074	6.785E-004	1.4525	-9.885E-005
101	CASE-4	1.906	-1.903E-003	-2.074	6.785E-004	1.4525	-9.885E-005
108	CASE-4	0.401	4.691	72.278	-18.6300	1.5897	-0.0090
108	CASE-4	0.401	4.691	72.278	-18.6300	1.5897	-0.0090
110	CASE-4	0.256	-0.025	40.537	0.1072	1.0337	7.368E-005
110	CASE-4	0.256	-0.025	40.537	0.1072	1.0337	7.368E-005
112	CASE-4	-0.248	0.013	35.142	-0.0455	-0.9811	3.845E-004
112	CASE-4	-0.248	0.013	35.142	-0.0455	-0.9811	3.845E-004
3	CASE-5	0.765	-3.250	18.506	12.9288	3.0034	-0.0030
3	CASE-5	0.765	-3.250	18.506	12.9288	3.0034	-0.0030
5	CASE-5	-0.822	-4.353	17.488	17.3040	-3.2539	0.0246
5	CASE-5	-0.822	-4.353	17.488	17.3040	-3.2539	0.0246
7	CASE-5	1.143	-4.672	21.687	18.5742	4.5188	-0.0101
7	CASE-5	1.143	-4.672	21.687	18.5742	4.5188	-0.0101
9	CASE-5	-1.069	-3.488	21.257	13.8620	-4.2058	0.0073
9	CASE-5	-1.069	-3.488	21.257	13.8620	-4.2058	0.0073
11	CASE-5	-0.277	0.451	9.717	-1.8381	-1.0774	0.0068
11	CASE-5	-0.277	0.451	9.717	-1.8381	-1.0774	0.0068
13	CASE-5	-2.220	-0.367	14.760	1.3990	-8.8210	-6.370E-004
13	CASE-5	-2.220	-0.367	14.760	1.3990	-8.8210	-6.370E-004
19	CASE-5	-22.111	-6.137	87.160	24.4185	-82.1864	0.1223
19	CASE-5	-22.111	-6.137	87.160	24.4185	-82.1864	0.1223
27	CASE-5	13.088	-9.856	94.726	39.2549	48.9419	0.0075
27	CASE-5	13.088	-9.856	94.726	39.2549	48.9419	0.0075
31	CASE-5	-2.016	-3.461	40.821	14.1722	-7.9919	0.0158
31	CASE-5	-2.016	-3.461	40.821	14.1722	-7.9919	0.0158
33	CASE-5	3.650	3.772	24.287	-11.2503	14.5240	-0.0880
33	CASE-5	3.650	3.772	24.287	-11.2503	14.5240	-0.0880
35	CASE-5	0.295	-0.673	12.355	2.7200	1.1628	-0.0176
35	CASE-5	0.295	-0.673	12.355	2.7200	1.1628	-0.0176
41	CASE-5	-0.985	-0.155	3.069	0.7959	-3.9781	-8.080E-004
41	CASE-5	-0.985	-0.155	3.069	0.7959	-3.9781	-8.080E-004
45	CASE-5	17.878	51.779	102.042	-168.9999	55.5459	-0.1278
45	CASE-5	17.878	51.779	102.042	-168.9999	55.5459	-0.1278
47	CASE-5	0.220	-0.321	9.652	1.3036	0.8715	0.0026
47	CASE-5	0.220	-0.321	9.652	1.3036	0.8715	0.0026
49	CASE-5	0.281	0.717	4.140	0.1012	-0.0358	-4.908E-004
49	CASE-5	0.281	0.717	4.140	0.1012	-0.0358	-4.908E-004
50	CASE-5	1.654	-2.950	11.424	1.7807	0.8141	-8.769E-008
50	CASE-5	1.654	-2.950	11.424	1.7807	0.8141	-8.769E-008
53	CASE-5	0.655	-2.734E-003	3.836	0.0062	0.3248	9.830E-004
53	CASE-5	0.655	-2.734E-003	3.836	0.0062	0.3248	9.830E-004
55	CASE-5	0.553	-2.308E-003	5.008	0.0103	0.1096	-0.0016
55	CASE-5	0.553	-2.308E-003	5.008	0.0103	0.1096	-0.0016
56	CASE-5	-0.133	-2.451E-003	4.621	0.0105	0.3307	0.0016
56	CASE-5	-0.133	-2.451E-003	4.621	0.0105	0.3307	0.0016
59	CASE-5	-0.011	-2.724E-003	5.380	0.0081	-0.8782	-0.0011
59	CASE-5	-0.011	-2.724E-003	5.380	0.0081	-0.8782	-0.0011

Table 28: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
60	CASE-5	-1.313	-0.965	7.093	0.2191	-0.5471	7.604E-006
60	CASE-5	-1.313	-0.965	7.093	0.2191	-0.5471	7.604E-006
63	CASE-5	-2.314E-003	1.320	7.672	0.1241	-0.0074	0.0010
63	CASE-5	-2.314E-003	1.320	7.672	0.1241	-0.0074	0.0010
65	CASE-5	3.098E-004	0.148	-0.583	-0.1440	-1.814E-005	6.774E-005
65	CASE-5	3.098E-004	0.148	-0.583	-0.1440	-1.814E-005	6.774E-005
66	CASE-5	-0.408	0.253	3.642	-0.0218	-0.1490	-4.352E-005
66	CASE-5	-0.408	0.253	3.642	-0.0218	-0.1490	-4.352E-005
69	CASE-5	1.302	-8.390E-004	5.546	9.828E-004	0.1474	-1.617E-004
69	CASE-5	1.302	-8.390E-004	5.546	9.828E-004	0.1474	-1.617E-004
75	CASE-5	0.572	1.558E-004	2.725	1.819E-005	-0.2935	1.400E-006
75	CASE-5	0.572	1.558E-004	2.725	1.819E-005	-0.2935	1.400E-006
76	CASE-5	-0.935	-2.058E-003	5.933	0.0055	0.4072	0.0010
76	CASE-5	-0.935	-2.058E-003	5.933	0.0055	0.4072	0.0010
79	CASE-5	1.202E-003	-1.624	7.506	-0.6611	0.0044	5.775E-004
79	CASE-5	1.202E-003	-1.624	7.506	-0.6611	0.0044	5.775E-004
80	CASE-5	-1.875E-004	-1.566	0.917	1.8362	0.0016	-2.400E-004
80	CASE-5	-1.875E-004	-1.566	0.917	1.8362	0.0016	-2.400E-004
83	CASE-5	-1.201	-5.957E-004	5.762	0.0027	0.3215	4.707E-004
83	CASE-5	-1.201	-5.957E-004	5.762	0.0027	0.3215	4.707E-004
84	CASE-5	1.492	-1.088	15.664	-0.1896	0.3173	4.693E-006
84	CASE-5	1.492	-1.088	15.664	-0.1896	0.3173	4.693E-006
87	CASE-5	0.311	0.237	7.169	0.3064	-0.8358	-6.794E-006
87	CASE-5	0.311	0.237	7.169	0.3064	-0.8358	-6.794E-006
89	CASE-5	-1.964	0.036	4.923	-0.7906	-0.5688	-1.025E-006
89	CASE-5	-1.964	0.036	4.923	-0.7906	-0.5688	-1.025E-006
91	CASE-5	-1.404	-3.304	11.777	0.8796	-0.3781	-5.399E-004
91	CASE-5	-1.404	-3.304	11.777	0.8796	-0.3781	-5.399E-004
93	CASE-5	-0.353	-5.511E-003	3.466	-0.0786	4.3516	-4.718E-004
93	CASE-5	-0.353	-5.511E-003	3.466	-0.0786	4.3516	-4.718E-004
94	CASE-5	3.438	4.978	5.174	-6.1885	2.8529	8.647E-005
94	CASE-5	3.438	4.978	5.174	-6.1885	2.8529	8.647E-005
97	CASE-5	-1.173	0.162	-0.650	-0.2141	-1.5053	-1.560E-005
97	CASE-5	-1.173	0.162	-0.650	-0.2141	-1.5053	-1.560E-005
101	CASE-5	2.005	-1.959E-003	-2.656	5.435E-004	1.4746	-1.169E-004
101	CASE-5	2.005	-1.959E-003	-2.656	5.435E-004	1.4746	-1.169E-004
108	CASE-5	0.588	5.951	80.380	-23.6335	2.3313	-0.0132
108	CASE-5	0.588	5.951	80.380	-23.6335	2.3313	-0.0132
110	CASE-5	0.211	-0.035	45.981	0.1461	0.8552	3.768E-005
110	CASE-5	0.211	-0.035	45.981	0.1461	0.8552	3.768E-005
112	CASE-5	-0.521	0.021	39.629	-0.0753	-2.0646	4.059E-004
112	CASE-5	-0.521	0.021	39.629	-0.0753	-2.0646	4.059E-004

Table 29: Integrated Wall Forces

Table 29: Integrated Wall Forces

Area	OutputCase	CaseType	Fx	Fy	Fz	Mx	My	Mz
			kip	kip	kip	kip-ft	kip-ft	kip-ft
1	CASE-1	NonStatic	-4.901	-12.543	68.414	31.2384	-12.1937	0.0122
1	CASE-1	NonStatic	-4.901	-12.543	68.414	31.2384	-12.1937	0.0122
2	CASE-1	NonStatic	8.701	-2.246E-003	28.860	0.0102	17.1745	-0.0084
2	CASE-1	NonStatic	8.701	-2.246E-003	28.860	0.0102	17.1745	-0.0084

Table 29: Integrated Wall Forces

Area	OutputCase	CaseType	Fx	Fy	Fz	Mx	My	Mz
			kip	kip	kip	kip-ft	kip-ft	kip-ft
3	CASE-1	NonStatic	0.757	-5.672E-003	27.845	0.0229	2.1442	-1.039E-004
3	CASE-1	NonStatic	0.757	-5.672E-003	27.845	0.0229	2.1442	-1.039E-004
4	CASE-1	NonStatic	-9.315	-4.257E-003	39.929	0.0179	7.7129	0.0166
4	CASE-1	NonStatic	-9.315	-4.257E-003	39.929	0.0179	7.7129	0.0166
5	CASE-1	NonStatic	-4.265E-003	-6.075	61.150	-59.0900	-0.0327	-0.0221
5	CASE-1	NonStatic	-4.265E-003	-6.075	61.150	-59.0900	-0.0327	-0.0221
6	CASE-1	NonStatic	-2.400E-004	0.169	1.391	-0.5707	-7.547E-004	-7.096E-005
6	CASE-1	NonStatic	-2.400E-004	0.169	1.391	-0.5707	-7.547E-004	-7.096E-005
7	CASE-1	NonStatic	-2.812	-9.734E-004	52.404	0.0040	61.7289	0.0087
7	CASE-1	NonStatic	-2.812	-9.734E-004	52.404	0.0040	61.7289	0.0087
9	CASE-1	NonStatic	15.300	-6.839E-003	94.166	0.0273	-147.1544	-0.0363
9	CASE-1	NonStatic	15.300	-6.839E-003	94.166	0.0273	-147.1544	-0.0363
10	CASE-1	NonStatic	2.073E-003	-4.993	22.355	30.6840	0.0068	-0.0047
10	CASE-1	NonStatic	2.073E-003	-4.993	22.355	30.6840	0.0068	-0.0047
11	CASE-1	NonStatic	1.565	-3.496E-003	108.007	0.0147	7.2941	-0.0164
11	CASE-1	NonStatic	1.565	-3.496E-003	108.007	0.0147	7.2941	-0.0164
12	CASE-1	NonStatic	-2.021	-4.772	90.222	159.6545	-67.7484	-0.0056
12	CASE-1	NonStatic	-2.021	-4.772	90.222	159.6545	-67.7484	-0.0056
13	CASE-1	NonStatic	-9.902	0.018	148.086	-0.0710	-109.2095	0.0346
13	CASE-1	NonStatic	-9.902	0.018	148.086	-0.0710	-109.2095	0.0346
14	CASE-1	NonStatic	-3.777	-8.891	47.773	67.8482	-28.8107	0.0078
14	CASE-1	NonStatic	-3.777	-8.891	47.773	67.8482	-28.8107	0.0078
15	CASE-1	NonStatic	-11.350	-0.195	23.934	0.1161	-7.2510	-0.0044
15	CASE-1	NonStatic	-11.350	-0.195	23.934	0.1161	-7.2510	-0.0044
16	CASE-1	NonStatic	2.015	1.596	4.940	17.0375	2.2583	-12.9958
16	CASE-1	NonStatic	2.015	1.596	4.940	17.0375	2.2583	-12.9958
18	CASE-1	NonStatic	2.052	-3.051E-004	-9.054	0.0047	11.4201	0.0032
18	CASE-1	NonStatic	2.052	-3.051E-004	-9.054	0.0047	11.4201	0.0032
1	CASE-2	NonStatic	-4.090	-10.465	61.639	29.3275	-11.4539	0.0087
1	CASE-2	NonStatic	-4.090	-10.465	61.639	29.3275	-11.4539	0.0087
2	CASE-2	NonStatic	6.809	-1.712E-003	26.933	0.0077	13.5725	-0.0063
2	CASE-2	NonStatic	6.809	-1.712E-003	26.933	0.0077	13.5725	-0.0063
3	CASE-2	NonStatic	0.793	-4.323E-003	25.253	0.0175	2.3191	-8.871E-005
3	CASE-2	NonStatic	0.793	-4.323E-003	25.253	0.0175	2.3191	-8.871E-005
4	CASE-2	NonStatic	-7.587	-3.272E-003	37.377	0.0137	4.4607	0.0126
4	CASE-2	NonStatic	-7.587	-3.272E-003	37.377	0.0137	4.4607	0.0126
5	CASE-2	NonStatic	-3.097E-003	-5.410	54.448	-39.5516	-0.0238	-0.0168
5	CASE-2	NonStatic	-3.097E-003	-5.410	54.448	-39.5516	-0.0238	-0.0168
6	CASE-2	NonStatic	-2.402E-004	0.179	1.511	-0.5725	-7.380E-004	-9.346E-005
6	CASE-2	NonStatic	-2.402E-004	0.179	1.511	-0.5725	-7.380E-004	-9.346E-005
7	CASE-2	NonStatic	-2.668	-6.300E-004	47.533	0.0027	46.6466	0.0076
7	CASE-2	NonStatic	-2.668	-6.300E-004	47.533	0.0027	46.6466	0.0076
9	CASE-2	NonStatic	13.177	-6.582E-003	83.197	0.0263	-139.7085	-0.0346
9	CASE-2	NonStatic	13.177	-6.582E-003	83.197	0.0263	-139.7085	-0.0346
10	CASE-2	NonStatic	1.581E-003	-4.950	21.273	26.6240	0.0054	-0.0041
10	CASE-2	NonStatic	1.581E-003	-4.950	21.273	26.6240	0.0054	-0.0041
11	CASE-2	NonStatic	-0.035	-2.500E-003	97.835	0.0104	17.2324	-0.0129
11	CASE-2	NonStatic	-0.035	-2.500E-003	97.835	0.0104	17.2324	-0.0129
12	CASE-2	NonStatic	-1.525	-3.603	82.334	139.7342	-59.2961	-0.0048
12	CASE-2	NonStatic	-1.525	-3.603	82.334	139.7342	-59.2961	-0.0048
13	CASE-2	NonStatic	-8.071	0.016	135.891	-0.0629	-95.5845	0.0312
13	CASE-2	NonStatic	-8.071	0.016	135.891	-0.0629	-95.5845	0.0312
14	CASE-2	NonStatic	-3.298	-7.764	42.363	52.3616	-22.2348	0.0061
14	CASE-2	NonStatic	-3.298	-7.764	42.363	52.3616	-22.2348	0.0061

Table 29: Integrated Wall Forces

Area	OutputCase	CaseType	Fx	Fy	Fz	Mx	My	Mz
			kip	kip	kip	kip-ft	kip-ft	kip-ft
15	CASE-2	NonStatic	-9.203	-0.158	22.044	0.1121	-6.8983	-0.0034
15	CASE-2	NonStatic	-9.203	-0.158	22.044	0.1121	-6.8983	-0.0034
16	CASE-2	NonStatic	1.451	1.221	4.506	14.9321	1.7095	-10.0435
16	CASE-2	NonStatic	1.451	1.221	4.506	14.9321	1.7095	-10.0435
18	CASE-2	NonStatic	1.866	-3.962E-004	-6.177	0.0036	9.2883	0.0026
18	CASE-2	NonStatic	1.866	-3.962E-004	-6.177	0.0036	9.2883	0.0026
1	CASE-3	NonStatic	-3.014	-7.744	59.398	11.8831	-4.5844	0.0345
1	CASE-3	NonStatic	-3.014	-7.744	59.398	11.8831	-4.5844	0.0345
2	CASE-3	NonStatic	5.347	-4.140E-003	25.436	0.0196	8.0279	-0.0187
2	CASE-3	NonStatic	5.347	-4.140E-003	25.436	0.0196	8.0279	-0.0187
3	CASE-3	NonStatic	0.431	-0.011	19.339	0.0444	1.0872	-4.659E-004
3	CASE-3	NonStatic	0.431	-0.011	19.339	0.0444	1.0872	-4.659E-004
4	CASE-3	NonStatic	-4.284	-9.292E-003	34.704	0.0397	19.1034	0.0387
4	CASE-3	NonStatic	-4.284	-9.292E-003	34.704	0.0397	19.1034	0.0387
5	CASE-3	NonStatic	-0.011	0.372	51.137	-71.5440	-0.0595	-0.0457
5	CASE-3	NonStatic	-0.011	0.372	51.137	-71.5440	-0.0595	-0.0457
6	CASE-3	NonStatic	-4.657E-004	0.426	0.117	-1.2290	-0.0013	-5.652E-004
6	CASE-3	NonStatic	-4.657E-004	0.426	0.117	-1.2290	-0.0013	-5.652E-004
7	CASE-3	NonStatic	2.817	1.896E-003	42.617	-0.0077	69.3272	0.0048
7	CASE-3	NonStatic	2.817	1.896E-003	42.617	-0.0077	69.3272	0.0048
9	CASE-3	NonStatic	10.576	-8.839E-003	76.040	0.0356	-159.8359	-0.0615
9	CASE-3	NonStatic	10.576	-8.839E-003	76.040	0.0356	-159.8359	-0.0615
10	CASE-3	NonStatic	3.533E-003	-7.307	20.811	44.2027	0.0120	-0.0079
10	CASE-3	NonStatic	3.533E-003	-7.307	20.811	44.2027	0.0120	-0.0079
11	CASE-3	NonStatic	0.467	-8.260E-003	104.370	0.0347	-5.2677	-0.0376
11	CASE-3	NonStatic	0.467	-8.260E-003	104.370	0.0347	-5.2677	-0.0376
12	CASE-3	NonStatic	-2.171	-5.147	80.530	145.6949	-61.7859	-0.0126
12	CASE-3	NonStatic	-2.171	-5.147	80.530	145.6949	-61.7859	-0.0126
13	CASE-3	NonStatic	-10.319	0.052	134.054	-0.2110	-43.6037	0.0969
13	CASE-3	NonStatic	-10.319	0.052	134.054	-0.2110	-43.6037	0.0969
14	CASE-3	NonStatic	-5.156	-12.132	47.096	103.5463	-43.9818	0.0224
14	CASE-3	NonStatic	-5.156	-12.132	47.096	103.5463	-43.9818	0.0224
15	CASE-3	NonStatic	-9.396	-0.157	21.872	0.1701	-10.8775	-0.0121
15	CASE-3	NonStatic	-9.396	-0.157	21.872	0.1701	-10.8775	-0.0121
16	CASE-3	NonStatic	8.538	5.455	2.551	3.1625	10.8461	-42.6452
16	CASE-3	NonStatic	8.538	5.455	2.551	3.1625	10.8461	-42.6452
18	CASE-3	NonStatic	1.620	3.927E-004	-12.386	0.0026	9.9535	0.0027
18	CASE-3	NonStatic	1.620	3.927E-004	-12.386	0.0026	9.9535	0.0027
1	CASE-4	NonStatic	-2.967	-7.607	53.255	25.7250	-10.0271	0.0194
1	CASE-4	NonStatic	-2.967	-7.607	53.255	25.7250	-10.0271	0.0194
2	CASE-4	NonStatic	4.780	-2.534E-003	23.795	0.0118	9.2146	-0.0109
2	CASE-4	NonStatic	4.780	-2.534E-003	23.795	0.0118	9.2146	-0.0109
3	CASE-4	NonStatic	0.362	-7.052E-003	18.609	0.0284	0.9990	-1.306E-004
3	CASE-4	NonStatic	0.362	-7.052E-003	18.609	0.0284	0.9990	-1.306E-004
4	CASE-4	NonStatic	-4.248	-5.481E-003	32.583	0.0234	8.9853	0.0227
4	CASE-4	NonStatic	-4.248	-5.481E-003	32.583	0.0234	8.9853	0.0227
5	CASE-4	NonStatic	-6.847E-003	-1.160	45.760	-39.7086	-0.0366	-0.0286
5	CASE-4	NonStatic	-6.847E-003	-1.160	45.760	-39.7086	-0.0366	-0.0286
6	CASE-4	NonStatic	-2.973E-004	0.271	0.945	-0.7656	-8.579E-004	-2.494E-004
6	CASE-4	NonStatic	-2.973E-004	0.271	0.945	-0.7656	-8.579E-004	-2.494E-004
7	CASE-4	NonStatic	1.267	-3.431E-004	40.601	0.0014	53.0462	0.0093
7	CASE-4	NonStatic	1.267	-3.431E-004	40.601	0.0014	53.0462	0.0093
9	CASE-4	NonStatic	9.977	-8.178E-003	68.356	0.0329	-137.2537	-0.0497
9	CASE-4	NonStatic	9.977	-8.178E-003	68.356	0.0329	-137.2537	-0.0497

Table 29: Integrated Wall Forces

Area	OutputCase	CaseType	Fx	Fy	Fz	Mx	My	Mz
			kip	kip	kip	kip-ft	kip-ft	kip-ft
10	CASE-4	NonStatic	2.740E-003	-5.815	21.343	35.1105	0.0093	-0.0065
10	CASE-4	NonStatic	2.740E-003	-5.815	21.343	35.1105	0.0093	-0.0065
11	CASE-4	NonStatic	-0.142	-4.005E-003	88.629	0.0171	22.4344	-0.0186
11	CASE-4	NonStatic	-0.142	-4.005E-003	88.629	0.0171	22.4344	-0.0186
12	CASE-4	NonStatic	-1.869	-4.422	72.458	123.0569	-52.2005	-0.0078
12	CASE-4	NonStatic	-1.869	-4.422	72.458	123.0569	-52.2005	-0.0078
13	CASE-4	NonStatic	-8.609	0.031	121.204	-0.1243	-50.8162	0.0567
13	CASE-4	NonStatic	-8.609	0.031	121.204	-0.1243	-50.8162	0.0567
14	CASE-4	NonStatic	-4.419	-10.401	40.932	78.8389	-33.4813	0.0125
14	CASE-4	NonStatic	-4.419	-10.401	40.932	78.8389	-33.4813	0.0125
15	CASE-4	NonStatic	-8.232	-0.139	18.375	0.0946	-6.1768	-0.0082
15	CASE-4	NonStatic	-8.232	-0.139	18.375	0.0946	-6.1768	-0.0082
16	CASE-4	NonStatic	5.688	3.603	2.931	5.9981	7.5005	-28.1150
16	CASE-4	NonStatic	5.688	3.603	2.931	5.9981	7.5005	-28.1150
18	CASE-4	NonStatic	1.700	-5.321E-004	-7.481	0.0049	9.7899	0.0031
18	CASE-4	NonStatic	1.700	-5.321E-004	-7.481	0.0049	9.7899	0.0031
1	CASE-5	NonStatic	-2.634	-6.769	57.467	7.3929	-2.8344	0.0300
1	CASE-5	NonStatic	-2.634	-6.769	57.467	7.3929	-2.8344	0.0300
2	CASE-5	NonStatic	4.806	-3.830E-003	24.624	0.0181	6.7357	-0.0173
2	CASE-5	NonStatic	4.806	-3.830E-003	24.624	0.0181	6.7357	-0.0173
3	CASE-5	NonStatic	0.430	-0.010	18.651	0.0416	1.2113	-3.735E-004
3	CASE-5	NonStatic	0.430	-0.010	18.651	0.0416	1.2113	-3.735E-004
4	CASE-5	NonStatic	-4.441	-7.912E-003	33.702	0.0339	20.7635	0.0333
4	CASE-5	NonStatic	-4.441	-7.912E-003	33.702	0.0339	20.7635	0.0333
5	CASE-5	NonStatic	-0.010	0.261	49.856	-66.8928	-0.0552	-0.0422
5	CASE-5	NonStatic	-0.010	0.261	49.856	-66.8928	-0.0552	-0.0422
6	CASE-5	NonStatic	-4.416E-004	0.401	0.395	-1.1411	-0.0012	-5.376E-004
6	CASE-5	NonStatic	-4.416E-004	0.401	0.395	-1.1411	-0.0012	-5.376E-004
7	CASE-5	NonStatic	2.872	1.259E-003	41.564	-0.0052	61.9140	0.0071
7	CASE-5	NonStatic	2.872	1.259E-003	41.564	-0.0052	61.9140	0.0071
9	CASE-5	NonStatic	9.946	-9.759E-003	73.860	0.0392	-144.5269	-0.0608
9	CASE-5	NonStatic	9.946	-9.759E-003	73.860	0.0392	-144.5269	-0.0608
10	CASE-5	NonStatic	3.579E-003	-5.933	23.880	41.7949	0.0118	-0.0075
10	CASE-5	NonStatic	3.579E-003	-5.933	23.880	41.7949	0.0118	-0.0075
11	CASE-5	NonStatic	1.133	-8.080E-003	97.253	0.0341	14.3854	-0.0353
11	CASE-5	NonStatic	1.133	-8.080E-003	97.253	0.0341	14.3854	-0.0353
12	CASE-5	NonStatic	-1.963	-4.657	78.160	133.1900	-56.4777	-0.0124
12	CASE-5	NonStatic	-1.963	-4.657	78.160	133.1900	-56.4777	-0.0124
13	CASE-5	NonStatic	-9.514	0.052	130.169	-0.2097	-43.9816	0.0872
13	CASE-5	NonStatic	-9.514	0.052	130.169	-0.2097	-43.9816	0.0872
14	CASE-5	NonStatic	-5.470	-12.873	45.008	102.3384	-43.4650	0.0194
14	CASE-5	NonStatic	-5.470	-12.873	45.008	102.3384	-43.4650	0.0194
15	CASE-5	NonStatic	-9.412	-0.158	20.195	0.1239	-8.1375	-0.0104
15	CASE-5	NonStatic	-9.412	-0.158	20.195	0.1239	-8.1375	-0.0104
16	CASE-5	NonStatic	8.104	5.226	2.770	2.1976	10.2342	-40.9353
16	CASE-5	NonStatic	8.104	5.226	2.770	2.1976	10.2342	-40.9353
18	CASE-5	NonStatic	0.766	1.054E-003	-10.583	0.0047	10.8427	0.0015
18	CASE-5	NonStatic	0.766	1.054E-003	-10.583	0.0047	10.8427	0.0015
1	UDCONN1	Combination	-2.651	-6.784	58.442	12.4842	-4.8697	0.0064
2	UDCONN1	Combination	5.542	-1.195E-003	26.146	0.0054	9.7254	-0.0043
3	UDCONN1	Combination	0.658	-2.880E-003	23.325	0.0116	1.8675	-5.236E-005
4	UDCONN1	Combination	-6.277	-2.277E-003	37.561	0.0095	3.0304	0.0085
5	UDCONN1	Combination	-1.990E-003	-4.887	50.136	-21.9419	-0.0156	-0.0119
6	UDCONN1	Combination	-2.963E-004	0.238	1.686	-0.7210	-8.993E-004	-1.315E-004

Table 29: Integrated Wall Forces

Area	OutputCase	CaseType	Fx	Fy	Fz	Mx	My	Mz
			kip	kip	kip	kip-ft	kip-ft	kip-ft
7	UDCONN1	Combination	-4.425	8.789E-004	42.992	-0.0035	24.3981	3.624E-004
9	UDCONN1	Combination	11.717	-1.440E-003	80.823	0.0056	-166.8817	-0.0111
10	UDCONN1	Combination	5.845E-004	-4.064	18.483	16.7483	0.0024	-0.0018
11	UDCONN1	Combination	-0.120	-1.987E-003	100.202	0.0082	-12.0149	-0.0097
12	UDCONN1	Combination	-0.529	-1.253	77.771	103.8832	-44.0836	-0.0033
13	UDCONN1	Combination	-3.969	0.011	134.678	-0.0446	-95.5750	0.0219
14	UDCONN1	Combination	-2.459	-5.790	34.349	15.7516	-6.6920	0.0040
15	UDCONN1	Combination	-6.243	-0.107	22.235	0.1291	-7.7197	-0.0017
16	UDCONN1	Combination	0.695	0.695	3.981	13.6984	0.9992	-5.8564
18	UDCONN1	Combination	1.857	-6.106E-004	-3.596	8.103E-004	5.8044	0.0022
1	UDCONN2	Combination	-3.101	-7.937	65.891	11.4774	-4.4720	0.0083
2	UDCONN2	Combination	6.894	-1.483E-003	29.353	0.0067	11.8290	-0.0054
3	UDCONN2	Combination	0.786	-3.582E-003	27.050	0.0145	2.2110	-6.555E-005
4	UDCONN2	Combination	-7.809	-2.839E-003	42.103	0.0119	4.3463	0.0107
5	UDCONN2	Combination	-2.547E-003	-5.811	56.868	-29.9660	-0.0200	-0.0149
6	UDCONN2	Combination	-3.798E-004	0.306	1.598	-0.9346	-0.0012	-1.757E-004
7	UDCONN2	Combination	-5.444	1.286E-003	47.679	-0.0051	29.2876	-2.543E-004
9	UDCONN2	Combination	13.796	-1.500E-003	93.122	0.0058	-197.8143	-0.0125
10	UDCONN2	Combination	7.009E-004	-4.770	19.862	19.1672	0.0029	-0.0021
11	UDCONN2	Combination	0.206	-2.449E-003	117.063	0.0101	-20.5969	-0.0119
12	UDCONN2	Combination	-0.517	-1.226	87.257	123.8325	-52.5486	-0.0041
13	UDCONN2	Combination	-4.634	0.014	151.631	-0.0555	-119.2070	0.0271
14	UDCONN2	Combination	-2.673	-6.292	38.406	17.6562	-7.5020	0.0051
15	UDCONN2	Combination	-7.511	-0.129	24.984	0.1602	-9.5708	-0.0020
16	UDCONN2	Combination	0.938	0.859	4.188	15.8909	1.2205	-7.1541
18	UDCONN2	Combination	2.254	-6.823E-004	-6.511	5.886E-004	6.8905	0.0028
1	UDCONU1	Combination	-3.711	-9.497	81.819	17.4779	-6.8176	0.0090
2	UDCONU1	Combination	7.758	-1.673E-003	36.604	0.0075	13.6155	-0.0060
3	UDCONU1	Combination	0.922	-4.032E-003	32.654	0.0163	2.6145	-7.331E-005
4	UDCONU1	Combination	-8.788	-3.188E-003	52.585	0.0133	4.2425	0.0119
5	UDCONU1	Combination	-2.786E-003	-6.842	70.191	-30.7187	-0.0219	-0.0166
6	UDCONU1	Combination	-4.148E-004	0.333	2.360	-1.0094	-0.0013	-1.842E-004
7	UDCONU1	Combination	-6.195	1.230E-003	60.188	-0.0048	34.1573	5.073E-004
9	UDCONU1	Combination	16.404	-2.016E-003	113.153	0.0078	-233.6343	-0.0156
10	UDCONU1	Combination	8.183E-004	-5.690	25.877	23.4476	0.0033	-0.0026
11	UDCONU1	Combination	-0.168	-2.782E-003	140.283	0.0115	-16.8208	-0.0136
12	UDCONU1	Combination	-0.741	-1.754	108.879	145.4364	-61.7171	-0.0046
13	UDCONU1	Combination	-5.557	0.015	188.550	-0.0625	-133.8051	0.0307
14	UDCONU1	Combination	-3.442	-8.105	48.089	22.0522	-9.3688	0.0056
15	UDCONU1	Combination	-8.741	-0.150	31.128	0.1807	-10.8075	-0.0023
16	UDCONU1	Combination	0.972	0.973	5.573	19.1778	1.3989	-8.1989
18	UDCONU1	Combination	2.599	-8.548E-004	-5.034	0.0011	8.1262	0.0030
1	UDCONU2	Combination	-3.901	-9.986	82.049	13.3702	-5.2073	0.0107
2	UDCONU2	Combination	8.813	-1.895E-003	36.507	0.0085	15.0363	-0.0069
3	UDCONU2	Combination	0.994	-4.580E-003	33.950	0.0185	2.7906	-8.393E-005
4	UDCONU2	Combination	-9.984	-3.632E-003	52.341	0.0152	5.7419	0.0137
5	UDCONU2	Combination	-3.280E-003	-7.343	70.934	-39.1689	-0.0258	-0.0190
6	UDCONU2	Combination	-4.892E-004	0.395	1.882	-1.2070	-0.0015	-2.285E-004
7	UDCONU2	Combination	-6.940	1.706E-003	59.089	-0.0068	37.1009	-5.518E-004
9	UDCONU2	Combination	17.386	-1.824E-003	116.666	0.0070	-249.7501	-0.0155
10	UDCONU2	Combination	8.876E-004	-6.005	24.385	23.9682	0.0037	-0.0027
11	UDCONU2	Combination	0.378	-3.124E-003	147.220	0.0129	-28.1491	-0.0152
12	UDCONU2	Combination	-0.615	-1.461	108.502	156.5787	-66.4443	-0.0052
13	UDCONU2	Combination	-5.827	0.018	188.739	-0.0709	-152.5011	0.0345

Table 29: Integrated Wall Forces

Area	OutputCase	CaseType	Fx	Fy	Fz	Mx	My	Mz
			kip	kip	kip	kip-ft	kip-ft	kip-ft
14	UDCONU2	Combination	-3.293	-7.752	47.710	21.9492	-9.3264	0.0066
15	UDCONU2	Combination	-9.520	-0.163	31.081	0.2047	-12.2255	-0.0026
16	UDCONU2	Combination	1.223	1.096	5.109	19.9461	1.5531	-9.1041
18	UDCONU2	Combination	2.864	-8.475E-004	-8.979	6.177E-004	8.7030	0.0036
1	LONG TERM (ACI L/240 LIMIT)	Combination	-0.858	-2.215	12.919	-11.9310	4.7030	0.0185
2	LONG TERM (ACI L/240 LIMIT)	Combination	2.459	-2.141E-003	3.567	0.0102	2.4153	-0.0100
3	LONG TERM (ACI L/240 LIMIT)	Combination	0.033	-5.313E-003	3.322	0.0214	-0.0867	-3.505E-004
4	LONG TERM (ACI L/240 LIMIT)	Combination	-1.764	-4.796E-003	4.673	0.0205	13.3704	0.0200
5	LONG TERM (ACI L/240 LIMIT)	Combination	-5.512E-003	0.867	12.080	-51.3738	-0.0318	-0.0224
6	LONG TERM (ACI L/240 LIMIT)	Combination	-1.682E-004	0.145	-0.949	-0.4616	-4.964E-004	-2.933E-004
7	LONG TERM (ACI L/240 LIMIT)	Combination	1.407	1.896E-003	6.887	-0.0077	31.3632	-0.0034
9	LONG TERM (ACI L/240 LIMIT)	Combination	2.722	-9.181E-004	18.653	0.0037	-30.0281	-0.0134
10	LONG TERM (ACI L/240 LIMIT)	Combination	1.285E-003	-1.535	0.551	13.1522	0.0041	-0.0020
11	LONG TERM (ACI L/240 LIMIT)	Combination	2.209	-5.251E-003	25.913	0.0219	-37.6403	-0.0224
12	LONG TERM (ACI L/240 LIMIT)	Combination	-0.797	-1.894	15.961	42.5584	-18.0377	-0.0054
13	LONG TERM (ACI L/240 LIMIT)	Combination	-3.541	0.023	25.044	-0.0947	-6.4125	0.0437
14	LONG TERM (ACI L/240 LIMIT)	Combination	-1.217	-2.859	11.573	40.1938	-17.0764	0.0116
15	LONG TERM (ACI L/240 LIMIT)	Combination	-3.310	-0.055	5.387	0.0795	-5.0534	-0.0049
16	LONG TERM (ACI L/240 LIMIT)	Combination	3.415	2.227	0.054	-0.7302	3.8945	-17.4825
18	LONG TERM (ACI L/240 LIMIT)	Combination	0.106	1.016E-003	-7.781	-0.0012	2.2955	1.616E-004
1	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-1.191	-3.053	8.706	6.4012	-2.4897	0.0079
2	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	2.433	-8.443E-004	2.738	0.0039	4.8942	-0.0035
3	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-0.034	-2.033E-003	3.280	0.0082	-0.2990	-1.077E-004

Table 29: Integrated Wall Forces

Area	OutputCase	CaseType	Fx	Fy	Fz	Mx	My	Mz
			kip	kip	kip	kip-ft	kip-ft	kip-ft
4	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-1.571	-2.365E-003	3.554	0.0100	1.5921	0.0094
5	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-2.005E-003	-0.554	7.984	-24.1896	-0.0132	-0.0088
6	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-2.384E-005	0.015	-0.398	-0.0861	-1.124E-004	-5.034E-006
7	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-0.198	2.936E-004	5.924	-0.0012	22.4954	-0.0011
9	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	2.753	6.629E-004	13.149	-0.0026	-22.7549	-0.0023
10	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	4.459E-004	-1.417	-1.987	6.4678	0.0016	-0.0010
11	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	0.934	-1.176E-003	17.290	0.0049	-29.5914	-0.0057
12	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-0.703	-1.659	10.259	32.4253	-13.7605	-8.697E-004
13	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-2.635	2.276E-003	16.079	-0.0093	-13.2471	0.0132
14	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-0.166	-0.386	7.497	16.6944	-7.0927	0.0047
15	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-2.130	-0.036	3.567	0.0503	-3.0927	-0.0027
16	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	0.998	0.604	0.215	3.0703	1.1608	-4.6622
18	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	1.040	-5.698E-004	-4.679	-0.0010	1.2427	0.0018

5.2. Structure results

Table 30: Sum Of Reactions, Part 1 of 2

Table 30: Sum Of Reactions, Part 1 of 2

OutputCase	GlobalFX	GlobalFY	GlobalFZ	GlobalMX	GlobalMY	GlobalMZ
	kip	kip	kip	kip-ft	kip-ft	kip-ft
CASE-1	-4.526E-005	-2.672E-005	1550.796	86835.3541	-84470.2460	3.489E-004
CASE-1	-4.526E-005	-2.672E-005	1550.796	86835.3541	-84470.2460	3.489E-004
CASE-2	-3.953E-005	-1.113E-005	1402.920	78293.1633	-76287.9281	0.0012
CASE-2	-3.953E-005	-1.113E-005	1402.920	78293.1633	-76287.9281	0.0012
CASE-3	-6.358E-005	-3.236E-005	1402.920	78293.1604	-76287.9260	0.0011
CASE-3	-6.358E-005	-3.236E-005	1402.920	78293.1604	-76287.9260	0.0011
CASE-4	-1.045E-005	2.169E-005	1238.614	68801.8570	-67196.4920	0.0025
CASE-4	-1.045E-005	2.169E-005	1238.614	68801.8570	-67196.4920	0.0025
CASE-5	-2.259E-005	2.099E-005	1353.628	75445.7664	-73560.4907	0.0031
CASE-5	-2.259E-005	2.099E-005	1353.628	75445.7664	-73560.4907	0.0031

Table 30: Sum Of Reactions, Part 1 of 2

OutputCase	GlobalFX	GlobalFY	GlobalFZ	GlobalMX	GlobalMY	GlobalMZ
	kip	kip	kip	kip-ft	kip-ft	kip-ft
UDCONN1	-9.174E-006	-8.082E-006	1353.629	75445.7712	-73560.4981	-7.786E-005
UDCONN2	-9.686E-006	-9.010E-006	1550.796	86835.3535	-84470.2442	-1.120E-004
UDCONU1	-1.284E-005	-1.132E-005	1895.080	105624.0797	-102985	-1.090E-004
UDCONU2	-1.183E-005	-1.118E-005	1939.822	108758.2571	-105728	-1.480E-004
LONG TERM (ACI L/240 LIMIT)	-5.886E-005	-6.965E-005	312.182	18033.4943	-17273.7519	-0.0023
LONG TERM (ACI LIVE L/360 LIMIT)	-4.671E-005	-6.895E-005	197.168	11389.5849	-10909.7531	-0.0028

Table 30: Sum Of Reactions, Part 2 of 2

Table 30: Sum Of Reactions, Part 2 of 2

OutputCase	GlobalX	GlobalY	GlobalZ
	ft	ft	ft
CASE-1	0.0000	0.0000	0.0000
CASE-1	0.0000	0.0000	0.0000
CASE-2	0.0000	0.0000	0.0000
CASE-2	0.0000	0.0000	0.0000
CASE-3	0.0000	0.0000	0.0000
CASE-3	0.0000	0.0000	0.0000
CASE-4	0.0000	0.0000	0.0000
CASE-4	0.0000	0.0000	0.0000
CASE-5	0.0000	0.0000	0.0000
CASE-5	0.0000	0.0000	0.0000
UDCONN1	0.0000	0.0000	0.0000
UDCONN2	0.0000	0.0000	0.0000
UDCONU1	0.0000	0.0000	0.0000
UDCONU2	0.0000	0.0000	0.0000
LONG TERM (ACI L/240 LIMIT)	0.0000	0.0000	0.0000
LONG TERM (ACI LIVE L/360 LIMIT)	0.0000	0.0000	0.0000

Table 31: Nodal Displacements - Summary, Part 1 of 2

Table 31: Nodal Displacements - Summary, Part 1 of 2

Panel	Node	OutputCase	Ux	Uy	Uz	Rx	Ry	Rz
			in	in	in	Radians	Radians	Radians
1	~1829	CASE-1	0.000000	0.000000	0.022729	0.000000	0.000000	0.000000
2	~1296	CASE-1	0.000000	0.000000	0.047105	0.000000	0.000000	0.000000
3	~1859	CASE-1	0.000000	0.000000	0.028633	0.000000	0.000000	0.000000
11	~236	CASE-1	0.000000	0.000000	-0.180686	0.000000	0.000000	0.000000
12	~272	CASE-1	0.000000	0.000000	-0.230859	0.000000	0.000000	0.000000
13	~1861	CASE-1	0.000000	0.000000	-0.120919	0.000000	0.000000	0.000000
14	~1307	CASE-1	0.000000	0.000000	-0.711220	0.000000	0.000000	0.000000
15	~1835	CASE-1	0.000000	0.000000	-0.905823	0.000000	0.000000	0.000000
21	~239	CASE-1	0.000000	0.000000	-0.224707	0.000000	0.000000	0.000000
22	~311	CASE-1	0.000000	0.000000	-0.290303	0.000000	0.000000	0.000000
23	~418	CASE-1	0.000000	0.000000	-0.071695	0.000000	0.000000	0.000000
24	~629	CASE-1	0.000000	0.000000	-0.353686	0.000000	0.000000	0.000000
25	~1867	CASE-1	0.000000	0.000000	-0.906953	0.000000	0.000000	0.000000
26	~1837	CASE-1	0.000000	0.000000	-0.843374	0.000000	0.000000	0.000000

Table 31: Nodal Displacements - Summary, Part 1 of 2

Panel	Node	OutputCase	Ux	Uy	Uz	Rx	Ry	Rz
			in	in	in	Radians	Radians	Radians
27	~1873	CASE-1	0.000000	0.000000	-0.442224	0.000000	0.000000	0.000000
28	~1881	CASE-1	0.000000	0.000000	-0.061016	0.000000	0.000000	0.000000
31	~240	CASE-1	0.000000	0.000000	-0.220981	0.000000	0.000000	0.000000
32	~312	CASE-1	0.000000	0.000000	-0.286080	0.000000	0.000000	0.000000
33	~492	CASE-1	0.000000	0.000000	-0.078581	0.000000	0.000000	0.000000
34	~635	CASE-1	0.000000	0.000000	-0.277190	0.000000	0.000000	0.000000
35	~671	CASE-1	0.000000	0.000000	-0.290547	0.000000	0.000000	0.000000
36	~777	CASE-1	0.000000	0.000000	-0.202504	0.000000	0.000000	0.000000
37	~882	CASE-1	0.000000	0.000000	-0.113496	0.000000	0.000000	0.000000
38	~1061	CASE-1	0.000000	0.000000	-0.080193	0.000000	0.000000	0.000000
39	~1878	CASE-1	0.000000	0.000000	0.068333	0.000000	0.000000	0.000000
40	~1880	CASE-1	0.000000	0.000000	0.050948	0.000000	0.000000	0.000000
41	~244	CASE-1	0.000000	0.000000	-0.139655	0.000000	0.000000	0.000000
42	~280	CASE-1	0.000000	0.000000	-0.181577	0.000000	0.000000	0.000000
43	~496	CASE-1	0.000000	0.000000	-0.211271	0.000000	0.000000	0.000000
44	~639	CASE-1	0.000000	0.000000	-0.850483	0.000000	0.000000	0.000000
45	~1623	CASE-1	0.000000	0.000000	-0.955525	0.000000	0.000000	0.000000
46	~1312	CASE-1	0.000000	0.000000	-0.809318	0.000000	0.000000	0.000000
47	~993	CASE-1	0.000000	0.000000	-0.158623	0.000000	0.000000	0.000000
48	~1100	CASE-1	0.000000	0.000000	-0.412460	0.000000	0.000000	0.000000
49	~1572	CASE-1	0.000000	0.000000	-0.389815	0.000000	0.000000	0.000000
50	~1324	CASE-1	0.000000	0.000000	-0.143242	0.000000	0.000000	0.000000
51	~1787	CASE-1	0.000000	0.000000	0.037915	0.000000	0.000000	0.000000
52	~1796	CASE-1	0.000000	0.000000	0.044617	0.000000	0.000000	0.000000
53	~1589	CASE-1	0.000000	0.000000	-0.242118	0.000000	0.000000	0.000000
54	~644	CASE-1	0.000000	0.000000	-1.023615	0.000000	0.000000	0.000000
55	~715	CASE-1	0.000000	0.000000	-1.248879	0.000000	0.000000	0.000000
56	~786	CASE-1	0.000000	0.000000	-1.057703	0.000000	0.000000	0.000000
57	~894	CASE-1	0.000000	0.000000	-0.345138	0.000000	0.000000	0.000000
58	~1105	CASE-1	0.000000	0.000000	-0.649034	0.000000	0.000000	0.000000
59	~1141	CASE-1	0.000000	0.000000	-0.562328	0.000000	0.000000	0.000000
60	~1213	CASE-1	0.000000	0.000000	-0.197854	0.000000	0.000000	0.000000
64	~1808	CASE-1	0.000000	0.000000	-1.066669	0.000000	0.000000	0.000000
65	~1812	CASE-1	0.000000	0.000000	-1.277351	0.000000	0.000000	0.000000
66	~1606	CASE-1	0.000000	0.000000	-1.156649	0.000000	0.000000	0.000000
67	~896	CASE-1	0.000000	0.000000	-0.420939	0.000000	0.000000	0.000000
68	~1107	CASE-1	0.000000	0.000000	-0.676945	0.000000	0.000000	0.000000
69	~1143	CASE-1	0.000000	0.000000	-0.583167	0.000000	0.000000	0.000000
70	~1217	CASE-1	0.000000	0.000000	-0.218038	0.000000	0.000000	0.000000
76	~1698	CASE-1	0.000000	0.000000	-0.772865	0.000000	0.000000	0.000000
77	~1614	CASE-1	0.000000	0.000000	-0.275993	0.000000	0.000000	0.000000
78	~1110	CASE-1	0.000000	0.000000	-0.570623	0.000000	0.000000	0.000000
79	~1146	CASE-1	0.000000	0.000000	-0.507550	0.000000	0.000000	0.000000
80	~1218	CASE-1	0.000000	0.000000	-0.203010	0.000000	0.000000	0.000000
87	~1007	CASE-1	0.000000	0.000000	-0.107961	0.000000	0.000000	0.000000
88	~1114	CASE-1	0.000000	0.000000	-0.215301	0.000000	0.000000	0.000000
89	~1150	CASE-1	0.000000	0.000000	-0.196344	0.000000	0.000000	0.000000
90	~1222	CASE-1	0.000000	0.000000	-0.077077	0.000000	0.000000	0.000000
1	~1829	CASE-2	0.000000	0.000000	0.018711	0.000000	0.000000	0.000000
2	~1857	CASE-2	0.000000	0.000000	0.038963	0.000000	0.000000	0.000000
3	~1859	CASE-2	0.000000	0.000000	0.022946	0.000000	0.000000	0.000000
11	~236	CASE-2	0.000000	0.000000	-0.159824	0.000000	0.000000	0.000000
12	~272	CASE-2	0.000000	0.000000	-0.202107	0.000000	0.000000	0.000000
13	~1861	CASE-2	0.000000	0.000000	-0.081641	0.000000	0.000000	0.000000

Table 31: Nodal Displacements - Summary, Part 1 of 2

Panel	Node	OutputCase	Ux	Uy	Uz	Rx	Ry	Rz
			in	in	in	Radians	Radians	Radians
14	~1307	CASE-2	0.000000	0.000000	-0.553233	0.000000	0.000000	0.000000
15	~1835	CASE-2	0.000000	0.000000	-0.715213	0.000000	0.000000	0.000000
21	~239	CASE-2	0.000000	0.000000	-0.207048	0.000000	0.000000	0.000000
22	~275	CASE-2	0.000000	0.000000	-0.269108	0.000000	0.000000	0.000000
23	~418	CASE-2	0.000000	0.000000	-0.060039	0.000000	0.000000	0.000000
24	~629	CASE-2	0.000000	0.000000	-0.279028	0.000000	0.000000	0.000000
25	~1867	CASE-2	0.000000	0.000000	-0.714087	0.000000	0.000000	0.000000
26	~1837	CASE-2	0.000000	0.000000	-0.663355	0.000000	0.000000	0.000000
27	~1873	CASE-2	0.000000	0.000000	-0.334598	0.000000	0.000000	0.000000
28	~1881	CASE-2	0.000000	0.000000	-0.043824	0.000000	0.000000	0.000000
31	~240	CASE-2	0.000000	0.000000	-0.202407	0.000000	0.000000	0.000000
32	~276	CASE-2	0.000000	0.000000	-0.262694	0.000000	0.000000	0.000000
33	~492	CASE-2	0.000000	0.000000	-0.063803	0.000000	0.000000	0.000000
34	~635	CASE-2	0.000000	0.000000	-0.229335	0.000000	0.000000	0.000000
35	~671	CASE-2	0.000000	0.000000	-0.242053	0.000000	0.000000	0.000000
36	~777	CASE-2	0.000000	0.000000	-0.169794	0.000000	0.000000	0.000000
37	~882	CASE-2	0.000000	0.000000	-0.086173	0.000000	0.000000	0.000000
38	~1061	CASE-2	0.000000	0.000000	-0.057268	0.000000	0.000000	0.000000
39	~1878	CASE-2	0.000000	0.000000	0.049144	0.000000	0.000000	0.000000
40	~1880	CASE-2	0.000000	0.000000	0.036416	0.000000	0.000000	0.000000
41	~244	CASE-2	0.000000	0.000000	-0.118959	0.000000	0.000000	0.000000
42	~280	CASE-2	0.000000	0.000000	-0.150950	0.000000	0.000000	0.000000
43	~496	CASE-2	0.000000	0.000000	-0.172621	0.000000	0.000000	0.000000
44	~639	CASE-2	0.000000	0.000000	-0.705228	0.000000	0.000000	0.000000
45	~1623	CASE-2	0.000000	0.000000	-0.798743	0.000000	0.000000	0.000000
46	~1312	CASE-2	0.000000	0.000000	-0.679604	0.000000	0.000000	0.000000
47	~889	CASE-2	0.000000	0.000000	-0.123563	0.000000	0.000000	0.000000
48	~1100	CASE-2	0.000000	0.000000	-0.295419	0.000000	0.000000	0.000000
49	~1572	CASE-2	0.000000	0.000000	-0.284983	0.000000	0.000000	0.000000
50	~1324	CASE-2	0.000000	0.000000	-0.109107	0.000000	0.000000	0.000000
51	~1787	CASE-2	0.000000	0.000000	0.029769	0.000000	0.000000	0.000000
52	~1796	CASE-2	0.000000	0.000000	0.032151	0.000000	0.000000	0.000000
53	~1589	CASE-2	0.000000	0.000000	-0.207090	0.000000	0.000000	0.000000
54	~644	CASE-2	0.000000	0.000000	-0.871248	0.000000	0.000000	0.000000
55	~715	CASE-2	0.000000	0.000000	-1.052609	0.000000	0.000000	0.000000
56	~786	CASE-2	0.000000	0.000000	-0.891763	0.000000	0.000000	0.000000
57	~894	CASE-2	0.000000	0.000000	-0.299033	0.000000	0.000000	0.000000
58	~1105	CASE-2	0.000000	0.000000	-0.447150	0.000000	0.000000	0.000000
59	~1141	CASE-2	0.000000	0.000000	-0.405549	0.000000	0.000000	0.000000
60	~1213	CASE-2	0.000000	0.000000	-0.150788	0.000000	0.000000	0.000000
64	~1808	CASE-2	0.000000	0.000000	-0.934450	0.000000	0.000000	0.000000
65	~1812	CASE-2	0.000000	0.000000	-1.115155	0.000000	0.000000	0.000000
66	~1605	CASE-2	0.000000	0.000000	-0.987815	0.000000	0.000000	0.000000
67	~896	CASE-2	0.000000	0.000000	-0.361062	0.000000	0.000000	0.000000
68	~1107	CASE-2	0.000000	0.000000	-0.473928	0.000000	0.000000	0.000000
69	~1143	CASE-2	0.000000	0.000000	-0.424139	0.000000	0.000000	0.000000
70	~1217	CASE-2	0.000000	0.000000	-0.167843	0.000000	0.000000	0.000000
76	~1698	CASE-2	0.000000	0.000000	-0.671146	0.000000	0.000000	0.000000
77	~1614	CASE-2	0.000000	0.000000	-0.240895	0.000000	0.000000	0.000000
78	~1110	CASE-2	0.000000	0.000000	-0.403350	0.000000	0.000000	0.000000
79	~1146	CASE-2	0.000000	0.000000	-0.372680	0.000000	0.000000	0.000000
80	~1218	CASE-2	0.000000	0.000000	-0.156377	0.000000	0.000000	0.000000
87	~1007	CASE-2	0.000000	0.000000	-0.079409	0.000000	0.000000	0.000000
88	~1114	CASE-2	0.000000	0.000000	-0.162314	0.000000	0.000000	0.000000

Table 31: Nodal Displacements - Summary, Part 1 of 2

Panel	Node	OutputCase	Ux	Uy	Uz	Rx	Ry	Rz
			in	in	in	Radians	Radians	Radians
89	~1150	CASE-2	0.000000	0.000000	-0.149317	0.000000	0.000000	0.000000
90	~1222	CASE-2	0.000000	0.000000	-0.059391	0.000000	0.000000	0.000000
1	~1829	CASE-3	0.000000	0.000000	0.067457	0.000000	0.000000	0.000000
2	~1857	CASE-3	0.000000	0.000000	0.138558	0.000000	0.000000	0.000000
3	~1859	CASE-3	0.000000	0.000000	0.076696	0.000000	0.000000	0.000000
11	~236	CASE-3	0.000000	0.000000	-0.526314	0.000000	0.000000	0.000000
12	~272	CASE-3	0.000000	0.000000	-0.664772	0.000000	0.000000	0.000000
13	~1861	CASE-3	0.000000	0.000000	-0.284960	0.000000	0.000000	0.000000
14	~1307	CASE-3	0.000000	0.000000	-1.820594	0.000000	0.000000	0.000000
15	~1835	CASE-3	0.000000	0.000000	-2.305654	0.000000	0.000000	0.000000
21	~239	CASE-3	0.000000	0.000000	-0.658110	0.000000	0.000000	0.000000
22	~311	CASE-3	0.000000	0.000000	-0.855084	0.000000	0.000000	0.000000
23	~418	CASE-3	0.000000	0.000000	-0.184793	0.000000	0.000000	0.000000
24	~629	CASE-3	0.000000	0.000000	-0.851602	0.000000	0.000000	0.000000
25	~1867	CASE-3	0.000000	0.000000	-2.296257	0.000000	0.000000	0.000000
26	~1837	CASE-3	0.000000	0.000000	-2.112625	0.000000	0.000000	0.000000
27	~1873	CASE-3	0.000000	0.000000	-1.022055	0.000000	0.000000	0.000000
28	~1881	CASE-3	0.000000	0.000000	-0.109078	0.000000	0.000000	0.000000
31	~240	CASE-3	0.000000	0.000000	-0.639256	0.000000	0.000000	0.000000
32	~312	CASE-3	0.000000	0.000000	-0.835106	0.000000	0.000000	0.000000
33	~419	CASE-3	0.000000	0.000000	-0.179003	0.000000	0.000000	0.000000
34	~635	CASE-3	0.000000	0.000000	-0.512298	0.000000	0.000000	0.000000
35	~671	CASE-3	0.000000	0.000000	-0.530475	0.000000	0.000000	0.000000
36	~777	CASE-3	0.000000	0.000000	-0.339752	0.000000	0.000000	0.000000
37	~882	CASE-3	0.000000	0.000000	-0.247225	0.000000	0.000000	0.000000
38	~1061	CASE-3	0.000000	0.000000	-0.183809	0.000000	0.000000	0.000000
39	~1878	CASE-3	0.000000	0.000000	0.166452	0.000000	0.000000	0.000000
40	~1880	CASE-3	0.000000	0.000000	0.111534	0.000000	0.000000	0.000000
41	~244	CASE-3	0.000000	0.000000	-0.393779	0.000000	0.000000	0.000000
42	~280	CASE-3	0.000000	0.000000	-0.511387	0.000000	0.000000	0.000000
43	~496	CASE-3	0.000000	0.000000	-0.384561	0.000000	0.000000	0.000000
44	~639	CASE-3	0.000000	0.000000	-1.592280	0.000000	0.000000	0.000000
45	~1623	CASE-3	0.000000	0.000000	-1.761176	0.000000	0.000000	0.000000
46	~1312	CASE-3	0.000000	0.000000	-1.467041	0.000000	0.000000	0.000000
47	~993	CASE-3	0.000000	0.000000	-0.369944	0.000000	0.000000	0.000000
48	~1100	CASE-3	0.000000	0.000000	-0.945545	0.000000	0.000000	0.000000
49	~1572	CASE-3	0.000000	0.000000	-0.889422	0.000000	0.000000	0.000000
50	~1324	CASE-3	0.000000	0.000000	-0.312448	0.000000	0.000000	0.000000
51	~1787	CASE-3	0.000000	0.000000	0.108804	0.000000	0.000000	0.000000
52	~1824	CASE-3	0.000000	0.000000	0.097921	0.000000	0.000000	0.000000
53	~1589	CASE-3	0.000000	0.000000	-0.372245	0.000000	0.000000	0.000000
54	~642	CASE-3	0.000000	0.000000	-1.729148	0.000000	0.000000	0.000000
55	~714	CASE-3	0.000000	0.000000	-2.127005	0.000000	0.000000	0.000000
56	~786	CASE-3	0.000000	0.000000	-1.837662	0.000000	0.000000	0.000000
57	~998	CASE-3	0.000000	0.000000	-0.705230	0.000000	0.000000	0.000000
58	~1105	CASE-3	0.000000	0.000000	-1.433019	0.000000	0.000000	0.000000
59	~1141	CASE-3	0.000000	0.000000	-1.273204	0.000000	0.000000	0.000000
60	~1211	CASE-3	0.000000	0.000000	-0.430963	0.000000	0.000000	0.000000
64	~2028	CASE-3	0.000000	0.000000	-1.686160	0.000000	0.000000	0.000000
65	~716	CASE-3	0.000000	0.000000	-2.091683	0.000000	0.000000	0.000000
66	~1606	CASE-3	0.000000	0.000000	-1.928602	0.000000	0.000000	0.000000
67	~1000	CASE-3	0.000000	0.000000	-0.808834	0.000000	0.000000	0.000000
68	~1107	CASE-3	0.000000	0.000000	-1.493423	0.000000	0.000000	0.000000
69	~1143	CASE-3	0.000000	0.000000	-1.327061	0.000000	0.000000	0.000000

Table 31: Nodal Displacements - Summary, Part 1 of 2

Panel	Node	OutputCase	Ux	Uy	Uz	Rx	Ry	Rz
			in	in	in	Radians	Radians	Radians
70	~1217	CASE-3	0.000000	0.000000	-0.498102	0.000000	0.000000	0.000000
76	~1698	CASE-3	0.000000	0.000000	-1.246853	0.000000	0.000000	0.000000
77	~1003	CASE-3	0.000000	0.000000	-0.559455	0.000000	0.000000	0.000000
78	~1110	CASE-3	0.000000	0.000000	-1.300981	0.000000	0.000000	0.000000
79	~1146	CASE-3	0.000000	0.000000	-1.185523	0.000000	0.000000	0.000000
80	~1218	CASE-3	0.000000	0.000000	-0.467768	0.000000	0.000000	0.000000
87	~1007	CASE-3	0.000000	0.000000	-0.264718	0.000000	0.000000	0.000000
88	~1114	CASE-3	0.000000	0.000000	-0.515956	0.000000	0.000000	0.000000
89	~1150	CASE-3	0.000000	0.000000	-0.466435	0.000000	0.000000	0.000000
90	~1222	CASE-3	0.000000	0.000000	-0.183669	0.000000	0.000000	0.000000
1	18	CASE-4	0.000000	0.000000	-0.062568	0.000000	0.000000	0.000000
2	~1857	CASE-4	0.000000	0.000000	0.079922	0.000000	0.000000	0.000000
3	~1859	CASE-4	0.000000	0.000000	0.043884	0.000000	0.000000	0.000000
11	~236	CASE-4	0.000000	0.000000	-0.309209	0.000000	0.000000	0.000000
12	~272	CASE-4	0.000000	0.000000	-0.388626	0.000000	0.000000	0.000000
13	~1861	CASE-4	0.000000	0.000000	-0.166264	0.000000	0.000000	0.000000
14	~1307	CASE-4	0.000000	0.000000	-1.116139	0.000000	0.000000	0.000000
15	~1835	CASE-4	0.000000	0.000000	-1.442373	0.000000	0.000000	0.000000
21	~239	CASE-4	0.000000	0.000000	-0.386280	0.000000	0.000000	0.000000
22	~311	CASE-4	0.000000	0.000000	-0.501137	0.000000	0.000000	0.000000
23	~418	CASE-4	0.000000	0.000000	-0.107122	0.000000	0.000000	0.000000
24	~629	CASE-4	0.000000	0.000000	-0.537315	0.000000	0.000000	0.000000
25	~1867	CASE-4	0.000000	0.000000	-1.443227	0.000000	0.000000	0.000000
26	~1837	CASE-4	0.000000	0.000000	-1.338039	0.000000	0.000000	0.000000
27	~1873	CASE-4	0.000000	0.000000	-0.645027	0.000000	0.000000	0.000000
28	~1881	CASE-4	0.000000	0.000000	-0.078402	0.000000	0.000000	0.000000
31	~240	CASE-4	0.000000	0.000000	-0.377043	0.000000	0.000000	0.000000
32	~312	CASE-4	0.000000	0.000000	-0.485860	0.000000	0.000000	0.000000
33	~492	CASE-4	0.000000	0.000000	-0.105476	0.000000	0.000000	0.000000
34	~635	CASE-4	0.000000	0.000000	-0.376513	0.000000	0.000000	0.000000
35	~671	CASE-4	0.000000	0.000000	-0.391614	0.000000	0.000000	0.000000
36	~777	CASE-4	0.000000	0.000000	-0.260077	0.000000	0.000000	0.000000
37	~882	CASE-4	0.000000	0.000000	-0.154061	0.000000	0.000000	0.000000
38	~1061	CASE-4	0.000000	0.000000	-0.107720	0.000000	0.000000	0.000000
39	~1878	CASE-4	0.000000	0.000000	0.095371	0.000000	0.000000	0.000000
40	~1880	CASE-4	0.000000	0.000000	0.064162	0.000000	0.000000	0.000000
41	~244	CASE-4	0.000000	0.000000	-0.231011	0.000000	0.000000	0.000000
42	~280	CASE-4	0.000000	0.000000	-0.295487	0.000000	0.000000	0.000000
43	~496	CASE-4	0.000000	0.000000	-0.285964	0.000000	0.000000	0.000000
44	~639	CASE-4	0.000000	0.000000	-1.169823	0.000000	0.000000	0.000000
45	~1623	CASE-4	0.000000	0.000000	-1.304805	0.000000	0.000000	0.000000
46	~1312	CASE-4	0.000000	0.000000	-1.087638	0.000000	0.000000	0.000000
47	~993	CASE-4	0.000000	0.000000	-0.205611	0.000000	0.000000	0.000000
48	~1100	CASE-4	0.000000	0.000000	-0.536940	0.000000	0.000000	0.000000
49	~1572	CASE-4	0.000000	0.000000	-0.506988	0.000000	0.000000	0.000000
50	~1324	CASE-4	0.000000	0.000000	-0.183640	0.000000	0.000000	0.000000
51	~1787	CASE-4	0.000000	0.000000	0.058530	0.000000	0.000000	0.000000
52	~1824	CASE-4	0.000000	0.000000	0.052497	0.000000	0.000000	0.000000
53	~1589	CASE-4	0.000000	0.000000	-0.288780	0.000000	0.000000	0.000000
54	~642	CASE-4	0.000000	0.000000	-1.317964	0.000000	0.000000	0.000000
55	~715	CASE-4	0.000000	0.000000	-1.639188	0.000000	0.000000	0.000000
56	~786	CASE-4	0.000000	0.000000	-1.382209	0.000000	0.000000	0.000000
57	~894	CASE-4	0.000000	0.000000	-0.429114	0.000000	0.000000	0.000000
58	~1105	CASE-4	0.000000	0.000000	-0.787893	0.000000	0.000000	0.000000

Table 31: Nodal Displacements - Summary, Part 1 of 2

Panel	Node	OutputCase	Ux	Uy	Uz	Rx	Ry	Rz
			in	in	in	Radians	Radians	Radians
59	~1141	CASE-4	0.000000	0.000000	-0.713044	0.000000	0.000000	0.000000
60	~1211	CASE-4	0.000000	0.000000	-0.254391	0.000000	0.000000	0.000000
64	~1808	CASE-4	0.000000	0.000000	-1.346218	0.000000	0.000000	0.000000
65	~1812	CASE-4	0.000000	0.000000	-1.638756	0.000000	0.000000	0.000000
66	~1606	CASE-4	0.000000	0.000000	-1.496082	0.000000	0.000000	0.000000
67	~896	CASE-4	0.000000	0.000000	-0.506815	0.000000	0.000000	0.000000
68	~1107	CASE-4	0.000000	0.000000	-0.810297	0.000000	0.000000	0.000000
69	~1143	CASE-4	0.000000	0.000000	-0.731071	0.000000	0.000000	0.000000
70	~1217	CASE-4	0.000000	0.000000	-0.289933	0.000000	0.000000	0.000000
76	~1698	CASE-4	0.000000	0.000000	-0.999869	0.000000	0.000000	0.000000
77	~1614	CASE-4	0.000000	0.000000	-0.347990	0.000000	0.000000	0.000000
78	~1110	CASE-4	0.000000	0.000000	-0.715802	0.000000	0.000000	0.000000
79	~1146	CASE-4	0.000000	0.000000	-0.655502	0.000000	0.000000	0.000000
80	~1218	CASE-4	0.000000	0.000000	-0.270733	0.000000	0.000000	0.000000
87	~1007	CASE-4	0.000000	0.000000	-0.148130	0.000000	0.000000	0.000000
88	~1114	CASE-4	0.000000	0.000000	-0.303872	0.000000	0.000000	0.000000
89	~1150	CASE-4	0.000000	0.000000	-0.279536	0.000000	0.000000	0.000000
90	~1222	CASE-4	0.000000	0.000000	-0.108380	0.000000	0.000000	0.000000
1	18	CASE-5	0.000000	0.000000	-0.094804	0.000000	0.000000	0.000000
2	~1857	CASE-5	0.000000	0.000000	0.140128	0.000000	0.000000	0.000000
3	~1859	CASE-5	0.000000	0.000000	0.081315	0.000000	0.000000	0.000000
11	~236	CASE-5	0.000000	0.000000	-0.513226	0.000000	0.000000	0.000000
12	~272	CASE-5	0.000000	0.000000	-0.643436	0.000000	0.000000	0.000000
13	~1861	CASE-5	0.000000	0.000000	-0.257691	0.000000	0.000000	0.000000
14	~1307	CASE-5	0.000000	0.000000	-1.641765	0.000000	0.000000	0.000000
15	~1835	CASE-5	0.000000	0.000000	-2.072723	0.000000	0.000000	0.000000
21	~239	CASE-5	0.000000	0.000000	-0.648482	0.000000	0.000000	0.000000
22	~275	CASE-5	0.000000	0.000000	-0.827774	0.000000	0.000000	0.000000
23	~418	CASE-5	0.000000	0.000000	-0.186834	0.000000	0.000000	0.000000
24	~629	CASE-5	0.000000	0.000000	-0.757290	0.000000	0.000000	0.000000
25	~1867	CASE-5	0.000000	0.000000	-2.074019	0.000000	0.000000	0.000000
26	~1837	CASE-5	0.000000	0.000000	-1.915606	0.000000	0.000000	0.000000
27	~1873	CASE-5	0.000000	0.000000	-0.927516	0.000000	0.000000	0.000000
28	~1881	CASE-5	0.000000	0.000000	-0.125481	0.000000	0.000000	0.000000
31	~240	CASE-5	0.000000	0.000000	-0.632607	0.000000	0.000000	0.000000
32	~276	CASE-5	0.000000	0.000000	-0.804175	0.000000	0.000000	0.000000
33	~419	CASE-5	0.000000	0.000000	-0.177846	0.000000	0.000000	0.000000
34	~635	CASE-5	0.000000	0.000000	-0.482920	0.000000	0.000000	0.000000
35	~671	CASE-5	0.000000	0.000000	-0.502095	0.000000	0.000000	0.000000
36	~777	CASE-5	0.000000	0.000000	-0.326092	0.000000	0.000000	0.000000
37	~882	CASE-5	0.000000	0.000000	-0.225727	0.000000	0.000000	0.000000
38	~1061	CASE-5	0.000000	0.000000	-0.167647	0.000000	0.000000	0.000000
39	~1878	CASE-5	0.000000	0.000000	0.150124	0.000000	0.000000	0.000000
40	~1880	CASE-5	0.000000	0.000000	0.100908	0.000000	0.000000	0.000000
41	~244	CASE-5	0.000000	0.000000	-0.367400	0.000000	0.000000	0.000000
42	~280	CASE-5	0.000000	0.000000	-0.473214	0.000000	0.000000	0.000000
43	~496	CASE-5	0.000000	0.000000	-0.360083	0.000000	0.000000	0.000000
44	~639	CASE-5	0.000000	0.000000	-1.510288	0.000000	0.000000	0.000000
45	~1623	CASE-5	0.000000	0.000000	-1.685051	0.000000	0.000000	0.000000
46	~1312	CASE-5	0.000000	0.000000	-1.410938	0.000000	0.000000	0.000000
47	~993	CASE-5	0.000000	0.000000	-0.338017	0.000000	0.000000	0.000000
48	~1100	CASE-5	0.000000	0.000000	-0.856363	0.000000	0.000000	0.000000
49	~1572	CASE-5	0.000000	0.000000	-0.806998	0.000000	0.000000	0.000000
50	~1324	CASE-5	0.000000	0.000000	-0.288422	0.000000	0.000000	0.000000

Table 31: Nodal Displacements - Summary, Part 1 of 2

Panel	Node	OutputCase	Ux	Uy	Uz	Rx	Ry	Rz
			in	in	in	Radians	Radians	Radians
51	~1787	CASE-5	0.000000	0.000000	0.089493	0.000000	0.000000	0.000000
52	~1824	CASE-5	0.000000	0.000000	0.081767	0.000000	0.000000	0.000000
53	~1589	CASE-5	0.000000	0.000000	-0.350034	0.000000	0.000000	0.000000
54	~642	CASE-5	0.000000	0.000000	-1.656551	0.000000	0.000000	0.000000
55	~714	CASE-5	0.000000	0.000000	-2.064033	0.000000	0.000000	0.000000
56	~786	CASE-5	0.000000	0.000000	-1.771899	0.000000	0.000000	0.000000
57	~998	CASE-5	0.000000	0.000000	-0.639187	0.000000	0.000000	0.000000
58	~1105	CASE-5	0.000000	0.000000	-1.302900	0.000000	0.000000	0.000000
59	~1141	CASE-5	0.000000	0.000000	-1.174187	0.000000	0.000000	0.000000
60	~1211	CASE-5	0.000000	0.000000	-0.401249	0.000000	0.000000	0.000000
64	~2028	CASE-5	0.000000	0.000000	-1.622703	0.000000	0.000000	0.000000
65	~716	CASE-5	0.000000	0.000000	-2.021436	0.000000	0.000000	0.000000
66	~1606	CASE-5	0.000000	0.000000	-1.866052	0.000000	0.000000	0.000000
67	~1000	CASE-5	0.000000	0.000000	-0.736136	0.000000	0.000000	0.000000
68	~1107	CASE-5	0.000000	0.000000	-1.358909	0.000000	0.000000	0.000000
69	~1574	CASE-5	0.000000	0.000000	-1.224372	0.000000	0.000000	0.000000
70	~1217	CASE-5	0.000000	0.000000	-0.460580	0.000000	0.000000	0.000000
76	~1698	CASE-5	0.000000	0.000000	-1.201709	0.000000	0.000000	0.000000
77	~1003	CASE-5	0.000000	0.000000	-0.503308	0.000000	0.000000	0.000000
78	~1110	CASE-5	0.000000	0.000000	-1.185379	0.000000	0.000000	0.000000
79	~1146	CASE-5	0.000000	0.000000	-1.081734	0.000000	0.000000	0.000000
80	~1218	CASE-5	0.000000	0.000000	-0.429470	0.000000	0.000000	0.000000
87	~1007	CASE-5	0.000000	0.000000	-0.244836	0.000000	0.000000	0.000000
88	~1114	CASE-5	0.000000	0.000000	-0.471124	0.000000	0.000000	0.000000
89	~1150	CASE-5	0.000000	0.000000	-0.424824	0.000000	0.000000	0.000000
90	~1222	CASE-5	0.000000	0.000000	-0.157249	0.000000	0.000000	0.000000

Table 31: Nodal Displacements - Summary, Part 2 of 2

Table 31: Nodal Displacements - Summary, Part 2 of 2

Panel	Node	OutputCase	GlobalX	GlobalY
			ft	ft
1	~1829	CASE-1	4.8912	11.5230
2	~1296	CASE-1	8.6641	20.4113
3	~1859	CASE-1	11.2812	26.5769
11	~236	CASE-1	23.3635	10.3073
12	~272	CASE-1	23.3635	13.7877
13	~1861	CASE-1	14.1470	33.3284
14	~1307	CASE-1	19.7429	46.5114
15	~1835	CASE-1	23.4727	55.2982
21	~239	CASE-1	33.7411	10.3073
22	~311	CASE-1	33.7411	17.2681
23	~418	CASE-1	33.7411	26.5769
24	~629	CASE-1	26.9398	46.4926
25	~1867	CASE-1	24.7671	58.3477
26	~1837	CASE-1	26.4281	62.2606
27	~1873	CASE-1	31.1635	73.4167
28	~1881	CASE-1	35.6789	84.0542
31	~240	CASE-1	36.9660	10.3073
32	~312	CASE-1	36.9660	17.2681
33	~492	CASE-1	45.5736	33.3284
34	~635	CASE-1	45.5736	46.4926
35	~671	CASE-1	45.5736	50.4828

Table 31: Nodal Displacements - Summary, Part 2 of 2

Panel	Node	OutputCase	GlobalX	GlobalY
			ft	ft
36	~777	CASE-1	45.5736	62.2224
37	~882	CASE-1	36.9660	73.4167
38	~1061	CASE-1	45.5736	89.9259
39	~1878	CASE-1	41.4516	97.6539
40	~1880	CASE-1	43.9172	103.4623
41	~244	CASE-1	48.2650	10.3073
42	~280	CASE-1	48.2650	13.7877
43	~496	CASE-1	57.3615	33.3284
44	~639	CASE-1	57.3615	46.4926
45	~1623	CASE-1	58.0082	54.4730
46	~1312	CASE-1	59.9638	62.2797
47	~993	CASE-1	57.3615	83.5878
48	~1100	CASE-1	57.3615	93.3100
49	~1572	CASE-1	58.2863	96.6941
50	~1324	CASE-1	58.2863	103.4623
51	~1787	CASE-1	60.8351	10.2299
52	~1796	CASE-1	66.4569	23.4739
53	~1589	CASE-1	65.5475	34.9546
54	~644	CASE-1	69.8538	46.4926
55	~715	CASE-1	69.8538	54.4730
56	~786	CASE-1	69.8538	62.2224
57	~894	CASE-1	69.8538	73.4167
58	~1105	CASE-1	69.8538	93.3100
59	~1141	CASE-1	69.8538	96.6941
60	~1213	CASE-1	69.8538	103.4623
64	~1808	CASE-1	76.2277	46.4926
65	~1812	CASE-1	79.6152	54.4730
66	~1606	CASE-1	75.3615	60.3589
67	~896	CASE-1	75.3615	73.4167
68	~1107	CASE-1	75.3615	93.3100
69	~1143	CASE-1	75.3615	96.6941
70	~1217	CASE-1	81.6111	103.4623
76	~1698	CASE-1	84.1136	66.0302
77	~1614	CASE-1	84.0301	73.4167
78	~1110	CASE-1	84.7359	93.3100
79	~1146	CASE-1	84.7359	96.6941
80	~1218	CASE-1	84.7359	103.4623
87	~1007	CASE-1	97.1065	83.5878
88	~1114	CASE-1	97.1065	93.3100
89	~1150	CASE-1	97.1065	96.6941
90	~1222	CASE-1	97.1065	103.4623
1	~1829	CASE-2	4.8912	11.5230
2	~1857	CASE-2	8.2824	19.5120
3	~1859	CASE-2	11.2812	26.5769
11	~236	CASE-2	23.3635	10.3073
12	~272	CASE-2	23.3635	13.7877
13	~1861	CASE-2	14.1470	33.3284
14	~1307	CASE-2	19.7429	46.5114
15	~1835	CASE-2	23.4727	55.2982
21	~239	CASE-2	33.7411	10.3073
22	~275	CASE-2	33.7411	13.7877
23	~418	CASE-2	33.7411	26.5769
24	~629	CASE-2	26.9398	46.4926
25	~1867	CASE-2	24.7671	58.3477

Table 31: Nodal Displacements - Summary, Part 2 of 2

Panel	Node	OutputCase	GlobalX	GlobalY
			ft	ft
26	~1837	CASE-2	26.4281	62.2606
27	~1873	CASE-2	31.1635	73.4167
28	~1881	CASE-2	35.6789	84.0542
31	~240	CASE-2	36.9660	10.3073
32	~276	CASE-2	36.9660	13.7877
33	~492	CASE-2	45.5736	33.3284
34	~635	CASE-2	45.5736	46.4926
35	~671	CASE-2	45.5736	50.4828
36	~777	CASE-2	45.5736	62.2224
37	~882	CASE-2	36.9660	73.4167
38	~1061	CASE-2	45.5736	89.9259
39	~1878	CASE-2	41.4516	97.6539
40	~1880	CASE-2	43.9172	103.4623
41	~244	CASE-2	48.2650	10.3073
42	~280	CASE-2	48.2650	13.7877
43	~496	CASE-2	57.3615	33.3284
44	~639	CASE-2	57.3615	46.4926
45	~1623	CASE-2	58.0082	54.4730
46	~1312	CASE-2	59.9638	62.2797
47	~889	CASE-2	57.3615	73.4167
48	~1100	CASE-2	57.3615	93.3100
49	~1572	CASE-2	58.2863	96.6941
50	~1324	CASE-2	58.2863	103.4623
51	~1787	CASE-2	60.8351	10.2299
52	~1796	CASE-2	66.4569	23.4739
53	~1589	CASE-2	65.5475	34.9546
54	~644	CASE-2	69.8538	46.4926
55	~715	CASE-2	69.8538	54.4730
56	~786	CASE-2	69.8538	62.2224
57	~894	CASE-2	69.8538	73.4167
58	~1105	CASE-2	69.8538	93.3100
59	~1141	CASE-2	69.8538	96.6941
60	~1213	CASE-2	69.8538	103.4623
64	~1808	CASE-2	76.2277	46.4926
65	~1812	CASE-2	79.6152	54.4730
66	~1605	CASE-2	78.4863	60.3589
67	~896	CASE-2	75.3615	73.4167
68	~1107	CASE-2	75.3615	93.3100
69	~1143	CASE-2	75.3615	96.6941
70	~1217	CASE-2	81.6111	103.4623
76	~1698	CASE-2	84.1136	66.0302
77	~1614	CASE-2	84.0301	73.4167
78	~1110	CASE-2	84.7359	93.3100
79	~1146	CASE-2	84.7359	96.6941
80	~1218	CASE-2	84.7359	103.4623
87	~1007	CASE-2	97.1065	83.5878
88	~1114	CASE-2	97.1065	93.3100
89	~1150	CASE-2	97.1065	96.6941
90	~1222	CASE-2	97.1065	103.4623
1	~1829	CASE-3	4.8912	11.5230
2	~1857	CASE-3	8.2824	19.5120
3	~1859	CASE-3	11.2812	26.5769
11	~236	CASE-3	23.3635	10.3073
12	~272	CASE-3	23.3635	13.7877

Table 31: Nodal Displacements - Summary, Part 2 of 2

Panel	Node	OutputCase	GlobalX	GlobalY
			ft	ft
13	~1861	CASE-3	14.1470	33.3284
14	~1307	CASE-3	19.7429	46.5114
15	~1835	CASE-3	23.4727	55.2982
21	~239	CASE-3	33.7411	10.3073
22	~311	CASE-3	33.7411	17.2681
23	~418	CASE-3	33.7411	26.5769
24	~629	CASE-3	26.9398	46.4926
25	~1867	CASE-3	24.7671	58.3477
26	~1837	CASE-3	26.4281	62.2606
27	~1873	CASE-3	31.1635	73.4167
28	~1881	CASE-3	35.6789	84.0542
31	~240	CASE-3	36.9660	10.3073
32	~312	CASE-3	36.9660	17.2681
33	~419	CASE-3	36.9660	26.5769
34	~635	CASE-3	45.5736	46.4926
35	~671	CASE-3	45.5736	50.4828
36	~777	CASE-3	45.5736	62.2224
37	~882	CASE-3	36.9660	73.4167
38	~1061	CASE-3	45.5736	89.9259
39	~1878	CASE-3	41.4516	97.6539
40	~1880	CASE-3	43.9172	103.4623
41	~244	CASE-3	48.2650	10.3073
42	~280	CASE-3	48.2650	13.7877
43	~496	CASE-3	57.3615	33.3284
44	~639	CASE-3	57.3615	46.4926
45	~1623	CASE-3	58.0082	54.4730
46	~1312	CASE-3	59.9638	62.2797
47	~993	CASE-3	57.3615	83.5878
48	~1100	CASE-3	57.3615	93.3100
49	~1572	CASE-3	58.2863	96.6941
50	~1324	CASE-3	58.2863	103.4623
51	~1787	CASE-3	60.8351	10.2299
52	~1824	CASE-3	62.0005	12.9753
53	~1589	CASE-3	65.5475	34.9546
54	~642	CASE-3	65.5475	46.4926
55	~714	CASE-3	67.7006	54.4730
56	~786	CASE-3	69.8538	62.2224
57	~998	CASE-3	69.8538	83.5878
58	~1105	CASE-3	69.8538	93.3100
59	~1141	CASE-3	69.8538	96.6941
60	~1211	CASE-3	65.5475	103.4623
64	~2028	CASE-3	72.7949	46.9536
65	~716	CASE-3	72.6076	54.4730
66	~1606	CASE-3	75.3615	60.3589
67	~1000	CASE-3	75.3615	83.5878
68	~1107	CASE-3	75.3615	93.3100
69	~1143	CASE-3	75.3615	96.6941
70	~1217	CASE-3	81.6111	103.4623
76	~1698	CASE-3	84.1136	66.0302
77	~1003	CASE-3	84.7359	83.5878
78	~1110	CASE-3	84.7359	93.3100
79	~1146	CASE-3	84.7359	96.6941
80	~1218	CASE-3	84.7359	103.4623
87	~1007	CASE-3	97.1065	83.5878

Table 31: Nodal Displacements - Summary, Part 2 of 2

Panel	Node	OutputCase	GlobalX	GlobalY
			ft	ft
88	~1114	CASE-3	97.1065	93.3100
89	~1150	CASE-3	97.1065	96.6941
90	~1222	CASE-3	97.1065	103.4623
1	18	CASE-4	0.0000	0.0000
2	~1857	CASE-4	8.2824	19.5120
3	~1859	CASE-4	11.2812	26.5769
11	~236	CASE-4	23.3635	10.3073
12	~272	CASE-4	23.3635	13.7877
13	~1861	CASE-4	14.1470	33.3284
14	~1307	CASE-4	19.7429	46.5114
15	~1835	CASE-4	23.4727	55.2982
21	~239	CASE-4	33.7411	10.3073
22	~311	CASE-4	33.7411	17.2681
23	~418	CASE-4	33.7411	26.5769
24	~629	CASE-4	26.9398	46.4926
25	~1867	CASE-4	24.7671	58.3477
26	~1837	CASE-4	26.4281	62.2606
27	~1873	CASE-4	31.1635	73.4167
28	~1881	CASE-4	35.6789	84.0542
31	~240	CASE-4	36.9660	10.3073
32	~312	CASE-4	36.9660	17.2681
33	~492	CASE-4	45.5736	33.3284
34	~635	CASE-4	45.5736	46.4926
35	~671	CASE-4	45.5736	50.4828
36	~777	CASE-4	45.5736	62.2224
37	~882	CASE-4	36.9660	73.4167
38	~1061	CASE-4	45.5736	89.9259
39	~1878	CASE-4	41.4516	97.6539
40	~1880	CASE-4	43.9172	103.4623
41	~244	CASE-4	48.2650	10.3073
42	~280	CASE-4	48.2650	13.7877
43	~496	CASE-4	57.3615	33.3284
44	~639	CASE-4	57.3615	46.4926
45	~1623	CASE-4	58.0082	54.4730
46	~1312	CASE-4	59.9638	62.2797
47	~993	CASE-4	57.3615	83.5878
48	~1100	CASE-4	57.3615	93.3100
49	~1572	CASE-4	58.2863	96.6941
50	~1324	CASE-4	58.2863	103.4623
51	~1787	CASE-4	60.8351	10.2299
52	~1824	CASE-4	62.0005	12.9753
53	~1589	CASE-4	65.5475	34.9546
54	~642	CASE-4	65.5475	46.4926
55	~715	CASE-4	69.8538	54.4730
56	~786	CASE-4	69.8538	62.2224
57	~894	CASE-4	69.8538	73.4167
58	~1105	CASE-4	69.8538	93.3100
59	~1141	CASE-4	69.8538	96.6941
60	~1211	CASE-4	65.5475	103.4623
64	~1808	CASE-4	76.2277	46.4926
65	~1812	CASE-4	79.6152	54.4730
66	~1606	CASE-4	75.3615	60.3589
67	~896	CASE-4	75.3615	73.4167
68	~1107	CASE-4	75.3615	93.3100

Table 31: Nodal Displacements - Summary, Part 2 of 2

Panel	Node	OutputCase	GlobalX	GlobalY
			ft	ft
69	~1143	CASE-4	75.3615	96.6941
70	~1217	CASE-4	81.6111	103.4623
76	~1698	CASE-4	84.1136	66.0302
77	~1614	CASE-4	84.0301	73.4167
78	~1110	CASE-4	84.7359	93.3100
79	~1146	CASE-4	84.7359	96.6941
80	~1218	CASE-4	84.7359	103.4623
87	~1007	CASE-4	97.1065	83.5878
88	~1114	CASE-4	97.1065	93.3100
89	~1150	CASE-4	97.1065	96.6941
90	~1222	CASE-4	97.1065	103.4623
1	18	CASE-5	0.0000	0.0000
2	~1857	CASE-5	8.2824	19.5120
3	~1859	CASE-5	11.2812	26.5769
11	~236	CASE-5	23.3635	10.3073
12	~272	CASE-5	23.3635	13.7877
13	~1861	CASE-5	14.1470	33.3284
14	~1307	CASE-5	19.7429	46.5114
15	~1835	CASE-5	23.4727	55.2982
21	~239	CASE-5	33.7411	10.3073
22	~275	CASE-5	33.7411	13.7877
23	~418	CASE-5	33.7411	26.5769
24	~629	CASE-5	26.9398	46.4926
25	~1867	CASE-5	24.7671	58.3477
26	~1837	CASE-5	26.4281	62.2606
27	~1873	CASE-5	31.1635	73.4167
28	~1881	CASE-5	35.6789	84.0542
31	~240	CASE-5	36.9660	10.3073
32	~276	CASE-5	36.9660	13.7877
33	~419	CASE-5	36.9660	26.5769
34	~635	CASE-5	45.5736	46.4926
35	~671	CASE-5	45.5736	50.4828
36	~777	CASE-5	45.5736	62.2224
37	~882	CASE-5	36.9660	73.4167
38	~1061	CASE-5	45.5736	89.9259
39	~1878	CASE-5	41.4516	97.6539
40	~1880	CASE-5	43.9172	103.4623
41	~244	CASE-5	48.2650	10.3073
42	~280	CASE-5	48.2650	13.7877
43	~496	CASE-5	57.3615	33.3284
44	~639	CASE-5	57.3615	46.4926
45	~1623	CASE-5	58.0082	54.4730
46	~1312	CASE-5	59.9638	62.2797
47	~993	CASE-5	57.3615	83.5878
48	~1100	CASE-5	57.3615	93.3100
49	~1572	CASE-5	58.2863	96.6941
50	~1324	CASE-5	58.2863	103.4623
51	~1787	CASE-5	60.8351	10.2299
52	~1824	CASE-5	62.0005	12.9753
53	~1589	CASE-5	65.5475	34.9546
54	~642	CASE-5	65.5475	46.4926
55	~714	CASE-5	67.7006	54.4730
56	~786	CASE-5	69.8538	62.2224
57	~998	CASE-5	69.8538	83.5878

Table 31: Nodal Displacements - Summary, Part 2 of 2

Panel	Node	OutputCase	GlobalX	GlobalY
			ft	ft
58	~1105	CASE-5	69.8538	93.3100
59	~1141	CASE-5	69.8538	96.6941
60	~1211	CASE-5	65.5475	103.4623
64	~2028	CASE-5	72.7949	46.9536
65	~716	CASE-5	72.6076	54.4730
66	~1606	CASE-5	75.3615	60.3589
67	~1000	CASE-5	75.3615	83.5878
68	~1107	CASE-5	75.3615	93.3100
69	~1574	CASE-5	75.8596	96.6941
70	~1217	CASE-5	81.6111	103.4623
76	~1698	CASE-5	84.1136	66.0302
77	~1003	CASE-5	84.7359	83.5878
78	~1110	CASE-5	84.7359	93.3100
79	~1146	CASE-5	84.7359	96.6941
80	~1218	CASE-5	84.7359	103.4623
87	~1007	CASE-5	97.1065	83.5878
88	~1114	CASE-5	97.1065	93.3100
89	~1150	CASE-5	97.1065	96.6941
90	~1222	CASE-5	97.1065	103.4623

Design

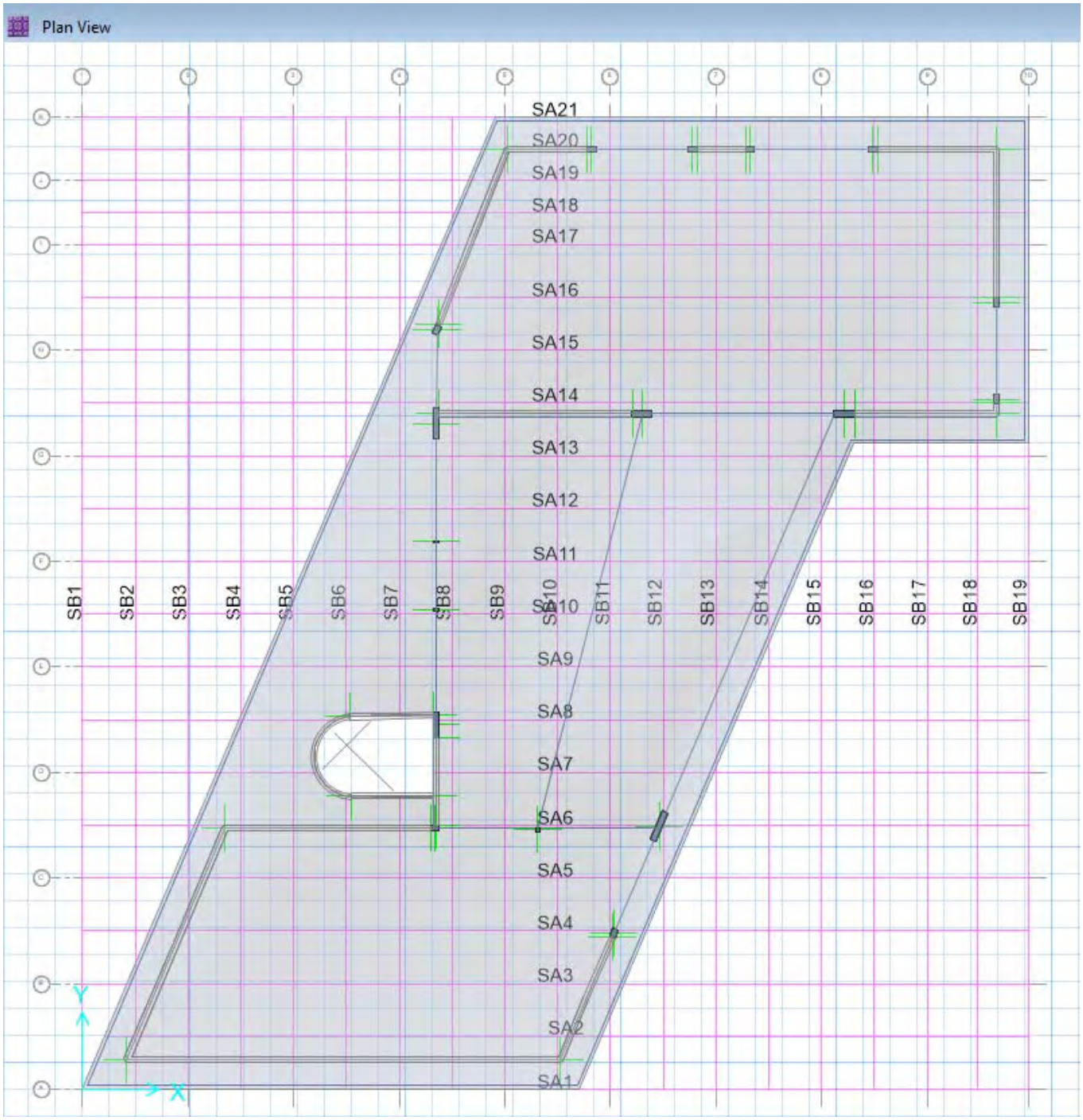


Figure 3: Finite element model

6. Design summary

This section provides design information for beams, strips, and punching checks.

6.1. Preferences

Table 32: Design Preferences 01 - Resistance Factors

Table 32: Design Preferences 01 - Resistance Factors

PhiTen	PhiComp	PhiShear
0.900000	0.650000	0.750000

Table 33: Design Preferences 02 - Rebar Cover - Slabs

Table 33: Design Preferences 02 - Rebar Cover - Slabs

CoverTop	CoverBot	BarSize	InnerLayer	PTCGSTop	PTCGSBotExt	PTCGSBotInt	SlabType
in	in			in	in	in	
0.7500	0.7500	#6	B	1.0000	1.7500	1.0000	Two Way

Table 34: Design Preferences 03 - Rebar Cover - Beams

Table 34: Design Preferences 03 - Rebar Cover - Beams

CoverTop	CoverBot	BarSizeF	BarSizeS	PTCGSTop	PTCGSBot
in	in			in	in
1.5000	1.5000	#9	#4	2.0000	2.0000

Table 35: Design Preferences 04 - Prestress Data

Table 35: Design Preferences 04 - Prestress Data

UserStress	InitConcRat	LLFraction
No	0.800000	0.500000

6.2. Overwrites

Table 36: Slab Design Overwrites 01 - Strip Based

Table 36: Slab Design Overwrites 01 - Strip Based

Strip	Layer	DesignType	RLLF	IgnorePT	RebarMat
SA1	A	Column	1.000000	No	A615Gr60
SA2	A	Middle	1.000000	No	A615Gr60
SA3	A	Column	1.000000	No	A615Gr60
SA4	A	Middle	1.000000	No	A615Gr60
SA5	A	Column	1.000000	No	A615Gr60

Table 36: Slab Design Overwrites 01 - Strip Based

Strip	Layer	DesignType	RLLF	IgnorePT	RebarMat
SA6	A	Middle	1.000000	No	A615Gr60
SA7	A	Column	1.000000	No	A615Gr60
SA8	A	Middle	1.000000	No	A615Gr60
SA9	A	Column	1.000000	No	A615Gr60
SB1	B	Column	1.000000	No	A615Gr60
SB2	B	Middle	1.000000	No	A615Gr60
SB3	B	Column	1.000000	No	A615Gr60
SB4	B	Middle	1.000000	No	A615Gr60
SB5	B	Column	1.000000	No	A615Gr60
SB6	B	Middle	1.000000	No	A615Gr60
SB7	B	Column	1.000000	No	A615Gr60
SB8	B	Middle	1.000000	No	A615Gr60
SB9	B	Column	1.000000	No	A615Gr60
SA10	A	Middle	1.000000	No	A615Gr60
SA11	A	Column	1.000000	No	A615Gr60
SA12	A	Middle	1.000000	No	A615Gr60
SA13	A	Column	1.000000	No	A615Gr60
SA14	A	Middle	1.000000	No	A615Gr60
SA15	A	Column	1.000000	No	A615Gr60
SA16	A	Middle	1.000000	No	A615Gr60
SA17	A	Column	1.000000	No	A615Gr60
SA18	A	Middle	1.000000	No	A615Gr60
SA19	A	Column	1.000000	No	A615Gr60
SA20	A	Middle	1.000000	No	A615Gr60
SA21	A	Column	1.000000	No	A615Gr60
SB10	B	Middle	1.000000	No	A615Gr60
SB11	B	Column	1.000000	No	A615Gr60
SB12	B	Middle	1.000000	No	A615Gr60
SB13	B	Column	1.000000	No	A615Gr60
SB14	B	Middle	1.000000	No	A615Gr60
SB15	B	Column	1.000000	No	A615Gr60
SB16	B	Middle	1.000000	No	A615Gr60
SB17	B	Column	1.000000	No	A615Gr60
SB18	B	Middle	1.000000	No	A615Gr60
SB19	B	Column	1.000000	No	A615Gr60

Table 37: Slab Design Overwrites 02 - Finite Element Based, Part 1 of 2

Table 37: Slab Design Overwrites 02 - Finite Element Based, Part 1 of 2

Area	RebarMat
8	A615Gr60
23	A615Gr60

Table 37: Slab Design Overwrites 02 - Finite Element Based, Part 2 of 2

Table 37: Slab Design Overwrites 02 - Finite Element Based, Part 2 of 2

Area	RLLF	Design	IgnorePT
8	1.000000	Yes	No

Table 37: Slab Design Overwrites 02 - Finite Element Based, Part 2 of 2

Area	RLLF	Design	IgnorePT
23	1.000000	Yes	No

Table 38: Beam Design Overwrites

Table 38: Beam Design Overwrites

Line	CoverType	RLLF	IgnorePT
1	Section	1.000000	No
8	Section	1.000000	No
9	Section	1.000000	No
11	Section	1.000000	No
12	Section	1.000000	No
13	Section	1.000000	No
25	Section	1.000000	No
26	Section	1.000000	No
27	Section	1.000000	No
28	Section	1.000000	No
33	Section	1.000000	No
34	Section	1.000000	No
35	Section	1.000000	No
36	Section	1.000000	No
37	Section	1.000000	No
38	Section	1.000000	No
39	Section	1.000000	No
40	Section	1.000000	No

Table 39: Punching Shear Design Overwrites 01 - General

Table 39: Punching Shear Design Overwrites 01 - General

Point	Check	LocType	EffDepth	ReinfType
4	Program Determined	Auto	Auto	None
6	Program Determined	Auto	Auto	None
8	Program Determined	Auto	Auto	None
10	Program Determined	Auto	Auto	None
12	Program Determined	Auto	Auto	None
14	Program Determined	Auto	Auto	None
20	Program Determined	Auto	Auto	None
28	Program Determined	Auto	Auto	None
32	Program Determined	Auto	Auto	None
34	Program Determined	Auto	Auto	None

Table 39: Punching Shear Design Overwrites 01 - General

Point	Check	LocType	EffDepth	ReinfType
36	Program Determined	Auto	Auto	None
42	Program Determined	Auto	Auto	None
46	Program Determined	Auto	Auto	None
48	Program Determined	Auto	Auto	None
109	Program Determined	Auto	Auto	None
111	Program Determined	Auto	Auto	None
113	Program Determined	Auto	Auto	None

6.3. Slab design

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA1	Span 1	Start		0.0000	0.0000
SA1	Span 1	Middle	UDCONU2	-5.3840	0.1654
SA1	Span 1	End	UDCONU2	-5.7927	0.1734
SA1	Span 2	Start	UDCONU2	-5.7927	0.1734
SA1	Span 2	Middle	UDCONU2	-8.0445	0.2318
SA1	Span 2	End	UDCONU2	-5.9369	0.1682
SA1	Span 3	Start	UDCONU2	-5.9369	0.1682
SA1	Span 3	Middle	UDCONU2	-4.4304	0.1254
SA1	Span 3	End	UDCONU2	-3.0800	0.0871
SA1	Span 4	Start	UDCONU2	-3.0800	0.0871
SA1	Span 4	Middle	UDCONU2	-7.7457	0.2198
SA1	Span 4	End	UDCONU2	-8.1593	0.2316
SA1	Span 5	Start	UDCONU2	-8.1593	0.2316
SA1	Span 5	Middle	UDCONU2	-1.9324	0.0628
SA1	Span 5	End		0.0000	0.0000
SA1	Span 6	Start		0.0000	0.0000
SA1	Span 6	Middle		0.0000	0.0000
SA1	Span 6	End		0.0000	0.0000
SA1	Span 7	Start		0.0000	0.0000
SA1	Span 7	Middle		0.0000	0.0000
SA1	Span 7	End		0.0000	0.0000
SA1	Span 8	Start		0.0000	0.0000
SA1	Span 8	Middle		0.0000	0.0000
SA1	Span 8	End		0.0000	0.0000
SA1	Span 9	Start		0.0000	0.0000
SA1	Span 9	Middle		0.0000	0.0000
SA1	Span 9	End		0.0000	0.0000
SA2	Cantilever Start	Start	UDCONU2	-4.9370	0.1415

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA2	Cantilever Start	Middle		0.0000	0.0000
SA2	Cantilever Start	End	UDCONU2	-14.3306	0.4128
SA2	Span 1	Start	UDCONU2	-14.9505	0.4551
SA2	Span 1	Middle	UDCONU2	-15.2869	0.4611
SA2	Span 1	End	UDCONU2	-14.1338	0.4251
SA2	Span 2	Start	UDCONU2	-14.1338	0.4251
SA2	Span 2	Middle	UDCONU2	-12.0058	0.3534
SA2	Span 2	End	UDCONU2	-4.3188	0.1221
SA2	Span 3	Start	UDCONU2	-4.3188	0.1221
SA2	Span 3	Middle	UDCONU2	-3.3999	0.0961
SA2	Span 3	End	UDCONU2	-2.2410	0.0640
SA2	Span 4	Start	UDCONU2	-2.2410	0.0640
SA2	Span 4	Middle	UDCONU2	-5.7318	0.1621
SA2	Span 4	End	UDCONU2	-9.6857	0.2743
SA2	Span 5	Start	UDCONU2	-9.6857	0.2743
SA2	Span 5	Middle	UDCONU2	-12.6700	0.3591
SA2	Span 5	End	UDCONU2	-9.3976	0.2749
SA2	Span 6	Start	UDCONU2	-16.1264	0.4705
SA2	Span 6	Middle		0.0000	0.0000
SA2	Span 6	End	UDCONU2	-1.6995	0.0531
SA2	Span 7	Start	UDCONU2	-0.3590	0.0106
SA2	Span 7	Middle		0.0000	0.0000
SA2	Span 7	End		0.0000	0.0000
SA2	Span 8	Start		0.0000	0.0000
SA2	Span 8	Middle		0.0000	0.0000
SA2	Span 8	End		0.0000	0.0000
SA2	Span 9	Start		0.0000	0.0000
SA2	Span 9	Middle		0.0000	0.0000
SA2	Span 9	End		0.0000	0.0000
SA2	Span 10	Start		0.0000	0.0000
SA2	Span 10	Middle		0.0000	0.0000
SA2	Span 10	End		0.0000	0.0000
SA3	Span 1	Start		0.0000	0.0000
SA3	Span 1	Middle	UDCONU2	-17.2146	0.5015
SA3	Span 1	End	UDCONU2	-11.4393	0.3468
SA3	Span 2	Start	UDCONU2	-11.4393	0.3468
SA3	Span 2	Middle	UDCONU2	-1.0083	0.0501
SA3	Span 2	End		0.0000	0.0000
SA3	Span 3	Start		0.0000	0.0000
SA3	Span 3	Middle		0.0000	0.0000
SA3	Span 3	End		0.0000	0.0000
SA3	Span 4	Start		0.0000	0.0000
SA3	Span 4	Middle		0.0000	0.0000
SA3	Span 4	End	UDCONU2	-0.3315	0.0000
SA3	Span 5	Start	UDCONU2	-0.3315	0.0000
SA3	Span 5	Middle	UDCONU2	-5.0505	0.1459
SA3	Span 5	End	UDCONU2	-21.6866	0.6284
SA3	Span 6	Start	UDCONU2	-7.8902	0.2682
SA3	Span 6	Middle		0.0000	0.0000
SA3	Span 6	End		0.0000	0.0000
SA3	Span 7	Start		0.0000	0.0000
SA3	Span 7	Middle		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA3	Span 7	End		0.0000	0.0000
SA3	Span 8	Start		0.0000	0.0000
SA3	Span 8	Middle		0.0000	0.0000
SA3	Span 8	End		0.0000	0.0000
SA3	Span 9	Start		0.0000	0.0000
SA3	Span 9	Middle		0.0000	0.0000
SA3	Span 9	End		0.0000	0.0000
SA4	Span 1	Start		0.0000	0.0000
SA4	Span 1	Middle	UDCONU2	-6.2746	0.1830
SA4	Span 1	End	UDCONU2	-21.0696	0.6153
SA4	Span 2	Start	UDCONU2	-19.9256	0.5930
SA4	Span 2	Middle	UDCONU1	-1.7469	0.0810
SA4	Span 2	End		0.0000	0.0000
SA4	Span 3	Start		0.0000	0.0000
SA4	Span 3	Middle		0.0000	0.0000
SA4	Span 3	End		0.0000	0.0000
SA4	Span 4	Start		0.0000	0.0000
SA4	Span 4	Middle		0.0000	0.0000
SA4	Span 4	End	UDCONU2	-0.1097	0.0293
SA4	Span 5	Start	UDCONU2	-0.1097	0.0293
SA4	Span 5	Middle	UDCONU2	-9.8424	0.3085
SA4	Span 5	End	UDCONU2	-15.6953	0.4951
SA4	Span 6	Start	UDCONU2	-15.6953	0.4951
SA4	Span 6	Middle		0.0000	0.0000
SA4	Span 6	End	UDCONU2	-13.8631	0.5132
SA4	Span 7	Start	UDCONU2	-13.8631	0.5132
SA4	Span 7	Middle		0.0000	0.0000
SA4	Span 7	End	UDCONU2	-15.7536	0.5703
SA4	Span 8	Start	UDCONU2	-15.7536	0.5703
SA4	Span 8	Middle		0.0000	0.0000
SA4	Span 8	End	UDCONU2	-6.6375	0.2489
SA4	Span 9	Start	UDCONU2	-6.6375	0.2489
SA4	Span 9	Middle	UDCONU2	-1.6385	0.0690
SA4	Span 9	End		0.0000	0.0000
SA4	Span 10	Start		0.0000	0.0000
SA4	Span 10	Middle		0.0000	0.0000
SA4	Span 10	End		0.0000	0.0000
SA4	Span 11	Start		0.0000	0.0000
SA4	Span 11	Middle		0.0000	0.0000
SA4	Span 11	End		0.0000	0.0000
SA4	Span 12	Start		0.0000	0.0000
SA4	Span 12	Middle		0.0000	0.0000
SA4	Span 12	End		0.0000	0.0000
SA5	Span 1	Start		0.0000	0.0000
SA5	Span 1	Middle		0.0000	0.0000
SA5	Span 1	End	UDCONU2	-10.7165	0.3225
SA5	Span 2	Start	UDCONU2	-20.0300	0.6129
SA5	Span 2	Middle	UDCONU2	-15.6490	0.4709
SA5	Span 2	End	UDCONU2	-4.1914	0.1403
SA5	Span 3	Start	UDCONU2	-4.1914	0.1403
SA5	Span 3	Middle	UDCONU2	-3.0124	0.1046
SA5	Span 3	End	UDCONU2	-0.5778	0.0388
SA5	Span 4	Start	UDCONU2	-0.5778	0.0388

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA5	Span 4	Middle	UDCONU2	-6.9080	0.2379
SA5	Span 4	End	UDCONU2	-6.9563	0.2197
SA5	Span 5	Start	UDCONU2	-6.9563	0.2197
SA5	Span 5	Middle	UDCONU2	-30.0723	0.8846
SA5	Span 5	End	UDCONU2	-8.6475	0.2996
SA5	Span 6	Start	UDCONU2	-8.6475	0.2996
SA5	Span 6	Middle		0.0000	0.0000
SA5	Span 6	End	UDCONU2	-2.1919	0.1218
SA5	Span 7	Start	UDCONU2	-2.1919	0.1218
SA5	Span 7	Middle	UDCONU1	-0.2474	0.0000
SA5	Span 7	End		0.0000	0.0000
SA5	Span 8	Start		0.0000	0.0000
SA5	Span 8	Middle		0.0000	0.0000
SA5	Span 8	End		0.0000	0.0000
SA5	Span 9	Start		0.0000	0.0000
SA5	Span 9	Middle		0.0000	0.0000
SA5	Span 9	End		0.0000	0.0000
SA5	Span 10	Start		0.0000	0.0000
SA5	Span 10	Middle		0.0000	0.0000
SA5	Span 10	End		0.0000	0.0000
SA6	Span 1	Start		0.0000	0.0000
SA6	Span 1	Middle		0.0000	0.0000
SA6	Span 1	End	UDCONU2	-0.0606	0.0000
SA6	Span 2	Start	UDCONU2	-1.9817	0.0697
SA6	Span 2	Middle		0.0000	0.0000
SA6	Span 2	End	UDCONU2	-29.6435	0.8570
SA6	Span 3	Start	UDCONU2	-29.6435	0.8570
SA6	Span 3	Middle	UDCONU2	-24.5474	0.7344
SA6	Span 3	End	UDCONU2	-27.5911	0.8356
SA6	Span 4	Start	UDCONU2	-27.5911	0.8356
SA6	Span 4	Middle	UDCONU2	-20.1460	0.6200
SA6	Span 4	End	UDCONU2	-18.7118	0.5529
SA6	Span 5	Start	UDCONU2	-18.7118	0.5529
SA6	Span 5	Middle		0.0000	0.0000
SA6	Span 5	End	UDCONU2	-29.9380	0.8558
SA6	Span 6	Start	UDCONU2	-29.9380	0.8558
SA6	Span 6	Middle		0.0000	0.0000
SA6	Span 6	End	UDCONU2	-36.1344	1.0938
SA6	Span 7	Start	UDCONU2	-36.1344	1.0938
SA6	Span 7	Middle		0.0000	0.0000
SA6	Span 7	End	UDCONU2	-24.4998	0.7139
SA6	Span 8	Start	UDCONU2	-24.4998	0.7139
SA6	Span 8	Middle	UDCONU2	-22.2944	0.6490
SA6	Span 8	End	UDCONU2	-35.9610	1.0367
SA6	Span 9	Start	UDCONU2	-35.9610	1.0367
SA6	Span 9	Middle		0.0000	0.0000
SA6	Span 9	End	UDCONU2	-45.2115	1.3066
SA6	Span 10	Start	UDCONU2	-45.2115	1.3066
SA6	Span 10	Middle		0.0000	0.0000
SA6	Span 10	End	UDCONU2	-30.6631	0.8866
SA6	Span 11	Start	UDCONU2	-30.6631	0.8866
SA6	Span 11	Middle	UDCONU2	-13.1880	0.3926
SA6	Span 11	End	UDCONU1	-1.7169	0.0821

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA6	Span 12	Start	UDCONU1	-1.7169	0.0821
SA6	Span 12	Middle	UDCONU2	-5.1414	0.2518
SA6	Span 12	End	UDCONU2	-13.0766	0.3952
SA6	Span 13	Start	UDCONU2	-13.0766	0.3952
SA6	Span 13	Middle		0.0000	0.0000
SA6	Span 13	End	UDCONU1	-12.3551	0.3635
SA6	Span 14	Start	UDCONU1	-12.3551	0.3635
SA6	Span 14	Middle	UDCONU1	-5.1090	0.1505
SA6	Span 14	End		0.0000	0.0000
SA6	Span 15	Start		0.0000	0.0000
SA6	Span 15	Middle		0.0000	0.0000
SA6	Span 15	End		0.0000	0.0000
SA6	Span 16	Start		0.0000	0.0000
SA6	Span 16	Middle		0.0000	0.0000
SA6	Span 16	End		0.0000	0.0000
SA6	Span 17	Start		0.0000	0.0000
SA6	Span 17	Middle		0.0000	0.0000
SA6	Span 17	End		0.0000	0.0000
SA7	Span 1	Start		0.0000	0.0000
SA7	Span 1	Middle		0.0000	0.0000
SA7	Span 1	End		0.0000	0.0000
SA7	Span 2	Start		0.0000	0.0000
SA7	Span 2	Middle	UDCONU2	-7.1990	0.2158
SA7	Span 2	End	UDCONU2	-31.3590	0.9223
SA7	Span 3	Start	UDCONU2	-31.3590	0.9223
SA7	Span 3	Middle	UDCONU2	-24.0551	0.7105
SA7	Span 3	End	UDCONU2	-0.3277	0.0161
SA7	Span 4	Start	UDCONU2	-0.3061	0.0155
SA7	Span 4	Middle	UDCONU2	-0.5604	0.0159
SA7	Span 4	End	UDCONU2	-1.4329	0.0410
SA7	Span 5	Start	UDCONU2	-1.4329	0.0410
SA7	Span 5	Middle		0.0000	0.0000
SA7	Span 5	End	UDCONU2	-26.9895	0.8009
SA7	Span 6	Start	UDCONU2	-27.6622	0.8109
SA7	Span 6	Middle	UDCONU2	-24.1003	0.6987
SA7	Span 6	End	UDCONU2	-11.8666	0.3437
SA7	Span 7	Start	UDCONU2	-11.8666	0.3437
SA7	Span 7	Middle	UDCONU2	-0.9998	0.0306
SA7	Span 7	End	UDCONU2	-1.2582	0.0370
SA7	Span 8	Start	UDCONU2	-1.2582	0.0370
SA7	Span 8	Middle	UDCONU2	-0.2967	0.0000
SA7	Span 8	End		0.0000	0.0000
SA7	Span 9	Start		0.0000	0.0000
SA7	Span 9	Middle	UDCONU1	-7.2499	0.2260
SA7	Span 9	End	UDCONU2	-18.0221	0.5392
SA7	Span 10	Start	UDCONU2	-18.0221	0.5392
SA7	Span 10	Middle		0.0000	0.0000
SA7	Span 10	End	UDCONU1	-4.3141	0.1315
SA7	Span 11	Start	UDCONU1	-3.1736	0.0969
SA7	Span 11	Middle		0.0000	0.0000
SA7	Span 11	End		0.0000	0.0000
SA7	Span 12	Start		0.0000	0.0000
SA7	Span 12	Middle		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA7	Span 12	End		0.0000	0.0000
SA7	Span 13	Start		0.0000	0.0000
SA7	Span 13	Middle		0.0000	0.0000
SA7	Span 13	End		0.0000	0.0000
SA8	Span 1	Start		0.0000	0.0000
SA8	Span 1	Middle		0.0000	0.0000
SA8	Span 1	End		0.0000	0.0000
SA8	Span 2	Start		0.0000	0.0000
SA8	Span 2	Middle	UDCONU1	-1.7681	0.0499
SA8	Span 2	End	UDCONU2	-3.6170	0.1022
SA8	Span 3	Start	UDCONU2	-3.6170	0.1022
SA8	Span 3	Middle	UDCONU2	-10.8616	0.3077
SA8	Span 3	End	UDCONU2	-51.2444	1.5100
SA8	Span 4	Start	UDCONU2	-51.2444	1.5100
SA8	Span 4	Middle	UDCONU2	-33.5992	1.0789
SA8	Span 4	End	UDCONU2	-9.2918	0.3808
SA8	Span 5	Start	UDCONU2	-9.2918	0.3808
SA8	Span 5	Middle		0.0000	0.0000
SA8	Span 5	End	UDCONU2	-58.3821	1.6827
SA8	Span 6	Start	UDCONU2	-58.3821	1.6827
SA8	Span 6	Middle		0.0000	0.0000
SA8	Span 6	End	UDCONU2	-67.3226	1.9469
SA8	Span 7	Start	UDCONU2	-67.3226	1.9469
SA8	Span 7	Middle	UDCONU2	-21.4738	0.6106
SA8	Span 7	End	UDCONU1	-1.7848	0.0504
SA8	Span 8	Start	UDCONU1	-1.7848	0.0504
SA8	Span 8	Middle	UDCONU1	-0.0925	0.0000
SA8	Span 8	End		0.0000	0.0000
SA8	Span 9	Start		0.0000	0.0000
SA8	Span 9	Middle		0.0000	0.0000
SA8	Span 9	End		0.0000	0.0000
SA8	Span 10	Start		0.0000	0.0000
SA8	Span 10	Middle	UDCONU1	-0.8197	0.0307
SA8	Span 10	End	UDCONU2	-17.7872	0.5240
SA8	Span 11	Start	UDCONU2	-17.7872	0.5240
SA8	Span 11	Middle		0.0000	0.0000
SA8	Span 11	End	UDCONU1	-9.5405	0.2701
SA8	Span 12	Start	UDCONU1	-9.5405	0.2701
SA8	Span 12	Middle		0.0000	0.0000
SA8	Span 12	End		0.0000	0.0000
SA8	Span 13	Start		0.0000	0.0000
SA8	Span 13	Middle		0.0000	0.0000
SA8	Span 13	End		0.0000	0.0000
SA8	Span 14	Start		0.0000	0.0000
SA8	Span 14	Middle		0.0000	0.0000
SA8	Span 14	End		0.0000	0.0000
SA9	Span 1	Start		0.0000	0.0000
SA9	Span 1	Middle		0.0000	0.0000
SA9	Span 1	End		0.0000	0.0000
SA9	Span 2	Start		0.0000	0.0000
SA9	Span 2	Middle	UDCONU2	-1.0253	0.0379
SA9	Span 2	End	UDCONU2	-4.8319	0.1728
SA9	Span 3	Start	UDCONU2	-4.8319	0.1728

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA9	Span 3	Middle	UDCONU2	-51.3588	1.5303
SA9	Span 3	End	UDCONU2	-58.0733	1.7282
SA9	Span 4	Start	UDCONU2	-58.0733	1.7282
SA9	Span 4	Middle	UDCONU2	-71.3371	2.1335
SA9	Span 4	End	UDCONU2	-89.2143	2.6867
SA9	Span 5	Start	UDCONU2	-89.2143	2.6867
SA9	Span 5	Middle	UDCONU2	-32.6924	0.9630
SA9	Span 5	End	UDCONU2	-1.4211	0.0516
SA9	Span 6	Start	UDCONU2	-1.4211	0.0516
SA9	Span 6	Middle	UDCONU1	-0.0418	0.0000
SA9	Span 6	End		0.0000	0.0000
SA9	Span 7	Start		0.0000	0.0000
SA9	Span 7	Middle		0.0000	0.0000
SA9	Span 7	End		0.0000	0.0000
SA9	Span 8	Start		0.0000	0.0000
SA9	Span 8	Middle		0.0000	0.0000
SA9	Span 8	End	UDCONU2	-7.7366	0.2305
SA9	Span 9	Start	UDCONU2	-7.7366	0.2305
SA9	Span 9	Middle		0.0000	0.0000
SA9	Span 9	End	UDCONU2	-9.9768	0.2919
SA9	Span 10	Start	UDCONU2	-9.9768	0.2919
SA9	Span 10	Middle	UDCONU2	-0.9761	0.0343
SA9	Span 10	End		0.0000	0.0000
SA9	Span 11	Start		0.0000	0.0000
SA9	Span 11	Middle		0.0000	0.0000
SA9	Span 11	End		0.0000	0.0000
SA9	Span 12	Start		0.0000	0.0000
SA9	Span 12	Middle		0.0000	0.0000
SA9	Span 12	End		0.0000	0.0000
SB1	Span 1	Start	UDCONU2	-0.7128	0.0240
SB1	Span 1	Middle	UDCONU1	-0.8445	0.0316
SB1	Span 1	End		0.0000	0.0000
SB1	Span 2	Start		0.0000	0.0000
SB1	Span 2	Middle		0.0000	0.0000
SB1	Span 2	End		0.0000	0.0000
SB1	Span 3	Start		0.0000	0.0000
SB1	Span 3	Middle		0.0000	0.0000
SB1	Span 3	End		0.0000	0.0000
SB1	Span 4	Start		0.0000	0.0000
SB1	Span 4	Middle		0.0000	0.0000
SB1	Span 4	End		0.0000	0.0000
SB1	Span 5	Start		0.0000	0.0000
SB1	Span 5	Middle		0.0000	0.0000
SB1	Span 5	End		0.0000	0.0000
SB1	Span 6	Start		0.0000	0.0000
SB1	Span 6	Middle		0.0000	0.0000
SB1	Span 6	End		0.0000	0.0000
SB1	Span 7	Start		0.0000	0.0000
SB1	Span 7	Middle		0.0000	0.0000
SB1	Span 7	End		0.0000	0.0000
SB1	Span 8	Start		0.0000	0.0000
SB1	Span 8	Middle		0.0000	0.0000
SB1	Span 8	End		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB1	Span 9	Start		0.0000	0.0000
SB1	Span 9	Middle		0.0000	0.0000
SB1	Span 9	End		0.0000	0.0000
SB1	Span 10	Start		0.0000	0.0000
SB1	Span 10	Middle		0.0000	0.0000
SB1	Span 10	End		0.0000	0.0000
SB2	Span 1	Start	UDCONU2	-6.8624	0.2200
SB2	Span 1	Middle		0.0000	0.0000
SB2	Span 1	End	UDCONU2	-14.1816	0.4501
SB2	Span 2	Start	UDCONU2	-14.1816	0.4501
SB2	Span 2	Middle	UDCONU1	-3.5527	0.1443
SB2	Span 2	End	UDCONU2	-0.3324	0.0261
SB2	Span 3	Start	UDCONU2	-0.7204	0.0348
SB2	Span 3	Middle	UDCONU2	-2.1259	0.0750
SB2	Span 3	End		0.0000	0.0000
SB2	Span 4	Start		0.0000	0.0000
SB2	Span 4	Middle		0.0000	0.0000
SB2	Span 4	End		0.0000	0.0000
SB2	Span 5	Start		0.0000	0.0000
SB2	Span 5	Middle		0.0000	0.0000
SB2	Span 5	End		0.0000	0.0000
SB2	Span 6	Start		0.0000	0.0000
SB2	Span 6	Middle		0.0000	0.0000
SB2	Span 6	End		0.0000	0.0000
SB2	Span 7	Start		0.0000	0.0000
SB2	Span 7	Middle		0.0000	0.0000
SB2	Span 7	End		0.0000	0.0000
SB2	Span 8	Start		0.0000	0.0000
SB2	Span 8	Middle		0.0000	0.0000
SB2	Span 8	End		0.0000	0.0000
SB2	Span 9	Start		0.0000	0.0000
SB2	Span 9	Middle		0.0000	0.0000
SB2	Span 9	End		0.0000	0.0000
SB2	Span 10	Start		0.0000	0.0000
SB2	Span 10	Middle		0.0000	0.0000
SB2	Span 10	End		0.0000	0.0000
SB2	Span 11	Start		0.0000	0.0000
SB2	Span 11	Middle		0.0000	0.0000
SB2	Span 11	End		0.0000	0.0000
SB3	Span 1	Start	UDCONU2	-11.4081	0.3603
SB3	Span 1	Middle	UDCONU2	-20.0665	0.6370
SB3	Span 1	End	UDCONU1	-0.5233	0.0377
SB3	Span 2	Start	UDCONU2	-0.4312	0.0371
SB3	Span 2	Middle	UDCONU2	-11.7736	0.3997
SB3	Span 2	End	UDCONU2	-18.4275	0.6480
SB3	Span 3	Start	UDCONU2	-18.4275	0.6480
SB3	Span 3	Middle	UDCONU2	-14.5378	0.4921
SB3	Span 3	End		0.0000	0.0000
SB3	Span 4	Start		0.0000	0.0000
SB3	Span 4	Middle		0.0000	0.0000
SB3	Span 4	End		0.0000	0.0000
SB3	Span 5	Start		0.0000	0.0000
SB3	Span 5	Middle		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB3	Span 5	End		0.0000	0.0000
SB3	Span 6	Start		0.0000	0.0000
SB3	Span 6	Middle		0.0000	0.0000
SB3	Span 6	End		0.0000	0.0000
SB3	Span 7	Start		0.0000	0.0000
SB3	Span 7	Middle		0.0000	0.0000
SB3	Span 7	End		0.0000	0.0000
SB3	Span 8	Start		0.0000	0.0000
SB3	Span 8	Middle		0.0000	0.0000
SB3	Span 8	End		0.0000	0.0000
SB3	Span 9	Start		0.0000	0.0000
SB3	Span 9	Middle		0.0000	0.0000
SB3	Span 9	End		0.0000	0.0000
SB3	Span 10	Start		0.0000	0.0000
SB3	Span 10	Middle		0.0000	0.0000
SB3	Span 10	End		0.0000	0.0000
SB4	Span 1	Start	UDCONU2	-12.9348	0.4070
SB4	Span 1	Middle	UDCONU2	-26.2287	0.8321
SB4	Span 1	End	UDCONU2	-0.3886	0.0000
SB4	Span 2	Start		0.0000	0.0000
SB4	Span 2	Middle	UDCONU2	-2.1487	0.1009
SB4	Span 2	End	UDCONU2	-21.6311	0.7349
SB4	Span 3	Start	UDCONU2	-21.6311	0.7349
SB4	Span 3	Middle	UDCONU2	-42.9145	1.4132
SB4	Span 3	End	UDCONU2	-96.9316	3.1659
SB4	Span 4	Start	UDCONU2	-96.9316	3.1659
SB4	Span 4	Middle	UDCONU2	-57.2280	1.8310
SB4	Span 4	End	UDCONU2	-39.4847	1.2560
SB4	Span 5	Start	UDCONU2	-39.4847	1.2560
SB4	Span 5	Middle	UDCONU2	-25.3989	0.8172
SB4	Span 5	End	UDCONU2	-2.8399	0.1030
SB4	Span 6	Start	UDCONU2	-0.3615	0.0188
SB4	Span 6	Middle		0.0000	0.0000
SB4	Span 6	End		0.0000	0.0000
SB4	Span 7	Start		0.0000	0.0000
SB4	Span 7	Middle		0.0000	0.0000
SB4	Span 7	End		0.0000	0.0000
SB4	Span 8	Start		0.0000	0.0000
SB4	Span 8	Middle		0.0000	0.0000
SB4	Span 8	End		0.0000	0.0000
SB4	Span 9	Start		0.0000	0.0000
SB4	Span 9	Middle		0.0000	0.0000
SB4	Span 9	End		0.0000	0.0000
SB4	Span 10	Start		0.0000	0.0000
SB4	Span 10	Middle		0.0000	0.0000
SB4	Span 10	End		0.0000	0.0000
SB4	Span 11	Start		0.0000	0.0000
SB4	Span 11	Middle		0.0000	0.0000
SB4	Span 11	End		0.0000	0.0000
SB5	Span 1	Start	UDCONU2	-9.5081	0.2981
SB5	Span 1	Middle	UDCONU2	-26.5734	0.8408
SB5	Span 1	End		0.0000	0.0000
SB5	Span 2	Start		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB5	Span 2	Middle		0.0000	0.0000
SB5	Span 2	End	UDCONU2	-8.4510	0.2908
SB5	Span 3	Start	UDCONU2	-37.6472	1.2237
SB5	Span 3	Middle	UDCONU2	-82.5630	2.7055
SB5	Span 3	End	UDCONU2	-51.8373	1.7017
SB5	Span 4	Start	UDCONU2	-51.8373	1.7017
SB5	Span 4	Middle	UDCONU2	-50.5362	1.6403
SB5	Span 4	End	UDCONU2	-1.0373	0.0471
SB5	Span 5	Start	UDCONU2	-1.0373	0.0471
SB5	Span 5	Middle		0.0000	0.0000
SB5	Span 5	End		0.0000	0.0000
SB5	Span 6	Start		0.0000	0.0000
SB5	Span 6	Middle		0.0000	0.0000
SB5	Span 6	End		0.0000	0.0000
SB5	Span 7	Start		0.0000	0.0000
SB5	Span 7	Middle		0.0000	0.0000
SB5	Span 7	End		0.0000	0.0000
SB5	Span 8	Start		0.0000	0.0000
SB5	Span 8	Middle		0.0000	0.0000
SB5	Span 8	End		0.0000	0.0000
SB5	Span 9	Start		0.0000	0.0000
SB5	Span 9	Middle		0.0000	0.0000
SB5	Span 9	End		0.0000	0.0000
SB5	Span 10	Start		0.0000	0.0000
SB5	Span 10	Middle		0.0000	0.0000
SB5	Span 10	End		0.0000	0.0000
SB6	Span 1	Start	UDCONU2	-4.2480	0.1334
SB6	Span 1	Middle	UDCONU2	-22.1808	0.6987
SB6	Span 1	End		0.0000	0.0000
SB6	Span 2	Start		0.0000	0.0000
SB6	Span 2	Middle		0.0000	0.0000
SB6	Span 2	End	UDCONU2	-4.4989	0.1520
SB6	Span 3	Start	UDCONU2	-4.4989	0.1520
SB6	Span 3	Middle	UDCONU2	-71.3084	2.3178
SB6	Span 3	End	UDCONU2	-11.2960	0.3853
SB6	Span 4	Start	UDCONU2	-11.2960	0.3853
SB6	Span 4	Middle		0.0000	0.0000
SB6	Span 4	End		0.0000	0.0000
SB6	Span 5	Start		0.0000	0.0000
SB6	Span 5	Middle		0.0000	0.0000
SB6	Span 5	End	UDCONU2	-37.5789	1.2185
SB6	Span 6	Start	UDCONU2	-33.1249	1.0767
SB6	Span 6	Middle	UDCONU2	-21.0246	0.6784
SB6	Span 6	End	UDCONU2	-12.3404	0.3869
SB6	Span 7	Start	UDCONU2	-12.3404	0.3869
SB6	Span 7	Middle	UDCONU1	-2.9056	0.0907
SB6	Span 7	End	UDCONU1	-0.1071	0.0000
SB6	Span 8	Start	UDCONU1	-0.1071	0.0000
SB6	Span 8	Middle	UDCONU2	-1.3091	0.0409
SB6	Span 8	End	UDCONU2	-2.5725	0.0805
SB6	Span 9	Start	UDCONU2	-3.2847	0.1032
SB6	Span 9	Middle	UDCONU2	-3.2847	0.1032
SB6	Span 9	End		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB6	Span 10	Start		0.0000	0.0000
SB6	Span 10	Middle		0.0000	0.0000
SB6	Span 10	End		0.0000	0.0000
SB6	Span 11	Start		0.0000	0.0000
SB6	Span 11	Middle		0.0000	0.0000
SB6	Span 11	End		0.0000	0.0000
SB6	Span 12	Start		0.0000	0.0000
SB6	Span 12	Middle		0.0000	0.0000
SB6	Span 12	End		0.0000	0.0000
SB7	Span 1	Start	UDCONU2	-3.6946	0.1159
SB7	Span 1	Middle	UDCONU2	-21.3741	0.6729
SB7	Span 1	End		0.0000	0.0000
SB7	Span 2	Start		0.0000	0.0000
SB7	Span 2	Middle		0.0000	0.0000
SB7	Span 2	End	UDCONU2	-4.3155	0.1348
SB7	Span 3	Start	UDCONU2	-23.2065	0.7311
SB7	Span 3	Middle	UDCONU2	-67.6467	2.1749
SB7	Span 3	End	UDCONU2	-13.8295	0.4353
SB7	Span 4	Start		0.0000	0.0000
SB7	Span 4	Middle	UDCONU2	-23.9707	0.8195
SB7	Span 4	End	UDCONU2	-23.1317	0.7564
SB7	Span 5	Start	UDCONU2	-22.1750	0.7443
SB7	Span 5	Middle	UDCONU2	-18.3949	0.5878
SB7	Span 5	End	UDCONU2	-10.2478	0.3210
SB7	Span 6	Start	UDCONU2	-10.2478	0.3210
SB7	Span 6	Middle	UDCONU2	-8.1673	0.2556
SB7	Span 6	End	UDCONU2	-10.5741	0.3313
SB7	Span 7	Start	UDCONU2	-22.7659	0.7171
SB7	Span 7	Middle	UDCONU2	-22.7659	0.7171
SB7	Span 7	End	UDCONU2	-13.4190	0.4217
SB7	Span 8	Start	UDCONU2	-7.9983	0.2510
SB7	Span 8	Middle	UDCONU1	-1.6330	0.0511
SB7	Span 8	End		0.0000	0.0000
SB7	Span 9	Start		0.0000	0.0000
SB7	Span 9	Middle		0.0000	0.0000
SB7	Span 9	End		0.0000	0.0000
SB7	Span 10	Start		0.0000	0.0000
SB7	Span 10	Middle		0.0000	0.0000
SB7	Span 10	End		0.0000	0.0000
SB8	Span 1	Start	UDCONU2	-9.4923	0.2978
SB8	Span 1	Middle	UDCONU2	-27.0248	0.8532
SB8	Span 1	End		0.0000	0.0000
SB8	Span 2	Start		0.0000	0.0000
SB8	Span 2	Middle		0.0000	0.0000
SB8	Span 2	End	UDCONU2	-8.7302	0.2733
SB8	Span 3	Start	UDCONU2	-8.7302	0.2733
SB8	Span 3	Middle	UDCONU2	-39.4982	1.2535
SB8	Span 3	End	UDCONU2	-80.2994	2.5973
SB8	Span 4	Start	UDCONU2	-80.2994	2.5973
SB8	Span 4	Middle		0.0000	0.0000
SB8	Span 4	End	UDCONU2	-70.8239	2.2856
SB8	Span 5	Start	UDCONU2	-70.8239	2.2856
SB8	Span 5	Middle		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB8	Span 5	End	UDCONU2	-27.5120	0.9204
SB8	Span 6	Start	UDCONU2	-27.5120	0.9204
SB8	Span 6	Middle		0.0000	0.0000
SB8	Span 6	End	UDCONU2	-13.9425	0.4822
SB8	Span 7	Start	UDCONU2	-13.9425	0.4822
SB8	Span 7	Middle	UDCONU2	-7.3222	0.2594
SB8	Span 7	End	UDCONU2	-10.2245	0.3481
SB8	Span 8	Start	UDCONU2	-10.2245	0.3481
SB8	Span 8	Middle		0.0000	0.0000
SB8	Span 8	End	UDCONU2	-22.1163	0.7071
SB8	Span 9	Start	UDCONU2	-22.1163	0.7071
SB8	Span 9	Middle		0.0000	0.0000
SB8	Span 9	End	UDCONU2	-15.0815	0.4781
SB8	Span 10	Start	UDCONU2	-15.0815	0.4781
SB8	Span 10	Middle	UDCONU2	-10.1201	0.3287
SB8	Span 10	End	UDCONU2	-7.0704	0.2212
SB8	Span 11	Start	UDCONU2	-7.0704	0.2212
SB8	Span 11	Middle	UDCONU2	-11.6065	0.3638
SB8	Span 11	End	UDCONU2	-30.2652	0.9565
SB8	Span 12	Start	UDCONU2	-30.2652	0.9565
SB8	Span 12	Middle	UDCONU2	-23.8983	0.7532
SB8	Span 12	End	UDCONU2	-24.9104	0.7854
SB8	Span 13	Start	UDCONU2	-24.9104	0.7854
SB8	Span 13	Middle		0.0000	0.0000
SB8	Span 13	End	UDCONU2	-31.7645	1.0046
SB8	Span 14	Start	UDCONU2	-31.7645	1.0046
SB8	Span 14	Middle	UDCONU2	-16.3777	0.5144
SB8	Span 14	End	UDCONU2	-17.2210	0.5411
SB8	Span 15	Start	UDCONU2	-17.2210	0.5411
SB8	Span 15	Middle		0.0000	0.0000
SB8	Span 15	End	UDCONU2	-29.5121	0.9324
SB8	Span 16	Start	UDCONU2	-29.5121	0.9324
SB8	Span 16	Middle		0.0000	0.0000
SB8	Span 16	End	UDCONU2	-20.7507	0.6531
SB8	Span 17	Start	UDCONU2	-20.7507	0.6531
SB8	Span 17	Middle	UDCONU2	-16.3693	0.5142
SB8	Span 17	End	UDCONU2	-12.1649	0.3814
SB8	Span 18	Start	UDCONU2	-12.1649	0.3814
SB8	Span 18	Middle		0.0000	0.0000
SB8	Span 18	End	UDCONU2	-8.8127	0.2759
SB8	Span 19	Start	UDCONU2	-8.8127	0.2759
SB8	Span 19	Middle		0.0000	0.0000
SB8	Span 19	End	UDCONU1	-3.1736	0.0991
SB8	Span 20	Start	UDCONU1	-3.1736	0.0991
SB8	Span 20	Middle	UDCONU2	-3.9207	0.1225
SB8	Span 20	End	UDCONU2	-3.3893	0.1059
SB8	Span 21	Start	UDCONU2	-3.3893	0.1059
SB8	Span 21	Middle	UDCONU2	-6.3879	0.2008
SB8	Span 21	End	UDCONU2	-3.0168	0.0947
SB8	Span 22	Start	UDCONU2	-3.0168	0.0947
SB8	Span 22	Middle		0.0000	0.0000
SB8	Span 22	End		0.0000	0.0000
SB9	Span 1	Start	UDCONU2	-14.2635	0.4495

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB9	Span 1	Middle	UDCONU2	-29.3286	0.9297
SB9	Span 1	End	UDCONU2	-1.7487	0.0610
SB9	Span 2	Start		0.0000	0.0000
SB9	Span 2	Middle		0.0000	0.0000
SB9	Span 2	End	UDCONU2	-22.2036	0.7028
SB9	Span 3	Start	UDCONU2	-22.2036	0.7028
SB9	Span 3	Middle	UDCONU2	-62.4638	2.0062
SB9	Span 3	End	UDCONU2	-84.2543	2.7304
SB9	Span 4	Start	UDCONU2	-84.2543	2.7304
SB9	Span 4	Middle	UDCONU2	-33.4148	1.0576
SB9	Span 4	End	UDCONU2	-5.1233	0.1601
SB9	Span 5	Start	UDCONU2	-5.1233	0.1601
SB9	Span 5	Middle	UDCONU2	-1.7033	0.0532
SB9	Span 5	End		0.0000	0.0000
SB9	Span 6	Start		0.0000	0.0000
SB9	Span 6	Middle	UDCONU2	-3.2709	0.1022
SB9	Span 6	End	UDCONU2	-7.5780	0.2371
SB9	Span 7	Start	UDCONU2	-7.5780	0.2371
SB9	Span 7	Middle	UDCONU2	-18.1064	0.5692
SB9	Span 7	End	UDCONU2	-29.0312	0.9170
SB9	Span 8	Start	UDCONU2	-33.7741	1.0691
SB9	Span 8	Middle	UDCONU2	-42.4065	1.3476
SB9	Span 8	End	UDCONU2	-10.1269	0.3172
SB9	Span 9	Start	UDCONU2	-10.1269	0.3172
SB9	Span 9	Middle	UDCONU1	-0.0801	0.0000
SB9	Span 9	End	UDCONU2	-0.6163	0.0000
SB9	Span 10	Start	UDCONU2	-0.6163	0.0000
SB9	Span 10	Middle	UDCONU2	-11.4631	0.3593
SB9	Span 10	End	UDCONU2	-30.4144	0.9613
SB9	Span 11	Start	UDCONU2	-30.4144	0.9613
SB9	Span 11	Middle		0.0000	0.0000
SB9	Span 11	End	UDCONU2	-17.0035	0.5343
SB9	Span 12	Start	UDCONU2	-12.5192	0.3927
SB9	Span 12	Middle		0.0000	0.0000
SB9	Span 12	End	UDCONU2	-5.2275	0.1658
SA10	Span 1	Start		0.0000	0.0000
SA10	Span 1	Middle		0.0000	0.0000
SA10	Span 1	End		0.0000	0.0000
SA10	Span 2	Start		0.0000	0.0000
SA10	Span 2	Middle		0.0000	0.0000
SA10	Span 2	End	UDCONU2	-1.2948	0.0518
SA10	Span 3	Start	UDCONU2	-1.2948	0.0518
SA10	Span 3	Middle	UDCONU2	-30.4362	0.8818
SA10	Span 3	End	UDCONU2	-64.6066	1.8782
SA10	Span 4	Start	UDCONU2	-64.6066	1.8782
SA10	Span 4	Middle	UDCONU2	-88.8538	2.6111
SA10	Span 4	End	UDCONU2	-119.2270	3.5347
SA10	Span 5	Start	UDCONU2	-119.2270	3.5347
SA10	Span 5	Middle	UDCONU2	-45.2642	1.3192
SA10	Span 5	End	UDCONU2	-3.3661	0.1086
SA10	Span 6	Start	UDCONU2	-3.3661	0.1086
SA10	Span 6	Middle	UDCONU1	-0.2771	0.0000
SA10	Span 6	End		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA10	Span 7	Start		0.0000	0.0000
SA10	Span 7	Middle		0.0000	0.0000
SA10	Span 7	End		0.0000	0.0000
SA10	Span 8	Start		0.0000	0.0000
SA10	Span 8	Middle		0.0000	0.0000
SA10	Span 8	End		0.0000	0.0000
SA10	Span 9	Start		0.0000	0.0000
SA10	Span 9	Middle		0.0000	0.0000
SA10	Span 9	End	UDCONU2	-8.6877	0.2577
SA10	Span 10	Start	UDCONU2	-8.6877	0.2577
SA10	Span 10	Middle	UDCONU2	-0.6843	0.0242
SA10	Span 10	End		0.0000	0.0000
SA10	Span 11	Start		0.0000	0.0000
SA10	Span 11	Middle		0.0000	0.0000
SA10	Span 11	End		0.0000	0.0000
SA10	Span 12	Start		0.0000	0.0000
SA10	Span 12	Middle		0.0000	0.0000
SA10	Span 12	End		0.0000	0.0000
SA11	Span 1	Start		0.0000	0.0000
SA11	Span 1	Middle		0.0000	0.0000
SA11	Span 1	End		0.0000	0.0000
SA11	Span 2	Start		0.0000	0.0000
SA11	Span 2	Middle		0.0000	0.0000
SA11	Span 2	End		0.0000	0.0000
SA11	Span 3	Start	UDCONU2	-0.6376	0.0209
SA11	Span 3	Middle	UDCONU2	-22.0368	0.6283
SA11	Span 3	End	UDCONU2	-53.7113	1.5489
SA11	Span 4	Start	UDCONU2	-53.7113	1.5489
SA11	Span 4	Middle	UDCONU2	-83.3580	2.4335
SA11	Span 4	End	UDCONU2	-119.6651	3.5399
SA11	Span 5	Start	UDCONU2	-119.6651	3.5399
SA11	Span 5	Middle	UDCONU2	-54.6754	1.5893
SA11	Span 5	End	UDCONU1	-12.0931	0.3572
SA11	Span 6	Start	UDCONU1	-12.0931	0.3572
SA11	Span 6	Middle	UDCONU1	-0.2676	0.0000
SA11	Span 6	End	UDCONU2	-0.3848	0.0000
SA11	Span 7	Start	UDCONU2	-0.3848	0.0000
SA11	Span 7	Middle		0.0000	0.0000
SA11	Span 7	End	UDCONU2	-2.0018	0.0565
SA11	Span 8	Start	UDCONU2	-2.0018	0.0565
SA11	Span 8	Middle		0.0000	0.0000
SA11	Span 8	End		0.0000	0.0000
SA11	Span 9	Start		0.0000	0.0000
SA11	Span 9	Middle	UDCONU2	-0.0977	0.0000
SA11	Span 9	End	UDCONU2	-2.3472	0.0765
SA11	Span 10	Start	UDCONU2	-2.3472	0.0765
SA11	Span 10	Middle	UDCONU2	-0.2988	0.0000
SA11	Span 10	End		0.0000	0.0000
SA11	Span 11	Start		0.0000	0.0000
SA11	Span 11	Middle		0.0000	0.0000
SA11	Span 11	End		0.0000	0.0000
SA11	Span 12	Start		0.0000	0.0000
SA11	Span 12	Middle		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA11	Span 12	End		0.0000	0.0000
SA12	Span 1	Start		0.0000	0.0000
SA12	Span 1	Middle		0.0000	0.0000
SA12	Span 1	End		0.0000	0.0000
SA12	Span 2	Start		0.0000	0.0000
SA12	Span 2	Middle		0.0000	0.0000
SA12	Span 2	End		0.0000	0.0000
SA12	Span 3	Start		0.0000	0.0000
SA12	Span 3	Middle	UDCONU2	-21.8963	0.6265
SA12	Span 3	End	UDCONU2	-49.1443	1.4145
SA12	Span 4	Start	UDCONU2	-49.1443	1.4145
SA12	Span 4	Middle	UDCONU2	-67.8775	1.9701
SA12	Span 4	End	UDCONU2	-93.5596	2.7404
SA12	Span 5	Start	UDCONU2	-93.5596	2.7404
SA12	Span 5	Middle	UDCONU2	-50.3901	1.4657
SA12	Span 5	End	UDCONU2	-22.1151	0.6476
SA12	Span 6	Start	UDCONU2	-22.1151	0.6476
SA12	Span 6	Middle	UDCONU1	-7.5992	0.2313
SA12	Span 6	End	UDCONU2	-3.6679	0.1053
SA12	Span 7	Start	UDCONU2	-3.6679	0.1053
SA12	Span 7	Middle		0.0000	0.0000
SA12	Span 7	End	UDCONU2	-8.8753	0.2512
SA12	Span 8	Start	UDCONU2	-8.8753	0.2512
SA12	Span 8	Middle	UDCONU2	-0.7430	0.0000
SA12	Span 8	End		0.0000	0.0000
SA12	Span 9	Start		0.0000	0.0000
SA12	Span 9	Middle		0.0000	0.0000
SA12	Span 9	End	UDCONU2	-0.4696	0.0000
SA12	Span 10	Start	UDCONU2	-0.4696	0.0000
SA12	Span 10	Middle		0.0000	0.0000
SA12	Span 10	End	UDCONU2	-1.2357	0.0400
SA12	Span 11	Start	UDCONU2	-1.2357	0.0400
SA12	Span 11	Middle		0.0000	0.0000
SA12	Span 11	End		0.0000	0.0000
SA12	Span 12	Start		0.0000	0.0000
SA12	Span 12	Middle		0.0000	0.0000
SA12	Span 12	End		0.0000	0.0000
SA13	Span 1	Start		0.0000	0.0000
SA13	Span 1	Middle		0.0000	0.0000
SA13	Span 1	End		0.0000	0.0000
SA13	Span 2	Start		0.0000	0.0000
SA13	Span 2	Middle		0.0000	0.0000
SA13	Span 2	End		0.0000	0.0000
SA13	Span 3	Start		0.0000	0.0000
SA13	Span 3	Middle	UDCONU2	-11.4816	0.3296
SA13	Span 3	End	UDCONU2	-43.9260	1.2677
SA13	Span 4	Start	UDCONU2	-43.9260	1.2677
SA13	Span 4	Middle	UDCONU2	-55.7640	1.6245
SA13	Span 4	End	UDCONU2	-76.3937	2.2313
SA13	Span 5	Start	UDCONU2	-76.3937	2.2313
SA13	Span 5	Middle	UDCONU2	-29.4628	0.8717
SA13	Span 5	End	UDCONU2	-22.7500	0.6816
SA13	Span 6	Start	UDCONU2	-22.7500	0.6816

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA13	Span 6	Middle	UDCONU2	-20.1478	0.6095
SA13	Span 6	End	UDCONU2	-22.6253	0.6807
SA13	Span 7	Start	UDCONU2	-22.6253	0.6807
SA13	Span 7	Middle		0.0000	0.0000
SA13	Span 7	End	UDCONU2	-7.1471	0.2140
SA13	Span 8	Start	UDCONU2	-7.1471	0.2140
SA13	Span 8	Middle	UDCONU2	-23.8314	0.6782
SA13	Span 8	End	UDCONU2	-0.7528	0.0000
SA13	Span 9	Start	UDCONU2	-0.7528	0.0000
SA13	Span 9	Middle		0.0000	0.0000
SA13	Span 9	End	UDCONU2	-1.4432	0.0407
SA13	Span 10	Start	UDCONU2	-1.4432	0.0407
SA13	Span 10	Middle		0.0000	0.0000
SA13	Span 10	End	UDCONU2	-1.8927	0.0534
SA13	Span 11	Start	UDCONU2	-1.8927	0.0534
SA13	Span 11	Middle	UDCONU2	-18.9862	0.5500
SA13	Span 11	End	UDCONU2	-9.2871	0.2680
SA13	Span 12	Start	UDCONU2	-3.6293	0.1105
SA13	Span 12	Middle	UDCONU2	-2.0066	0.0607
SA13	Span 12	End	UDCONU2	-0.5181	0.0150
SA14	Span 1	Start		0.0000	0.0000
SA14	Span 1	Middle		0.0000	0.0000
SA14	Span 1	End		0.0000	0.0000
SA14	Span 2	Start		0.0000	0.0000
SA14	Span 2	Middle		0.0000	0.0000
SA14	Span 2	End		0.0000	0.0000
SA14	Span 3	Start		0.0000	0.0000
SA14	Span 3	Middle		0.0000	0.0000
SA14	Span 3	End	UDCONU2	-42.3202	1.2142
SA14	Span 4	Start	UDCONU2	-42.3202	1.2142
SA14	Span 4	Middle	UDCONU2	-51.9709	1.4944
SA14	Span 4	End	UDCONU2	-66.6347	1.9282
SA14	Span 5	Start	UDCONU2	-66.6347	1.9282
SA14	Span 5	Middle		0.0000	0.0000
SA14	Span 5	End	UDCONU2	-60.3743	1.7509
SA14	Span 6	Start	UDCONU2	-60.3743	1.7509
SA14	Span 6	Middle		0.0000	0.0000
SA14	Span 6	End	UDCONU2	-53.9081	1.5629
SA14	Span 7	Start	UDCONU2	-53.9081	1.5629
SA14	Span 7	Middle	UDCONU2	-11.5738	0.3964
SA14	Span 7	End	UDCONU2	-15.3808	0.4870
SA14	Span 8	Start	UDCONU2	-15.3808	0.4870
SA14	Span 8	Middle	UDCONU2	-20.3298	0.6209
SA14	Span 8	End	UDCONU2	-43.6944	1.3323
SA14	Span 9	Start	UDCONU2	-43.6944	1.3323
SA14	Span 9	Middle		0.0000	0.0000
SA14	Span 9	End	UDCONU2	-78.7234	2.3793
SA14	Span 10	Start	UDCONU2	-78.7234	2.3793
SA14	Span 10	Middle		0.0000	0.0000
SA14	Span 10	End	UDCONU2	-83.5270	2.4306
SA14	Span 11	Start	UDCONU2	-83.5270	2.4306
SA14	Span 11	Middle		0.0000	0.0000
SA14	Span 11	End	UDCONU2	-92.5496	2.7026

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA14	Span 12	Start	UDCONU2	-92.5496	2.7026
SA14	Span 12	Middle	UDCONU2	-41.7938	1.1972
SA14	Span 12	End	UDCONU2	-1.0550	0.0298
SA14	Span 13	Start	UDCONU2	-1.0550	0.0298
SA14	Span 13	Middle	UDCONU2	-4.3775	0.1237
SA14	Span 13	End	UDCONU2	-39.5464	1.1319
SA14	Span 14	Start	UDCONU2	-39.5464	1.1319
SA14	Span 14	Middle		0.0000	0.0000
SA14	Span 14	End	UDCONU2	-68.7837	1.9903
SA14	Span 15	Start	UDCONU2	-68.7837	1.9903
SA14	Span 15	Middle		0.0000	0.0000
SA14	Span 15	End	UDCONU2	-117.8302	3.5064
SA14	Span 16	Start	UDCONU2	-117.8302	3.5064
SA14	Span 16	Middle		0.0000	0.0000
SA14	Span 16	End	UDCONU2	-90.2999	2.6657
SA14	Span 17	Start	UDCONU2	-90.2999	2.6657
SA14	Span 17	Middle	UDCONU2	-34.4485	1.0210
SA14	Span 17	End	UDCONU2	-7.0583	0.2130
SA14	Span 18	Start	UDCONU2	-7.0583	0.2130
SA14	Span 18	Middle	UDCONU2	-8.3513	0.2364
SA14	Span 18	End	UDCONU2	-17.5790	0.5016
SA14	Span 19	Start	UDCONU2	-17.5790	0.5016
SA14	Span 19	Middle		0.0000	0.0000
SA14	Span 19	End	UDCONU2	-7.1366	0.2034
SA15	Span 1	Start		0.0000	0.0000
SA15	Span 1	Middle		0.0000	0.0000
SA15	Span 1	End		0.0000	0.0000
SA15	Span 2	Start		0.0000	0.0000
SA15	Span 2	Middle		0.0000	0.0000
SA15	Span 2	End		0.0000	0.0000
SA15	Span 3	Start		0.0000	0.0000
SA15	Span 3	Middle		0.0000	0.0000
SA15	Span 3	End	UDCONU2	-23.0164	0.6618
SA15	Span 4	Start	UDCONU2	-23.0164	0.6618
SA15	Span 4	Middle	UDCONU2	-37.3634	1.0788
SA15	Span 4	End	UDCONU2	-42.5367	1.2306
SA15	Span 5	Start	UDCONU2	-42.5367	1.2306
SA15	Span 5	Middle		0.0000	0.0000
SA15	Span 5	End	UDCONU2	-43.0466	1.2525
SA15	Span 6	Start	UDCONU2	-43.0466	1.2525
SA15	Span 6	Middle		0.0000	0.0000
SA15	Span 6	End	UDCONU2	-43.8773	1.2701
SA15	Span 7	Start	UDCONU2	-43.8773	1.2701
SA15	Span 7	Middle	UDCONU2	-20.0757	0.5801
SA15	Span 7	End	UDCONU2	-8.8863	0.2624
SA15	Span 8	Start	UDCONU2	-8.8863	0.2624
SA15	Span 8	Middle	UDCONU2	-8.0662	0.2400
SA15	Span 8	End	UDCONU2	-19.3692	0.5515
SA15	Span 9	Start	UDCONU2	-19.3692	0.5515
SA15	Span 9	Middle	UDCONU2	-22.3393	0.6354
SA15	Span 9	End		0.0000	0.0000
SA15	Span 10	Start		0.0000	0.0000
SA15	Span 10	Middle		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA15	Span 10	End	UDCONU2	-15.4098	0.4372
SA15	Span 11	Start	UDCONU2	-20.0796	0.5707
SA15	Span 11	Middle	UDCONU2	-20.0796	0.5707
SA15	Span 11	End	UDCONU2	-8.5679	0.2425
SA15	Span 12	Start	UDCONU2	-6.0563	0.1713
SA15	Span 12	Middle	UDCONU2	-12.9028	0.3658
SA15	Span 12	End	UDCONU2	-22.3029	0.6362
SA15	Span 13	Start	UDCONU2	-22.3029	0.6362
SA15	Span 13	Middle		0.0000	0.0000
SA15	Span 13	End	UDCONU2	-10.9815	0.3117
SA16	Span 1	Start		0.0000	0.0000
SA16	Span 1	Middle		0.0000	0.0000
SA16	Span 1	End		0.0000	0.0000
SA16	Span 2	Start		0.0000	0.0000
SA16	Span 2	Middle		0.0000	0.0000
SA16	Span 2	End		0.0000	0.0000
SA16	Span 3	Start		0.0000	0.0000
SA16	Span 3	Middle		0.0000	0.0000
SA16	Span 3	End		0.0000	0.0000
SA16	Span 4	Start		0.0000	0.0000
SA16	Span 4	Middle	UDCONU2	-13.3399	0.3820
SA16	Span 4	End	UDCONU2	-27.4400	0.7858
SA16	Span 5	Start	UDCONU2	-30.2348	0.8660
SA16	Span 5	Middle	UDCONU2	-13.6165	0.3879
SA16	Span 5	End	UDCONU2	-1.7140	0.0493
SA16	Span 6	Start	UDCONU2	-1.7140	0.0493
SA16	Span 6	Middle	UDCONU1	-0.6169	0.0000
SA16	Span 6	End	UDCONU1	-0.0341	0.0000
SA16	Span 7	Start	UDCONU1	-0.0341	0.0000
SA16	Span 7	Middle	UDCONU1	-0.2968	0.0000
SA16	Span 7	End		0.0000	0.0000
SA16	Span 8	Start		0.0000	0.0000
SA16	Span 8	Middle		0.0000	0.0000
SA16	Span 8	End		0.0000	0.0000
SA16	Span 9	Start		0.0000	0.0000
SA16	Span 9	Middle	UDCONU1	-1.2091	0.0341
SA16	Span 9	End	UDCONU2	-1.2112	0.0342
SA16	Span 10	Start	UDCONU2	-1.2112	0.0342
SA16	Span 10	Middle	UDCONU2	-12.0967	0.3451
SA16	Span 10	End	UDCONU2	-40.8388	1.1767
SA16	Span 11	Start	UDCONU2	-40.8388	1.1767
SA16	Span 11	Middle		0.0000	0.0000
SA16	Span 11	End	UDCONU2	-5.2481	0.1535
SA17	Span 1	Start		0.0000	0.0000
SA17	Span 1	Middle		0.0000	0.0000
SA17	Span 1	End		0.0000	0.0000
SA17	Span 2	Start		0.0000	0.0000
SA17	Span 2	Middle		0.0000	0.0000
SA17	Span 2	End		0.0000	0.0000
SA17	Span 3	Start		0.0000	0.0000
SA17	Span 3	Middle		0.0000	0.0000
SA17	Span 3	End		0.0000	0.0000
SA17	Span 4	Start		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA17	Span 4	Middle	UDCONU2	-24.4822	0.6975
SA17	Span 4	End	UDCONU2	-2.6235	0.0741
SA17	Span 5	Start	UDCONU2	-2.6235	0.0741
SA17	Span 5	Middle	UDCONU2	-0.5545	0.0000
SA17	Span 5	End		0.0000	0.0000
SA17	Span 6	Start		0.0000	0.0000
SA17	Span 6	Middle		0.0000	0.0000
SA17	Span 6	End		0.0000	0.0000
SA17	Span 7	Start		0.0000	0.0000
SA17	Span 7	Middle		0.0000	0.0000
SA17	Span 7	End		0.0000	0.0000
SA17	Span 8	Start		0.0000	0.0000
SA17	Span 8	Middle		0.0000	0.0000
SA17	Span 8	End	UDCONU2	-1.3343	0.0468
SA17	Span 9	Start	UDCONU2	-1.3343	0.0468
SA17	Span 9	Middle	UDCONU2	-37.0748	1.0704
SA17	Span 9	End	UDCONU2	-7.0010	0.1998
SA18	Span 1	Start		0.0000	0.0000
SA18	Span 1	Middle		0.0000	0.0000
SA18	Span 1	End		0.0000	0.0000
SA18	Span 2	Start		0.0000	0.0000
SA18	Span 2	Middle		0.0000	0.0000
SA18	Span 2	End		0.0000	0.0000
SA18	Span 3	Start		0.0000	0.0000
SA18	Span 3	Middle		0.0000	0.0000
SA18	Span 3	End		0.0000	0.0000
SA18	Span 4	Start		0.0000	0.0000
SA18	Span 4	Middle	UDCONU2	-4.1604	0.1194
SA18	Span 4	End	UDCONU2	-19.9621	0.5709
SA18	Span 5	Start	UDCONU2	-7.5173	0.2161
SA18	Span 5	Middle	UDCONU2	-4.7252	0.1400
SA18	Span 5	End		0.0000	0.0000
SA18	Span 6	Start		0.0000	0.0000
SA18	Span 6	Middle	UDCONU2	-1.9165	0.0541
SA18	Span 6	End	UDCONU1	-0.3135	0.0000
SA18	Span 7	Start	UDCONU1	-0.3135	0.0000
SA18	Span 7	Middle	UDCONU2	-1.0405	0.0294
SA18	Span 7	End		0.0000	0.0000
SA18	Span 8	Start		0.0000	0.0000
SA18	Span 8	Middle	UDCONU2	-3.5748	0.1106
SA18	Span 8	End	UDCONU2	-7.3009	0.2218
SA18	Span 9	Start	UDCONU2	-7.3009	0.2218
SA18	Span 9	Middle	UDCONU2	-37.1737	1.0762
SA18	Span 9	End	UDCONU2	-11.5705	0.3297
SA19	Span 1	Start		0.0000	0.0000
SA19	Span 1	Middle		0.0000	0.0000
SA19	Span 1	End		0.0000	0.0000
SA19	Span 2	Start		0.0000	0.0000
SA19	Span 2	Middle		0.0000	0.0000
SA19	Span 2	End		0.0000	0.0000
SA19	Span 3	Start		0.0000	0.0000
SA19	Span 3	Middle		0.0000	0.0000
SA19	Span 3	End		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA19	Span 4	Start		0.0000	0.0000
SA19	Span 4	Middle	UDCONU2	-1.9452	0.0593
SA19	Span 4	End	UDCONU2	-18.5912	0.5371
SA19	Span 5	Start	UDCONU2	-12.4573	0.3732
SA19	Span 5	Middle	UDCONU2	-14.4660	0.4162
SA19	Span 5	End	UDCONU2	-1.9978	0.0564
SA19	Span 6	Start	UDCONU2	-1.9978	0.0564
SA19	Span 6	Middle	UDCONU2	-6.1968	0.1752
SA19	Span 6	End	UDCONU2	-4.5331	0.1360
SA19	Span 7	Start	UDCONU2	-4.5331	0.1360
SA19	Span 7	Middle	UDCONU2	-11.8582	0.3360
SA19	Span 7	End		0.0000	0.0000
SA19	Span 8	Start	UDCONU2	-1.6601	0.0469
SA19	Span 8	Middle	UDCONU2	-21.5371	0.6201
SA19	Span 8	End	UDCONU2	-16.7150	0.4989
SA19	Span 9	Start	UDCONU2	-14.9007	0.4494
SA19	Span 9	Middle	UDCONU2	-28.0601	0.8079
SA19	Span 9	End	UDCONU2	-10.7197	0.3066
SA20	Span 1	Start		0.0000	0.0000
SA20	Span 1	Middle		0.0000	0.0000
SA20	Span 1	End		0.0000	0.0000
SA20	Span 2	Start		0.0000	0.0000
SA20	Span 2	Middle		0.0000	0.0000
SA20	Span 2	End		0.0000	0.0000
SA20	Span 3	Start		0.0000	0.0000
SA20	Span 3	Middle		0.0000	0.0000
SA20	Span 3	End		0.0000	0.0000
SA20	Span 4	Start		0.0000	0.0000
SA20	Span 4	Middle		0.0000	0.0000
SA20	Span 4	End	UDCONU2	-5.9642	0.1849
SA20	Span 5	Start	UDCONU2	-5.9642	0.1849
SA20	Span 5	Middle		0.0000	0.0000
SA20	Span 5	End	UDCONU2	-5.2239	0.1652
SA20	Span 6	Start	UDCONU2	-5.2239	0.1652
SA20	Span 6	Middle	UDCONU2	-19.8495	0.5790
SA20	Span 6	End	UDCONU2	-32.7933	0.9363
SA20	Span 7	Start	UDCONU2	-32.7933	0.9363
SA20	Span 7	Middle		0.0000	0.0000
SA20	Span 7	End	UDCONU2	-33.9753	0.9705
SA20	Span 8	Start	UDCONU2	-33.9753	0.9705
SA20	Span 8	Middle		0.0000	0.0000
SA20	Span 8	End	UDCONU2	-9.2370	0.2615
SA20	Span 9	Start	UDCONU2	-9.2370	0.2615
SA20	Span 9	Middle	UDCONU2	-2.3142	0.0654
SA20	Span 9	End	UDCONU2	-25.3627	0.7222
SA20	Span 10	Start	UDCONU2	-25.3627	0.7222
SA20	Span 10	Middle		0.0000	0.0000
SA20	Span 10	End	UDCONU2	-26.9264	0.7672
SA20	Span 11	Start	UDCONU2	-26.9264	0.7672
SA20	Span 11	Middle		0.0000	0.0000
SA20	Span 11	End	UDCONU2	-13.9342	0.4320
SA20	Span 12	Start	UDCONU2	-13.9342	0.4320
SA20	Span 12	Middle		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA20	Span 12	End	UDCONU2	-33.6379	0.9607
SA20	Span 13	Start	UDCONU2	-33.6379	0.9607
SA20	Span 13	Middle		0.0000	0.0000
SA20	Span 13	End	UDCONU2	-34.1974	0.9769
SA20	Span 14	Start	UDCONU2	-34.1974	0.9769
SA20	Span 14	Middle	UDCONU2	-0.6658	0.0000
SA20	Span 14	End		0.0000	0.0000
SA20	Span 15	Start		0.0000	0.0000
SA20	Span 15	Middle	UDCONU2	-7.3451	0.2078
SA20	Span 15	End	UDCONU2	-42.8661	1.2284
SA20	Span 16	Start	UDCONU2	-42.8661	1.2284
SA20	Span 16	Middle		0.0000	0.0000
SA20	Span 16	End	UDCONU2	-42.7372	1.2247
SA20	Span 17	Start	UDCONU2	-42.7372	1.2247
SA20	Span 17	Middle	UDCONU2	-23.5013	0.7046
SA20	Span 17	End	UDCONU2	-19.4918	0.5854
SA20	Span 18	Start	UDCONU2	-19.4918	0.5854
SA20	Span 18	Middle	UDCONU2	-18.8529	0.5514
SA20	Span 18	End	UDCONU2	-16.0926	0.4674
SA20	Span 19	Start	UDCONU2	-16.0926	0.4674
SA20	Span 19	Middle		0.0000	0.0000
SA20	Span 19	End	UDCONU2	-7.7249	0.2210
SA21	Span 1	Start		0.0000	0.0000
SA21	Span 1	Middle		0.0000	0.0000
SA21	Span 1	End		0.0000	0.0000
SA21	Span 2	Start		0.0000	0.0000
SA21	Span 2	Middle		0.0000	0.0000
SA21	Span 2	End		0.0000	0.0000
SA21	Span 3	Start		0.0000	0.0000
SA21	Span 3	Middle		0.0000	0.0000
SA21	Span 3	End		0.0000	0.0000
SA21	Span 4	Start		0.0000	0.0000
SA21	Span 4	Middle		0.0000	0.0000
SA21	Span 4	End	UDCONU2	-3.4287	0.1138
SA21	Span 5	Start	UDCONU2	-0.0419	0.0000
SA21	Span 5	Middle	UDCONU2	-13.9668	0.4032
SA21	Span 5	End	UDCONU2	-6.7917	0.1926
SA21	Span 6	Start	UDCONU2	-6.7917	0.1926
SA21	Span 6	Middle	UDCONU2	-2.5968	0.0747
SA21	Span 6	End	UDCONU2	-8.3539	0.2509
SA21	Span 7	Start	UDCONU2	-8.3539	0.2509
SA21	Span 7	Middle	UDCONU2	-10.4783	0.2979
SA21	Span 7	End		0.0000	0.0000
SA21	Span 8	Start		0.0000	0.0000
SA21	Span 8	Middle	UDCONU2	-14.5535	0.4258
SA21	Span 8	End	UDCONU2	-10.4426	0.3073
SA21	Span 9	Start	UDCONU2	-10.4426	0.3073
SA21	Span 9	Middle	UDCONU2	-7.2882	0.2137
SA21	Span 9	End	UDCONU2	-2.9746	0.0853
SB10	Span 1	Start	UDCONU2	-7.0193	0.2219
SB10	Span 1	Middle		0.0000	0.0000
SB10	Span 1	End	UDCONU2	-14.5916	0.4580
SB10	Span 2	Start	UDCONU2	-14.5916	0.4580

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB10	Span 2	Middle	UDCONU2	-21.3333	0.6716
SB10	Span 2	End	UDCONU2	-2.6134	0.0906
SB10	Span 3	Start	UDCONU2	-2.6134	0.0906
SB10	Span 3	Middle	UDCONU1	-1.9518	0.0875
SB10	Span 3	End	UDCONU2	-31.4853	1.0072
SB10	Span 4	Start	UDCONU2	-31.4853	1.0072
SB10	Span 4	Middle	UDCONU2	-83.4882	2.7078
SB10	Span 4	End	UDCONU2	-117.1276	3.8579
SB10	Span 5	Start	UDCONU2	-117.1276	3.8579
SB10	Span 5	Middle		0.0000	0.0000
SB10	Span 5	End	UDCONU2	-56.4573	1.8057
SB10	Span 6	Start	UDCONU2	-56.4573	1.8057
SB10	Span 6	Middle	UDCONU2	-11.0877	0.3475
SB10	Span 6	End	UDCONU2	-1.4669	0.0458
SB10	Span 7	Start	UDCONU2	-1.4669	0.0458
SB10	Span 7	Middle		0.0000	0.0000
SB10	Span 7	End	UDCONU2	-0.0930	0.0000
SB10	Span 8	Start	UDCONU2	-0.0930	0.0000
SB10	Span 8	Middle		0.0000	0.0000
SB10	Span 8	End		0.0000	0.0000
SB10	Span 9	Start		0.0000	0.0000
SB10	Span 9	Middle		0.0000	0.0000
SB10	Span 9	End	UDCONU1	-0.2533	0.0000
SB10	Span 10	Start	UDCONU1	-0.2533	0.0000
SB10	Span 10	Middle	UDCONU2	-10.5697	0.3312
SB10	Span 10	End	UDCONU2	-33.0463	1.0457
SB10	Span 11	Start	UDCONU2	-53.0775	1.6950
SB10	Span 11	Middle	UDCONU2	-84.6453	2.7436
SB10	Span 11	End	UDCONU2	-13.9064	0.4363
SB10	Span 12	Start	UDCONU2	-13.9064	0.4363
SB10	Span 12	Middle		0.0000	0.0000
SB10	Span 12	End		0.0000	0.0000
SB10	Span 13	Start		0.0000	0.0000
SB10	Span 13	Middle	UDCONU2	-0.2254	0.0000
SB10	Span 13	End	UDCONU2	-15.8976	0.4993
SB10	Span 14	Start	UDCONU2	-15.8976	0.4993
SB10	Span 14	Middle	UDCONU2	-41.1168	1.3133
SB10	Span 14	End	UDCONU2	-14.0420	0.4476
SB11	Span 1	Start	UDCONU2	-0.3981	0.0124
SB11	Span 1	Middle	UDCONU2	-6.3366	0.1987
SB11	Span 1	End	UDCONU2	-5.0371	0.1816
SB11	Span 2	Start	UDCONU2	-5.0371	0.1816
SB11	Span 2	Middle	UDCONU1	-0.1619	0.0581
SB11	Span 2	End	UDCONU2	-0.0904	0.1326
SB11	Span 3	Start	UDCONU2	-0.0904	0.1326
SB11	Span 3	Middle		0.0000	0.0000
SB11	Span 3	End	UDCONU2	-0.7374	0.1652
SB11	Span 4	Start	UDCONU2	-0.7374	0.1652
SB11	Span 4	Middle	UDCONU2	-7.8418	0.3219
SB11	Span 4	End	UDCONU2	-18.8783	0.6562
SB11	Span 5	Start	UDCONU2	-18.8783	0.6562
SB11	Span 5	Middle	UDCONU2	-30.4253	0.9978
SB11	Span 5	End	UDCONU2	-58.9044	1.9058

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB11	Span 6	Start	UDCONU2	-58.9044	1.9058
SB11	Span 6	Middle	UDCONU2	-22.2359	0.7003
SB11	Span 6	End	UDCONU1	-4.1398	0.1293
SB11	Span 7	Start	UDCONU1	-4.1398	0.1293
SB11	Span 7	Middle		0.0000	0.0000
SB11	Span 7	End		0.0000	0.0000
SB11	Span 8	Start		0.0000	0.0000
SB11	Span 8	Middle		0.0000	0.0000
SB11	Span 8	End		0.0000	0.0000
SB11	Span 9	Start		0.0000	0.0000
SB11	Span 9	Middle		0.0000	0.0000
SB11	Span 9	End	UDCONU2	-0.0475	0.0000
SB11	Span 10	Start	UDCONU2	-0.0475	0.0000
SB11	Span 10	Middle	UDCONU2	-4.8413	0.1513
SB11	Span 10	End	UDCONU2	-20.4759	0.6469
SB11	Span 11	Start	UDCONU2	-20.4759	0.6469
SB11	Span 11	Middle	UDCONU2	-54.3017	1.7481
SB11	Span 11	End	UDCONU2	-182.9030	6.2746
SB11	Span 12	Start	UDCONU2	-182.9030	6.2746
SB11	Span 12	Middle	UDCONU2	-70.4650	2.2847
SB11	Span 12	End	UDCONU2	-14.1185	0.4430
SB11	Span 13	Start	UDCONU2	-14.1185	0.4430
SB11	Span 13	Middle		0.0000	0.0000
SB11	Span 13	End		0.0000	0.0000
SB11	Span 14	Start		0.0000	0.0000
SB11	Span 14	Middle		0.0000	0.0000
SB11	Span 14	End	UDCONU2	-0.4125	0.0000
SB11	Span 15	Start	UDCONU2	-0.4125	0.0000
SB11	Span 15	Middle		0.0000	0.0000
SB11	Span 15	End	UDCONU2	-38.5772	1.2257
SB11	Span 16	Start	UDCONU2	-38.5772	1.2257
SB11	Span 16	Middle		0.0000	0.0000
SB11	Span 16	End	UDCONU2	-14.6671	0.4623
SB12	Span 1	Start		0.0000	0.0000
SB12	Span 1	Middle		0.0000	0.0000
SB12	Span 1	End		0.0000	0.0000
SB12	Span 2	Start		0.0000	0.0000
SB12	Span 2	Middle	UDCONU2	-0.5492	0.0465
SB12	Span 2	End	UDCONU2	-5.7048	0.2442
SB12	Span 3	Start	UDCONU2	-5.7048	0.2442
SB12	Span 3	Middle	UDCONU2	-13.8052	0.5323
SB12	Span 3	End	UDCONU2	-42.7610	1.4551
SB12	Span 4	Start	UDCONU2	-42.7610	1.4551
SB12	Span 4	Middle		0.0000	0.0000
SB12	Span 4	End	UDCONU2	-62.4091	2.0016
SB12	Span 5	Start	UDCONU2	-62.4091	2.0016
SB12	Span 5	Middle		0.0000	0.0000
SB12	Span 5	End	UDCONU2	-52.2431	1.6677
SB12	Span 6	Start	UDCONU2	-52.2431	1.6677
SB12	Span 6	Middle	UDCONU2	-37.1599	1.1781
SB12	Span 6	End	UDCONU2	-15.4167	0.4841
SB12	Span 7	Start	UDCONU2	-15.4167	0.4841
SB12	Span 7	Middle	UDCONU2	-1.0615	0.0331

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB12	Span 7	End		0.0000	0.0000
SB12	Span 8	Start		0.0000	0.0000
SB12	Span 8	Middle		0.0000	0.0000
SB12	Span 8	End		0.0000	0.0000
SB12	Span 9	Start		0.0000	0.0000
SB12	Span 9	Middle	UDCONU2	-0.8851	0.0276
SB12	Span 9	End	UDCONU2	-18.7818	0.5906
SB12	Span 10	Start	UDCONU2	-18.7818	0.5906
SB12	Span 10	Middle	UDCONU2	-56.1074	1.8158
SB12	Span 10	End	UDCONU2	-371.3733	14.1818
SB12	Span 11	Start	UDCONU2	-371.3733	14.1818
SB12	Span 11	Middle		0.0000	0.0000
SB12	Span 11	End	UDCONU2	-354.7348	13.3690
SB12	Span 12	Start	UDCONU2	-354.7348	13.3690
SB12	Span 12	Middle	UDCONU2	-72.3938	2.3328
SB12	Span 12	End	UDCONU1	-11.6666	0.3657
SB12	Span 13	Start	UDCONU1	-11.6666	0.3657
SB12	Span 13	Middle		0.0000	0.0000
SB12	Span 13	End		0.0000	0.0000
SB12	Span 14	Start		0.0000	0.0000
SB12	Span 14	Middle		0.0000	0.0000
SB12	Span 14	End	UDCONU1	-0.2629	0.0000
SB12	Span 15	Start	UDCONU1	-0.2629	0.0000
SB12	Span 15	Middle		0.0000	0.0000
SB12	Span 15	End	UDCONU2	-28.4569	0.8987
SB12	Span 16	Start	UDCONU2	-28.4569	0.8987
SB12	Span 16	Middle		0.0000	0.0000
SB12	Span 16	End	UDCONU2	-6.9543	0.2203
SB13	Span 1	Start		0.0000	0.0000
SB13	Span 1	Middle		0.0000	0.0000
SB13	Span 1	End		0.0000	0.0000
SB13	Span 2	Start		0.0000	0.0000
SB13	Span 2	Middle		0.0000	0.0000
SB13	Span 2	End		0.0000	0.0000
SB13	Span 3	Start		0.0000	0.0000
SB13	Span 3	Middle	UDCONU2	-5.0276	0.1587
SB13	Span 3	End	UDCONU2	-10.4259	0.3284
SB13	Span 4	Start	UDCONU2	-10.4259	0.3284
SB13	Span 4	Middle	UDCONU2	-2.1194	0.0662
SB13	Span 4	End	UDCONU2	-0.7085	0.0000
SB13	Span 5	Start	UDCONU2	-0.7085	0.0000
SB13	Span 5	Middle		0.0000	0.0000
SB13	Span 5	End		0.0000	0.0000
SB13	Span 6	Start		0.0000	0.0000
SB13	Span 6	Middle		0.0000	0.0000
SB13	Span 6	End		0.0000	0.0000
SB13	Span 7	Start		0.0000	0.0000
SB13	Span 7	Middle		0.0000	0.0000
SB13	Span 7	End	UDCONU2	-22.5368	0.7098
SB13	Span 8	Start	UDCONU2	-22.5368	0.7098
SB13	Span 8	Middle	UDCONU2	-56.7511	1.8154
SB13	Span 8	End	UDCONU2	-87.8762	2.8528
SB13	Span 9	Start	UDCONU2	-87.8762	2.8528

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB13	Span 9	Middle	UDCONU2	-50.0754	1.5969
SB13	Span 9	End	UDCONU1	-8.3188	0.2604
SB13	Span 10	Start	UDCONU1	-8.3188	0.2604
SB13	Span 10	Middle		0.0000	0.0000
SB13	Span 10	End		0.0000	0.0000
SB13	Span 11	Start		0.0000	0.0000
SB13	Span 11	Middle		0.0000	0.0000
SB13	Span 11	End		0.0000	0.0000
SB13	Span 12	Start		0.0000	0.0000
SB13	Span 12	Middle		0.0000	0.0000
SB13	Span 12	End	UDCONU2	-43.4777	1.3823
SB13	Span 13	Start	UDCONU2	-43.4777	1.3823
SB13	Span 13	Middle		0.0000	0.0000
SB13	Span 13	End	UDCONU2	-2.0009	0.0756
SB14	Span 1	Start		0.0000	0.0000
SB14	Span 1	Middle		0.0000	0.0000
SB14	Span 1	End		0.0000	0.0000
SB14	Span 2	Start		0.0000	0.0000
SB14	Span 2	Middle		0.0000	0.0000
SB14	Span 2	End		0.0000	0.0000
SB14	Span 3	Start		0.0000	0.0000
SB14	Span 3	Middle		0.0000	0.0000
SB14	Span 3	End		0.0000	0.0000
SB14	Span 4	Start		0.0000	0.0000
SB14	Span 4	Middle		0.0000	0.0000
SB14	Span 4	End		0.0000	0.0000
SB14	Span 5	Start		0.0000	0.0000
SB14	Span 5	Middle		0.0000	0.0000
SB14	Span 5	End		0.0000	0.0000
SB14	Span 6	Start		0.0000	0.0000
SB14	Span 6	Middle		0.0000	0.0000
SB14	Span 6	End		0.0000	0.0000
SB14	Span 7	Start		0.0000	0.0000
SB14	Span 7	Middle	UDCONU2	-0.1123	0.0000
SB14	Span 7	End	UDCONU2	-14.4087	0.4522
SB14	Span 8	Start	UDCONU2	-14.4087	0.4522
SB14	Span 8	Middle	UDCONU2	-40.6829	1.2998
SB14	Span 8	End	UDCONU2	-89.2106	2.8980
SB14	Span 9	Start	UDCONU2	-89.2106	2.8980
SB14	Span 9	Middle	UDCONU2	-48.5790	1.5481
SB14	Span 9	End	UDCONU1	-6.8877	0.2155
SB14	Span 10	Start	UDCONU1	-6.8877	0.2155
SB14	Span 10	Middle		0.0000	0.0000
SB14	Span 10	End		0.0000	0.0000
SB14	Span 11	Start		0.0000	0.0000
SB14	Span 11	Middle		0.0000	0.0000
SB14	Span 11	End	UDCONU1	-0.4702	0.0000
SB14	Span 12	Start	UDCONU1	-0.4702	0.0000
SB14	Span 12	Middle		0.0000	0.0000
SB14	Span 12	End	UDCONU2	-32.4538	1.0267
SB14	Span 13	Start	UDCONU2	-32.4538	1.0267
SB14	Span 13	Middle		0.0000	0.0000
SB14	Span 13	End	UDCONU2	-2.3713	0.0777

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB15	Span 1	Start		0.0000	0.0000
SB15	Span 1	Middle		0.0000	0.0000
SB15	Span 1	End		0.0000	0.0000
SB15	Span 2	Start		0.0000	0.0000
SB15	Span 2	Middle		0.0000	0.0000
SB15	Span 2	End		0.0000	0.0000
SB15	Span 3	Start		0.0000	0.0000
SB15	Span 3	Middle		0.0000	0.0000
SB15	Span 3	End		0.0000	0.0000
SB15	Span 4	Start		0.0000	0.0000
SB15	Span 4	Middle		0.0000	0.0000
SB15	Span 4	End		0.0000	0.0000
SB15	Span 5	Start		0.0000	0.0000
SB15	Span 5	Middle		0.0000	0.0000
SB15	Span 5	End		0.0000	0.0000
SB15	Span 6	Start		0.0000	0.0000
SB15	Span 6	Middle	UDCONU2	-4.0590	0.1269
SB15	Span 6	End	UDCONU2	-13.4089	0.4206
SB15	Span 7	Start	UDCONU2	-13.4089	0.4206
SB15	Span 7	Middle		0.0000	0.0000
SB15	Span 7	End	UDCONU2	-16.2135	0.5096
SB15	Span 8	Start	UDCONU2	-16.2135	0.5096
SB15	Span 8	Middle		0.0000	0.0000
SB15	Span 8	End	UDCONU2	-185.9191	6.3542
SB15	Span 9	Start	UDCONU2	-185.9191	6.3542
SB15	Span 9	Middle	UDCONU2	-70.5254	2.2706
SB15	Span 9	End	UDCONU1	-7.9399	0.2485
SB15	Span 10	Start	UDCONU1	-7.9399	0.2485
SB15	Span 10	Middle		0.0000	0.0000
SB15	Span 10	End		0.0000	0.0000
SB15	Span 11	Start		0.0000	0.0000
SB15	Span 11	Middle		0.0000	0.0000
SB15	Span 11	End		0.0000	0.0000
SB15	Span 12	Start		0.0000	0.0000
SB15	Span 12	Middle		0.0000	0.0000
SB15	Span 12	End	UDCONU2	-19.3615	0.6145
SB15	Span 13	Start	UDCONU2	-19.3615	0.6145
SB15	Span 13	Middle		0.0000	0.0000
SB15	Span 13	End	UDCONU2	-6.3740	0.2012
SB16	Span 1	Start		0.0000	0.0000
SB16	Span 1	Middle		0.0000	0.0000
SB16	Span 1	End		0.0000	0.0000
SB16	Span 2	Start		0.0000	0.0000
SB16	Span 2	Middle		0.0000	0.0000
SB16	Span 2	End		0.0000	0.0000
SB16	Span 3	Start		0.0000	0.0000
SB16	Span 3	Middle		0.0000	0.0000
SB16	Span 3	End		0.0000	0.0000
SB16	Span 4	Start		0.0000	0.0000
SB16	Span 4	Middle		0.0000	0.0000
SB16	Span 4	End		0.0000	0.0000
SB16	Span 5	Start		0.0000	0.0000
SB16	Span 5	Middle		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB16	Span 5	End		0.0000	0.0000
SB16	Span 6	Start		0.0000	0.0000
SB16	Span 6	Middle		0.0000	0.0000
SB16	Span 6	End	UDCONU2	-0.0856	0.0027
SB16	Span 7	Start	UDCONU2	-0.0856	0.0027
SB16	Span 7	Middle	UDCONU2	-37.3490	1.1956
SB16	Span 7	End	UDCONU2	-133.6911	4.4411
SB16	Span 8	Start	UDCONU2	-133.6911	4.4411
SB16	Span 8	Middle	UDCONU2	-55.5092	1.7746
SB16	Span 8	End	UDCONU2	-12.3135	0.3861
SB16	Span 9	Start	UDCONU2	-12.3135	0.3861
SB16	Span 9	Middle		0.0000	0.0000
SB16	Span 9	End		0.0000	0.0000
SB16	Span 10	Start		0.0000	0.0000
SB16	Span 10	Middle		0.0000	0.0000
SB16	Span 10	End	UDCONU2	-3.3482	0.1130
SB16	Span 11	Start	UDCONU2	-3.3482	0.1130
SB16	Span 11	Middle		0.0000	0.0000
SB16	Span 11	End	UDCONU2	-45.9906	1.4805
SB16	Span 12	Start	UDCONU2	-45.9906	1.4805
SB16	Span 12	Middle		0.0000	0.0000
SB16	Span 12	End	UDCONU2	-13.4822	0.4331
SB17	Span 1	Start		0.0000	0.0000
SB17	Span 1	Middle		0.0000	0.0000
SB17	Span 1	End		0.0000	0.0000
SB17	Span 2	Start		0.0000	0.0000
SB17	Span 2	Middle		0.0000	0.0000
SB17	Span 2	End		0.0000	0.0000
SB17	Span 3	Start		0.0000	0.0000
SB17	Span 3	Middle		0.0000	0.0000
SB17	Span 3	End		0.0000	0.0000
SB17	Span 4	Start		0.0000	0.0000
SB17	Span 4	Middle		0.0000	0.0000
SB17	Span 4	End		0.0000	0.0000
SB17	Span 5	Start		0.0000	0.0000
SB17	Span 5	Middle		0.0000	0.0000
SB17	Span 5	End		0.0000	0.0000
SB17	Span 6	Start		0.0000	0.0000
SB17	Span 6	Middle		0.0000	0.0000
SB17	Span 6	End		0.0000	0.0000
SB17	Span 7	Start	UDCONU2	-6.9309	0.2247
SB17	Span 7	Middle	UDCONU2	-40.8145	1.2998
SB17	Span 7	End	UDCONU2	-12.5237	0.3931
SB17	Span 8	Start	UDCONU2	-12.5237	0.3931
SB17	Span 8	Middle	UDCONU1	-1.4227	0.0497
SB17	Span 8	End	UDCONU2	-0.0338	0.0000
SB17	Span 9	Start	UDCONU2	-0.0338	0.0000
SB17	Span 9	Middle	UDCONU2	-1.3831	0.0603
SB17	Span 9	End	UDCONU2	-14.2446	0.4702
SB17	Span 10	Start	UDCONU2	-14.2446	0.4702
SB17	Span 10	Middle	UDCONU2	-36.2723	1.1643
SB17	Span 10	End	UDCONU2	-17.5600	0.5557
SB18	Span 1	Start		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB18	Span 1	Middle		0.0000	0.0000
SB18	Span 1	End		0.0000	0.0000
SB18	Span 2	Start		0.0000	0.0000
SB18	Span 2	Middle		0.0000	0.0000
SB18	Span 2	End		0.0000	0.0000
SB18	Span 3	Start		0.0000	0.0000
SB18	Span 3	Middle		0.0000	0.0000
SB18	Span 3	End		0.0000	0.0000
SB18	Span 4	Start		0.0000	0.0000
SB18	Span 4	Middle		0.0000	0.0000
SB18	Span 4	End		0.0000	0.0000
SB18	Span 5	Start		0.0000	0.0000
SB18	Span 5	Middle		0.0000	0.0000
SB18	Span 5	End		0.0000	0.0000
SB18	Span 6	Start		0.0000	0.0000
SB18	Span 6	Middle		0.0000	0.0000
SB18	Span 6	End		0.0000	0.0000
SB18	Span 7	Start		0.0000	0.0000
SB18	Span 7	Middle	UDCONU2	-4.6913	0.1484
SB18	Span 7	End	UDCONU2	-18.6629	0.5941
SB18	Span 8	Start	UDCONU2	-18.6629	0.5941
SB18	Span 8	Middle		0.0000	0.0000
SB18	Span 8	End	UDCONU2	-19.7276	0.6285
SB18	Span 9	Start	UDCONU2	-19.7276	0.6285
SB18	Span 9	Middle		0.0000	0.0000
SB18	Span 9	End	UDCONU2	-20.4694	0.6512
SB18	Span 10	Start	UDCONU2	-20.4694	0.6512
SB18	Span 10	Middle	UDCONU2	-11.6559	0.3654
SB18	Span 10	End	UDCONU2	-6.4196	0.2008
SB18	Span 11	Start	UDCONU2	-6.4196	0.2008
SB18	Span 11	Middle	UDCONU2	-7.7892	0.2438
SB18	Span 11	End	UDCONU2	-20.0209	0.6299
SB18	Span 12	Start	UDCONU2	-20.0209	0.6299
SB18	Span 12	Middle		0.0000	0.0000
SB18	Span 12	End	UDCONU2	-18.5749	0.5840
SB18	Span 13	Start	UDCONU2	-18.5749	0.5840
SB18	Span 13	Middle	UDCONU2	-4.7228	0.1743
SB18	Span 13	End	UDCONU2	-4.6888	0.1730
SB18	Span 14	Start	UDCONU2	-4.6888	0.1730
SB18	Span 14	Middle	UDCONU2	-10.9197	0.3623
SB18	Span 14	End	UDCONU2	-17.9809	0.5789
SB18	Span 15	Start	UDCONU2	-17.9809	0.5789
SB18	Span 15	Middle		0.0000	0.0000
SB18	Span 15	End	UDCONU2	-22.1562	0.7131
SB18	Span 16	Start	UDCONU2	-22.1562	0.7131
SB18	Span 16	Middle		0.0000	0.0000
SB18	Span 16	End	UDCONU2	-12.3723	0.3895
SB19	Span 1	Start		0.0000	0.0000
SB19	Span 1	Middle		0.0000	0.0000
SB19	Span 1	End		0.0000	0.0000
SB19	Span 2	Start		0.0000	0.0000
SB19	Span 2	Middle		0.0000	0.0000
SB19	Span 2	End		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB19	Span 3	Start		0.0000	0.0000
SB19	Span 3	Middle		0.0000	0.0000
SB19	Span 3	End		0.0000	0.0000
SB19	Span 4	Start		0.0000	0.0000
SB19	Span 4	Middle		0.0000	0.0000
SB19	Span 4	End		0.0000	0.0000
SB19	Span 5	Start		0.0000	0.0000
SB19	Span 5	Middle		0.0000	0.0000
SB19	Span 5	End		0.0000	0.0000
SB19	Span 6	Start		0.0000	0.0000
SB19	Span 6	Middle		0.0000	0.0000
SB19	Span 6	End		0.0000	0.0000
SB19	Span 7	Start	UDCONU2	-1.4550	0.0469
SB19	Span 7	Middle	UDCONU2	-3.9067	0.1223
SB19	Span 7	End	UDCONU2	-2.0607	0.0644
SB19	Span 8	Start	UDCONU2	-2.0607	0.0644
SB19	Span 8	Middle	UDCONU2	-7.3633	0.2420
SB19	Span 8	End	UDCONU2	-6.8430	0.2254
SB19	Span 9	Start	UDCONU2	-6.8430	0.2254
SB19	Span 9	Middle	UDCONU2	-8.3303	0.2693
SB19	Span 9	End	UDCONU2	-6.1893	0.1952
SB19	Span 10	Start	UDCONU2	-6.1893	0.1952
SB19	Span 10	Middle	UDCONU1	-3.9162	0.1246
SB19	Span 10	End	UDCONU2	-3.3532	0.1064

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA1	Span 1	Start	UDCONU2	0.0168	4.781E-004
SA1	Span 1	Middle	UDCONU2	4.4049	0.1376
SA1	Span 1	End	UDCONU2	6.6782	0.1986
SA1	Span 2	Start	UDCONU2	6.6782	0.1986
SA1	Span 2	Middle	UDCONU2	5.9375	0.1717
SA1	Span 2	End	UDCONU2	3.1030	0.0878
SA1	Span 3	Start	UDCONU2	3.1030	0.0878
SA1	Span 3	Middle	UDCONU2	0.8954	0.0253
SA1	Span 3	End	UDCONU2	0.0000	0.0000
SA1	Span 4	Start	UDCONU2	0.0000	0.0000
SA1	Span 4	Middle	UDCONU2	3.1764	0.0898
SA1	Span 4	End	UDCONU2	7.6197	0.2162
SA1	Span 5	Start	UDCONU2	9.0914	0.2582
SA1	Span 5	Middle	UDCONU2	14.9653	0.4268
SA1	Span 5	End		0.0000	0.0000
SA1	Span 6	Start		0.0000	0.0000
SA1	Span 6	Middle		0.0000	0.0000
SA1	Span 6	End		0.0000	0.0000
SA1	Span 7	Start		0.0000	0.0000
SA1	Span 7	Middle		0.0000	0.0000
SA1	Span 7	End		0.0000	0.0000
SA1	Span 8	Start		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA1	Span 8	Middle		0.0000	0.0000
SA1	Span 8	End		0.0000	0.0000
SA1	Span 9	Start		0.0000	0.0000
SA1	Span 9	Middle		0.0000	0.0000
SA1	Span 9	End		0.0000	0.0000
SA2	Cantilever Start	Start	UDCONU2	0.9607	0.0288
SA2	Cantilever Start	Middle		0.0000	0.0000
SA2	Cantilever Start	End	UDCONU2	0.6988	0.0359
SA2	Span 1	Start	UDCONU2	1.2479	0.0665
SA2	Span 1	Middle	UDCONU2	7.1038	0.2284
SA2	Span 1	End	UDCONU2	14.7806	0.4435
SA2	Span 2	Start	UDCONU2	14.7806	0.4435
SA2	Span 2	Middle	UDCONU2	17.5757	0.5122
SA2	Span 2	End	UDCONU2	13.1198	0.3719
SA2	Span 3	Start	UDCONU2	13.1198	0.3719
SA2	Span 3	Middle	UDCONU2	10.5891	0.2999
SA2	Span 3	End	UDCONU2	7.8433	0.2226
SA2	Span 4	Start	UDCONU2	7.8433	0.2226
SA2	Span 4	Middle	UDCONU2	13.7678	0.3904
SA2	Span 4	End	UDCONU2	18.2840	0.5193
SA2	Span 5	Start	UDCONU2	18.2840	0.5193
SA2	Span 5	Middle	UDCONU2	22.2468	0.6328
SA2	Span 5	End	UDCONU2	23.1720	0.6636
SA2	Span 6	Start	UDCONU2	23.1720	0.6636
SA2	Span 6	Middle		0.0000	0.0000
SA2	Span 6	End	UDCONU2	3.0739	0.0920
SA2	Span 7	Start	UDCONU2	0.1039	0.0000
SA2	Span 7	Middle		0.0000	0.0000
SA2	Span 7	End		0.0000	0.0000
SA2	Span 8	Start		0.0000	0.0000
SA2	Span 8	Middle		0.0000	0.0000
SA2	Span 8	End		0.0000	0.0000
SA2	Span 9	Start		0.0000	0.0000
SA2	Span 9	Middle		0.0000	0.0000
SA2	Span 9	End		0.0000	0.0000
SA2	Span 10	Start		0.0000	0.0000
SA2	Span 10	Middle		0.0000	0.0000
SA2	Span 10	End		0.0000	0.0000
SA3	Span 1	Start		0.0000	0.0000
SA3	Span 1	Middle	UDCONU2	0.4419	0.0185
SA3	Span 1	End	UDCONU2	7.1098	0.2239
SA3	Span 2	Start	UDCONU2	7.1098	0.2239
SA3	Span 2	Middle	UDCONU2	21.5666	0.6246
SA3	Span 2	End	UDCONU2	22.1525	0.6371
SA3	Span 3	Start	UDCONU2	22.1525	0.6371
SA3	Span 3	Middle	UDCONU2	21.0530	0.6043
SA3	Span 3	End	UDCONU2	19.4011	0.5585
SA3	Span 4	Start	UDCONU2	19.4011	0.5585
SA3	Span 4	Middle	UDCONU2	21.5968	0.6299
SA3	Span 4	End	UDCONU2	18.0875	0.5326
SA3	Span 5	Start	UDCONU2	18.0875	0.5326

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA3	Span 5	Middle	UDCONU2	12.5643	0.3753
SA3	Span 5	End	UDCONU2	0.5431	0.0573
SA3	Span 6	Start	UDCONU2	0.5431	0.0573
SA3	Span 6	Middle		0.0000	0.0000
SA3	Span 6	End		0.0000	0.0000
SA3	Span 7	Start		0.0000	0.0000
SA3	Span 7	Middle		0.0000	0.0000
SA3	Span 7	End		0.0000	0.0000
SA3	Span 8	Start		0.0000	0.0000
SA3	Span 8	Middle		0.0000	0.0000
SA3	Span 8	End		0.0000	0.0000
SA3	Span 9	Start		0.0000	0.0000
SA3	Span 9	Middle		0.0000	0.0000
SA3	Span 9	End		0.0000	0.0000
SA4	Span 1	Start		0.0000	0.0000
SA4	Span 1	Middle	UDCONU2	1.0963	0.0361
SA4	Span 1	End	UDCONU2	0.2715	0.0331
SA4	Span 2	Start	UDCONU2	0.2715	0.0331
SA4	Span 2	Middle	UDCONU2	16.2612	0.4786
SA4	Span 2	End	UDCONU2	19.4095	0.5640
SA4	Span 3	Start	UDCONU2	19.4095	0.5640
SA4	Span 3	Middle	UDCONU2	20.0175	0.5838
SA4	Span 3	End	UDCONU2	19.5685	0.5710
SA4	Span 4	Start	UDCONU2	19.5685	0.5710
SA4	Span 4	Middle	UDCONU2	18.8960	0.5554
SA4	Span 4	End	UDCONU2	12.5861	0.3860
SA4	Span 5	Start	UDCONU2	12.5861	0.3860
SA4	Span 5	Middle	UDCONU2	12.6907	0.3881
SA4	Span 5	End	UDCONU2	0.4175	0.0576
SA4	Span 6	Start	UDCONU2	0.4175	0.0576
SA4	Span 6	Middle		0.0000	0.0000
SA4	Span 6	End	UDCONU2	5.3630	0.2722
SA4	Span 7	Start	UDCONU2	5.3630	0.2722
SA4	Span 7	Middle		0.0000	0.0000
SA4	Span 7	End	UDCONU2	4.5994	0.2540
SA4	Span 8	Start	UDCONU2	4.5994	0.2540
SA4	Span 8	Middle		0.0000	0.0000
SA4	Span 8	End	UDCONU2	1.9050	0.1157
SA4	Span 9	Start	UDCONU2	1.9050	0.1157
SA4	Span 9	Middle	UDCONU2	0.3622	0.0318
SA4	Span 9	End		0.0000	0.0000
SA4	Span 10	Start		0.0000	0.0000
SA4	Span 10	Middle		0.0000	0.0000
SA4	Span 10	End		0.0000	0.0000
SA4	Span 11	Start		0.0000	0.0000
SA4	Span 11	Middle		0.0000	0.0000
SA4	Span 11	End		0.0000	0.0000
SA4	Span 12	Start		0.0000	0.0000
SA4	Span 12	Middle		0.0000	0.0000
SA4	Span 12	End		0.0000	0.0000
SA5	Span 1	Start		0.0000	0.0000
SA5	Span 1	Middle		0.0000	0.0000
SA5	Span 1	End	UDCONU2	7.1745	0.2218

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA5	Span 2	Start	UDCONU2	7.1745	0.2218
SA5	Span 2	Middle	UDCONU2	15.4648	0.4656
SA5	Span 2	End	UDCONU2	11.8834	0.3585
SA5	Span 3	Start	UDCONU2	11.8834	0.3585
SA5	Span 3	Middle	UDCONU2	10.3355	0.3121
SA5	Span 3	End	UDCONU2	9.6210	0.2949
SA5	Span 4	Start	UDCONU2	9.6210	0.2949
SA5	Span 4	Middle	UDCONU2	16.2737	0.5041
SA5	Span 4	End	UDCONU2	4.9512	0.1629
SA5	Span 5	Start	UDCONU2	4.9512	0.1629
SA5	Span 5	Middle	UDCONU2	8.4930	0.2766
SA5	Span 5	End	UDCONU2	5.1352	0.2001
SA5	Span 6	Start	UDCONU2	5.1352	0.2001
SA5	Span 6	Middle		0.0000	0.0000
SA5	Span 6	End	UDCONU2	3.6836	0.1640
SA5	Span 7	Start	UDCONU2	3.6836	0.1640
SA5	Span 7	Middle	UDCONU1	1.8694	0.0574
SA5	Span 7	End		0.0000	0.0000
SA5	Span 8	Start		0.0000	0.0000
SA5	Span 8	Middle		0.0000	0.0000
SA5	Span 8	End		0.0000	0.0000
SA5	Span 9	Start		0.0000	0.0000
SA5	Span 9	Middle		0.0000	0.0000
SA5	Span 9	End		0.0000	0.0000
SA5	Span 10	Start		0.0000	0.0000
SA5	Span 10	Middle		0.0000	0.0000
SA5	Span 10	End		0.0000	0.0000
SA6	Span 1	Start		0.0000	0.0000
SA6	Span 1	Middle		0.0000	0.0000
SA6	Span 1	End	UDCONU2	2.4750	0.0745
SA6	Span 2	Start	UDCONU2	7.2041	0.2183
SA6	Span 2	Middle		0.0000	0.0000
SA6	Span 2	End	UDCONU2	4.0610	0.1439
SA6	Span 3	Start	UDCONU2	4.0610	0.1439
SA6	Span 3	Middle	UDCONU2	2.2965	0.1010
SA6	Span 3	End	UDCONU2	2.3494	0.1163
SA6	Span 4	Start	UDCONU2	2.3494	0.1163
SA6	Span 4	Middle	UDCONU2	5.0895	0.1650
SA6	Span 4	End	UDCONU1	0.4457	0.0342
SA6	Span 5	Start	UDCONU1	0.4457	0.0342
SA6	Span 5	Middle		0.0000	0.0000
SA6	Span 5	End	UDCONU2	6.6178	0.1891
SA6	Span 6	Start	UDCONU2	6.6178	0.1891
SA6	Span 6	Middle		0.0000	0.0000
SA6	Span 6	End	UDCONU2	4.3013	0.1835
SA6	Span 7	Start	UDCONU2	4.3013	0.1835
SA6	Span 7	Middle		0.0000	0.0000
SA6	Span 7	End	UDCONU2	1.8963	0.0703
SA6	Span 8	Start	UDCONU2	1.8963	0.0703
SA6	Span 8	Middle	UDCONU2	5.3200	0.1654
SA6	Span 8	End	UDCONU1	9.2823	0.2724
SA6	Span 9	Start	UDCONU1	9.2823	0.2724
SA6	Span 9	Middle		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA6	Span 9	End	UDCONU2	12.8632	0.3761
SA6	Span 10	Start	UDCONU2	12.8632	0.3761
SA6	Span 10	Middle		0.0000	0.0000
SA6	Span 10	End	UDCONU2	23.7071	0.6863
SA6	Span 11	Start	UDCONU2	23.7071	0.6863
SA6	Span 11	Middle	UDCONU2	7.2329	0.2247
SA6	Span 11	End	UDCONU2	2.4197	0.1015
SA6	Span 12	Start	UDCONU2	2.4197	0.1015
SA6	Span 12	Middle	UDCONU2	5.9789	0.2758
SA6	Span 12	End	UDCONU2	3.6246	0.1152
SA6	Span 13	Start	UDCONU2	3.6246	0.1152
SA6	Span 13	Middle		0.0000	0.0000
SA6	Span 13	End	UDCONU2	3.5485	0.1133
SA6	Span 14	Start	UDCONU2	3.5485	0.1133
SA6	Span 14	Middle	UDCONU2	5.4136	0.1594
SA6	Span 14	End		0.0000	0.0000
SA6	Span 15	Start		0.0000	0.0000
SA6	Span 15	Middle		0.0000	0.0000
SA6	Span 15	End		0.0000	0.0000
SA6	Span 16	Start		0.0000	0.0000
SA6	Span 16	Middle		0.0000	0.0000
SA6	Span 16	End		0.0000	0.0000
SA6	Span 17	Start		0.0000	0.0000
SA6	Span 17	Middle		0.0000	0.0000
SA6	Span 17	End		0.0000	0.0000
SA7	Span 1	Start		0.0000	0.0000
SA7	Span 1	Middle		0.0000	0.0000
SA7	Span 1	End		0.0000	0.0000
SA7	Span 2	Start		0.0000	0.0000
SA7	Span 2	Middle	UDCONU2	21.3727	0.6198
SA7	Span 2	End	UDCONU2	24.1227	0.7110
SA7	Span 3	Start	UDCONU2	23.8681	0.7068
SA7	Span 3	Middle	UDCONU2	10.7425	0.3460
SA7	Span 3	End	UDCONU2	1.2838	0.0366
SA7	Span 4	Start	UDCONU2	1.2838	0.0366
SA7	Span 4	Middle	UDCONU2	0.6648	0.0189
SA7	Span 4	End	UDCONU2	0.0000	0.0000
SA7	Span 5	Start	UDCONU2	0.0000	0.0000
SA7	Span 5	Middle		0.0000	0.0000
SA7	Span 5	End	UDCONU2	6.8759	0.2268
SA7	Span 6	Start	UDCONU2	10.1201	0.3018
SA7	Span 6	Middle	UDCONU2	9.5145	0.2823
SA7	Span 6	End	UDCONU2	15.5407	0.4483
SA7	Span 7	Start	UDCONU2	15.5407	0.4483
SA7	Span 7	Middle	UDCONU2	24.9386	0.7123
SA7	Span 7	End	UDCONU2	35.6479	1.0203
SA7	Span 8	Start	UDCONU2	35.6479	1.0203
SA7	Span 8	Middle	UDCONU2	28.9528	0.8316
SA7	Span 8	End	UDCONU2	21.0204	0.6093
SA7	Span 9	Start	UDCONU2	21.0204	0.6093
SA7	Span 9	Middle	UDCONU2	14.4964	0.4424
SA7	Span 9	End	UDCONU2	2.2102	0.0899
SA7	Span 10	Start	UDCONU2	2.2102	0.0899

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA7	Span 10	Middle		0.0000	0.0000
SA7	Span 10	End	UDCONU1	0.2490	0.0165
SA7	Span 11	Start	UDCONU1	0.3069	0.0155
SA7	Span 11	Middle		0.0000	0.0000
SA7	Span 11	End		0.0000	0.0000
SA7	Span 12	Start		0.0000	0.0000
SA7	Span 12	Middle		0.0000	0.0000
SA7	Span 12	End		0.0000	0.0000
SA7	Span 13	Start		0.0000	0.0000
SA7	Span 13	Middle		0.0000	0.0000
SA7	Span 13	End		0.0000	0.0000
SA8	Span 1	Start		0.0000	0.0000
SA8	Span 1	Middle		0.0000	0.0000
SA8	Span 1	End		0.0000	0.0000
SA8	Span 2	Start		0.0000	0.0000
SA8	Span 2	Middle	UDCONU2	36.8620	1.0540
SA8	Span 2	End	UDCONU2	41.0411	1.1753
SA8	Span 3	Start	UDCONU2	41.0411	1.1753
SA8	Span 3	Middle	UDCONU2	23.6385	0.6728
SA8	Span 3	End	UDCONU2	0.7142	0.0202
SA8	Span 4	Start	UDCONU2	0.0811	0.0000
SA8	Span 4	Middle	UDCONU2	10.3694	0.4020
SA8	Span 4	End	UDCONU2	14.7116	0.5360
SA8	Span 5	Start	UDCONU2	14.7116	0.5360
SA8	Span 5	Middle		0.0000	0.0000
SA8	Span 5	End	UDCONU2	6.8603	0.1941
SA8	Span 6	Start	UDCONU2	6.8603	0.1941
SA8	Span 6	Middle		0.0000	0.0000
SA8	Span 6	End	UDCONU2	5.2639	0.1488
SA8	Span 7	Start	UDCONU2	5.2639	0.1488
SA8	Span 7	Middle	UDCONU2	5.4647	0.1545
SA8	Span 7	End	UDCONU2	23.0714	0.6564
SA8	Span 8	Start	UDCONU2	23.0714	0.6564
SA8	Span 8	Middle	UDCONU2	32.2484	0.9206
SA8	Span 8	End	UDCONU1	33.5639	0.9586
SA8	Span 9	Start	UDCONU1	33.5639	0.9586
SA8	Span 9	Middle	UDCONU2	36.6971	1.0572
SA8	Span 9	End	UDCONU2	40.8240	1.1746
SA8	Span 10	Start	UDCONU2	40.8240	1.1746
SA8	Span 10	Middle	UDCONU2	29.3663	0.8455
SA8	Span 10	End	UDCONU2	1.4087	0.0588
SA8	Span 11	Start	UDCONU2	1.4087	0.0588
SA8	Span 11	Middle		0.0000	0.0000
SA8	Span 11	End	UDCONU2	0.2562	0.0000
SA8	Span 12	Start	UDCONU2	0.2562	0.0000
SA8	Span 12	Middle		0.0000	0.0000
SA8	Span 12	End		0.0000	0.0000
SA8	Span 13	Start		0.0000	0.0000
SA8	Span 13	Middle		0.0000	0.0000
SA8	Span 13	End		0.0000	0.0000
SA8	Span 14	Start		0.0000	0.0000
SA8	Span 14	Middle		0.0000	0.0000
SA8	Span 14	End		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA9	Span 1	Start		0.0000	0.0000
SA9	Span 1	Middle		0.0000	0.0000
SA9	Span 1	End		0.0000	0.0000
SA9	Span 2	Start		0.0000	0.0000
SA9	Span 2	Middle	UDCONU2	16.1812	0.4707
SA9	Span 2	End	UDCONU2	14.9848	0.4613
SA9	Span 3	Start	UDCONU2	14.9848	0.4613
SA9	Span 3	Middle	UDCONU2	9.3745	0.3283
SA9	Span 3	End	UDCONU2	4.0531	0.1708
SA9	Span 4	Start	UDCONU2	4.0531	0.1708
SA9	Span 4	Middle	UDCONU2	0.0000	0.0614
SA9	Span 4	End	UDCONU2	0.0000	0.0783
SA9	Span 5	Start	UDCONU2	0.0000	0.0783
SA9	Span 5	Middle	UDCONU2	0.1941	0.0328
SA9	Span 5	End	UDCONU2	11.3913	0.3341
SA9	Span 6	Start	UDCONU2	11.3913	0.3341
SA9	Span 6	Middle	UDCONU2	31.1331	0.8884
SA9	Span 6	End	UDCONU1	28.3031	0.8079
SA9	Span 7	Start	UDCONU1	28.3031	0.8079
SA9	Span 7	Middle		0.0000	0.0000
SA9	Span 7	End	UDCONU2	41.3740	1.1879
SA9	Span 8	Start	UDCONU2	41.3740	1.1879
SA9	Span 8	Middle	UDCONU2	39.3309	1.1283
SA9	Span 8	End	UDCONU2	4.2809	0.1326
SA9	Span 9	Start	UDCONU2	4.2809	0.1326
SA9	Span 9	Middle		0.0000	0.0000
SA9	Span 9	End	UDCONU2	1.3740	0.0481
SA9	Span 10	Start	UDCONU2	1.3740	0.0481
SA9	Span 10	Middle	UDCONU2	2.2372	0.0699
SA9	Span 10	End		0.0000	0.0000
SA9	Span 11	Start		0.0000	0.0000
SA9	Span 11	Middle		0.0000	0.0000
SA9	Span 11	End		0.0000	0.0000
SA9	Span 12	Start		0.0000	0.0000
SA9	Span 12	Middle		0.0000	0.0000
SA9	Span 12	End		0.0000	0.0000
SB1	Span 1	Start	UDCONU2	1.0324	0.0340
SB1	Span 1	Middle	UDCONU2	1.5637	0.0540
SB1	Span 1	End		0.0000	0.0000
SB1	Span 2	Start		0.0000	0.0000
SB1	Span 2	Middle		0.0000	0.0000
SB1	Span 2	End		0.0000	0.0000
SB1	Span 3	Start		0.0000	0.0000
SB1	Span 3	Middle		0.0000	0.0000
SB1	Span 3	End		0.0000	0.0000
SB1	Span 4	Start		0.0000	0.0000
SB1	Span 4	Middle		0.0000	0.0000
SB1	Span 4	End		0.0000	0.0000
SB1	Span 5	Start		0.0000	0.0000
SB1	Span 5	Middle		0.0000	0.0000
SB1	Span 5	End		0.0000	0.0000
SB1	Span 6	Start		0.0000	0.0000
SB1	Span 6	Middle		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB1	Span 6	End		0.0000	0.0000
SB1	Span 7	Start		0.0000	0.0000
SB1	Span 7	Middle		0.0000	0.0000
SB1	Span 7	End		0.0000	0.0000
SB1	Span 8	Start		0.0000	0.0000
SB1	Span 8	Middle		0.0000	0.0000
SB1	Span 8	End		0.0000	0.0000
SB1	Span 9	Start		0.0000	0.0000
SB1	Span 9	Middle		0.0000	0.0000
SB1	Span 9	End		0.0000	0.0000
SB1	Span 10	Start		0.0000	0.0000
SB1	Span 10	Middle		0.0000	0.0000
SB1	Span 10	End		0.0000	0.0000
SB2	Span 1	Start	UDCONU2	8.0149	0.2562
SB2	Span 1	Middle		0.0000	0.0000
SB2	Span 1	End	UDCONU2	7.9022	0.2749
SB2	Span 2	Start	UDCONU2	7.9022	0.2749
SB2	Span 2	Middle	UDCONU2	10.6491	0.3607
SB2	Span 2	End	UDCONU2	4.9207	0.1697
SB2	Span 3	Start	UDCONU2	4.9207	0.1697
SB2	Span 3	Middle	UDCONU2	0.3109	0.0182
SB2	Span 3	End		0.0000	0.0000
SB2	Span 4	Start		0.0000	0.0000
SB2	Span 4	Middle		0.0000	0.0000
SB2	Span 4	End		0.0000	0.0000
SB2	Span 5	Start		0.0000	0.0000
SB2	Span 5	Middle		0.0000	0.0000
SB2	Span 5	End		0.0000	0.0000
SB2	Span 6	Start		0.0000	0.0000
SB2	Span 6	Middle		0.0000	0.0000
SB2	Span 6	End		0.0000	0.0000
SB2	Span 7	Start		0.0000	0.0000
SB2	Span 7	Middle		0.0000	0.0000
SB2	Span 7	End		0.0000	0.0000
SB2	Span 8	Start		0.0000	0.0000
SB2	Span 8	Middle		0.0000	0.0000
SB2	Span 8	End		0.0000	0.0000
SB2	Span 9	Start		0.0000	0.0000
SB2	Span 9	Middle		0.0000	0.0000
SB2	Span 9	End		0.0000	0.0000
SB2	Span 10	Start		0.0000	0.0000
SB2	Span 10	Middle		0.0000	0.0000
SB2	Span 10	End		0.0000	0.0000
SB2	Span 11	Start		0.0000	0.0000
SB2	Span 11	Middle		0.0000	0.0000
SB2	Span 11	End		0.0000	0.0000
SB3	Span 1	Start	UDCONU2	11.9098	0.3761
SB3	Span 1	Middle	UDCONU2	23.0209	0.7350
SB3	Span 1	End	UDCONU2	26.7469	0.8642
SB3	Span 2	Start	UDCONU2	20.2577	0.6609
SB3	Span 2	Middle	UDCONU2	8.7267	0.2947
SB3	Span 2	End	UDCONU2	1.7428	0.1021
SB3	Span 3	Start	UDCONU2	0.6370	0.0810

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB3	Span 3	Middle	UDCONU2	0.6831	0.0546
SB3	Span 3	End		0.0000	0.0000
SB3	Span 4	Start		0.0000	0.0000
SB3	Span 4	Middle		0.0000	0.0000
SB3	Span 4	End		0.0000	0.0000
SB3	Span 5	Start		0.0000	0.0000
SB3	Span 5	Middle		0.0000	0.0000
SB3	Span 5	End		0.0000	0.0000
SB3	Span 6	Start		0.0000	0.0000
SB3	Span 6	Middle		0.0000	0.0000
SB3	Span 6	End		0.0000	0.0000
SB3	Span 7	Start		0.0000	0.0000
SB3	Span 7	Middle		0.0000	0.0000
SB3	Span 7	End		0.0000	0.0000
SB3	Span 8	Start		0.0000	0.0000
SB3	Span 8	Middle		0.0000	0.0000
SB3	Span 8	End		0.0000	0.0000
SB3	Span 9	Start		0.0000	0.0000
SB3	Span 9	Middle		0.0000	0.0000
SB3	Span 9	End		0.0000	0.0000
SB3	Span 10	Start		0.0000	0.0000
SB3	Span 10	Middle		0.0000	0.0000
SB3	Span 10	End		0.0000	0.0000
SB4	Span 1	Start	UDCONU2	12.8084	0.4031
SB4	Span 1	Middle	UDCONU2	25.8043	0.8206
SB4	Span 1	End	UDCONU2	39.2236	1.2630
SB4	Span 2	Start	UDCONU2	39.2236	1.2630
SB4	Span 2	Middle	UDCONU2	29.0672	0.9434
SB4	Span 2	End	UDCONU2	9.5826	0.3544
SB4	Span 3	Start	UDCONU2	9.5826	0.3544
SB4	Span 3	Middle	UDCONU2	2.5907	0.1310
SB4	Span 3	End	UDCONU2	0.0000	0.0499
SB4	Span 4	Start	UDCONU2	0.0000	0.0499
SB4	Span 4	Middle	UDCONU2	0.0000	0.0000
SB4	Span 4	End	UDCONU1	1.5168	0.0499
SB4	Span 5	Start	UDCONU1	1.5168	0.0499
SB4	Span 5	Middle	UDCONU2	7.7875	0.2567
SB4	Span 5	End	UDCONU2	6.3977	0.2154
SB4	Span 6	Start	UDCONU2	2.7486	0.0941
SB4	Span 6	Middle		0.0000	0.0000
SB4	Span 6	End		0.0000	0.0000
SB4	Span 7	Start		0.0000	0.0000
SB4	Span 7	Middle		0.0000	0.0000
SB4	Span 7	End		0.0000	0.0000
SB4	Span 8	Start		0.0000	0.0000
SB4	Span 8	Middle		0.0000	0.0000
SB4	Span 8	End		0.0000	0.0000
SB4	Span 9	Start		0.0000	0.0000
SB4	Span 9	Middle		0.0000	0.0000
SB4	Span 9	End		0.0000	0.0000
SB4	Span 10	Start		0.0000	0.0000
SB4	Span 10	Middle		0.0000	0.0000
SB4	Span 10	End		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB4	Span 11	Start		0.0000	0.0000
SB4	Span 11	Middle		0.0000	0.0000
SB4	Span 11	End		0.0000	0.0000
SB5	Span 1	Start	UDCONU2	8.8149	0.2763
SB5	Span 1	Middle	UDCONU2	24.0975	0.7636
SB5	Span 1	End	UDCONU2	46.1269	1.4776
SB5	Span 2	Start	UDCONU2	46.1269	1.4776
SB5	Span 2	Middle	UDCONU2	42.9446	1.3783
SB5	Span 2	End	UDCONU2	12.3511	0.4135
SB5	Span 3	Start	UDCONU2	12.3511	0.4135
SB5	Span 3	Middle	UDCONU1	0.2888	0.0342
SB5	Span 3	End	UDCONU2	9.1063	0.3319
SB5	Span 4	Start	UDCONU2	9.1063	0.3319
SB5	Span 4	Middle	UDCONU2	9.0994	0.3266
SB5	Span 4	End	UDCONU2	21.2734	0.6844
SB5	Span 5	Start	UDCONU2	34.9044	1.1073
SB5	Span 5	Middle	UDCONU2	26.8060	0.8502
SB5	Span 5	End	UDCONU2	8.1377	0.2574
SB5	Span 6	Start	UDCONU2	8.1377	0.2574
SB5	Span 6	Middle		0.0000	0.0000
SB5	Span 6	End		0.0000	0.0000
SB5	Span 7	Start		0.0000	0.0000
SB5	Span 7	Middle		0.0000	0.0000
SB5	Span 7	End		0.0000	0.0000
SB5	Span 8	Start		0.0000	0.0000
SB5	Span 8	Middle		0.0000	0.0000
SB5	Span 8	End		0.0000	0.0000
SB5	Span 9	Start		0.0000	0.0000
SB5	Span 9	Middle		0.0000	0.0000
SB5	Span 9	End		0.0000	0.0000
SB5	Span 10	Start		0.0000	0.0000
SB5	Span 10	Middle		0.0000	0.0000
SB5	Span 10	End		0.0000	0.0000
SB6	Span 1	Start	UDCONU2	2.4339	0.0767
SB6	Span 1	Middle	UDCONU2	20.9335	0.6600
SB6	Span 1	End	UDCONU2	47.6661	1.5197
SB6	Span 2	Start	UDCONU2	47.6661	1.5197
SB6	Span 2	Middle	UDCONU2	48.7241	1.5557
SB6	Span 2	End	UDCONU2	12.7215	0.4103
SB6	Span 3	Start	UDCONU2	12.7215	0.4103
SB6	Span 3	Middle	UDCONU2	0.0000	0.0000
SB6	Span 3	End	UDCONU2	2.1145	0.0750
SB6	Span 4	Start	UDCONU2	2.0957	0.0776
SB6	Span 4	Middle		0.0000	0.0000
SB6	Span 4	End		0.0000	0.0000
SB6	Span 5	Start		0.0000	0.0000
SB6	Span 5	Middle		0.0000	0.0000
SB6	Span 5	End	UDCONU2	0.7976	0.0363
SB6	Span 6	Start	UDCONU2	0.7976	0.0363
SB6	Span 6	Middle	UDCONU2	26.9557	0.8672
SB6	Span 6	End	UDCONU2	32.2689	1.0208
SB6	Span 7	Start	UDCONU2	32.2689	1.0208
SB6	Span 7	Middle	UDCONU2	32.8023	1.0379

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB6	Span 7	End	UDCONU2	20.2625	0.6376
SB6	Span 8	Start	UDCONU2	20.2625	0.6376
SB6	Span 8	Middle	UDCONU2	17.7836	0.5590
SB6	Span 8	End	UDCONU2	9.3195	0.2930
SB6	Span 9	Start	UDCONU2	8.0084	0.2519
SB6	Span 9	Middle	UDCONU2	3.5934	0.1130
SB6	Span 9	End		0.0000	0.0000
SB6	Span 10	Start		0.0000	0.0000
SB6	Span 10	Middle		0.0000	0.0000
SB6	Span 10	End		0.0000	0.0000
SB6	Span 11	Start		0.0000	0.0000
SB6	Span 11	Middle		0.0000	0.0000
SB6	Span 11	End		0.0000	0.0000
SB6	Span 12	Start		0.0000	0.0000
SB6	Span 12	Middle		0.0000	0.0000
SB6	Span 12	End		0.0000	0.0000
SB7	Span 1	Start	UDCONU2	2.0252	0.0637
SB7	Span 1	Middle	UDCONU2	19.7905	0.6227
SB7	Span 1	End	UDCONU2	47.5788	1.5155
SB7	Span 2	Start	UDCONU2	47.5788	1.5155
SB7	Span 2	Middle	UDCONU2	49.0582	1.5637
SB7	Span 2	End	UDCONU2	12.8941	0.4044
SB7	Span 3	Start	UDCONU2	12.8941	0.4044
SB7	Span 3	Middle	UDCONU2	0.0000	0.0000
SB7	Span 3	End	UDCONU2	1.1925	0.0372
SB7	Span 4	Start		0.0000	0.0000
SB7	Span 4	Middle	UDCONU2	23.9103	0.8176
SB7	Span 4	End	UDCONU2	23.9084	0.7811
SB7	Span 5	Start	UDCONU2	16.6065	0.5678
SB7	Span 5	Middle	UDCONU2	14.0289	0.4498
SB7	Span 5	End	UDCONU2	2.4927	0.0778
SB7	Span 6	Start	UDCONU2	2.4927	0.0778
SB7	Span 6	Middle	UDCONU2	5.3435	0.1670
SB7	Span 6	End	UDCONU2	6.3075	0.1973
SB7	Span 7	Start	UDCONU2	6.7687	0.2117
SB7	Span 7	Middle	UDCONU2	10.9476	0.3436
SB7	Span 7	End	UDCONU2	10.9476	0.3436
SB7	Span 8	Start	UDCONU2	7.6729	0.2409
SB7	Span 8	Middle	UDCONU2	3.8397	0.1206
SB7	Span 8	End		0.0000	0.0000
SB7	Span 9	Start		0.0000	0.0000
SB7	Span 9	Middle		0.0000	0.0000
SB7	Span 9	End		0.0000	0.0000
SB7	Span 10	Start		0.0000	0.0000
SB7	Span 10	Middle		0.0000	0.0000
SB7	Span 10	End		0.0000	0.0000
SB8	Span 1	Start	UDCONU2	8.7062	0.2731
SB8	Span 1	Middle	UDCONU2	21.9653	0.6935
SB8	Span 1	End	UDCONU2	45.9924	1.4640
SB8	Span 2	Start	UDCONU2	45.9924	1.4640
SB8	Span 2	Middle	UDCONU2	45.3965	1.4446
SB8	Span 2	End	UDCONU2	11.7485	0.3683
SB8	Span 3	Start	UDCONU2	11.7485	0.3683

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB8	Span 3	Middle	UDCONU2	0.0000	0.0000
SB8	Span 3	End	UDCONU2	0.0000	0.0000
SB8	Span 4	Start	UDCONU2	0.0000	0.0000
SB8	Span 4	Middle		0.0000	0.0000
SB8	Span 4	End	UDCONU2	0.0000	0.0473
SB8	Span 5	Start	UDCONU2	0.0000	0.0473
SB8	Span 5	Middle		0.0000	0.0000
SB8	Span 5	End	UDCONU2	16.0421	0.5485
SB8	Span 6	Start	UDCONU2	16.0421	0.5485
SB8	Span 6	Middle		0.0000	0.0000
SB8	Span 6	End	UDCONU2	14.9192	0.5131
SB8	Span 7	Start	UDCONU2	14.9192	0.5131
SB8	Span 7	Middle	UDCONU2	17.5352	0.5818
SB8	Span 7	End	UDCONU2	12.2605	0.4021
SB8	Span 8	Start	UDCONU2	12.2605	0.4021
SB8	Span 8	Middle		0.0000	0.0000
SB8	Span 8	End	UDCONU2	4.8177	0.1506
SB8	Span 9	Start	UDCONU2	4.8177	0.1506
SB8	Span 9	Middle		0.0000	0.0000
SB8	Span 9	End	UDCONU2	2.9355	0.0917
SB8	Span 10	Start	UDCONU2	2.9355	0.0917
SB8	Span 10	Middle	UDCONU2	2.2959	0.0835
SB8	Span 10	End	UDCONU2	1.9963	0.0623
SB8	Span 11	Start	UDCONU2	1.9963	0.0623
SB8	Span 11	Middle	UDCONU2	2.5653	0.0801
SB8	Span 11	End	UDCONU2	0.0000	0.0000
SB8	Span 12	Start	UDCONU2	0.0000	0.0000
SB8	Span 12	Middle	UDCONU1	0.2088	0.0000
SB8	Span 12	End	UDCONU2	0.2304	0.0000
SB8	Span 13	Start	UDCONU2	0.2304	0.0000
SB8	Span 13	Middle		0.0000	0.0000
SB8	Span 13	End	UDCONU2	0.0000	0.0000
SB8	Span 14	Start	UDCONU2	0.0000	0.0000
SB8	Span 14	Middle	UDCONU2	2.6067	0.0814
SB8	Span 14	End	UDCONU2	0.2367	0.0000
SB8	Span 15	Start	UDCONU2	0.2367	0.0000
SB8	Span 15	Middle		0.0000	0.0000
SB8	Span 15	End	UDCONU2	0.0000	0.0000
SB8	Span 16	Start	UDCONU2	0.0000	0.0000
SB8	Span 16	Middle		0.0000	0.0000
SB8	Span 16	End	UDCONU1	0.3414	0.0000
SB8	Span 17	Start	UDCONU1	0.3414	0.0000
SB8	Span 17	Middle	UDCONU2	1.5257	0.0476
SB8	Span 17	End	UDCONU2	11.3014	0.3542
SB8	Span 18	Start	UDCONU2	11.3014	0.3542
SB8	Span 18	Middle		0.0000	0.0000
SB8	Span 18	End	UDCONU2	18.2039	0.5723
SB8	Span 19	Start	UDCONU2	18.2039	0.5723
SB8	Span 19	Middle		0.0000	0.0000
SB8	Span 19	End	UDCONU2	18.6561	0.5866
SB8	Span 20	Start	UDCONU2	18.6561	0.5866
SB8	Span 20	Middle	UDCONU2	11.5837	0.3631
SB8	Span 20	End	UDCONU2	2.1350	0.0667

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB8	Span 21	Start	UDCONU2	2.1350	0.0667
SB8	Span 21	Middle	UDCONU2	0.4455	0.0139
SB8	Span 21	End	UDCONU2	0.0000	0.0000
SB8	Span 22	Start	UDCONU2	0.0000	0.0000
SB8	Span 22	Middle		0.0000	0.0000
SB8	Span 22	End		0.0000	0.0000
SB9	Span 1	Start	UDCONU2	14.3526	0.4523
SB9	Span 1	Middle	UDCONU2	20.9141	0.6679
SB9	Span 1	End	UDCONU2	36.0878	1.1474
SB9	Span 2	Start	UDCONU2	36.0878	1.1474
SB9	Span 2	Middle	UDCONU2	37.0446	1.1773
SB9	Span 2	End	UDCONU2	4.7795	0.1530
SB9	Span 3	Start	UDCONU2	4.7795	0.1530
SB9	Span 3	Middle	UDCONU2	0.0000	0.0000
SB9	Span 3	End	UDCONU2	23.4013	0.7373
SB9	Span 4	Start	UDCONU2	23.4013	0.7373
SB9	Span 4	Middle	UDCONU1	6.9348	0.2169
SB9	Span 4	End	UDCONU2	27.2744	0.8609
SB9	Span 5	Start	UDCONU2	27.2744	0.8609
SB9	Span 5	Middle	UDCONU2	30.5845	0.9668
SB9	Span 5	End	UDCONU2	16.6715	0.5237
SB9	Span 6	Start	UDCONU2	16.6715	0.5237
SB9	Span 6	Middle	UDCONU2	12.2921	0.3854
SB9	Span 6	End	UDCONU2	11.8400	0.3712
SB9	Span 7	Start	UDCONU2	11.8400	0.3712
SB9	Span 7	Middle	UDCONU2	14.9468	0.4692
SB9	Span 7	End	UDCONU2	5.3582	0.1675
SB9	Span 8	Start	UDCONU2	5.3582	0.1675
SB9	Span 8	Middle	UDCONU2	1.8655	0.0582
SB9	Span 8	End	UDCONU2	20.2694	0.6378
SB9	Span 9	Start	UDCONU2	20.2694	0.6378
SB9	Span 9	Middle	UDCONU2	33.1587	1.0493
SB9	Span 9	End	UDCONU2	17.0258	0.5350
SB9	Span 10	Start	UDCONU2	17.0258	0.5350
SB9	Span 10	Middle	UDCONU2	11.9119	0.3734
SB9	Span 10	End	UDCONU2	9.6257	0.3015
SB9	Span 11	Start	UDCONU2	9.6257	0.3015
SB9	Span 11	Middle		0.0000	0.0000
SB9	Span 11	End	UDCONU2	29.1383	0.9536
SB9	Span 12	Start	UDCONU2	29.1383	0.9536
SB9	Span 12	Middle		0.0000	0.0000
SB9	Span 12	End	UDCONU2	7.9138	0.2503
SA10	Span 1	Start		0.0000	0.0000
SA10	Span 1	Middle		0.0000	0.0000
SA10	Span 1	End		0.0000	0.0000
SA10	Span 2	Start		0.0000	0.0000
SA10	Span 2	Middle		0.0000	0.0000
SA10	Span 2	End	UDCONU2	18.9903	0.5549
SA10	Span 3	Start	UDCONU2	18.9903	0.5549
SA10	Span 3	Middle	UDCONU2	15.5676	0.4562
SA10	Span 3	End	UDCONU2	0.0381	0.0000
SA10	Span 4	Start	UDCONU2	0.0381	0.0000
SA10	Span 4	Middle	UDCONU2	0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA10	Span 4	End	UDCONU2	0.0000	0.0000
SA10	Span 5	Start	UDCONU2	0.0000	0.0000
SA10	Span 5	Middle	UDCONU2	0.0000	0.0000
SA10	Span 5	End	UDCONU2	6.7196	0.2035
SA10	Span 6	Start	UDCONU2	6.7196	0.2035
SA10	Span 6	Middle	UDCONU2	21.8642	0.6287
SA10	Span 6	End	UDCONU1	17.1986	0.4891
SA10	Span 7	Start	UDCONU1	17.1986	0.4891
SA10	Span 7	Middle		0.0000	0.0000
SA10	Span 7	End	UDCONU2	26.2885	0.7490
SA10	Span 8	Start	UDCONU2	26.2885	0.7490
SA10	Span 8	Middle	UDCONU2	38.7165	1.1089
SA10	Span 8	End	UDCONU2	18.5041	0.5341
SA10	Span 9	Start	UDCONU2	18.5041	0.5341
SA10	Span 9	Middle		0.0000	0.0000
SA10	Span 9	End	UDCONU2	1.4464	0.0526
SA10	Span 10	Start	UDCONU2	1.4464	0.0526
SA10	Span 10	Middle	UDCONU1	2.6676	0.0801
SA10	Span 10	End		0.0000	0.0000
SA10	Span 11	Start		0.0000	0.0000
SA10	Span 11	Middle		0.0000	0.0000
SA10	Span 11	End		0.0000	0.0000
SA10	Span 12	Start		0.0000	0.0000
SA10	Span 12	Middle		0.0000	0.0000
SA10	Span 12	End		0.0000	0.0000
SA11	Span 1	Start		0.0000	0.0000
SA11	Span 1	Middle		0.0000	0.0000
SA11	Span 1	End		0.0000	0.0000
SA11	Span 2	Start		0.0000	0.0000
SA11	Span 2	Middle		0.0000	0.0000
SA11	Span 2	End		0.0000	0.0000
SA11	Span 3	Start	UDCONU2	6.3889	0.1839
SA11	Span 3	Middle	UDCONU2	4.0757	0.1184
SA11	Span 3	End	UDCONU2	0.0000	0.0000
SA11	Span 4	Start	UDCONU2	0.0000	0.0000
SA11	Span 4	Middle	UDCONU2	0.0000	0.0000
SA11	Span 4	End	UDCONU2	0.0000	0.0000
SA11	Span 5	Start	UDCONU2	0.0000	0.0000
SA11	Span 5	Middle	UDCONU2	0.0000	0.0000
SA11	Span 5	End	UDCONU2	9.8789	0.2942
SA11	Span 6	Start	UDCONU2	9.8789	0.2942
SA11	Span 6	Middle	UDCONU2	21.5876	0.6221
SA11	Span 6	End	UDCONU1	9.1457	0.2589
SA11	Span 7	Start	UDCONU1	9.1457	0.2589
SA11	Span 7	Middle		0.0000	0.0000
SA11	Span 7	End	UDCONU1	10.8661	0.3078
SA11	Span 8	Start	UDCONU1	10.8661	0.3078
SA11	Span 8	Middle	UDCONU2	35.2186	1.0064
SA11	Span 8	End	UDCONU2	30.2799	0.8657
SA11	Span 9	Start	UDCONU2	30.2799	0.8657
SA11	Span 9	Middle	UDCONU2	17.8249	0.5115
SA11	Span 9	End	UDCONU1	5.3127	0.1598
SA11	Span 10	Start	UDCONU1	5.3127	0.1598

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA11	Span 10	Middle	UDCONU1	5.3233	0.1548
SA11	Span 10	End		0.0000	0.0000
SA11	Span 11	Start		0.0000	0.0000
SA11	Span 11	Middle		0.0000	0.0000
SA11	Span 11	End		0.0000	0.0000
SA11	Span 12	Start		0.0000	0.0000
SA11	Span 12	Middle		0.0000	0.0000
SA11	Span 12	End		0.0000	0.0000
SA12	Span 1	Start		0.0000	0.0000
SA12	Span 1	Middle		0.0000	0.0000
SA12	Span 1	End		0.0000	0.0000
SA12	Span 2	Start		0.0000	0.0000
SA12	Span 2	Middle		0.0000	0.0000
SA12	Span 2	End		0.0000	0.0000
SA12	Span 3	Start		0.0000	0.0000
SA12	Span 3	Middle	UDCONU2	0.5015	0.0162
SA12	Span 3	End	UDCONU2	0.0000	0.0000
SA12	Span 4	Start	UDCONU2	0.0000	0.0000
SA12	Span 4	Middle	UDCONU2	0.0000	0.0000
SA12	Span 4	End	UDCONU2	0.0000	0.0000
SA12	Span 5	Start	UDCONU2	0.0000	0.0000
SA12	Span 5	Middle	UDCONU2	0.0000	0.0000
SA12	Span 5	End	UDCONU2	10.5029	0.3162
SA12	Span 6	Start	UDCONU2	10.5029	0.3162
SA12	Span 6	Middle	UDCONU2	20.7851	0.6071
SA12	Span 6	End	UDCONU1	7.8886	0.2259
SA12	Span 7	Start	UDCONU1	7.8886	0.2259
SA12	Span 7	Middle		0.0000	0.0000
SA12	Span 7	End	UDCONU1	4.5120	0.1275
SA12	Span 8	Start	UDCONU1	4.5120	0.1275
SA12	Span 8	Middle	UDCONU2	24.8031	0.7061
SA12	Span 8	End	UDCONU2	30.3234	0.8650
SA12	Span 9	Start	UDCONU2	30.3234	0.8650
SA12	Span 9	Middle	UDCONU2	25.5888	0.7287
SA12	Span 9	End	UDCONU1	11.8606	0.3405
SA12	Span 10	Start	UDCONU1	11.8606	0.3405
SA12	Span 10	Middle		0.0000	0.0000
SA12	Span 10	End	UDCONU2	13.6092	0.3917
SA12	Span 11	Start	UDCONU2	13.6092	0.3917
SA12	Span 11	Middle		0.0000	0.0000
SA12	Span 11	End		0.0000	0.0000
SA12	Span 12	Start		0.0000	0.0000
SA12	Span 12	Middle		0.0000	0.0000
SA12	Span 12	End		0.0000	0.0000
SA13	Span 1	Start		0.0000	0.0000
SA13	Span 1	Middle		0.0000	0.0000
SA13	Span 1	End		0.0000	0.0000
SA13	Span 2	Start		0.0000	0.0000
SA13	Span 2	Middle		0.0000	0.0000
SA13	Span 2	End		0.0000	0.0000
SA13	Span 3	Start		0.0000	0.0000
SA13	Span 3	Middle	UDCONU2	1.5142	0.0458
SA13	Span 3	End	UDCONU2	0.8726	0.0292

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA13	Span 4	Start	UDCONU2	0.2127	0.0000
SA13	Span 4	Middle	UDCONU2	0.0000	0.0000
SA13	Span 4	End	UDCONU2	0.0000	0.0000
SA13	Span 5	Start	UDCONU2	0.0000	0.0000
SA13	Span 5	Middle	UDCONU1	0.0398	0.0286
SA13	Span 5	End	UDCONU2	4.9413	0.1744
SA13	Span 6	Start	UDCONU2	4.9413	0.1744
SA13	Span 6	Middle	UDCONU2	10.7491	0.3394
SA13	Span 6	End	UDCONU1	0.9337	0.0623
SA13	Span 7	Start	UDCONU1	0.9337	0.0623
SA13	Span 7	Middle		0.0000	0.0000
SA13	Span 7	End	UDCONU2	10.3901	0.3060
SA13	Span 8	Start	UDCONU2	10.3901	0.3060
SA13	Span 8	Middle	UDCONU2	12.3012	0.3486
SA13	Span 8	End	UDCONU2	21.6969	0.6170
SA13	Span 9	Start	UDCONU2	21.6969	0.6170
SA13	Span 9	Middle	UDCONU2	23.0079	0.6546
SA13	Span 9	End	UDCONU2	22.5745	0.6422
SA13	Span 10	Start	UDCONU2	22.5745	0.6422
SA13	Span 10	Middle		0.0000	0.0000
SA13	Span 10	End	UDCONU2	23.1193	0.6578
SA13	Span 11	Start	UDCONU2	23.1193	0.6578
SA13	Span 11	Middle	UDCONU2	6.9535	0.1972
SA13	Span 11	End	UDCONU2	0.0805	0.0094
SA13	Span 12	Start	UDCONU2	0.0805	0.0094
SA13	Span 12	Middle	UDCONU2	2.9075	0.0828
SA13	Span 12	End	UDCONU2	1.0691	0.0306
SA14	Span 1	Start		0.0000	0.0000
SA14	Span 1	Middle		0.0000	0.0000
SA14	Span 1	End		0.0000	0.0000
SA14	Span 2	Start		0.0000	0.0000
SA14	Span 2	Middle		0.0000	0.0000
SA14	Span 2	End		0.0000	0.0000
SA14	Span 3	Start		0.0000	0.0000
SA14	Span 3	Middle		0.0000	0.0000
SA14	Span 3	End	UDCONU2	1.5943	0.0471
SA14	Span 4	Start	UDCONU2	1.1784	0.0350
SA14	Span 4	Middle	UDCONU2	0.3579	0.0000
SA14	Span 4	End	UDCONU2	0.0000	0.0000
SA14	Span 5	Start	UDCONU2	0.0000	0.0000
SA14	Span 5	Middle		0.0000	0.0000
SA14	Span 5	End	UDCONU2	0.0000	0.0000
SA14	Span 6	Start	UDCONU2	0.0000	0.0000
SA14	Span 6	Middle		0.0000	0.0000
SA14	Span 6	End	UDCONU2	0.0000	0.0662
SA14	Span 7	Start	UDCONU2	0.0000	0.0662
SA14	Span 7	Middle	UDCONU2	2.5186	0.1399
SA14	Span 7	End	UDCONU2	4.0254	0.1647
SA14	Span 8	Start	UDCONU2	4.0254	0.1647
SA14	Span 8	Middle	UDCONU2	4.2681	0.1489
SA14	Span 8	End	UDCONU2	0.2081	0.0781
SA14	Span 9	Start	UDCONU2	0.2081	0.0781
SA14	Span 9	Middle		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA14	Span 9	End	UDCONU2	0.1133	0.0886
SA14	Span 10	Start	UDCONU2	0.1133	0.0886
SA14	Span 10	Middle		0.0000	0.0000
SA14	Span 10	End	UDCONU2	10.5259	0.2981
SA14	Span 11	Start	UDCONU2	10.5259	0.2981
SA14	Span 11	Middle		0.0000	0.0000
SA14	Span 11	End	UDCONU2	8.5216	0.2412
SA14	Span 12	Start	UDCONU2	8.5216	0.2412
SA14	Span 12	Middle	UDCONU2	9.1771	0.2598
SA14	Span 12	End	UDCONU2	17.5072	0.4971
SA14	Span 13	Start	UDCONU2	17.5072	0.4971
SA14	Span 13	Middle	UDCONU2	24.2294	0.6896
SA14	Span 13	End	UDCONU2	3.4218	0.0967
SA14	Span 14	Start	UDCONU2	3.4218	0.0967
SA14	Span 14	Middle		0.0000	0.0000
SA14	Span 14	End	UDCONU2	16.2002	0.4598
SA14	Span 15	Start	UDCONU2	16.2002	0.4598
SA14	Span 15	Middle		0.0000	0.0000
SA14	Span 15	End	UDCONU1	0.2643	0.0366
SA14	Span 16	Start	UDCONU1	0.2643	0.0366
SA14	Span 16	Middle		0.0000	0.0000
SA14	Span 16	End	UDCONU2	0.0411	0.0731
SA14	Span 17	Start	UDCONU2	0.0411	0.0731
SA14	Span 17	Middle	UDCONU2	9.9489	0.3212
SA14	Span 17	End	UDCONU2	8.0817	0.2420
SA14	Span 18	Start	UDCONU2	8.0817	0.2420
SA14	Span 18	Middle	UDCONU2	13.5382	0.3839
SA14	Span 18	End	UDCONU2	9.8100	0.2778
SA14	Span 19	Start	UDCONU2	9.8100	0.2778
SA14	Span 19	Middle		0.0000	0.0000
SA14	Span 19	End	UDCONU2	8.2214	0.2342
SA15	Span 1	Start		0.0000	0.0000
SA15	Span 1	Middle		0.0000	0.0000
SA15	Span 1	End		0.0000	0.0000
SA15	Span 2	Start		0.0000	0.0000
SA15	Span 2	Middle		0.0000	0.0000
SA15	Span 2	End		0.0000	0.0000
SA15	Span 3	Start		0.0000	0.0000
SA15	Span 3	Middle		0.0000	0.0000
SA15	Span 3	End	UDCONU2	1.1108	0.0352
SA15	Span 4	Start	UDCONU2	1.1108	0.0352
SA15	Span 4	Middle	UDCONU2	1.3375	0.0482
SA15	Span 4	End	UDCONU2	0.0000	0.0000
SA15	Span 5	Start	UDCONU2	0.0000	0.0000
SA15	Span 5	Middle		0.0000	0.0000
SA15	Span 5	End	UDCONU2	0.0000	0.0000
SA15	Span 6	Start	UDCONU2	0.0000	0.0000
SA15	Span 6	Middle		0.0000	0.0000
SA15	Span 6	End	UDCONU2	0.0000	0.0000
SA15	Span 7	Start	UDCONU2	0.0000	0.0000
SA15	Span 7	Middle	UDCONU2	7.0472	0.2090
SA15	Span 7	End	UDCONU2	20.3017	0.5878
SA15	Span 8	Start	UDCONU2	20.3017	0.5878

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA15	Span 8	Middle	UDCONU2	21.0297	0.6111
SA15	Span 8	End	UDCONU2	5.3610	0.1527
SA15	Span 9	Start	UDCONU2	5.3610	0.1527
SA15	Span 9	Middle	UDCONU2	15.2872	0.4337
SA15	Span 9	End	UDCONU2	26.4202	0.7526
SA15	Span 10	Start	UDCONU2	26.4202	0.7526
SA15	Span 10	Middle	UDCONU2	28.5465	0.8138
SA15	Span 10	End	UDCONU2	5.0837	0.1437
SA15	Span 11	Start	UDCONU2	5.0837	0.1437
SA15	Span 11	Middle	UDCONU2	22.4254	0.6379
SA15	Span 11	End	UDCONU2	28.2286	0.8046
SA15	Span 12	Start	UDCONU2	28.2286	0.8046
SA15	Span 12	Middle	UDCONU2	21.9194	0.6234
SA15	Span 12	End	UDCONU2	5.7587	0.1646
SA15	Span 13	Start	UDCONU2	5.7587	0.1646
SA15	Span 13	Middle		0.0000	0.0000
SA15	Span 13	End	UDCONU2	10.5506	0.2995
SA16	Span 1	Start		0.0000	0.0000
SA16	Span 1	Middle		0.0000	0.0000
SA16	Span 1	End		0.0000	0.0000
SA16	Span 2	Start		0.0000	0.0000
SA16	Span 2	Middle		0.0000	0.0000
SA16	Span 2	End		0.0000	0.0000
SA16	Span 3	Start		0.0000	0.0000
SA16	Span 3	Middle		0.0000	0.0000
SA16	Span 3	End		0.0000	0.0000
SA16	Span 4	Start		0.0000	0.0000
SA16	Span 4	Middle	UDCONU2	0.1545	0.0000
SA16	Span 4	End	UDCONU2	0.6416	0.0000
SA16	Span 5	Start	UDCONU2	0.6416	0.0000
SA16	Span 5	Middle	UDCONU2	7.5075	0.2142
SA16	Span 5	End	UDCONU2	20.3808	0.5802
SA16	Span 6	Start	UDCONU2	20.3808	0.5802
SA16	Span 6	Middle	UDCONU2	24.5938	0.7001
SA16	Span 6	End	UDCONU2	19.1123	0.5430
SA16	Span 7	Start	UDCONU2	19.1123	0.5430
SA16	Span 7	Middle	UDCONU2	21.7547	0.6187
SA16	Span 7	End	UDCONU2	26.0796	0.7428
SA16	Span 8	Start	UDCONU2	26.0796	0.7428
SA16	Span 8	Middle	UDCONU2	26.7716	0.7627
SA16	Span 8	End	UDCONU2	18.9578	0.5386
SA16	Span 9	Start	UDCONU2	18.9578	0.5386
SA16	Span 9	Middle	UDCONU2	24.7876	0.7057
SA16	Span 9	End	UDCONU2	24.7876	0.7057
SA16	Span 10	Start	UDCONU2	24.7003	0.7032
SA16	Span 10	Middle	UDCONU2	11.8513	0.3381
SA16	Span 10	End	UDCONU2	0.0646	0.0000
SA16	Span 11	Start	UDCONU2	0.0646	0.0000
SA16	Span 11	Middle		0.0000	0.0000
SA16	Span 11	End	UDCONU2	2.6693	0.0805
SA17	Span 1	Start		0.0000	0.0000
SA17	Span 1	Middle		0.0000	0.0000
SA17	Span 1	End		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA17	Span 2	Start		0.0000	0.0000
SA17	Span 2	Middle		0.0000	0.0000
SA17	Span 2	End		0.0000	0.0000
SA17	Span 3	Start		0.0000	0.0000
SA17	Span 3	Middle		0.0000	0.0000
SA17	Span 3	End		0.0000	0.0000
SA17	Span 4	Start		0.0000	0.0000
SA17	Span 4	Middle	UDCONU2	1.2989	0.0373
SA17	Span 4	End	UDCONU2	13.7756	0.3906
SA17	Span 5	Start	UDCONU2	13.7756	0.3906
SA17	Span 5	Middle	UDCONU2	24.1844	0.6884
SA17	Span 5	End	UDCONU2	24.7445	0.7044
SA17	Span 6	Start	UDCONU2	24.7445	0.7044
SA17	Span 6	Middle	UDCONU2	23.7376	0.6755
SA17	Span 6	End	UDCONU2	21.5887	0.6139
SA17	Span 7	Start	UDCONU2	21.5887	0.6139
SA17	Span 7	Middle	UDCONU2	27.7534	0.7910
SA17	Span 7	End	UDCONU2	26.3078	0.7494
SA17	Span 8	Start	UDCONU2	26.3078	0.7494
SA17	Span 8	Middle	UDCONU2	24.5769	0.6996
SA17	Span 8	End	UDCONU2	23.1839	0.6650
SA17	Span 9	Start	UDCONU2	21.1943	0.6116
SA17	Span 9	Middle	UDCONU2	8.2848	0.2482
SA17	Span 9	End	UDCONU2	4.3155	0.1237
SA18	Span 1	Start		0.0000	0.0000
SA18	Span 1	Middle		0.0000	0.0000
SA18	Span 1	End		0.0000	0.0000
SA18	Span 2	Start		0.0000	0.0000
SA18	Span 2	Middle		0.0000	0.0000
SA18	Span 2	End		0.0000	0.0000
SA18	Span 3	Start		0.0000	0.0000
SA18	Span 3	Middle		0.0000	0.0000
SA18	Span 3	End		0.0000	0.0000
SA18	Span 4	Start		0.0000	0.0000
SA18	Span 4	Middle		0.0000	0.0000
SA18	Span 4	End	UDCONU2	16.6514	0.4761
SA18	Span 5	Start	UDCONU2	16.6514	0.4761
SA18	Span 5	Middle	UDCONU2	24.5113	0.6980
SA18	Span 5	End	UDCONU2	23.1572	0.6589
SA18	Span 6	Start	UDCONU2	23.1572	0.6589
SA18	Span 6	Middle	UDCONU2	25.4622	0.7251
SA18	Span 6	End	UDCONU2	13.8026	0.3914
SA18	Span 7	Start	UDCONU2	13.8026	0.3914
SA18	Span 7	Middle	UDCONU2	26.9924	0.7691
SA18	Span 7	End	UDCONU2	27.5505	0.7851
SA18	Span 8	Start	UDCONU2	27.5505	0.7851
SA18	Span 8	Middle	UDCONU2	24.5678	0.7088
SA18	Span 8	End	UDCONU2	26.0101	0.7559
SA18	Span 9	Start	UDCONU2	26.0101	0.7559
SA18	Span 9	Middle	UDCONU2	17.6452	0.5193
SA18	Span 9	End	UDCONU2	10.3127	0.2939
SA19	Span 1	Start		0.0000	0.0000
SA19	Span 1	Middle		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA19	Span 1	End		0.0000	0.0000
SA19	Span 2	Start		0.0000	0.0000
SA19	Span 2	Middle		0.0000	0.0000
SA19	Span 2	End		0.0000	0.0000
SA19	Span 3	Start		0.0000	0.0000
SA19	Span 3	Middle		0.0000	0.0000
SA19	Span 3	End		0.0000	0.0000
SA19	Span 4	Start		0.0000	0.0000
SA19	Span 4	Middle	UDCONU2	3.3455	0.0990
SA19	Span 4	End	UDCONU2	26.1325	0.7646
SA19	Span 5	Start	UDCONU2	26.1325	0.7646
SA19	Span 5	Middle	UDCONU2	29.6085	0.8537
SA19	Span 5	End	UDCONU2	15.7749	0.4476
SA19	Span 6	Start	UDCONU2	15.7749	0.4476
SA19	Span 6	Middle	UDCONU2	23.6317	0.6725
SA19	Span 6	End	UDCONU2	4.2051	0.1267
SA19	Span 7	Start	UDCONU2	4.2051	0.1267
SA19	Span 7	Middle	UDCONU2	24.1793	0.6882
SA19	Span 7	End	UDCONU2	26.2124	0.7466
SA19	Span 8	Start	UDCONU2	26.2124	0.7466
SA19	Span 8	Middle	UDCONU2	19.2736	0.5720
SA19	Span 8	End	UDCONU2	25.8033	0.7614
SA19	Span 9	Start	UDCONU2	25.8033	0.7614
SA19	Span 9	Middle	UDCONU2	24.2569	0.7109
SA19	Span 9	End	UDCONU2	11.8982	0.3401
SA20	Span 1	Start		0.0000	0.0000
SA20	Span 1	Middle		0.0000	0.0000
SA20	Span 1	End		0.0000	0.0000
SA20	Span 2	Start		0.0000	0.0000
SA20	Span 2	Middle		0.0000	0.0000
SA20	Span 2	End		0.0000	0.0000
SA20	Span 3	Start		0.0000	0.0000
SA20	Span 3	Middle		0.0000	0.0000
SA20	Span 3	End		0.0000	0.0000
SA20	Span 4	Start		0.0000	0.0000
SA20	Span 4	Middle		0.0000	0.0000
SA20	Span 4	End	UDCONU2	37.3071	1.0831
SA20	Span 5	Start	UDCONU2	37.3071	1.0831
SA20	Span 5	Middle		0.0000	0.0000
SA20	Span 5	End	UDCONU2	43.2485	1.2458
SA20	Span 6	Start	UDCONU2	43.2485	1.2458
SA20	Span 6	Middle	UDCONU2	26.2848	0.7577
SA20	Span 6	End	UDCONU2	6.3037	0.1783
SA20	Span 7	Start	UDCONU2	6.3037	0.1783
SA20	Span 7	Middle		0.0000	0.0000
SA20	Span 7	End	UDCONU2	5.5243	0.1562
SA20	Span 8	Start	UDCONU2	5.5243	0.1562
SA20	Span 8	Middle		0.0000	0.0000
SA20	Span 8	End	UDCONU2	12.8572	0.3645
SA20	Span 9	Start	UDCONU2	12.8572	0.3645
SA20	Span 9	Middle	UDCONU2	21.4904	0.6111
SA20	Span 9	End	UDCONU2	0.8787	0.0000
SA20	Span 10	Start	UDCONU2	0.8787	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA20	Span 10	Middle		0.0000	0.0000
SA20	Span 10	End	UDCONU2	0.2359	0.0489
SA20	Span 11	Start	UDCONU2	0.2359	0.0489
SA20	Span 11	Middle		0.0000	0.0000
SA20	Span 11	End	UDCONU2	0.3843	0.0451
SA20	Span 12	Start	UDCONU2	0.3843	0.0451
SA20	Span 12	Middle		0.0000	0.0000
SA20	Span 12	End	UDCONU2	0.0000	0.0468
SA20	Span 13	Start	UDCONU2	0.0000	0.0468
SA20	Span 13	Middle		0.0000	0.0000
SA20	Span 13	End	UDCONU2	0.0000	0.0000
SA20	Span 14	Start	UDCONU2	0.0000	0.0000
SA20	Span 14	Middle	UDCONU2	23.4114	0.6662
SA20	Span 14	End	UDCONU2	24.0556	0.6847
SA20	Span 15	Start	UDCONU2	24.0556	0.6847
SA20	Span 15	Middle	UDCONU2	13.9363	0.3952
SA20	Span 15	End	UDCONU2	2.3776	0.0672
SA20	Span 16	Start	UDCONU2	2.3776	0.0672
SA20	Span 16	Middle		0.0000	0.0000
SA20	Span 16	End	UDCONU2	1.4690	0.0869
SA20	Span 17	Start	UDCONU2	1.4690	0.0869
SA20	Span 17	Middle	UDCONU2	9.4462	0.3035
SA20	Span 17	End	UDCONU2	20.0584	0.6016
SA20	Span 18	Start	UDCONU2	20.0584	0.6016
SA20	Span 18	Middle	UDCONU2	23.3768	0.6810
SA20	Span 18	End	UDCONU2	20.8395	0.6009
SA20	Span 19	Start	UDCONU2	20.8395	0.6009
SA20	Span 19	Middle		0.0000	0.0000
SA20	Span 19	End	UDCONU2	9.8471	0.2813
SA21	Span 1	Start		0.0000	0.0000
SA21	Span 1	Middle		0.0000	0.0000
SA21	Span 1	End		0.0000	0.0000
SA21	Span 2	Start		0.0000	0.0000
SA21	Span 2	Middle		0.0000	0.0000
SA21	Span 2	End		0.0000	0.0000
SA21	Span 3	Start		0.0000	0.0000
SA21	Span 3	Middle		0.0000	0.0000
SA21	Span 3	End		0.0000	0.0000
SA21	Span 4	Start		0.0000	0.0000
SA21	Span 4	Middle		0.0000	0.0000
SA21	Span 4	End	UDCONU2	19.3985	0.5742
SA21	Span 5	Start	UDCONU2	18.3450	0.5245
SA21	Span 5	Middle	UDCONU2	9.9983	0.2872
SA21	Span 5	End	UDCONU2	7.3946	0.2098
SA21	Span 6	Start	UDCONU2	7.3946	0.2098
SA21	Span 6	Middle	UDCONU2	9.1061	0.2586
SA21	Span 6	End	UDCONU2	0.0000	0.0000
SA21	Span 7	Start	UDCONU2	0.0000	0.0000
SA21	Span 7	Middle	UDCONU2	6.2924	0.1784
SA21	Span 7	End	UDCONU2	12.0590	0.3432
SA21	Span 8	Start	UDCONU2	12.0590	0.3432
SA21	Span 8	Middle	UDCONU2	4.8333	0.1369
SA21	Span 8	End	UDCONU2	8.1327	0.2413

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA21	Span 9	Start	UDCONU2	8.1327	0.2413
SA21	Span 9	Middle	UDCONU2	12.5103	0.3566
SA21	Span 9	End	UDCONU2	4.5557	0.1301
SB10	Span 1	Start	UDCONU2	10.3635	0.3270
SB10	Span 1	Middle		0.0000	0.0000
SB10	Span 1	End	UDCONU2	18.4848	0.5984
SB10	Span 2	Start	UDCONU2	18.4848	0.5984
SB10	Span 2	Middle	UDCONU2	12.1153	0.3799
SB10	Span 2	End	UDCONU2	15.9121	0.5087
SB10	Span 3	Start	UDCONU2	15.9121	0.5087
SB10	Span 3	Middle	UDCONU2	25.1439	0.8180
SB10	Span 3	End	UDCONU2	4.5825	0.1549
SB10	Span 4	Start	UDCONU2	4.5825	0.1549
SB10	Span 4	Middle	UDCONU2	0.0000	0.0000
SB10	Span 4	End	UDCONU2	1.1002	0.0343
SB10	Span 5	Start	UDCONU2	1.1002	0.0343
SB10	Span 5	Middle		0.0000	0.0000
SB10	Span 5	End	UDCONU2	6.9005	0.2159
SB10	Span 6	Start	UDCONU2	6.9005	0.2159
SB10	Span 6	Middle	UDCONU2	15.9558	0.5011
SB10	Span 6	End	UDCONU2	29.2299	0.9234
SB10	Span 7	Start	UDCONU2	29.2299	0.9234
SB10	Span 7	Middle		0.0000	0.0000
SB10	Span 7	End	UDCONU2	31.1612	0.9852
SB10	Span 8	Start	UDCONU2	31.1612	0.9852
SB10	Span 8	Middle	UDCONU2	34.5978	1.0956
SB10	Span 8	End	UDCONU2	30.9802	0.9794
SB10	Span 9	Start	UDCONU2	30.9802	0.9794
SB10	Span 9	Middle	UDCONU2	25.4828	0.8037
SB10	Span 9	End	UDCONU2	23.0397	0.7258
SB10	Span 10	Start	UDCONU2	23.0397	0.7258
SB10	Span 10	Middle	UDCONU2	22.3992	0.7055
SB10	Span 10	End	UDCONU2	6.1525	0.1924
SB10	Span 11	Start	UDCONU2	6.1525	0.1924
SB10	Span 11	Middle	UDCONU2	0.9623	0.0300
SB10	Span 11	End	UDCONU2	17.1102	0.5376
SB10	Span 12	Start	UDCONU2	17.1102	0.5376
SB10	Span 12	Middle	UDCONU2	45.7917	1.4574
SB10	Span 12	End	UDCONU2	40.0563	1.2716
SB10	Span 13	Start	UDCONU2	40.0563	1.2716
SB10	Span 13	Middle	UDCONU2	36.0553	1.1425
SB10	Span 13	End	UDCONU2	27.3492	0.8633
SB10	Span 14	Start	UDCONU2	27.3492	0.8633
SB10	Span 14	Middle	UDCONU2	4.3152	0.1348
SB10	Span 14	End	UDCONU2	15.2959	0.4872
SB11	Span 1	Start	UDCONU2	2.4271	0.0764
SB11	Span 1	Middle	UDCONU2	1.9380	0.0606
SB11	Span 1	End	UDCONU2	0.4424	0.0380
SB11	Span 2	Start	UDCONU2	0.4424	0.0380
SB11	Span 2	Middle	UDCONU2	5.5063	0.2363
SB11	Span 2	End	UDCONU2	15.9197	0.6614
SB11	Span 3	Start	UDCONU2	15.9197	0.6614
SB11	Span 3	Middle		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB11	Span 3	End	UDCONU2	10.7371	0.5049
SB11	Span 4	Start	UDCONU2	10.7371	0.5049
SB11	Span 4	Middle	UDCONU2	8.6758	0.3480
SB11	Span 4	End	UDCONU2	1.2602	0.1025
SB11	Span 5	Start	UDCONU2	1.2602	0.1025
SB11	Span 5	Middle	UDCONU1	0.0000	0.0298
SB11	Span 5	End	UDCONU2	0.0000	0.0000
SB11	Span 6	Start	UDCONU2	0.0000	0.0000
SB11	Span 6	Middle	UDCONU2	0.0000	0.0000
SB11	Span 6	End	UDCONU2	11.1118	0.3482
SB11	Span 7	Start	UDCONU2	11.1118	0.3482
SB11	Span 7	Middle	UDCONU2	38.6591	1.2264
SB11	Span 7	End	UDCONU2	41.2948	1.3116
SB11	Span 8	Start	UDCONU2	41.2948	1.3116
SB11	Span 8	Middle	UDCONU2	37.8963	1.2018
SB11	Span 8	End	UDCONU2	25.2046	0.7948
SB11	Span 9	Start	UDCONU2	25.2046	0.7948
SB11	Span 9	Middle		0.0000	0.0000
SB11	Span 9	End	UDCONU2	24.7326	0.7798
SB11	Span 10	Start	UDCONU2	24.7326	0.7798
SB11	Span 10	Middle	UDCONU2	14.2403	0.4469
SB11	Span 10	End	UDCONU2	6.3042	0.1997
SB11	Span 11	Start	UDCONU2	6.3042	0.1997
SB11	Span 11	Middle	UDCONU2	10.9149	0.3554
SB11	Span 11	End	UDCONU2	0.7980	0.0352
SB11	Span 12	Start	UDCONU2	0.7980	0.0352
SB11	Span 12	Middle	UDCONU2	0.1626	0.0000
SB11	Span 12	End	UDCONU2	9.6294	0.3016
SB11	Span 13	Start	UDCONU2	9.6294	0.3016
SB11	Span 13	Middle	UDCONU2	54.0947	1.7283
SB11	Span 13	End	UDCONU2	54.7926	1.7511
SB11	Span 14	Start	UDCONU2	54.7926	1.7511
SB11	Span 14	Middle	UDCONU2	45.4492	1.4463
SB11	Span 14	End	UDCONU2	21.4457	0.6751
SB11	Span 15	Start	UDCONU2	21.4457	0.6751
SB11	Span 15	Middle		0.0000	0.0000
SB11	Span 15	End	UDCONU2	0.1091	0.0000
SB11	Span 16	Start	UDCONU2	0.1091	0.0000
SB11	Span 16	Middle		0.0000	0.0000
SB11	Span 16	End	UDCONU2	13.5334	0.4265
SB12	Span 1	Start		0.0000	0.0000
SB12	Span 1	Middle		0.0000	0.0000
SB12	Span 1	End		0.0000	0.0000
SB12	Span 2	Start		0.0000	0.0000
SB12	Span 2	Middle	UDCONU2	1.1346	0.0646
SB12	Span 2	End	UDCONU2	0.0212	0.0530
SB12	Span 3	Start	UDCONU2	0.0212	0.0530
SB12	Span 3	Middle	UDCONU2	0.0000	0.0786
SB12	Span 3	End	UDCONU2	0.0000	0.0777
SB12	Span 4	Start	UDCONU2	0.0000	0.0777
SB12	Span 4	Middle		0.0000	0.0000
SB12	Span 4	End	UDCONU2	0.0000	0.0859
SB12	Span 5	Start	UDCONU2	0.0000	0.0859

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB12	Span 5	Middle		0.0000	0.0000
SB12	Span 5	End	UDCONU2	0.0000	0.0000
SB12	Span 6	Start	UDCONU2	0.0000	0.0000
SB12	Span 6	Middle	UDCONU1	0.1637	0.0000
SB12	Span 6	End	UDCONU2	0.4836	0.0000
SB12	Span 7	Start	UDCONU2	0.4836	0.0000
SB12	Span 7	Middle	UDCONU2	28.6197	0.9039
SB12	Span 7	End	UDCONU2	40.8103	1.2959
SB12	Span 8	Start	UDCONU2	40.8103	1.2959
SB12	Span 8	Middle	UDCONU2	44.5985	1.4187
SB12	Span 8	End	UDCONU2	36.4303	1.1546
SB12	Span 9	Start	UDCONU2	36.4303	1.1546
SB12	Span 9	Middle	UDCONU2	27.1440	0.8567
SB12	Span 9	End	UDCONU2	6.0409	0.1889
SB12	Span 10	Start	UDCONU2	6.0409	0.1889
SB12	Span 10	Middle	UDCONU2	4.3306	0.1578
SB12	Span 10	End		0.0000	8.3174
SB12	Span 11	Start		0.0000	8.3174
SB12	Span 11	Middle		0.0000	0.0000
SB12	Span 11	End		0.0000	6.6872
SB12	Span 12	Start		0.0000	6.6872
SB12	Span 12	Middle	UDCONU2	0.0536	0.0000
SB12	Span 12	End	UDCONU2	6.6391	0.2077
SB12	Span 13	Start	UDCONU2	6.6391	0.2077
SB12	Span 13	Middle	UDCONU2	60.1908	1.9285
SB12	Span 13	End	UDCONU2	62.8074	2.0148
SB12	Span 14	Start	UDCONU2	62.8074	2.0148
SB12	Span 14	Middle	UDCONU2	51.0448	1.6285
SB12	Span 14	End	UDCONU2	25.9844	0.8197
SB12	Span 15	Start	UDCONU2	25.9844	0.8197
SB12	Span 15	Middle		0.0000	0.0000
SB12	Span 15	End	UDCONU2	0.1375	0.0000
SB12	Span 16	Start	UDCONU2	0.1375	0.0000
SB12	Span 16	Middle		0.0000	0.0000
SB12	Span 16	End	UDCONU2	6.2990	0.1997
SB13	Span 1	Start		0.0000	0.0000
SB13	Span 1	Middle		0.0000	0.0000
SB13	Span 1	End		0.0000	0.0000
SB13	Span 2	Start		0.0000	0.0000
SB13	Span 2	Middle		0.0000	0.0000
SB13	Span 2	End		0.0000	0.0000
SB13	Span 3	Start		0.0000	0.0000
SB13	Span 3	Middle		0.0000	0.0000
SB13	Span 3	End	UDCONU2	0.0000	0.0000
SB13	Span 4	Start	UDCONU2	0.0000	0.0000
SB13	Span 4	Middle	UDCONU2	15.9642	0.5017
SB13	Span 4	End	UDCONU2	21.0988	0.6641
SB13	Span 5	Start	UDCONU2	21.0988	0.6641
SB13	Span 5	Middle		0.0000	0.0000
SB13	Span 5	End	UDCONU2	26.4025	0.8330
SB13	Span 6	Start	UDCONU2	26.4025	0.8330
SB13	Span 6	Middle	UDCONU2	38.0572	1.2070
SB13	Span 6	End	UDCONU2	36.8837	1.1692

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB13	Span 7	Start	UDCONU2	36.8837	1.1692
SB13	Span 7	Middle	UDCONU2	32.5591	1.0301
SB13	Span 7	End	UDCONU2	1.6783	0.0524
SB13	Span 8	Start	UDCONU2	1.6783	0.0524
SB13	Span 8	Middle	UDCONU2	0.0000	0.0000
SB13	Span 8	End	UDCONU2	0.0000	0.0000
SB13	Span 9	Start	UDCONU2	0.0000	0.0000
SB13	Span 9	Middle	UDCONU2	0.0583	0.0000
SB13	Span 9	End	UDCONU2	6.2032	0.1940
SB13	Span 10	Start	UDCONU2	6.2032	0.1940
SB13	Span 10	Middle	UDCONU2	59.8170	1.9162
SB13	Span 10	End	UDCONU2	62.0958	1.9913
SB13	Span 11	Start	UDCONU2	62.0958	1.9913
SB13	Span 11	Middle	UDCONU2	50.4063	1.6077
SB13	Span 11	End	UDCONU2	18.6800	0.5874
SB13	Span 12	Start	UDCONU2	18.6800	0.5874
SB13	Span 12	Middle		0.0000	0.0000
SB13	Span 12	End	UDCONU2	0.0837	0.0000
SB13	Span 13	Start	UDCONU2	0.0837	0.0000
SB13	Span 13	Middle		0.0000	0.0000
SB13	Span 13	End	UDCONU2	1.5127	0.0603
SB14	Span 1	Start		0.0000	0.0000
SB14	Span 1	Middle		0.0000	0.0000
SB14	Span 1	End		0.0000	0.0000
SB14	Span 2	Start		0.0000	0.0000
SB14	Span 2	Middle		0.0000	0.0000
SB14	Span 2	End		0.0000	0.0000
SB14	Span 3	Start		0.0000	0.0000
SB14	Span 3	Middle		0.0000	0.0000
SB14	Span 3	End		0.0000	0.0000
SB14	Span 4	Start		0.0000	0.0000
SB14	Span 4	Middle	UDCONU2	0.5595	0.0176
SB14	Span 4	End	UDCONU2	7.7653	0.2448
SB14	Span 5	Start	UDCONU2	15.5549	0.4918
SB14	Span 5	Middle	UDCONU2	27.5548	0.8710
SB14	Span 5	End	UDCONU2	28.7215	0.9071
SB14	Span 6	Start	UDCONU2	28.7215	0.9071
SB14	Span 6	Middle		0.0000	0.0000
SB14	Span 6	End	UDCONU2	25.0588	0.7902
SB14	Span 7	Start	UDCONU2	25.0588	0.7902
SB14	Span 7	Middle	UDCONU2	24.4441	0.7706
SB14	Span 7	End	UDCONU2	0.7960	0.0000
SB14	Span 8	Start	UDCONU2	0.7960	0.0000
SB14	Span 8	Middle	UDCONU2	0.0000	0.0000
SB14	Span 8	End	UDCONU2	0.0000	0.0000
SB14	Span 9	Start	UDCONU2	0.0000	0.0000
SB14	Span 9	Middle	UDCONU2	0.0000	0.0000
SB14	Span 9	End	UDCONU2	7.2035	0.2254
SB14	Span 10	Start	UDCONU2	7.2035	0.2254
SB14	Span 10	Middle	UDCONU2	58.6427	1.8775
SB14	Span 10	End	UDCONU2	61.7795	1.9808
SB14	Span 11	Start	UDCONU2	61.7795	1.9808
SB14	Span 11	Middle	UDCONU2	50.4859	1.6103

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB14	Span 11	End	UDCONU2	25.1077	0.7917
SB14	Span 12	Start	UDCONU2	25.1077	0.7917
SB14	Span 12	Middle		0.0000	0.0000
SB14	Span 12	End	UDCONU2	0.1201	0.0000
SB14	Span 13	Start	UDCONU2	0.1201	0.0000
SB14	Span 13	Middle		0.0000	0.0000
SB14	Span 13	End	UDCONU2	2.4642	0.0806
SB15	Span 1	Start		0.0000	0.0000
SB15	Span 1	Middle		0.0000	0.0000
SB15	Span 1	End		0.0000	0.0000
SB15	Span 2	Start		0.0000	0.0000
SB15	Span 2	Middle		0.0000	0.0000
SB15	Span 2	End		0.0000	0.0000
SB15	Span 3	Start		0.0000	0.0000
SB15	Span 3	Middle		0.0000	0.0000
SB15	Span 3	End		0.0000	0.0000
SB15	Span 4	Start		0.0000	0.0000
SB15	Span 4	Middle		0.0000	0.0000
SB15	Span 4	End		0.0000	0.0000
SB15	Span 5	Start		0.0000	0.0000
SB15	Span 5	Middle		0.0000	0.0000
SB15	Span 5	End	UDCONU2	4.3810	0.1383
SB15	Span 6	Start	UDCONU2	9.2606	0.2924
SB15	Span 6	Middle	UDCONU1	10.4094	0.3266
SB15	Span 6	End	UDCONU1	2.1695	0.0677
SB15	Span 7	Start	UDCONU1	2.1695	0.0677
SB15	Span 7	Middle		0.0000	0.0000
SB15	Span 7	End	UDCONU1	0.1954	0.0000
SB15	Span 8	Start	UDCONU1	0.1954	0.0000
SB15	Span 8	Middle		0.0000	0.0000
SB15	Span 8	End	UDCONU1	0.0330	0.0000
SB15	Span 9	Start	UDCONU1	0.0330	0.0000
SB15	Span 9	Middle	UDCONU2	0.5417	0.0000
SB15	Span 9	End	UDCONU2	8.7940	0.2753
SB15	Span 10	Start	UDCONU2	8.7940	0.2753
SB15	Span 10	Middle	UDCONU2	57.5622	1.8420
SB15	Span 10	End	UDCONU2	60.0409	1.9235
SB15	Span 11	Start	UDCONU2	60.0409	1.9235
SB15	Span 11	Middle	UDCONU2	47.9368	1.5272
SB15	Span 11	End	UDCONU2	22.2854	0.7018
SB15	Span 12	Start	UDCONU2	22.2854	0.7018
SB15	Span 12	Middle		0.0000	0.0000
SB15	Span 12	End	UDCONU2	0.1263	0.0000
SB15	Span 13	Start	UDCONU2	0.1263	0.0000
SB15	Span 13	Middle		0.0000	0.0000
SB15	Span 13	End	UDCONU2	8.0036	0.2524
SB16	Span 1	Start		0.0000	0.0000
SB16	Span 1	Middle		0.0000	0.0000
SB16	Span 1	End		0.0000	0.0000
SB16	Span 2	Start		0.0000	0.0000
SB16	Span 2	Middle		0.0000	0.0000
SB16	Span 2	End		0.0000	0.0000
SB16	Span 3	Start		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB16	Span 3	Middle		0.0000	0.0000
SB16	Span 3	End		0.0000	0.0000
SB16	Span 4	Start		0.0000	0.0000
SB16	Span 4	Middle		0.0000	0.0000
SB16	Span 4	End		0.0000	0.0000
SB16	Span 5	Start		0.0000	0.0000
SB16	Span 5	Middle		0.0000	0.0000
SB16	Span 5	End		0.0000	0.0000
SB16	Span 6	Start		0.0000	0.0000
SB16	Span 6	Middle		0.0000	0.0000
SB16	Span 6	End	UDCONU1	0.0496	0.0015
SB16	Span 7	Start	UDCONU1	0.0496	0.0015
SB16	Span 7	Middle	UDCONU2	28.8555	0.9229
SB16	Span 7	End	UDCONU2	0.0000	0.0000
SB16	Span 8	Start	UDCONU2	0.0000	0.0000
SB16	Span 8	Middle	UDCONU2	1.0970	0.0342
SB16	Span 8	End	UDCONU2	18.7561	0.5898
SB16	Span 9	Start	UDCONU2	18.7561	0.5898
SB16	Span 9	Middle	UDCONU2	50.4597	1.6094
SB16	Span 9	End	UDCONU2	51.6182	1.6473
SB16	Span 10	Start	UDCONU2	51.6182	1.6473
SB16	Span 10	Middle	UDCONU2	45.2866	1.4429
SB16	Span 10	End	UDCONU2	24.3049	0.7744
SB16	Span 11	Start	UDCONU2	24.3049	0.7744
SB16	Span 11	Middle		0.0000	0.0000
SB16	Span 11	End	UDCONU2	0.1081	0.0000
SB16	Span 12	Start	UDCONU2	0.1081	0.0000
SB16	Span 12	Middle		0.0000	0.0000
SB16	Span 12	End	UDCONU2	12.3379	0.3970
SB17	Span 1	Start		0.0000	0.0000
SB17	Span 1	Middle		0.0000	0.0000
SB17	Span 1	End		0.0000	0.0000
SB17	Span 2	Start		0.0000	0.0000
SB17	Span 2	Middle		0.0000	0.0000
SB17	Span 2	End		0.0000	0.0000
SB17	Span 3	Start		0.0000	0.0000
SB17	Span 3	Middle		0.0000	0.0000
SB17	Span 3	End		0.0000	0.0000
SB17	Span 4	Start		0.0000	0.0000
SB17	Span 4	Middle		0.0000	0.0000
SB17	Span 4	End		0.0000	0.0000
SB17	Span 5	Start		0.0000	0.0000
SB17	Span 5	Middle		0.0000	0.0000
SB17	Span 5	End		0.0000	0.0000
SB17	Span 6	Start		0.0000	0.0000
SB17	Span 6	Middle		0.0000	0.0000
SB17	Span 6	End		0.0000	0.0000
SB17	Span 7	Start	UDCONU1	2.8028	0.0951
SB17	Span 7	Middle	UDCONU2	4.3408	0.1356
SB17	Span 7	End	UDCONU2	25.8399	0.8155
SB17	Span 8	Start	UDCONU2	25.8399	0.8155
SB17	Span 8	Middle	UDCONU2	36.7292	1.1685
SB17	Span 8	End	UDCONU2	36.4712	1.1676

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB17	Span 9	Start	UDCONU2	36.4712	1.1676
SB17	Span 9	Middle	UDCONU2	40.1491	1.2913
SB17	Span 9	End	UDCONU2	30.8288	0.9975
SB17	Span 10	Start	UDCONU2	30.8288	0.9975
SB17	Span 10	Middle	UDCONU2	6.2471	0.1963
SB17	Span 10	End	UDCONU2	16.3580	0.5177
SB18	Span 1	Start		0.0000	0.0000
SB18	Span 1	Middle		0.0000	0.0000
SB18	Span 1	End		0.0000	0.0000
SB18	Span 2	Start		0.0000	0.0000
SB18	Span 2	Middle		0.0000	0.0000
SB18	Span 2	End		0.0000	0.0000
SB18	Span 3	Start		0.0000	0.0000
SB18	Span 3	Middle		0.0000	0.0000
SB18	Span 3	End		0.0000	0.0000
SB18	Span 4	Start		0.0000	0.0000
SB18	Span 4	Middle		0.0000	0.0000
SB18	Span 4	End		0.0000	0.0000
SB18	Span 5	Start		0.0000	0.0000
SB18	Span 5	Middle		0.0000	0.0000
SB18	Span 5	End		0.0000	0.0000
SB18	Span 6	Start		0.0000	0.0000
SB18	Span 6	Middle		0.0000	0.0000
SB18	Span 6	End		0.0000	0.0000
SB18	Span 7	Start		0.0000	0.0000
SB18	Span 7	Middle	UDCONU2	4.6326	0.1466
SB18	Span 7	End	UDCONU2	2.9582	0.0997
SB18	Span 8	Start	UDCONU2	2.9582	0.0997
SB18	Span 8	Middle		0.0000	0.0000
SB18	Span 8	End	UDCONU2	4.7316	0.1608
SB18	Span 9	Start	UDCONU2	4.7316	0.1608
SB18	Span 9	Middle		0.0000	0.0000
SB18	Span 9	End	UDCONU2	5.4281	0.1697
SB18	Span 10	Start	UDCONU2	5.4281	0.1697
SB18	Span 10	Middle	UDCONU2	18.0880	0.5686
SB18	Span 10	End	UDCONU2	26.1369	0.8245
SB18	Span 11	Start	UDCONU2	26.1369	0.8245
SB18	Span 11	Middle	UDCONU2	23.0957	0.7276
SB18	Span 11	End	UDCONU2	10.9220	0.3459
SB18	Span 12	Start	UDCONU2	10.9220	0.3459
SB18	Span 12	Middle		0.0000	0.0000
SB18	Span 12	End	UDCONU2	8.9225	0.3079
SB18	Span 13	Start	UDCONU2	8.9225	0.3079
SB18	Span 13	Middle	UDCONU2	7.5099	0.2616
SB18	Span 13	End	UDCONU2	12.5903	0.4212
SB18	Span 14	Start	UDCONU2	12.5903	0.4212
SB18	Span 14	Middle	UDCONU2	19.5518	0.6350
SB18	Span 14	End	UDCONU2	25.0925	0.8049
SB18	Span 15	Start	UDCONU2	25.0925	0.8049
SB18	Span 15	Middle		0.0000	0.0000
SB18	Span 15	End	UDCONU2	20.7455	0.6652
SB18	Span 16	Start	UDCONU2	20.7455	0.6652
SB18	Span 16	Middle		0.0000	0.0000

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB18	Span 16	End	UDCONU2	14.1597	0.4459
SB19	Span 1	Start		0.0000	0.0000
SB19	Span 1	Middle		0.0000	0.0000
SB19	Span 1	End		0.0000	0.0000
SB19	Span 2	Start		0.0000	0.0000
SB19	Span 2	Middle		0.0000	0.0000
SB19	Span 2	End		0.0000	0.0000
SB19	Span 3	Start		0.0000	0.0000
SB19	Span 3	Middle		0.0000	0.0000
SB19	Span 3	End		0.0000	0.0000
SB19	Span 4	Start		0.0000	0.0000
SB19	Span 4	Middle		0.0000	0.0000
SB19	Span 4	End		0.0000	0.0000
SB19	Span 5	Start		0.0000	0.0000
SB19	Span 5	Middle		0.0000	0.0000
SB19	Span 5	End		0.0000	0.0000
SB19	Span 6	Start		0.0000	0.0000
SB19	Span 6	Middle		0.0000	0.0000
SB19	Span 6	End		0.0000	0.0000
SB19	Span 7	Start	UDCONU2	2.5689	0.0818
SB19	Span 7	Middle	UDCONU2	8.8009	0.2766
SB19	Span 7	End	UDCONU2	10.2321	0.3220
SB19	Span 8	Start	UDCONU2	10.2321	0.3220
SB19	Span 8	Middle	UDCONU2	3.5354	0.1112
SB19	Span 8	End	UDCONU2	0.3367	0.0213
SB19	Span 9	Start	UDCONU2	0.3367	0.0213
SB19	Span 9	Middle	UDCONU2	4.4497	0.1470
SB19	Span 9	End	UDCONU2	8.3641	0.2638
SB19	Span 10	Start	UDCONU2	8.3641	0.2638
SB19	Span 10	Middle	UDCONU2	7.6727	0.2436
SB19	Span 10	End	UDCONU2	5.0889	0.1609

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA1	Span 1	Start		0.000	0.0000	OK	A
SA1	Span 1	Middle	UDCONU1	2.013	0.0000	OK	A
SA1	Span 1	End	UDCONU2	0.375	0.0000	OK	A
SA1	Span 2	Start	UDCONU2	0.375	0.0000	OK	A
SA1	Span 2	Middle	UDCONU2	0.593	0.0000	OK	A
SA1	Span 2	End	UDCONU2	0.367	0.0000	OK	A
SA1	Span 3	Start	UDCONU2	0.367	0.0000	OK	A
SA1	Span 3	Middle	UDCONU2	0.186	0.0000	OK	A
SA1	Span 3	End	UDCONU2	0.146	0.0000	OK	A
SA1	Span 4	Start	UDCONU2	0.146	0.0000	OK	A
SA1	Span 4	Middle	UDCONU2	0.323	0.0000	OK	A
SA1	Span 4	End	UDCONU2	1.314	0.0000	OK	A
SA1	Span 5	Start	UDCONU2	1.314	0.0000	OK	A
SA1	Span 5	Middle	UDCONU2	3.931	0.0000	OK	A
SA1	Span 5	End		0.000	0.0000	OK	A
SA1	Span 6	Start		0.000	0.0000	OK	A

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA1	Span 6	Middle		0.000	0.0000	OK	A
SA1	Span 6	End		0.000	0.0000	OK	A
SA1	Span 7	Start		0.000	0.0000	OK	A
SA1	Span 7	Middle		0.000	0.0000	OK	A
SA1	Span 7	End		0.000	0.0000	OK	A
SA1	Span 8	Start		0.000	0.0000	OK	A
SA1	Span 8	Middle		0.000	0.0000	OK	A
SA1	Span 8	End		0.000	0.0000	OK	A
SA1	Span 9	Start		0.000	0.0000	OK	A
SA1	Span 9	Middle		0.000	0.0000	OK	A
SA1	Span 9	End		0.000	0.0000	OK	A
SA2	Cantilever Start	Start		0.000	0.0000	OK	A
SA2	Cantilever Start	Middle		0.000	0.0000	OK	A
SA2	Cantilever Start	End	UDCONU2	3.490	0.0000	OK	A
SA2	Span 1	Start	UDCONU2	3.490	0.0000	OK	A
SA2	Span 1	Middle	UDCONU2	1.600	0.0000	OK	A
SA2	Span 1	End	UDCONU2	1.600	0.0000	OK	A
SA2	Span 2	Start	UDCONU2	1.600	0.0000	OK	A
SA2	Span 2	Middle	UDCONU2	0.681	0.0000	OK	A
SA2	Span 2	End	UDCONU2	0.071	0.0000	OK	A
SA2	Span 3	Start	UDCONU2	0.071	0.0000	OK	A
SA2	Span 3	Middle	UDCONU2	0.032	0.0000	OK	A
SA2	Span 3	End	UDCONU1	5.102E-003	0.0000	OK	A
SA2	Span 4	Start	UDCONU1	0.046	0.0000	OK	A
SA2	Span 4	Middle	UDCONU1	0.060	0.0000	OK	A
SA2	Span 4	End	UDCONU2	0.083	0.0000	OK	A
SA2	Span 5	Start	UDCONU2	0.083	0.0000	OK	A
SA2	Span 5	Middle	UDCONU1	0.194	0.0000	OK	A
SA2	Span 5	End	UDCONU1	0.420	0.0000	OK	A
SA2	Span 6	Start	UDCONU1	0.420	0.0000	OK	A
SA2	Span 6	Middle		0.000	0.0000	OK	A
SA2	Span 6	End	UDCONU1	0.895	0.0000	OK	A
SA2	Span 7	Start	UDCONU2	0.074	0.0000	OK	A
SA2	Span 7	Middle		0.000	0.0000	OK	A
SA2	Span 7	End		0.000	0.0000	OK	A
SA2	Span 8	Start		0.000	0.0000	OK	A
SA2	Span 8	Middle		0.000	0.0000	OK	A
SA2	Span 8	End		0.000	0.0000	OK	A
SA2	Span 9	Start		0.000	0.0000	OK	A
SA2	Span 9	Middle		0.000	0.0000	OK	A
SA2	Span 9	End		0.000	0.0000	OK	A
SA2	Span 10	Start		0.000	0.0000	OK	A
SA2	Span 10	Middle		0.000	0.0000	OK	A
SA2	Span 10	End		0.000	0.0000	OK	A
SA3	Span 1	Start		0.000	0.0000	OK	A
SA3	Span 1	Middle	UDCONU2	1.690	0.0000	OK	A
SA3	Span 1	End	UDCONU2	5.571	0.0000	OK	A
SA3	Span 2	Start	UDCONU2	5.571	0.0000	OK	A
SA3	Span 2	Middle	UDCONU2	4.244	0.0000	OK	A
SA3	Span 2	End	UDCONU2	1.051	0.0000	OK	A
SA3	Span 3	Start	UDCONU2	1.051	0.0000	OK	A
SA3	Span 3	Middle	UDCONU2	0.577	0.0000	OK	A

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA3	Span 3	End	UDCONU2	0.193	0.0000	OK	A
SA3	Span 4	Start	UDCONU2	0.193	0.0000	OK	A
SA3	Span 4	Middle	UDCONU2	0.641	0.0000	OK	A
SA3	Span 4	End	UDCONU2	2.471	0.0000	OK	A
SA3	Span 5	Start	UDCONU2	2.471	0.0000	OK	A
SA3	Span 5	Middle	UDCONU2	3.247	0.0000	OK	A
SA3	Span 5	End	UDCONU1	0.199	0.0000	OK	A
SA3	Span 6	Start	UDCONU2	4.998	0.0000	OK	A
SA3	Span 6	Middle		0.000	0.0000	OK	A
SA3	Span 6	End		0.000	0.0000	OK	A
SA3	Span 7	Start		0.000	0.0000	OK	A
SA3	Span 7	Middle		0.000	0.0000	OK	A
SA3	Span 7	End		0.000	0.0000	OK	A
SA3	Span 8	Start		0.000	0.0000	OK	A
SA3	Span 8	Middle		0.000	0.0000	OK	A
SA3	Span 8	End		0.000	0.0000	OK	A
SA3	Span 9	Start		0.000	0.0000	OK	A
SA3	Span 9	Middle		0.000	0.0000	OK	A
SA3	Span 9	End		0.000	0.0000	OK	A
SA4	Span 1	Start		0.000	0.0000	OK	A
SA4	Span 1	Middle		0.000	0.0000	OK	A
SA4	Span 1	End	UDCONU2	2.166	0.0000	OK	A
SA4	Span 2	Start	UDCONU2	8.122	0.0000	OK	A
SA4	Span 2	Middle	UDCONU2	6.283	0.0000	OK	A
SA4	Span 2	End	UDCONU2	1.592	0.0000	OK	A
SA4	Span 3	Start	UDCONU2	1.592	0.0000	OK	A
SA4	Span 3	Middle	UDCONU2	0.964	0.0000	OK	A
SA4	Span 3	End	UDCONU2	0.195	0.0000	OK	A
SA4	Span 4	Start	UDCONU2	0.195	0.0000	OK	A
SA4	Span 4	Middle	UDCONU2	0.773	0.0000	OK	A
SA4	Span 4	End	UDCONU2	2.431	0.0000	OK	A
SA4	Span 5	Start	UDCONU2	2.431	0.0000	OK	A
SA4	Span 5	Middle	UDCONU2	3.376	0.0000	OK	A
SA4	Span 5	End	UDCONU2	1.383	0.0000	OK	A
SA4	Span 6	Start	UDCONU2	1.383	0.0000	OK	A
SA4	Span 6	Middle		0.000	0.0000	OK	A
SA4	Span 6	End	UDCONU2	0.605	0.0000	OK	A
SA4	Span 7	Start	UDCONU2	0.605	0.0000	OK	A
SA4	Span 7	Middle		0.000	0.0000	OK	A
SA4	Span 7	End	UDCONU2	12.072	0.0000	OK	A
SA4	Span 8	Start	UDCONU2	12.072	0.0000	OK	A
SA4	Span 8	Middle		0.000	0.0000	OK	A
SA4	Span 8	End	UDCONU2	3.106	0.0000	OK	A
SA4	Span 9	Start	UDCONU2	3.106	0.0000	OK	A
SA4	Span 9	Middle	UDCONU2	0.230	0.0000	OK	A
SA4	Span 9	End		0.000	0.0000	OK	A
SA4	Span 10	Start		0.000	0.0000	OK	A
SA4	Span 10	Middle		0.000	0.0000	OK	A
SA4	Span 10	End		0.000	0.0000	OK	A
SA4	Span 11	Start		0.000	0.0000	OK	A
SA4	Span 11	Middle		0.000	0.0000	OK	A
SA4	Span 11	End		0.000	0.0000	OK	A
SA4	Span 12	Start		0.000	0.0000	OK	A
SA4	Span 12	Middle		0.000	0.0000	OK	A

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA4	Span 12	End		0.000	0.0000	OK	A
SA5	Span 1	Start		0.000	0.0000	OK	A
SA5	Span 1	Middle		0.000	0.0000	OK	A
SA5	Span 1	End	UDCONU2	1.211	0.0000	OK	A
SA5	Span 2	Start	UDCONU1	2.450	0.0000	OK	A
SA5	Span 2	Middle	UDCONU2	2.877	0.0000	OK	A
SA5	Span 2	End	UDCONU2	0.984	0.0000	OK	A
SA5	Span 3	Start	UDCONU2	0.984	0.0000	OK	A
SA5	Span 3	Middle	UDCONU2	1.171	0.0000	OK	A
SA5	Span 3	End	UDCONU2	0.223	0.0000	OK	A
SA5	Span 4	Start	UDCONU2	0.223	0.0000	OK	A
SA5	Span 4	Middle	UDCONU2	0.570	0.0000	OK	A
SA5	Span 4	End	UDCONU2	2.444	0.0000	OK	A
SA5	Span 5	Start	UDCONU2	2.444	0.0000	OK	A
SA5	Span 5	Middle	UDCONU1	0.428	0.0000	OK	A
SA5	Span 5	End	UDCONU2	1.578	0.0000	OK	A
SA5	Span 6	Start	UDCONU2	1.578	0.0000	OK	A
SA5	Span 6	Middle		0.000	0.0000	OK	A
SA5	Span 6	End	UDCONU2	0.393	0.0000	OK	A
SA5	Span 7	Start	UDCONU2	0.393	0.0000	OK	A
SA5	Span 7	Middle	UDCONU2	0.242	0.0000	OK	A
SA5	Span 7	End		0.000	0.0000	OK	A
SA5	Span 8	Start		0.000	0.0000	OK	A
SA5	Span 8	Middle		0.000	0.0000	OK	A
SA5	Span 8	End		0.000	0.0000	OK	A
SA5	Span 9	Start		0.000	0.0000	OK	A
SA5	Span 9	Middle		0.000	0.0000	OK	A
SA5	Span 9	End		0.000	0.0000	OK	A
SA5	Span 10	Start		0.000	0.0000	OK	A
SA5	Span 10	Middle		0.000	0.0000	OK	A
SA5	Span 10	End		0.000	0.0000	OK	A
SA6	Span 1	Start		0.000	0.0000	OK	A
SA6	Span 1	Middle		0.000	0.0000	OK	A
SA6	Span 1	End	UDCONU2	0.214	0.0000	OK	A
SA6	Span 2	Start	UDCONU2	0.214	0.0000	OK	A
SA6	Span 2	Middle		0.000	0.0000	OK	A
SA6	Span 2	End	UDCONU2	51.283	0.0000	OK	A
SA6	Span 3	Start	UDCONU2	51.283	0.0000	OK	A
SA6	Span 3	Middle	UDCONU1	1.174	0.0000	OK	A
SA6	Span 3	End	UDCONU1	0.152	0.0000	OK	A
SA6	Span 4	Start	UDCONU1	0.152	0.0000	OK	A
SA6	Span 4	Middle	UDCONU2	3.822	0.0000	OK	A
SA6	Span 4	End	UDCONU2	0.642	0.0000	OK	A
SA6	Span 5	Start	UDCONU2	0.642	0.0000	OK	A
SA6	Span 5	Middle		0.000	0.0000	OK	A
SA6	Span 5	End	UDCONU1	0.712	0.0000	OK	A
SA6	Span 6	Start	UDCONU1	0.712	0.0000	OK	A
SA6	Span 6	Middle		0.000	0.0000	OK	A
SA6	Span 6	End	UDCONU1	0.377	0.0000	OK	A
SA6	Span 7	Start	UDCONU1	0.377	0.0000	OK	A
SA6	Span 7	Middle		0.000	0.0000	OK	A
SA6	Span 7	End	UDCONU1	0.518	0.0000	OK	A
SA6	Span 8	Start	UDCONU1	0.518	0.0000	OK	A
SA6	Span 8	Middle	UDCONU2	0.143	0.0000	OK	A

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA6	Span 8	End	UDCONU2	2.785	0.0000	OK	A
SA6	Span 9	Start	UDCONU2	2.785	0.0000	OK	A
SA6	Span 9	Middle		0.000	0.0000	OK	A
SA6	Span 9	End	UDCONU2	5.602	0.0000	OK	A
SA6	Span 10	Start	UDCONU2	5.602	0.0000	OK	A
SA6	Span 10	Middle		0.000	0.0000	OK	A
SA6	Span 10	End	UDCONU2	5.421	0.0000	OK	A
SA6	Span 11	Start	UDCONU2	5.421	0.0000	OK	A
SA6	Span 11	Middle	UDCONU2	3.297	0.0000	OK	A
SA6	Span 11	End	UDCONU2	0.485	0.0000	OK	A
SA6	Span 12	Start	UDCONU2	0.485	0.0000	OK	A
SA6	Span 12	Middle	UDCONU1	2.614	0.0000	OK	A
SA6	Span 12	End	UDCONU2	4.573	0.0000	OK	A
SA6	Span 13	Start	UDCONU2	4.573	0.0000	OK	A
SA6	Span 13	Middle		0.000	0.0000	OK	A
SA6	Span 13	End	UDCONU2	7.407	0.0000	OK	A
SA6	Span 14	Start	UDCONU2	7.407	0.0000	OK	A
SA6	Span 14	Middle	UDCONU1	3.145	0.0000	OK	A
SA6	Span 14	End		0.000	0.0000	OK	A
SA6	Span 15	Start		0.000	0.0000	OK	A
SA6	Span 15	Middle		0.000	0.0000	OK	A
SA6	Span 15	End		0.000	0.0000	OK	A
SA6	Span 16	Start		0.000	0.0000	OK	A
SA6	Span 16	Middle		0.000	0.0000	OK	A
SA6	Span 16	End		0.000	0.0000	OK	A
SA6	Span 17	Start		0.000	0.0000	OK	A
SA6	Span 17	Middle		0.000	0.0000	OK	A
SA6	Span 17	End		0.000	0.0000	OK	A
SA7	Span 1	Start		0.000	0.0000	OK	A
SA7	Span 1	Middle		0.000	0.0000	OK	A
SA7	Span 1	End		0.000	0.0000	OK	A
SA7	Span 2	Start		0.000	0.0000	OK	A
SA7	Span 2	Middle	UDCONU2	1.417	0.0000	OK	A
SA7	Span 2	End	UDCONU2	0.456	0.0000	OK	A
SA7	Span 3	Start	UDCONU2	2.188	0.0000	OK	A
SA7	Span 3	Middle	UDCONU2	1.365	0.0000	OK	A
SA7	Span 3	End	UDCONU2	0.315	0.0000	OK	A
SA7	Span 4	Start	UDCONU2	0.315	0.0000	OK	A
SA7	Span 4	Middle	UDCONU2	0.288	0.0000	OK	A
SA7	Span 4	End	UDCONU2	0.044	0.0000	OK	A
SA7	Span 5	Start	UDCONU2	0.044	0.0000	OK	A
SA7	Span 5	Middle		0.000	0.0000	OK	A
SA7	Span 5	End	UDCONU2	0.154	0.0000	OK	A
SA7	Span 6	Start	UDCONU2	0.154	0.0000	OK	A
SA7	Span 6	Middle	UDCONU2	3.234	0.0000	OK	A
SA7	Span 6	End	UDCONU1	1.463	0.0000	OK	A
SA7	Span 7	Start	UDCONU1	1.463	0.0000	OK	A
SA7	Span 7	Middle	UDCONU2	0.900	0.0000	OK	A
SA7	Span 7	End	UDCONU2	0.896	0.0000	OK	A
SA7	Span 8	Start	UDCONU2	0.896	0.0000	OK	A
SA7	Span 8	Middle	UDCONU2	1.763	0.0000	OK	A
SA7	Span 8	End	UDCONU1	0.684	0.0000	OK	A
SA7	Span 9	Start	UDCONU1	0.684	0.0000	OK	A
SA7	Span 9	Middle	UDCONU2	5.458	0.0000	OK	A

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA7	Span 9	End	UDCONU1	6.732	0.0000	OK	A
SA7	Span 10	Start	UDCONU1	6.732	0.0000	OK	A
SA7	Span 10	Middle		0.000	0.0000	OK	A
SA7	Span 10	End	UDCONU2	1.667	0.0000	OK	A
SA7	Span 11	Start	UDCONU2	1.558	0.0000	OK	A
SA7	Span 11	Middle		0.000	0.0000	OK	A
SA7	Span 11	End		0.000	0.0000	OK	A
SA7	Span 12	Start		0.000	0.0000	OK	A
SA7	Span 12	Middle		0.000	0.0000	OK	A
SA7	Span 12	End		0.000	0.0000	OK	A
SA7	Span 13	Start		0.000	0.0000	OK	A
SA7	Span 13	Middle		0.000	0.0000	OK	A
SA7	Span 13	End		0.000	0.0000	OK	A
SA8	Span 1	Start		0.000	0.0000	OK	A
SA8	Span 1	Middle		0.000	0.0000	OK	A
SA8	Span 1	End		0.000	0.0000	OK	A
SA8	Span 2	Start		0.000	0.0000	OK	A
SA8	Span 2	Middle	UDCONU2	2.005	0.0000	OK	A
SA8	Span 2	End	UDCONU2	6.230	0.0000	OK	A
SA8	Span 3	Start	UDCONU2	6.230	0.0000	OK	A
SA8	Span 3	Middle	UDCONU2	6.023	0.0000	OK	A
SA8	Span 3	End	UDCONU2	9.819	0.0000	OK	A
SA8	Span 4	Start	UDCONU2	10.065	0.0000	OK	A
SA8	Span 4	Middle	UDCONU2	7.422	0.0000	OK	A
SA8	Span 4	End	UDCONU2	0.831	0.0000	OK	A
SA8	Span 5	Start	UDCONU2	0.831	0.0000	OK	A
SA8	Span 5	Middle		0.000	0.0000	OK	A
SA8	Span 5	End	UDCONU2	5.658	0.0000	OK	A
SA8	Span 6	Start	UDCONU2	5.658	0.0000	OK	A
SA8	Span 6	Middle		0.000	0.0000	OK	A
SA8	Span 6	End	UDCONU2	0.409	0.0000	OK	A
SA8	Span 7	Start	UDCONU2	0.409	0.0000	OK	A
SA8	Span 7	Middle	UDCONU2	9.273	0.0000	OK	A
SA8	Span 7	End	UDCONU2	5.943	0.0000	OK	A
SA8	Span 8	Start	UDCONU2	5.943	0.0000	OK	A
SA8	Span 8	Middle	UDCONU1	2.876	0.0000	OK	A
SA8	Span 8	End	UDCONU2	0.612	0.0000	OK	A
SA8	Span 9	Start	UDCONU2	0.612	0.0000	OK	A
SA8	Span 9	Middle	UDCONU2	1.950	0.0000	OK	A
SA8	Span 9	End	UDCONU2	1.950	0.0000	OK	A
SA8	Span 10	Start	UDCONU2	1.950	0.0000	OK	A
SA8	Span 10	Middle	UDCONU2	2.400	0.0000	OK	A
SA8	Span 10	End	UDCONU1	1.580	0.0000	OK	A
SA8	Span 11	Start	UDCONU1	1.580	0.0000	OK	A
SA8	Span 11	Middle		0.000	0.0000	OK	A
SA8	Span 11	End	UDCONU2	4.987	0.0000	OK	A
SA8	Span 12	Start	UDCONU2	4.987	0.0000	OK	A
SA8	Span 12	Middle		0.000	0.0000	OK	A
SA8	Span 12	End		0.000	0.0000	OK	A
SA8	Span 13	Start		0.000	0.0000	OK	A
SA8	Span 13	Middle		0.000	0.0000	OK	A
SA8	Span 13	End		0.000	0.0000	OK	A
SA8	Span 14	Start		0.000	0.0000	OK	A
SA8	Span 14	Middle		0.000	0.0000	OK	A

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA8	Span 14	End		0.000	0.0000	OK	A
SA9	Span 1	Start		0.000	0.0000	OK	A
SA9	Span 1	Middle		0.000	0.0000	OK	A
SA9	Span 1	End		0.000	0.0000	OK	A
SA9	Span 2	Start		0.000	0.0000	OK	A
SA9	Span 2	Middle		0.000	0.0000	OK	A
SA9	Span 2	End	UDCONU2	4.056	0.0000	OK	A
SA9	Span 3	Start	UDCONU2	4.056	0.0000	OK	A
SA9	Span 3	Middle	UDCONU2	13.420	0.0000	OK	A
SA9	Span 3	End	UDCONU2	5.171	0.0000	OK	A
SA9	Span 4	Start	UDCONU2	5.171	0.0000	OK	A
SA9	Span 4	Middle	UDCONU2	11.650	0.0000	OK	A
SA9	Span 4	End	UDCONU2	6.393	0.0000	OK	A
SA9	Span 5	Start	UDCONU2	6.393	0.0000	OK	A
SA9	Span 5	Middle	UDCONU2	10.056	0.0000	OK	A
SA9	Span 5	End	UDCONU2	7.298	0.0000	OK	A
SA9	Span 6	Start	UDCONU2	7.298	0.0000	OK	A
SA9	Span 6	Middle	UDCONU2	4.637	0.0000	OK	A
SA9	Span 6	End	UDCONU2	2.385	0.0000	OK	A
SA9	Span 7	Start	UDCONU2	2.385	0.0000	OK	A
SA9	Span 7	Middle		0.000	0.0000	OK	A
SA9	Span 7	End	UDCONU2	4.088	0.0000	OK	A
SA9	Span 8	Start	UDCONU2	4.088	0.0000	OK	A
SA9	Span 8	Middle	UDCONU2	0.178	0.0000	OK	A
SA9	Span 8	End	UDCONU2	5.061	0.0000	OK	A
SA9	Span 9	Start	UDCONU2	5.061	0.0000	OK	A
SA9	Span 9	Middle		0.000	0.0000	OK	A
SA9	Span 9	End	UDCONU2	1.154	0.0000	OK	A
SA9	Span 10	Start	UDCONU2	1.154	0.0000	OK	A
SA9	Span 10	Middle	UDCONU2	1.574	0.0000	OK	A
SA9	Span 10	End		0.000	0.0000	OK	A
SA9	Span 11	Start		0.000	0.0000	OK	A
SA9	Span 11	Middle		0.000	0.0000	OK	A
SA9	Span 11	End		0.000	0.0000	OK	A
SA9	Span 12	Start		0.000	0.0000	OK	A
SA9	Span 12	Middle		0.000	0.0000	OK	A
SA9	Span 12	End		0.000	0.0000	OK	A
SB1	Span 1	Start		0.000	0.0000	OK	B
SB1	Span 1	Middle	UDCONU2	0.221	0.0000	OK	B
SB1	Span 1	End		0.000	0.0000	OK	B
SB1	Span 2	Start		0.000	0.0000	OK	B
SB1	Span 2	Middle		0.000	0.0000	OK	B
SB1	Span 2	End		0.000	0.0000	OK	B
SB1	Span 3	Start		0.000	0.0000	OK	B
SB1	Span 3	Middle		0.000	0.0000	OK	B
SB1	Span 3	End		0.000	0.0000	OK	B
SB1	Span 4	Start		0.000	0.0000	OK	B
SB1	Span 4	Middle		0.000	0.0000	OK	B
SB1	Span 4	End		0.000	0.0000	OK	B
SB1	Span 5	Start		0.000	0.0000	OK	B
SB1	Span 5	Middle		0.000	0.0000	OK	B
SB1	Span 5	End		0.000	0.0000	OK	B
SB1	Span 6	Start		0.000	0.0000	OK	B
SB1	Span 6	Middle		0.000	0.0000	OK	B

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB1	Span 6	End		0.000	0.0000	OK	B
SB1	Span 7	Start		0.000	0.0000	OK	B
SB1	Span 7	Middle		0.000	0.0000	OK	B
SB1	Span 7	End		0.000	0.0000	OK	B
SB1	Span 8	Start		0.000	0.0000	OK	B
SB1	Span 8	Middle		0.000	0.0000	OK	B
SB1	Span 8	End		0.000	0.0000	OK	B
SB1	Span 9	Start		0.000	0.0000	OK	B
SB1	Span 9	Middle		0.000	0.0000	OK	B
SB1	Span 9	End		0.000	0.0000	OK	B
SB1	Span 10	Start		0.000	0.0000	OK	B
SB1	Span 10	Middle		0.000	0.0000	OK	B
SB1	Span 10	End		0.000	0.0000	OK	B
SB2	Span 1	Start		0.000	0.0000	OK	B
SB2	Span 1	Middle		0.000	0.0000	OK	B
SB2	Span 1	End	UDCONU2	3.504	0.0000	OK	B
SB2	Span 2	Start	UDCONU2	3.504	0.0000	OK	B
SB2	Span 2	Middle	UDCONU1	0.621	0.0000	OK	B
SB2	Span 2	End	UDCONU2	1.623	0.0000	OK	B
SB2	Span 3	Start	UDCONU2	1.623	0.0000	OK	B
SB2	Span 3	Middle	UDCONU2	0.401	0.0000	OK	B
SB2	Span 3	End		0.000	0.0000	OK	B
SB2	Span 4	Start		0.000	0.0000	OK	B
SB2	Span 4	Middle		0.000	0.0000	OK	B
SB2	Span 4	End		0.000	0.0000	OK	B
SB2	Span 5	Start		0.000	0.0000	OK	B
SB2	Span 5	Middle		0.000	0.0000	OK	B
SB2	Span 5	End		0.000	0.0000	OK	B
SB2	Span 6	Start		0.000	0.0000	OK	B
SB2	Span 6	Middle		0.000	0.0000	OK	B
SB2	Span 6	End		0.000	0.0000	OK	B
SB2	Span 7	Start		0.000	0.0000	OK	B
SB2	Span 7	Middle		0.000	0.0000	OK	B
SB2	Span 7	End		0.000	0.0000	OK	B
SB2	Span 8	Start		0.000	0.0000	OK	B
SB2	Span 8	Middle		0.000	0.0000	OK	B
SB2	Span 8	End		0.000	0.0000	OK	B
SB2	Span 9	Start		0.000	0.0000	OK	B
SB2	Span 9	Middle		0.000	0.0000	OK	B
SB2	Span 9	End		0.000	0.0000	OK	B
SB2	Span 10	Start		0.000	0.0000	OK	B
SB2	Span 10	Middle		0.000	0.0000	OK	B
SB2	Span 10	End		0.000	0.0000	OK	B
SB2	Span 11	Start		0.000	0.0000	OK	B
SB2	Span 11	Middle		0.000	0.0000	OK	B
SB2	Span 11	End		0.000	0.0000	OK	B
SB3	Span 1	Start		0.000	0.0000	OK	B
SB3	Span 1	Middle	UDCONU2	3.142	0.0000	OK	B
SB3	Span 1	End	UDCONU2	0.748	0.0000	OK	B
SB3	Span 2	Start	UDCONU2	2.781	0.0000	OK	B
SB3	Span 2	Middle	UDCONU2	2.954	0.0000	OK	B
SB3	Span 2	End	UDCONU2	0.489	0.0000	OK	B
SB3	Span 3	Start	UDCONU2	1.611	0.0000	OK	B
SB3	Span 3	Middle	UDCONU2	2.993	0.0000	OK	B

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB3	Span 3	End		0.000	0.0000	OK	B
SB3	Span 4	Start		0.000	0.0000	OK	B
SB3	Span 4	Middle		0.000	0.0000	OK	B
SB3	Span 4	End		0.000	0.0000	OK	B
SB3	Span 5	Start		0.000	0.0000	OK	B
SB3	Span 5	Middle		0.000	0.0000	OK	B
SB3	Span 5	End		0.000	0.0000	OK	B
SB3	Span 6	Start		0.000	0.0000	OK	B
SB3	Span 6	Middle		0.000	0.0000	OK	B
SB3	Span 6	End		0.000	0.0000	OK	B
SB3	Span 7	Start		0.000	0.0000	OK	B
SB3	Span 7	Middle		0.000	0.0000	OK	B
SB3	Span 7	End		0.000	0.0000	OK	B
SB3	Span 8	Start		0.000	0.0000	OK	B
SB3	Span 8	Middle		0.000	0.0000	OK	B
SB3	Span 8	End		0.000	0.0000	OK	B
SB3	Span 9	Start		0.000	0.0000	OK	B
SB3	Span 9	Middle		0.000	0.0000	OK	B
SB3	Span 9	End		0.000	0.0000	OK	B
SB3	Span 10	Start		0.000	0.0000	OK	B
SB3	Span 10	Middle		0.000	0.0000	OK	B
SB3	Span 10	End		0.000	0.0000	OK	B
SB4	Span 1	Start		0.000	0.0000	OK	B
SB4	Span 1	Middle	UDCONU2	3.254	0.0000	OK	B
SB4	Span 1	End	UDCONU2	4.281	0.0000	OK	B
SB4	Span 2	Start	UDCONU2	0.574	0.0000	OK	B
SB4	Span 2	Middle	UDCONU2	2.096	0.0000	OK	B
SB4	Span 2	End	UDCONU2	6.646	0.0000	OK	B
SB4	Span 3	Start	UDCONU2	6.646	0.0000	OK	B
SB4	Span 3	Middle	UDCONU2	8.783	0.0000	OK	B
SB4	Span 3	End	UDCONU2	15.602	0.0000	OK	B
SB4	Span 4	Start	UDCONU2	15.602	0.0000	OK	B
SB4	Span 4	Middle	UDCONU2	12.795	0.0000	OK	B
SB4	Span 4	End	UDCONU2	7.748	0.0000	OK	B
SB4	Span 5	Start	UDCONU2	7.748	0.0000	OK	B
SB4	Span 5	Middle	UDCONU2	4.600	0.0000	OK	B
SB4	Span 5	End	UDCONU2	1.219	0.0000	OK	B
SB4	Span 6	Start	UDCONU2	0.136	0.0000	OK	B
SB4	Span 6	Middle		0.000	0.0000	OK	B
SB4	Span 6	End		0.000	0.0000	OK	B
SB4	Span 7	Start		0.000	0.0000	OK	B
SB4	Span 7	Middle		0.000	0.0000	OK	B
SB4	Span 7	End		0.000	0.0000	OK	B
SB4	Span 8	Start		0.000	0.0000	OK	B
SB4	Span 8	Middle		0.000	0.0000	OK	B
SB4	Span 8	End		0.000	0.0000	OK	B
SB4	Span 9	Start		0.000	0.0000	OK	B
SB4	Span 9	Middle		0.000	0.0000	OK	B
SB4	Span 9	End		0.000	0.0000	OK	B
SB4	Span 10	Start		0.000	0.0000	OK	B
SB4	Span 10	Middle		0.000	0.0000	OK	B
SB4	Span 10	End		0.000	0.0000	OK	B
SB4	Span 11	Start		0.000	0.0000	OK	B
SB4	Span 11	Middle		0.000	0.0000	OK	B

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB4	Span 11	End		0.000	0.0000	OK	B
SB5	Span 1	Start		0.000	0.0000	OK	B
SB5	Span 1	Middle	UDCONU2	3.386	0.0000	OK	B
SB5	Span 1	End	UDCONU2	6.244	0.0000	OK	B
SB5	Span 2	Start	UDCONU2	2.347	0.0000	OK	B
SB5	Span 2	Middle	UDCONU2	1.112	0.0000	OK	B
SB5	Span 2	End	UDCONU2	9.864	0.0000	OK	B
SB5	Span 3	Start	UDCONU2	9.864	0.0000	OK	B
SB5	Span 3	Middle	UDCONU2	14.258	0.0000	OK	B
SB5	Span 3	End	UDCONU2	4.202	0.0000	OK	B
SB5	Span 4	Start	UDCONU2	4.202	0.0000	OK	B
SB5	Span 4	Middle	UDCONU2	4.616	0.0000	OK	B
SB5	Span 4	End	UDCONU2	2.403	0.0000	OK	B
SB5	Span 5	Start	UDCONU2	2.403	0.0000	OK	B
SB5	Span 5	Middle	UDCONU2	0.159	0.0000	OK	B
SB5	Span 5	End	UDCONU2	0.738	0.0000	OK	B
SB5	Span 6	Start	UDCONU2	0.738	0.0000	OK	B
SB5	Span 6	Middle		0.000	0.0000	OK	B
SB5	Span 6	End		0.000	0.0000	OK	B
SB5	Span 7	Start		0.000	0.0000	OK	B
SB5	Span 7	Middle		0.000	0.0000	OK	B
SB5	Span 7	End		0.000	0.0000	OK	B
SB5	Span 8	Start		0.000	0.0000	OK	B
SB5	Span 8	Middle		0.000	0.0000	OK	B
SB5	Span 8	End		0.000	0.0000	OK	B
SB5	Span 9	Start		0.000	0.0000	OK	B
SB5	Span 9	Middle		0.000	0.0000	OK	B
SB5	Span 9	End		0.000	0.0000	OK	B
SB5	Span 10	Start		0.000	0.0000	OK	B
SB5	Span 10	Middle		0.000	0.0000	OK	B
SB5	Span 10	End		0.000	0.0000	OK	B
SB6	Span 1	Start		0.000	0.0000	OK	B
SB6	Span 1	Middle	UDCONU2	3.451	0.0000	OK	B
SB6	Span 1	End	UDCONU2	7.145	0.0000	OK	B
SB6	Span 2	Start	UDCONU2	3.095	0.0000	OK	B
SB6	Span 2	Middle	UDCONU2	0.717	0.0000	OK	B
SB6	Span 2	End	UDCONU2	10.978	0.0000	OK	B
SB6	Span 3	Start	UDCONU2	10.978	0.0000	OK	B
SB6	Span 3	Middle	UDCONU2	14.505	0.0000	OK	B
SB6	Span 3	End	UDCONU2	10.852	0.0000	OK	B
SB6	Span 4	Start	UDCONU2	10.852	0.0000	OK	B
SB6	Span 4	Middle		0.000	0.0000	OK	B
SB6	Span 4	End		0.000	0.0000	OK	B
SB6	Span 5	Start		0.000	0.0000	OK	B
SB6	Span 5	Middle		0.000	0.0000	OK	B
SB6	Span 5	End	UDCONU2	0.323	0.0000	OK	B
SB6	Span 6	Start	UDCONU2	0.238	0.0000	OK	B
SB6	Span 6	Middle	UDCONU2	12.788	0.0000	OK	B
SB6	Span 6	End	UDCONU2	4.867	0.0000	OK	B
SB6	Span 7	Start	UDCONU2	4.867	0.0000	OK	B
SB6	Span 7	Middle	UDCONU2	0.417	0.0000	OK	B
SB6	Span 7	End	UDCONU2	1.539	0.0000	OK	B
SB6	Span 8	Start	UDCONU2	1.539	0.0000	OK	B
SB6	Span 8	Middle	UDCONU2	1.465	0.0000	OK	B

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB6	Span 8	End	UDCONU2	2.842	0.0000	OK	B
SB6	Span 9	Start	UDCONU2	3.081	0.0000	OK	B
SB6	Span 9	Middle	UDCONU2	1.259	0.0000	OK	B
SB6	Span 9	End		0.000	0.0000	OK	B
SB6	Span 10	Start		0.000	0.0000	OK	B
SB6	Span 10	Middle		0.000	0.0000	OK	B
SB6	Span 10	End		0.000	0.0000	OK	B
SB6	Span 11	Start		0.000	0.0000	OK	B
SB6	Span 11	Middle		0.000	0.0000	OK	B
SB6	Span 11	End		0.000	0.0000	OK	B
SB6	Span 12	Start		0.000	0.0000	OK	B
SB6	Span 12	Middle		0.000	0.0000	OK	B
SB6	Span 12	End		0.000	0.0000	OK	B
SB7	Span 1	Start		0.000	0.0000	OK	B
SB7	Span 1	Middle	UDCONU2	3.521	0.0000	OK	B
SB7	Span 1	End	UDCONU2	7.146	0.0000	OK	B
SB7	Span 2	Start	UDCONU2	3.175	0.0000	OK	B
SB7	Span 2	Middle	UDCONU2	0.658	0.0000	OK	B
SB7	Span 2	End	UDCONU2	11.444	0.0000	OK	B
SB7	Span 3	Start	UDCONU2	11.444	0.0000	OK	B
SB7	Span 3	Middle	UDCONU2	15.333	0.0000	OK	B
SB7	Span 3	End	UDCONU2	6.737	0.0000	OK	B
SB7	Span 4	Start		0.000	0.0000	OK	B
SB7	Span 4	Middle		0.000	0.0000	OK	B
SB7	Span 4	End	UDCONU2	2.271	0.0000	OK	B
SB7	Span 5	Start	UDCONU1	0.539	0.0000	OK	B
SB7	Span 5	Middle	UDCONU2	4.081	0.0000	OK	B
SB7	Span 5	End	UDCONU2	1.869	0.0000	OK	B
SB7	Span 6	Start	UDCONU2	1.869	0.0000	OK	B
SB7	Span 6	Middle	UDCONU2	3.356	0.0000	OK	B
SB7	Span 6	End	UDCONU2	2.991	0.0000	OK	B
SB7	Span 7	Start	UDCONU2	2.991	0.0000	OK	B
SB7	Span 7	Middle	UDCONU2	5.521	0.0000	OK	B
SB7	Span 7	End	UDCONU2	0.114	0.0000	OK	B
SB7	Span 8	Start	UDCONU2	1.846	0.0000	OK	B
SB7	Span 8	Middle	UDCONU2	0.511	0.0000	OK	B
SB7	Span 8	End		0.000	0.0000	OK	B
SB7	Span 9	Start		0.000	0.0000	OK	B
SB7	Span 9	Middle		0.000	0.0000	OK	B
SB7	Span 9	End		0.000	0.0000	OK	B
SB7	Span 10	Start		0.000	0.0000	OK	B
SB7	Span 10	Middle		0.000	0.0000	OK	B
SB7	Span 10	End		0.000	0.0000	OK	B
SB8	Span 1	Start		0.000	0.0000	OK	B
SB8	Span 1	Middle	UDCONU2	3.619	0.0000	OK	B
SB8	Span 1	End	UDCONU2	6.526	0.0000	OK	B
SB8	Span 2	Start	UDCONU2	2.932	0.0000	OK	B
SB8	Span 2	Middle	UDCONU2	0.735	0.0000	OK	B
SB8	Span 2	End	UDCONU2	11.268	0.0000	OK	B
SB8	Span 3	Start	UDCONU2	11.268	0.0000	OK	B
SB8	Span 3	Middle	UDCONU2	14.298	0.0000	OK	B
SB8	Span 3	End	UDCONU2	14.256	0.0000	OK	B
SB8	Span 4	Start	UDCONU2	14.256	0.0000	OK	B
SB8	Span 4	Middle		0.000	0.0000	OK	B

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB8	Span 4	End	UDCONU2	1.874	0.0000	OK	B
SB8	Span 5	Start	UDCONU2	1.874	0.0000	OK	B
SB8	Span 5	Middle		0.000	0.0000	OK	B
SB8	Span 5	End	UDCONU2	9.444	0.0000	OK	B
SB8	Span 6	Start	UDCONU2	9.444	0.0000	OK	B
SB8	Span 6	Middle		0.000	0.0000	OK	B
SB8	Span 6	End	UDCONU2	0.675	0.0000	OK	B
SB8	Span 7	Start	UDCONU2	0.675	0.0000	OK	B
SB8	Span 7	Middle	UDCONU2	3.469	0.0000	OK	B
SB8	Span 7	End	UDCONU2	3.485	0.0000	OK	B
SB8	Span 8	Start	UDCONU2	3.485	0.0000	OK	B
SB8	Span 8	Middle		0.000	0.0000	OK	B
SB8	Span 8	End	UDCONU2	9.711	0.0000	OK	B
SB8	Span 9	Start	UDCONU2	9.711	0.0000	OK	B
SB8	Span 9	Middle		0.000	0.0000	OK	B
SB8	Span 9	End	UDCONU2	3.447	0.0000	OK	B
SB8	Span 10	Start	UDCONU2	3.447	0.0000	OK	B
SB8	Span 10	Middle	UDCONU2	2.317	0.0000	OK	B
SB8	Span 10	End	UDCONU2	1.613	0.0000	OK	B
SB8	Span 11	Start	UDCONU2	1.613	0.0000	OK	B
SB8	Span 11	Middle	UDCONU2	6.001	0.0000	OK	B
SB8	Span 11	End	UDCONU2	6.001	0.0000	OK	B
SB8	Span 12	Start	UDCONU2	6.001	0.0000	OK	B
SB8	Span 12	Middle	UDCONU2	2.003	0.0000	OK	B
SB8	Span 12	End	UDCONU2	2.313	0.0000	OK	B
SB8	Span 13	Start	UDCONU2	2.313	0.0000	OK	B
SB8	Span 13	Middle		0.000	0.0000	OK	B
SB8	Span 13	End	UDCONU2	2.313	0.0000	OK	B
SB8	Span 14	Start	UDCONU2	2.313	0.0000	OK	B
SB8	Span 14	Middle	UDCONU2	4.870	0.0000	OK	B
SB8	Span 14	End	UDCONU2	2.899	0.0000	OK	B
SB8	Span 15	Start	UDCONU2	2.899	0.0000	OK	B
SB8	Span 15	Middle		0.000	0.0000	OK	B
SB8	Span 15	End	UDCONU2	4.514	0.0000	OK	B
SB8	Span 16	Start	UDCONU2	4.514	0.0000	OK	B
SB8	Span 16	Middle		0.000	0.0000	OK	B
SB8	Span 16	End	UDCONU2	8.501	0.0000	OK	B
SB8	Span 17	Start	UDCONU2	8.501	0.0000	OK	B
SB8	Span 17	Middle	UDCONU2	2.340	0.0000	OK	B
SB8	Span 17	End	UDCONU2	0.250	0.0000	OK	B
SB8	Span 18	Start	UDCONU2	0.250	0.0000	OK	B
SB8	Span 18	Middle		0.000	0.0000	OK	B
SB8	Span 18	End	UDCONU2	0.850	0.0000	OK	B
SB8	Span 19	Start	UDCONU2	0.850	0.0000	OK	B
SB8	Span 19	Middle		0.000	0.0000	OK	B
SB8	Span 19	End	UDCONU2	0.110	0.0000	OK	B
SB8	Span 20	Start	UDCONU2	0.110	0.0000	OK	B
SB8	Span 20	Middle	UDCONU2	0.929	0.0000	OK	B
SB8	Span 20	End	UDCONU2	1.247	0.0000	OK	B
SB8	Span 21	Start	UDCONU2	1.247	0.0000	OK	B
SB8	Span 21	Middle	UDCONU2	0.769	0.0000	OK	B
SB8	Span 21	End	UDCONU2	0.774	0.0000	OK	B
SB8	Span 22	Start	UDCONU2	0.774	0.0000	OK	B
SB8	Span 22	Middle		0.000	0.0000	OK	B

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB8	Span 22	End		0.000	0.0000	OK	B
SB9	Span 1	Start		0.000	0.0000	OK	B
SB9	Span 1	Middle	UDCONU2	3.617	0.0000	OK	B
SB9	Span 1	End	UDCONU2	5.102	0.0000	OK	B
SB9	Span 2	Start	UDCONU2	2.596	0.0000	OK	B
SB9	Span 2	Middle	UDCONU2	0.711	0.0000	OK	B
SB9	Span 2	End	UDCONU2	11.262	0.0000	OK	B
SB9	Span 3	Start	UDCONU2	11.262	0.0000	OK	B
SB9	Span 3	Middle	UDCONU2	15.472	0.0000	OK	B
SB9	Span 3	End	UDCONU2	17.473	0.0000	OK	B
SB9	Span 4	Start	UDCONU2	17.473	0.0000	OK	B
SB9	Span 4	Middle	UDCONU2	6.725	0.0000	OK	B
SB9	Span 4	End	UDCONU2	4.031	0.0000	OK	B
SB9	Span 5	Start	UDCONU2	4.031	0.0000	OK	B
SB9	Span 5	Middle	UDCONU2	1.832	0.0000	OK	B
SB9	Span 5	End	UDCONU2	0.234	0.0000	OK	B
SB9	Span 6	Start	UDCONU2	0.234	0.0000	OK	B
SB9	Span 6	Middle	UDCONU2	2.235	0.0000	OK	B
SB9	Span 6	End	UDCONU2	1.687	0.0000	OK	B
SB9	Span 7	Start	UDCONU2	1.687	0.0000	OK	B
SB9	Span 7	Middle	UDCONU2	0.160	0.0000	OK	B
SB9	Span 7	End	UDCONU2	5.376	0.0000	OK	B
SB9	Span 8	Start	UDCONU2	5.376	0.0000	OK	B
SB9	Span 8	Middle	UDCONU2	5.553	0.0000	OK	B
SB9	Span 8	End	UDCONU2	3.844	0.0000	OK	B
SB9	Span 9	Start	UDCONU2	3.844	0.0000	OK	B
SB9	Span 9	Middle	UDCONU2	1.706	0.0000	OK	B
SB9	Span 9	End	UDCONU2	2.499	0.0000	OK	B
SB9	Span 10	Start	UDCONU2	2.499	0.0000	OK	B
SB9	Span 10	Middle	UDCONU2	3.288	0.0000	OK	B
SB9	Span 10	End	UDCONU2	1.721	0.0000	OK	B
SB9	Span 11	Start	UDCONU2	1.721	0.0000	OK	B
SB9	Span 11	Middle		0.000	0.0000	OK	B
SB9	Span 11	End	UDCONU2	5.563	0.0000	OK	B
SB9	Span 12	Start	UDCONU2	2.206	0.0000	OK	B
SB9	Span 12	Middle		0.000	0.0000	OK	B
SB9	Span 12	End	UDCONU2	0.814	0.0000	OK	B
SA10	Span 1	Start		0.000	0.0000	OK	A
SA10	Span 1	Middle		0.000	0.0000	OK	A
SA10	Span 1	End		0.000	0.0000	OK	A
SA10	Span 2	Start		0.000	0.0000	OK	A
SA10	Span 2	Middle		0.000	0.0000	OK	A
SA10	Span 2	End	UDCONU1	0.426	0.0000	OK	A
SA10	Span 3	Start	UDCONU2	2.964	0.0000	OK	A
SA10	Span 3	Middle	UDCONU2	6.774	0.0000	OK	A
SA10	Span 3	End	UDCONU2	11.072	0.0000	OK	A
SA10	Span 4	Start	UDCONU2	11.072	0.0000	OK	A
SA10	Span 4	Middle	UDCONU2	16.207	0.0000	OK	A
SA10	Span 4	End	UDCONU2	10.305	0.0000	OK	A
SA10	Span 5	Start	UDCONU2	10.305	0.0000	OK	A
SA10	Span 5	Middle	UDCONU2	13.869	0.0000	OK	A
SA10	Span 5	End	UDCONU2	10.147	0.0000	OK	A
SA10	Span 6	Start	UDCONU2	10.147	0.0000	OK	A
SA10	Span 6	Middle	UDCONU2	6.046	0.0000	OK	A

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA10	Span 6	End	UDCONU2	2.660	0.0000	OK	A
SA10	Span 7	Start	UDCONU2	2.660	0.0000	OK	A
SA10	Span 7	Middle		0.000	0.0000	OK	A
SA10	Span 7	End	UDCONU2	7.110	0.0000	OK	A
SA10	Span 8	Start	UDCONU2	7.110	0.0000	OK	A
SA10	Span 8	Middle	UDCONU2	2.251	0.0000	OK	A
SA10	Span 8	End	UDCONU2	4.077	0.0000	OK	A
SA10	Span 9	Start	UDCONU2	4.077	0.0000	OK	A
SA10	Span 9	Middle		0.000	0.0000	OK	A
SA10	Span 9	End	UDCONU2	2.943	0.0000	OK	A
SA10	Span 10	Start	UDCONU2	2.943	0.0000	OK	A
SA10	Span 10	Middle	UDCONU1	0.800	0.0000	OK	A
SA10	Span 10	End		0.000	0.0000	OK	A
SA10	Span 11	Start		0.000	0.0000	OK	A
SA10	Span 11	Middle		0.000	0.0000	OK	A
SA10	Span 11	End		0.000	0.0000	OK	A
SA10	Span 12	Start		0.000	0.0000	OK	A
SA10	Span 12	Middle		0.000	0.0000	OK	A
SA10	Span 12	End		0.000	0.0000	OK	A
SA11	Span 1	Start		0.000	0.0000	OK	A
SA11	Span 1	Middle		0.000	0.0000	OK	A
SA11	Span 1	End		0.000	0.0000	OK	A
SA11	Span 2	Start		0.000	0.0000	OK	A
SA11	Span 2	Middle		0.000	0.0000	OK	A
SA11	Span 2	End		0.000	0.0000	OK	A
SA11	Span 3	Start		0.000	0.0000	OK	A
SA11	Span 3	Middle	UDCONU2	4.897	0.0000	OK	A
SA11	Span 3	End	UDCONU2	11.915	0.0000	OK	A
SA11	Span 4	Start	UDCONU2	11.915	0.0000	OK	A
SA11	Span 4	Middle	UDCONU2	16.870	0.0000	OK	A
SA11	Span 4	End	UDCONU2	16.870	0.0000	OK	A
SA11	Span 5	Start	UDCONU2	16.870	0.0000	OK	A
SA11	Span 5	Middle	UDCONU2	13.508	0.0000	OK	A
SA11	Span 5	End	UDCONU2	9.792	0.0000	OK	A
SA11	Span 6	Start	UDCONU2	9.792	0.0000	OK	A
SA11	Span 6	Middle	UDCONU1	2.848	0.0000	OK	A
SA11	Span 6	End	UDCONU2	1.469	0.0000	OK	A
SA11	Span 7	Start	UDCONU2	1.469	0.0000	OK	A
SA11	Span 7	Middle		0.000	0.0000	OK	A
SA11	Span 7	End	UDCONU2	5.283	0.0000	OK	A
SA11	Span 8	Start	UDCONU2	5.283	0.0000	OK	A
SA11	Span 8	Middle	UDCONU2	3.890	0.0000	OK	A
SA11	Span 8	End	UDCONU2	1.296	0.0000	OK	A
SA11	Span 9	Start	UDCONU2	1.296	0.0000	OK	A
SA11	Span 9	Middle	UDCONU2	3.483	0.0000	OK	A
SA11	Span 9	End	UDCONU2	0.940	0.0000	OK	A
SA11	Span 10	Start	UDCONU2	0.940	0.0000	OK	A
SA11	Span 10	Middle	UDCONU2	0.025	0.0000	OK	A
SA11	Span 10	End		0.000	0.0000	OK	A
SA11	Span 11	Start		0.000	0.0000	OK	A
SA11	Span 11	Middle		0.000	0.0000	OK	A
SA11	Span 11	End		0.000	0.0000	OK	A
SA11	Span 12	Start		0.000	0.0000	OK	A
SA11	Span 12	Middle		0.000	0.0000	OK	A

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA11	Span 12	End		0.000	0.0000	OK	A
SA12	Span 1	Start		0.000	0.0000	OK	A
SA12	Span 1	Middle		0.000	0.0000	OK	A
SA12	Span 1	End		0.000	0.0000	OK	A
SA12	Span 2	Start		0.000	0.0000	OK	A
SA12	Span 2	Middle		0.000	0.0000	OK	A
SA12	Span 2	End		0.000	0.0000	OK	A
SA12	Span 3	Start		0.000	0.0000	OK	A
SA12	Span 3	Middle	UDCONU2	2.607	0.0000	OK	A
SA12	Span 3	End	UDCONU2	9.540	0.0000	OK	A
SA12	Span 4	Start	UDCONU2	9.540	0.0000	OK	A
SA12	Span 4	Middle	UDCONU2	13.078	0.0000	OK	A
SA12	Span 4	End	UDCONU2	13.078	0.0000	OK	A
SA12	Span 5	Start	UDCONU2	13.078	0.0000	OK	A
SA12	Span 5	Middle	UDCONU2	9.018	0.0000	OK	A
SA12	Span 5	End	UDCONU2	6.176	0.0000	OK	A
SA12	Span 6	Start	UDCONU2	6.176	0.0000	OK	A
SA12	Span 6	Middle	UDCONU1	1.508	0.0000	OK	A
SA12	Span 6	End	UDCONU2	1.961	0.0000	OK	A
SA12	Span 7	Start	UDCONU2	1.961	0.0000	OK	A
SA12	Span 7	Middle		0.000	0.0000	OK	A
SA12	Span 7	End	UDCONU2	2.484	0.0000	OK	A
SA12	Span 8	Start	UDCONU2	2.484	0.0000	OK	A
SA12	Span 8	Middle	UDCONU2	5.067	0.0000	OK	A
SA12	Span 8	End	UDCONU2	1.418	0.0000	OK	A
SA12	Span 9	Start	UDCONU2	1.418	0.0000	OK	A
SA12	Span 9	Middle	UDCONU2	0.885	0.0000	OK	A
SA12	Span 9	End	UDCONU2	0.958	0.0000	OK	A
SA12	Span 10	Start	UDCONU2	0.958	0.0000	OK	A
SA12	Span 10	Middle		0.000	0.0000	OK	A
SA12	Span 10	End	UDCONU2	1.627	0.0000	OK	A
SA12	Span 11	Start	UDCONU2	1.627	0.0000	OK	A
SA12	Span 11	Middle		0.000	0.0000	OK	A
SA12	Span 11	End		0.000	0.0000	OK	A
SA12	Span 12	Start		0.000	0.0000	OK	A
SA12	Span 12	Middle		0.000	0.0000	OK	A
SA12	Span 12	End		0.000	0.0000	OK	A
SA13	Span 1	Start		0.000	0.0000	OK	A
SA13	Span 1	Middle		0.000	0.0000	OK	A
SA13	Span 1	End		0.000	0.0000	OK	A
SA13	Span 2	Start		0.000	0.0000	OK	A
SA13	Span 2	Middle		0.000	0.0000	OK	A
SA13	Span 2	End		0.000	0.0000	OK	A
SA13	Span 3	Start		0.000	0.0000	OK	A
SA13	Span 3	Middle		0.000	0.0000	OK	A
SA13	Span 3	End	UDCONU2	4.952	0.0000	OK	A
SA13	Span 4	Start	UDCONU2	7.119	0.0000	OK	A
SA13	Span 4	Middle	UDCONU2	10.975	0.0000	OK	A
SA13	Span 4	End	UDCONU2	10.975	0.0000	OK	A
SA13	Span 5	Start	UDCONU2	10.975	0.0000	OK	A
SA13	Span 5	Middle	UDCONU2	3.726	0.0000	OK	A
SA13	Span 5	End	UDCONU1	0.119	0.0000	OK	A
SA13	Span 6	Start	UDCONU1	0.119	0.0000	OK	A
SA13	Span 6	Middle	UDCONU2	1.838	0.0000	OK	A

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA13	Span 6	End	UDCONU2	1.167	0.0000	OK	A
SA13	Span 7	Start	UDCONU2	1.167	0.0000	OK	A
SA13	Span 7	Middle		0.000	0.0000	OK	A
SA13	Span 7	End	UDCONU2	2.782	0.0000	OK	A
SA13	Span 8	Start	UDCONU2	2.782	0.0000	OK	A
SA13	Span 8	Middle	UDCONU2	7.297	0.0000	OK	A
SA13	Span 8	End	UDCONU2	3.666	0.0000	OK	A
SA13	Span 9	Start	UDCONU2	3.666	0.0000	OK	A
SA13	Span 9	Middle	UDCONU2	0.918	0.0000	OK	A
SA13	Span 9	End	UDCONU2	7.804	0.0000	OK	A
SA13	Span 10	Start	UDCONU2	7.804	0.0000	OK	A
SA13	Span 10	Middle		0.000	0.0000	OK	A
SA13	Span 10	End	UDCONU2	5.686	0.0000	OK	A
SA13	Span 11	Start	UDCONU2	5.686	0.0000	OK	A
SA13	Span 11	Middle	UDCONU1	2.019	0.0000	OK	A
SA13	Span 11	End	UDCONU2	1.849	0.0000	OK	A
SA13	Span 12	Start	UDCONU2	0.907	0.0000	OK	A
SA13	Span 12	Middle	UDCONU2	0.819	0.0000	OK	A
SA13	Span 12	End	UDCONU2	0.037	0.0000	OK	A
SA14	Span 1	Start		0.000	0.0000	OK	A
SA14	Span 1	Middle		0.000	0.0000	OK	A
SA14	Span 1	End		0.000	0.0000	OK	A
SA14	Span 2	Start		0.000	0.0000	OK	A
SA14	Span 2	Middle		0.000	0.0000	OK	A
SA14	Span 2	End		0.000	0.0000	OK	A
SA14	Span 3	Start		0.000	0.0000	OK	A
SA14	Span 3	Middle		0.000	0.0000	OK	A
SA14	Span 3	End	UDCONU2	2.734	0.0000	OK	A
SA14	Span 4	Start	UDCONU2	5.972	0.0000	OK	A
SA14	Span 4	Middle	UDCONU2	9.922	0.0000	OK	A
SA14	Span 4	End	UDCONU2	9.922	0.0000	OK	A
SA14	Span 5	Start	UDCONU2	9.922	0.0000	OK	A
SA14	Span 5	Middle		0.000	0.0000	OK	A
SA14	Span 5	End	UDCONU2	1.131	0.0000	OK	A
SA14	Span 6	Start	UDCONU2	1.131	0.0000	OK	A
SA14	Span 6	Middle		0.000	0.0000	OK	A
SA14	Span 6	End	UDCONU2	5.815	0.0000	OK	A
SA14	Span 7	Start	UDCONU2	5.815	0.0000	OK	A
SA14	Span 7	Middle	UDCONU2	1.494	0.0000	OK	A
SA14	Span 7	End	UDCONU2	3.182	0.0000	OK	A
SA14	Span 8	Start	UDCONU2	3.182	0.0000	OK	A
SA14	Span 8	Middle	UDCONU2	5.894	0.0000	OK	A
SA14	Span 8	End	UDCONU2	8.701	0.0000	OK	A
SA14	Span 9	Start	UDCONU2	8.701	0.0000	OK	A
SA14	Span 9	Middle		0.000	0.0000	OK	A
SA14	Span 9	End	UDCONU2	13.872	0.0000	OK	A
SA14	Span 10	Start	UDCONU2	13.872	0.0000	OK	A
SA14	Span 10	Middle		0.000	0.0000	OK	A
SA14	Span 10	End	UDCONU2	12.871	0.0000	OK	A
SA14	Span 11	Start	UDCONU2	12.871	0.0000	OK	A
SA14	Span 11	Middle		0.000	0.0000	OK	A
SA14	Span 11	End	UDCONU2	10.943	0.0000	OK	A
SA14	Span 12	Start	UDCONU2	10.943	0.0000	OK	A
SA14	Span 12	Middle	UDCONU2	10.696	0.0000	OK	A

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA14	Span 12	End	UDCONU2	5.023	0.0000	OK	A
SA14	Span 13	Start	UDCONU2	5.023	0.0000	OK	A
SA14	Span 13	Middle	UDCONU2	1.699	0.0000	OK	A
SA14	Span 13	End	UDCONU2	10.988	0.0000	OK	A
SA14	Span 14	Start	UDCONU2	10.988	0.0000	OK	A
SA14	Span 14	Middle		0.000	0.0000	OK	A
SA14	Span 14	End	UDCONU2	23.063	0.0000	OK	A
SA14	Span 15	Start	UDCONU2	23.063	0.0000	OK	A
SA14	Span 15	Middle		0.000	0.0000	OK	A
SA14	Span 15	End	UDCONU2	37.195	0.0000	OK	A
SA14	Span 16	Start	UDCONU2	37.195	0.0000	OK	A
SA14	Span 16	Middle		0.000	0.0000	OK	A
SA14	Span 16	End	UDCONU2	18.724	0.0000	OK	A
SA14	Span 17	Start	UDCONU2	18.724	0.0000	OK	A
SA14	Span 17	Middle	UDCONU2	6.859	0.0000	OK	A
SA14	Span 17	End	UDCONU2	3.445	0.0000	OK	A
SA14	Span 18	Start	UDCONU2	3.445	0.0000	OK	A
SA14	Span 18	Middle	UDCONU2	2.414	0.0000	OK	A
SA14	Span 18	End	UDCONU2	3.645	0.0000	OK	A
SA14	Span 19	Start	UDCONU2	3.645	0.0000	OK	A
SA14	Span 19	Middle		0.000	0.0000	OK	A
SA14	Span 19	End	UDCONU2	0.265	0.0000	OK	A
SA15	Span 1	Start		0.000	0.0000	OK	A
SA15	Span 1	Middle		0.000	0.0000	OK	A
SA15	Span 1	End		0.000	0.0000	OK	A
SA15	Span 2	Start		0.000	0.0000	OK	A
SA15	Span 2	Middle		0.000	0.0000	OK	A
SA15	Span 2	End		0.000	0.0000	OK	A
SA15	Span 3	Start		0.000	0.0000	OK	A
SA15	Span 3	Middle		0.000	0.0000	OK	A
SA15	Span 3	End	UDCONU2	1.353	0.0000	OK	A
SA15	Span 4	Start	UDCONU2	1.729	0.0000	OK	A
SA15	Span 4	Middle	UDCONU2	4.934	0.0000	OK	A
SA15	Span 4	End	UDCONU1	1.199	0.0000	OK	A
SA15	Span 5	Start	UDCONU1	1.199	0.0000	OK	A
SA15	Span 5	Middle		0.000	0.0000	OK	A
SA15	Span 5	End	UDCONU2	4.122	0.0000	OK	A
SA15	Span 6	Start	UDCONU2	4.122	0.0000	OK	A
SA15	Span 6	Middle		0.000	0.0000	OK	A
SA15	Span 6	End	UDCONU2	7.521	0.0000	OK	A
SA15	Span 7	Start	UDCONU2	7.521	0.0000	OK	A
SA15	Span 7	Middle	UDCONU2	5.380	0.0000	OK	A
SA15	Span 7	End	UDCONU2	2.864	0.0000	OK	A
SA15	Span 8	Start	UDCONU2	2.864	0.0000	OK	A
SA15	Span 8	Middle	UDCONU2	0.640	0.0000	OK	A
SA15	Span 8	End	UDCONU2	2.403	0.0000	OK	A
SA15	Span 9	Start	UDCONU2	2.403	0.0000	OK	A
SA15	Span 9	Middle	UDCONU2	2.273	0.0000	OK	A
SA15	Span 9	End	UDCONU2	2.744	0.0000	OK	A
SA15	Span 10	Start	UDCONU2	2.744	0.0000	OK	A
SA15	Span 10	Middle	UDCONU2	0.649	0.0000	OK	A
SA15	Span 10	End	UDCONU2	3.123	0.0000	OK	A
SA15	Span 11	Start	UDCONU2	3.123	0.0000	OK	A
SA15	Span 11	Middle	UDCONU2	0.718	0.0000	OK	A

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA15	Span 11	End	UDCONU2	1.447	0.0000	OK	A
SA15	Span 12	Start	UDCONU2	0.173	0.0000	OK	A
SA15	Span 12	Middle	UDCONU2	2.060	0.0000	OK	A
SA15	Span 12	End	UDCONU2	1.909	0.0000	OK	A
SA15	Span 13	Start	UDCONU2	1.909	0.0000	OK	A
SA15	Span 13	Middle		0.000	0.0000	OK	A
SA15	Span 13	End	UDCONU2	0.079	0.0000	OK	A
SA16	Span 1	Start		0.000	0.0000	OK	A
SA16	Span 1	Middle		0.000	0.0000	OK	A
SA16	Span 1	End		0.000	0.0000	OK	A
SA16	Span 2	Start		0.000	0.0000	OK	A
SA16	Span 2	Middle		0.000	0.0000	OK	A
SA16	Span 2	End		0.000	0.0000	OK	A
SA16	Span 3	Start		0.000	0.0000	OK	A
SA16	Span 3	Middle		0.000	0.0000	OK	A
SA16	Span 3	End		0.000	0.0000	OK	A
SA16	Span 4	Start		0.000	0.0000	OK	A
SA16	Span 4	Middle	UDCONU2	2.346	0.0000	OK	A
SA16	Span 4	End	UDCONU2	3.880	0.0000	OK	A
SA16	Span 5	Start	UDCONU2	3.880	0.0000	OK	A
SA16	Span 5	Middle	UDCONU2	8.468	0.0000	OK	A
SA16	Span 5	End	UDCONU2	6.363	0.0000	OK	A
SA16	Span 6	Start	UDCONU2	6.363	0.0000	OK	A
SA16	Span 6	Middle	UDCONU2	2.941	0.0000	OK	A
SA16	Span 6	End	UDCONU2	1.024	0.0000	OK	A
SA16	Span 7	Start	UDCONU2	1.024	0.0000	OK	A
SA16	Span 7	Middle	UDCONU2	1.378	0.0000	OK	A
SA16	Span 7	End	UDCONU2	0.817	0.0000	OK	A
SA16	Span 8	Start	UDCONU2	0.817	0.0000	OK	A
SA16	Span 8	Middle	UDCONU2	0.210	0.0000	OK	A
SA16	Span 8	End	UDCONU2	1.831	0.0000	OK	A
SA16	Span 9	Start	UDCONU2	1.831	0.0000	OK	A
SA16	Span 9	Middle	UDCONU2	1.067	0.0000	OK	A
SA16	Span 9	End	UDCONU2	2.337	0.0000	OK	A
SA16	Span 10	Start	UDCONU2	4.081	0.0000	OK	A
SA16	Span 10	Middle	UDCONU2	7.046	0.0000	OK	A
SA16	Span 10	End	UDCONU2	10.605	0.0000	OK	A
SA16	Span 11	Start	UDCONU2	10.605	0.0000	OK	A
SA16	Span 11	Middle		0.000	0.0000	OK	A
SA16	Span 11	End	UDCONU2	0.434	0.0000	OK	A
SA17	Span 1	Start		0.000	0.0000	OK	A
SA17	Span 1	Middle		0.000	0.0000	OK	A
SA17	Span 1	End		0.000	0.0000	OK	A
SA17	Span 2	Start		0.000	0.0000	OK	A
SA17	Span 2	Middle		0.000	0.0000	OK	A
SA17	Span 2	End		0.000	0.0000	OK	A
SA17	Span 3	Start		0.000	0.0000	OK	A
SA17	Span 3	Middle		0.000	0.0000	OK	A
SA17	Span 3	End		0.000	0.0000	OK	A
SA17	Span 4	Start		0.000	0.0000	OK	A
SA17	Span 4	Middle	UDCONU2	2.730	0.0000	OK	A
SA17	Span 4	End	UDCONU2	9.578	0.0000	OK	A
SA17	Span 5	Start	UDCONU2	9.578	0.0000	OK	A
SA17	Span 5	Middle	UDCONU2	4.306	0.0000	OK	A

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA17	Span 5	End	UDCONU2	1.968	0.0000	OK	A
SA17	Span 6	Start	UDCONU2	1.968	0.0000	OK	A
SA17	Span 6	Middle	UDCONU2	1.138	0.0000	OK	A
SA17	Span 6	End	UDCONU2	0.544	0.0000	OK	A
SA17	Span 7	Start	UDCONU2	0.544	0.0000	OK	A
SA17	Span 7	Middle	UDCONU2	0.467	0.0000	OK	A
SA17	Span 7	End	UDCONU2	1.210	0.0000	OK	A
SA17	Span 8	Start	UDCONU2	1.210	0.0000	OK	A
SA17	Span 8	Middle	UDCONU2	1.963	0.0000	OK	A
SA17	Span 8	End	UDCONU2	2.298	0.0000	OK	A
SA17	Span 9	Start	UDCONU2	3.985	0.0000	OK	A
SA17	Span 9	Middle	UDCONU2	6.468	0.0000	OK	A
SA17	Span 9	End	UDCONU2	0.293	0.0000	OK	A
SA18	Span 1	Start		0.000	0.0000	OK	A
SA18	Span 1	Middle		0.000	0.0000	OK	A
SA18	Span 1	End		0.000	0.0000	OK	A
SA18	Span 2	Start		0.000	0.0000	OK	A
SA18	Span 2	Middle		0.000	0.0000	OK	A
SA18	Span 2	End		0.000	0.0000	OK	A
SA18	Span 3	Start		0.000	0.0000	OK	A
SA18	Span 3	Middle		0.000	0.0000	OK	A
SA18	Span 3	End		0.000	0.0000	OK	A
SA18	Span 4	Start		0.000	0.0000	OK	A
SA18	Span 4	Middle		0.000	0.0000	OK	A
SA18	Span 4	End	UDCONU2	4.397	0.0000	OK	A
SA18	Span 5	Start	UDCONU2	13.086	0.0000	OK	A
SA18	Span 5	Middle	UDCONU2	3.880	0.0000	OK	A
SA18	Span 5	End	UDCONU2	2.422	0.0000	OK	A
SA18	Span 6	Start	UDCONU2	2.422	0.0000	OK	A
SA18	Span 6	Middle	UDCONU2	0.530	0.0000	OK	A
SA18	Span 6	End	UDCONU2	0.343	0.0000	OK	A
SA18	Span 7	Start	UDCONU2	0.343	0.0000	OK	A
SA18	Span 7	Middle	UDCONU2	1.676	0.0000	OK	A
SA18	Span 7	End	UDCONU2	0.785	0.0000	OK	A
SA18	Span 8	Start	UDCONU2	0.785	0.0000	OK	A
SA18	Span 8	Middle	UDCONU2	2.306	0.0000	OK	A
SA18	Span 8	End	UDCONU2	0.918	0.0000	OK	A
SA18	Span 9	Start	UDCONU2	2.593	0.0000	OK	A
SA18	Span 9	Middle	UDCONU2	4.617	0.0000	OK	A
SA18	Span 9	End	UDCONU2	0.301	0.0000	OK	A
SA19	Span 1	Start		0.000	0.0000	OK	A
SA19	Span 1	Middle		0.000	0.0000	OK	A
SA19	Span 1	End		0.000	0.0000	OK	A
SA19	Span 2	Start		0.000	0.0000	OK	A
SA19	Span 2	Middle		0.000	0.0000	OK	A
SA19	Span 2	End		0.000	0.0000	OK	A
SA19	Span 3	Start		0.000	0.0000	OK	A
SA19	Span 3	Middle		0.000	0.0000	OK	A
SA19	Span 3	End		0.000	0.0000	OK	A
SA19	Span 4	Start		0.000	0.0000	OK	A
SA19	Span 4	Middle		0.000	0.0000	OK	A
SA19	Span 4	End	UDCONU2	3.913	0.0000	OK	A
SA19	Span 5	Start	UDCONU2	14.227	0.0000	OK	A
SA19	Span 5	Middle	UDCONU2	0.228	0.0000	OK	A

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA19	Span 5	End	UDCONU2	2.408	0.0000	OK	A
SA19	Span 6	Start	UDCONU2	2.408	0.0000	OK	A
SA19	Span 6	Middle	UDCONU2	0.578	0.0000	OK	A
SA19	Span 6	End	UDCONU2	0.724	0.0000	OK	A
SA19	Span 7	Start	UDCONU2	0.724	0.0000	OK	A
SA19	Span 7	Middle	UDCONU2	5.900	0.0000	OK	A
SA19	Span 7	End	UDCONU2	0.423	0.0000	OK	A
SA19	Span 8	Start	UDCONU2	0.423	0.0000	OK	A
SA19	Span 8	Middle	UDCONU2	3.707	0.0000	OK	A
SA19	Span 8	End	UDCONU2	2.308	0.0000	OK	A
SA19	Span 9	Start	UDCONU2	0.037	0.0000	OK	A
SA19	Span 9	Middle	UDCONU2	1.222	0.0000	OK	A
SA19	Span 9	End	UDCONU2	0.266	0.0000	OK	A
SA20	Span 1	Start		0.000	0.0000	OK	A
SA20	Span 1	Middle		0.000	0.0000	OK	A
SA20	Span 1	End		0.000	0.0000	OK	A
SA20	Span 2	Start		0.000	0.0000	OK	A
SA20	Span 2	Middle		0.000	0.0000	OK	A
SA20	Span 2	End		0.000	0.0000	OK	A
SA20	Span 3	Start		0.000	0.0000	OK	A
SA20	Span 3	Middle		0.000	0.0000	OK	A
SA20	Span 3	End		0.000	0.0000	OK	A
SA20	Span 4	Start		0.000	0.0000	OK	A
SA20	Span 4	Middle		0.000	0.0000	OK	A
SA20	Span 4	End	UDCONU1	0.155	0.0000	OK	A
SA20	Span 5	Start	UDCONU2	4.080	0.0000	OK	A
SA20	Span 5	Middle		0.000	0.0000	OK	A
SA20	Span 5	End	UDCONU2	6.482	0.0000	OK	A
SA20	Span 6	Start	UDCONU2	6.482	0.0000	OK	A
SA20	Span 6	Middle	UDCONU2	5.905	0.0000	OK	A
SA20	Span 6	End	UDCONU2	4.804	0.0000	OK	A
SA20	Span 7	Start	UDCONU2	4.804	0.0000	OK	A
SA20	Span 7	Middle		0.000	0.0000	OK	A
SA20	Span 7	End	UDCONU2	2.884	0.0000	OK	A
SA20	Span 8	Start	UDCONU2	2.884	0.0000	OK	A
SA20	Span 8	Middle		0.000	0.0000	OK	A
SA20	Span 8	End	UDCONU2	4.609	0.0000	OK	A
SA20	Span 9	Start	UDCONU2	4.609	0.0000	OK	A
SA20	Span 9	Middle	UDCONU2	1.541	0.0000	OK	A
SA20	Span 9	End	UDCONU2	10.621	0.0000	OK	A
SA20	Span 10	Start	UDCONU2	10.621	0.0000	OK	A
SA20	Span 10	Middle		0.000	0.0000	OK	A
SA20	Span 10	End	UDCONU2	2.758	0.0000	OK	A
SA20	Span 11	Start	UDCONU2	2.758	0.0000	OK	A
SA20	Span 11	Middle		0.000	0.0000	OK	A
SA20	Span 11	End	UDCONU2	1.229	0.0000	OK	A
SA20	Span 12	Start	UDCONU2	1.229	0.0000	OK	A
SA20	Span 12	Middle		0.000	0.0000	OK	A
SA20	Span 12	End	UDCONU2	4.253	0.0000	OK	A
SA20	Span 13	Start	UDCONU2	4.253	0.0000	OK	A
SA20	Span 13	Middle		0.000	0.0000	OK	A
SA20	Span 13	End	UDCONU2	0.828	0.0000	OK	A
SA20	Span 14	Start	UDCONU2	0.828	0.0000	OK	A
SA20	Span 14	Middle	UDCONU2	4.904	0.0000	OK	A

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA20	Span 14	End	UDCONU1	0.132	0.0000	OK	A
SA20	Span 15	Start	UDCONU1	0.132	0.0000	OK	A
SA20	Span 15	Middle	UDCONU2	5.083	0.0000	OK	A
SA20	Span 15	End	UDCONU2	10.138	0.0000	OK	A
SA20	Span 16	Start	UDCONU2	10.138	0.0000	OK	A
SA20	Span 16	Middle		0.000	0.0000	OK	A
SA20	Span 16	End	UDCONU2	0.463	0.0000	OK	A
SA20	Span 17	Start	UDCONU2	0.463	0.0000	OK	A
SA20	Span 17	Middle	UDCONU2	5.502	0.0000	OK	A
SA20	Span 17	End	UDCONU2	2.474	0.0000	OK	A
SA20	Span 18	Start	UDCONU2	2.474	0.0000	OK	A
SA20	Span 18	Middle	UDCONU2	2.056	0.0000	OK	A
SA20	Span 18	End	UDCONU2	1.698	0.0000	OK	A
SA20	Span 19	Start	UDCONU2	1.698	0.0000	OK	A
SA20	Span 19	Middle		0.000	0.0000	OK	A
SA20	Span 19	End	UDCONU2	0.133	0.0000	OK	A
SA21	Span 1	Start		0.000	0.0000	OK	A
SA21	Span 1	Middle		0.000	0.0000	OK	A
SA21	Span 1	End		0.000	0.0000	OK	A
SA21	Span 2	Start		0.000	0.0000	OK	A
SA21	Span 2	Middle		0.000	0.0000	OK	A
SA21	Span 2	End		0.000	0.0000	OK	A
SA21	Span 3	Start		0.000	0.0000	OK	A
SA21	Span 3	Middle		0.000	0.0000	OK	A
SA21	Span 3	End		0.000	0.0000	OK	A
SA21	Span 4	Start		0.000	0.0000	OK	A
SA21	Span 4	Middle		0.000	0.0000	OK	A
SA21	Span 4	End	UDCONU2	0.882	0.0000	OK	A
SA21	Span 5	Start	UDCONU2	0.202	0.0000	OK	A
SA21	Span 5	Middle	UDCONU2	3.180	0.0000	OK	A
SA21	Span 5	End	UDCONU2	3.105	0.0000	OK	A
SA21	Span 6	Start	UDCONU2	3.105	0.0000	OK	A
SA21	Span 6	Middle	UDCONU2	0.993	0.0000	OK	A
SA21	Span 6	End	UDCONU2	0.441	0.0000	OK	A
SA21	Span 7	Start	UDCONU2	0.441	0.0000	OK	A
SA21	Span 7	Middle	UDCONU2	5.165	0.0000	OK	A
SA21	Span 7	End	UDCONU2	0.016	0.0000	OK	A
SA21	Span 8	Start	UDCONU2	0.016	0.0000	OK	A
SA21	Span 8	Middle	UDCONU2	4.708	0.0000	OK	A
SA21	Span 8	End	UDCONU2	1.589	0.0000	OK	A
SA21	Span 9	Start	UDCONU2	1.589	0.0000	OK	A
SA21	Span 9	Middle	UDCONU2	1.547	0.0000	OK	A
SA21	Span 9	End	UDCONU2	0.016	0.0000	OK	A
SB10	Span 1	Start		0.000	0.0000	OK	B
SB10	Span 1	Middle		0.000	0.0000	OK	B
SB10	Span 1	End	UDCONU2	3.389	0.0000	OK	B
SB10	Span 2	Start	UDCONU2	3.389	0.0000	OK	B
SB10	Span 2	Middle	UDCONU2	2.062	0.0000	OK	B
SB10	Span 2	End	UDCONU2	3.651	0.0000	OK	B
SB10	Span 3	Start	UDCONU2	3.651	0.0000	OK	B
SB10	Span 3	Middle	UDCONU2	0.434	0.0000	OK	B
SB10	Span 3	End	UDCONU2	10.798	0.0000	OK	B
SB10	Span 4	Start	UDCONU2	10.798	0.0000	OK	B
SB10	Span 4	Middle	UDCONU2	16.891	0.0000	OK	B

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB10	Span 4	End	UDCONU2	16.891	0.0000	OK	B
SB10	Span 5	Start	UDCONU2	16.891	0.0000	OK	B
SB10	Span 5	Middle		0.000	0.0000	OK	B
SB10	Span 5	End	UDCONU2	24.018	0.0000	OK	B
SB10	Span 6	Start	UDCONU2	24.018	0.0000	OK	B
SB10	Span 6	Middle	UDCONU2	10.075	0.0000	OK	B
SB10	Span 6	End	UDCONU2	5.639	0.0000	OK	B
SB10	Span 7	Start	UDCONU2	5.639	0.0000	OK	B
SB10	Span 7	Middle		0.000	0.0000	OK	B
SB10	Span 7	End	UDCONU2	3.885	0.0000	OK	B
SB10	Span 8	Start	UDCONU2	3.885	0.0000	OK	B
SB10	Span 8	Middle	UDCONU2	1.882	0.0000	OK	B
SB10	Span 8	End	UDCONU2	0.824	0.0000	OK	B
SB10	Span 9	Start	UDCONU2	0.824	0.0000	OK	B
SB10	Span 9	Middle	UDCONU2	0.793	0.0000	OK	B
SB10	Span 9	End	UDCONU2	2.161	0.0000	OK	B
SB10	Span 10	Start	UDCONU2	2.161	0.0000	OK	B
SB10	Span 10	Middle	UDCONU2	2.559	0.0000	OK	B
SB10	Span 10	End	UDCONU2	8.445	0.0000	OK	B
SB10	Span 11	Start	UDCONU2	8.445	0.0000	OK	B
SB10	Span 11	Middle	UDCONU2	15.941	0.0000	OK	B
SB10	Span 11	End	UDCONU2	7.128	0.0000	OK	B
SB10	Span 12	Start	UDCONU2	7.128	0.0000	OK	B
SB10	Span 12	Middle	UDCONU2	4.263	0.0000	OK	B
SB10	Span 12	End	UDCONU2	1.452	0.0000	OK	B
SB10	Span 13	Start	UDCONU2	1.452	0.0000	OK	B
SB10	Span 13	Middle	UDCONU2	4.110	0.0000	OK	B
SB10	Span 13	End	UDCONU2	6.205	0.0000	OK	B
SB10	Span 14	Start	UDCONU2	6.205	0.0000	OK	B
SB10	Span 14	Middle	UDCONU2	8.414	0.0000	OK	B
SB10	Span 14	End	UDCONU2	0.398	0.0000	OK	B
SB11	Span 1	Start		0.000	0.0000	OK	B
SB11	Span 1	Middle	UDCONU2	1.475	0.0000	OK	B
SB11	Span 1	End	UDCONU2	0.768	0.0000	OK	B
SB11	Span 2	Start	UDCONU2	0.768	0.0000	OK	B
SB11	Span 2	Middle	UDCONU2	1.790	0.0000	OK	B
SB11	Span 2	End	UDCONU2	1.882	0.0000	OK	B
SB11	Span 3	Start	UDCONU2	1.933	0.0000	OK	B
SB11	Span 3	Middle		0.000	0.0000	OK	B
SB11	Span 3	End	UDCONU2	11.517	0.0000	OK	B
SB11	Span 4	Start	UDCONU2	11.517	0.0000	OK	B
SB11	Span 4	Middle	UDCONU2	3.114	0.0000	OK	B
SB11	Span 4	End	UDCONU2	6.183	0.0000	OK	B
SB11	Span 5	Start	UDCONU2	6.183	0.0000	OK	B
SB11	Span 5	Middle	UDCONU2	9.897	0.0000	OK	B
SB11	Span 5	End	UDCONU2	9.897	0.0000	OK	B
SB11	Span 6	Start	UDCONU2	9.897	0.0000	OK	B
SB11	Span 6	Middle	UDCONU2	10.393	0.0000	OK	B
SB11	Span 6	End	UDCONU2	6.244	0.0000	OK	B
SB11	Span 7	Start	UDCONU2	6.244	0.0000	OK	B
SB11	Span 7	Middle	UDCONU2	6.527	0.0000	OK	B
SB11	Span 7	End	UDCONU1	1.067	0.0000	OK	B
SB11	Span 8	Start	UDCONU1	1.067	0.0000	OK	B
SB11	Span 8	Middle	UDCONU2	0.216	0.0000	OK	B

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB11	Span 8	End	UDCONU2	2.986	0.0000	OK	B
SB11	Span 9	Start	UDCONU2	2.986	0.0000	OK	B
SB11	Span 9	Middle		0.000	0.0000	OK	B
SB11	Span 9	End	UDCONU2	4.106	0.0000	OK	B
SB11	Span 10	Start	UDCONU2	4.106	0.0000	OK	B
SB11	Span 10	Middle	UDCONU2	3.730	0.0000	OK	B
SB11	Span 10	End	UDCONU2	8.460	0.0000	OK	B
SB11	Span 11	Start	UDCONU2	8.460	0.0000	OK	B
SB11	Span 11	Middle	UDCONU2	30.940	0.0000	OK	B
SB11	Span 11	End	UDCONU2	30.577	0.0000	OK	B
SB11	Span 12	Start	UDCONU2	30.577	0.0000	OK	B
SB11	Span 12	Middle	UDCONU2	15.411	0.0000	OK	B
SB11	Span 12	End	UDCONU2	10.380	0.0000	OK	B
SB11	Span 13	Start	UDCONU2	10.380	0.0000	OK	B
SB11	Span 13	Middle	UDCONU2	6.523	0.0000	OK	B
SB11	Span 13	End	UDCONU2	0.963	0.0000	OK	B
SB11	Span 14	Start	UDCONU2	0.963	0.0000	OK	B
SB11	Span 14	Middle	UDCONU2	4.403	0.0000	OK	B
SB11	Span 14	End	UDCONU2	7.799	0.0000	OK	B
SB11	Span 15	Start	UDCONU2	7.799	0.0000	OK	B
SB11	Span 15	Middle		0.000	0.0000	OK	B
SB11	Span 15	End	UDCONU2	10.648	0.0000	OK	B
SB11	Span 16	Start	UDCONU2	10.648	0.0000	OK	B
SB11	Span 16	Middle		0.000	0.0000	OK	B
SB11	Span 16	End	UDCONU2	0.204	0.0000	OK	B
SB12	Span 1	Start		0.000	0.0000	OK	B
SB12	Span 1	Middle		0.000	0.0000	OK	B
SB12	Span 1	End		0.000	0.0000	OK	B
SB12	Span 2	Start		0.000	0.0000	OK	B
SB12	Span 2	Middle	UDCONU2	0.028	0.0000	OK	B
SB12	Span 2	End	UDCONU1	1.312	0.0000	OK	B
SB12	Span 3	Start	UDCONU2	2.451	0.0000	OK	B
SB12	Span 3	Middle	UDCONU2	4.896	0.0000	OK	B
SB12	Span 3	End	UDCONU2	7.260	0.0000	OK	B
SB12	Span 4	Start	UDCONU2	7.260	0.0000	OK	B
SB12	Span 4	Middle		0.000	0.0000	OK	B
SB12	Span 4	End	UDCONU2	14.956	0.0000	OK	B
SB12	Span 5	Start	UDCONU2	14.956	0.0000	OK	B
SB12	Span 5	Middle		0.000	0.0000	OK	B
SB12	Span 5	End	UDCONU2	9.926	0.0000	OK	B
SB12	Span 6	Start	UDCONU2	9.926	0.0000	OK	B
SB12	Span 6	Middle	UDCONU2	13.169	0.0000	OK	B
SB12	Span 6	End	UDCONU2	10.919	0.0000	OK	B
SB12	Span 7	Start	UDCONU2	10.919	0.0000	OK	B
SB12	Span 7	Middle	UDCONU2	10.274	0.0000	OK	B
SB12	Span 7	End	UDCONU2	2.388	0.0000	OK	B
SB12	Span 8	Start	UDCONU2	2.388	0.0000	OK	B
SB12	Span 8	Middle	UDCONU2	0.877	0.0000	OK	B
SB12	Span 8	End	UDCONU2	2.463	0.0000	OK	B
SB12	Span 9	Start	UDCONU2	2.463	0.0000	OK	B
SB12	Span 9	Middle	UDCONU2	4.647	0.0000	OK	B
SB12	Span 9	End	UDCONU2	7.105	0.0000	OK	B
SB12	Span 10	Start	UDCONU2	7.105	0.0000	OK	B
SB12	Span 10	Middle	UDCONU2	12.107	0.0000	OK	B

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB12	Span 10	End	UDCONU2	85.962	1.1808	OK	B
SB12	Span 11	Start	UDCONU2	85.962	1.1808	OK	B
SB12	Span 11	Middle		0.000	0.0000	OK	B
SB12	Span 11	End	UDCONU2	48.549	0.0000	OK	B
SB12	Span 12	Start	UDCONU2	48.549	0.0000	OK	B
SB12	Span 12	Middle	UDCONU2	16.467	0.0000	OK	B
SB12	Span 12	End	UDCONU2	11.139	0.0000	OK	B
SB12	Span 13	Start	UDCONU2	11.139	0.0000	OK	B
SB12	Span 13	Middle	UDCONU2	6.874	0.0000	OK	B
SB12	Span 13	End	UDCONU2	0.965	0.0000	OK	B
SB12	Span 14	Start	UDCONU2	0.965	0.0000	OK	B
SB12	Span 14	Middle	UDCONU2	4.846	0.0000	OK	B
SB12	Span 14	End	UDCONU2	8.514	0.0000	OK	B
SB12	Span 15	Start	UDCONU2	8.514	0.0000	OK	B
SB12	Span 15	Middle		0.000	0.0000	OK	B
SB12	Span 15	End	UDCONU2	11.778	0.0000	OK	B
SB12	Span 16	Start	UDCONU2	11.778	0.0000	OK	B
SB12	Span 16	Middle		0.000	0.0000	OK	B
SB12	Span 16	End	UDCONU2	0.189	0.0000	OK	B
SB13	Span 1	Start		0.000	0.0000	OK	B
SB13	Span 1	Middle		0.000	0.0000	OK	B
SB13	Span 1	End		0.000	0.0000	OK	B
SB13	Span 2	Start		0.000	0.0000	OK	B
SB13	Span 2	Middle		0.000	0.0000	OK	B
SB13	Span 2	End		0.000	0.0000	OK	B
SB13	Span 3	Start		0.000	0.0000	OK	B
SB13	Span 3	Middle		0.000	0.0000	OK	B
SB13	Span 3	End	UDCONU2	3.083	0.0000	OK	B
SB13	Span 4	Start	UDCONU2	3.083	0.0000	OK	B
SB13	Span 4	Middle	UDCONU2	4.450	0.0000	OK	B
SB13	Span 4	End	UDCONU2	4.707	0.0000	OK	B
SB13	Span 5	Start	UDCONU2	4.707	0.0000	OK	B
SB13	Span 5	Middle		0.000	0.0000	OK	B
SB13	Span 5	End	UDCONU2	2.594	0.0000	OK	B
SB13	Span 6	Start	UDCONU2	2.594	0.0000	OK	B
SB13	Span 6	Middle	UDCONU2	2.628	0.0000	OK	B
SB13	Span 6	End	UDCONU1	0.799	0.0000	OK	B
SB13	Span 7	Start	UDCONU1	0.799	0.0000	OK	B
SB13	Span 7	Middle	UDCONU2	3.434	0.0000	OK	B
SB13	Span 7	End	UDCONU2	6.035	0.0000	OK	B
SB13	Span 8	Start	UDCONU2	6.035	0.0000	OK	B
SB13	Span 8	Middle	UDCONU2	8.589	0.0000	OK	B
SB13	Span 8	End	UDCONU2	8.589	0.0000	OK	B
SB13	Span 9	Start	UDCONU2	8.589	0.0000	OK	B
SB13	Span 9	Middle	UDCONU2	13.000	0.0000	OK	B
SB13	Span 9	End	UDCONU2	9.422	0.0000	OK	B
SB13	Span 10	Start	UDCONU2	9.422	0.0000	OK	B
SB13	Span 10	Middle	UDCONU2	5.899	0.0000	OK	B
SB13	Span 10	End	UDCONU2	1.635	0.0000	OK	B
SB13	Span 11	Start	UDCONU2	1.635	0.0000	OK	B
SB13	Span 11	Middle	UDCONU2	5.533	0.0000	OK	B
SB13	Span 11	End	UDCONU2	10.002	0.0000	OK	B
SB13	Span 12	Start	UDCONU2	10.002	0.0000	OK	B
SB13	Span 12	Middle		0.000	0.0000	OK	B

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB13	Span 12	End	UDCONU2	15.703	0.0000	OK	B
SB13	Span 13	Start	UDCONU2	15.703	0.0000	OK	B
SB13	Span 13	Middle		0.000	0.0000	OK	B
SB13	Span 13	End	UDCONU2	0.378	0.0000	OK	B
SB14	Span 1	Start		0.000	0.0000	OK	B
SB14	Span 1	Middle		0.000	0.0000	OK	B
SB14	Span 1	End		0.000	0.0000	OK	B
SB14	Span 2	Start		0.000	0.0000	OK	B
SB14	Span 2	Middle		0.000	0.0000	OK	B
SB14	Span 2	End		0.000	0.0000	OK	B
SB14	Span 3	Start		0.000	0.0000	OK	B
SB14	Span 3	Middle		0.000	0.0000	OK	B
SB14	Span 3	End		0.000	0.0000	OK	B
SB14	Span 4	Start		0.000	0.0000	OK	B
SB14	Span 4	Middle		0.000	0.0000	OK	B
SB14	Span 4	End	UDCONU2	0.801	0.0000	OK	B
SB14	Span 5	Start	UDCONU2	0.801	0.0000	OK	B
SB14	Span 5	Middle	UDCONU1	1.278	0.0000	OK	B
SB14	Span 5	End	UDCONU2	1.961	0.0000	OK	B
SB14	Span 6	Start	UDCONU2	1.961	0.0000	OK	B
SB14	Span 6	Middle		0.000	0.0000	OK	B
SB14	Span 6	End	UDCONU2	1.567	0.0000	OK	B
SB14	Span 7	Start	UDCONU2	1.567	0.0000	OK	B
SB14	Span 7	Middle	UDCONU2	2.914	0.0000	OK	B
SB14	Span 7	End	UDCONU2	4.993	0.0000	OK	B
SB14	Span 8	Start	UDCONU2	4.993	0.0000	OK	B
SB14	Span 8	Middle	UDCONU2	11.462	0.0000	OK	B
SB14	Span 8	End	UDCONU2	11.462	0.0000	OK	B
SB14	Span 9	Start	UDCONU2	11.462	0.0000	OK	B
SB14	Span 9	Middle	UDCONU2	12.145	0.0000	OK	B
SB14	Span 9	End	UDCONU2	8.910	0.0000	OK	B
SB14	Span 10	Start	UDCONU2	8.910	0.0000	OK	B
SB14	Span 10	Middle	UDCONU2	5.764	0.0000	OK	B
SB14	Span 10	End	UDCONU2	1.313	0.0000	OK	B
SB14	Span 11	Start	UDCONU2	1.313	0.0000	OK	B
SB14	Span 11	Middle	UDCONU2	5.139	0.0000	OK	B
SB14	Span 11	End	UDCONU2	9.248	0.0000	OK	B
SB14	Span 12	Start	UDCONU2	9.248	0.0000	OK	B
SB14	Span 12	Middle		0.000	0.0000	OK	B
SB14	Span 12	End	UDCONU2	13.677	0.0000	OK	B
SB14	Span 13	Start	UDCONU2	13.677	0.0000	OK	B
SB14	Span 13	Middle		0.000	0.0000	OK	B
SB14	Span 13	End	UDCONU2	0.328	0.0000	OK	B
SB15	Span 1	Start		0.000	0.0000	OK	B
SB15	Span 1	Middle		0.000	0.0000	OK	B
SB15	Span 1	End		0.000	0.0000	OK	B
SB15	Span 2	Start		0.000	0.0000	OK	B
SB15	Span 2	Middle		0.000	0.0000	OK	B
SB15	Span 2	End		0.000	0.0000	OK	B
SB15	Span 3	Start		0.000	0.0000	OK	B
SB15	Span 3	Middle		0.000	0.0000	OK	B
SB15	Span 3	End		0.000	0.0000	OK	B
SB15	Span 4	Start		0.000	0.0000	OK	B
SB15	Span 4	Middle		0.000	0.0000	OK	B

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB15	Span 4	End		0.000	0.0000	OK	B
SB15	Span 5	Start		0.000	0.0000	OK	B
SB15	Span 5	Middle		0.000	0.0000	OK	B
SB15	Span 5	End	UDCONU2	0.156	0.0000	OK	B
SB15	Span 6	Start	UDCONU2	0.156	0.0000	OK	B
SB15	Span 6	Middle	UDCONU2	2.081	0.0000	OK	B
SB15	Span 6	End	UDCONU2	5.285	0.0000	OK	B
SB15	Span 7	Start	UDCONU2	5.285	0.0000	OK	B
SB15	Span 7	Middle		0.000	0.0000	OK	B
SB15	Span 7	End	UDCONU2	4.142	0.0000	OK	B
SB15	Span 8	Start	UDCONU2	4.142	0.0000	OK	B
SB15	Span 8	Middle		0.000	0.0000	OK	B
SB15	Span 8	End	UDCONU2	8.764	0.0000	OK	B
SB15	Span 9	Start	UDCONU2	8.764	0.0000	OK	B
SB15	Span 9	Middle	UDCONU2	15.783	0.0000	OK	B
SB15	Span 9	End	UDCONU2	10.495	0.0000	OK	B
SB15	Span 10	Start	UDCONU2	10.495	0.0000	OK	B
SB15	Span 10	Middle	UDCONU2	6.817	0.0000	OK	B
SB15	Span 10	End	UDCONU2	0.641	0.0000	OK	B
SB15	Span 11	Start	UDCONU2	0.641	0.0000	OK	B
SB15	Span 11	Middle	UDCONU2	4.117	0.0000	OK	B
SB15	Span 11	End	UDCONU2	7.460	0.0000	OK	B
SB15	Span 12	Start	UDCONU2	7.460	0.0000	OK	B
SB15	Span 12	Middle		0.000	0.0000	OK	B
SB15	Span 12	End	UDCONU2	10.130	0.0000	OK	B
SB15	Span 13	Start	UDCONU2	10.130	0.0000	OK	B
SB15	Span 13	Middle		0.000	0.0000	OK	B
SB15	Span 13	End	UDCONU2	0.023	0.0000	OK	B
SB16	Span 1	Start		0.000	0.0000	OK	B
SB16	Span 1	Middle		0.000	0.0000	OK	B
SB16	Span 1	End		0.000	0.0000	OK	B
SB16	Span 2	Start		0.000	0.0000	OK	B
SB16	Span 2	Middle		0.000	0.0000	OK	B
SB16	Span 2	End		0.000	0.0000	OK	B
SB16	Span 3	Start		0.000	0.0000	OK	B
SB16	Span 3	Middle		0.000	0.0000	OK	B
SB16	Span 3	End		0.000	0.0000	OK	B
SB16	Span 4	Start		0.000	0.0000	OK	B
SB16	Span 4	Middle		0.000	0.0000	OK	B
SB16	Span 4	End		0.000	0.0000	OK	B
SB16	Span 5	Start		0.000	0.0000	OK	B
SB16	Span 5	Middle		0.000	0.0000	OK	B
SB16	Span 5	End		0.000	0.0000	OK	B
SB16	Span 6	Start		0.000	0.0000	OK	B
SB16	Span 6	Middle		0.000	0.0000	OK	B
SB16	Span 6	End	UDCONU2	0.256	0.0000	OK	B
SB16	Span 7	Start	UDCONU2	0.256	0.0000	OK	B
SB16	Span 7	Middle	UDCONU2	2.478	0.0000	OK	B
SB16	Span 7	End	UDCONU2	6.889	0.0000	OK	B
SB16	Span 8	Start	UDCONU2	6.889	0.0000	OK	B
SB16	Span 8	Middle	UDCONU2	13.542	0.0000	OK	B
SB16	Span 8	End	UDCONU2	9.145	0.0000	OK	B
SB16	Span 9	Start	UDCONU2	9.145	0.0000	OK	B
SB16	Span 9	Middle	UDCONU2	5.483	0.0000	OK	B

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

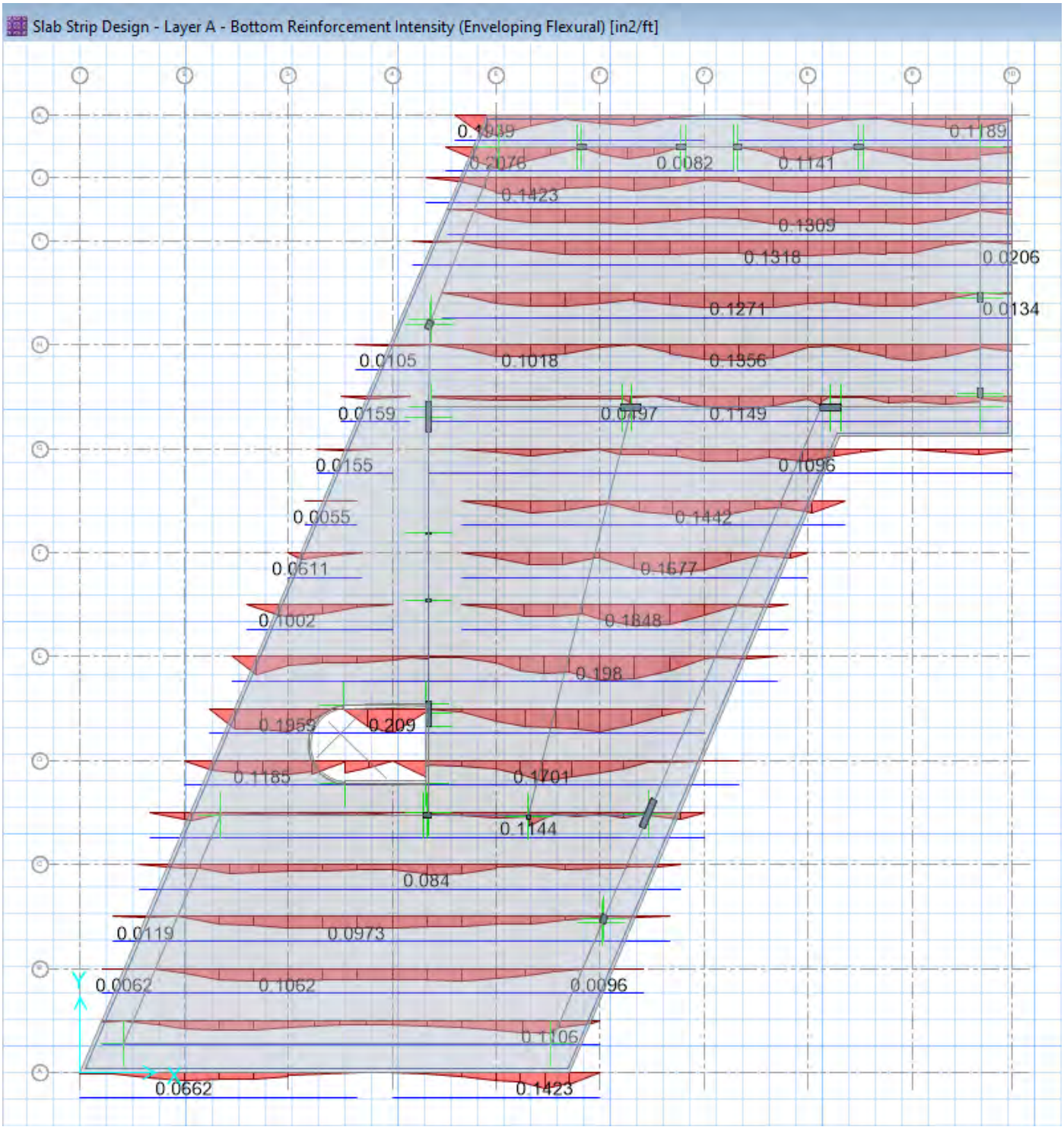
Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB16	Span 9	End	UDCONU2	0.746	0.0000	OK	B
SB16	Span 10	Start	UDCONU2	0.746	0.0000	OK	B
SB16	Span 10	Middle	UDCONU2	4.186	0.0000	OK	B
SB16	Span 10	End	UDCONU2	8.170	0.0000	OK	B
SB16	Span 11	Start	UDCONU2	8.170	0.0000	OK	B
SB16	Span 11	Middle		0.000	0.0000	OK	B
SB16	Span 11	End	UDCONU2	13.137	0.0000	OK	B
SB16	Span 12	Start	UDCONU2	13.137	0.0000	OK	B
SB16	Span 12	Middle		0.000	0.0000	OK	B
SB16	Span 12	End	UDCONU2	0.501	0.0000	OK	B
SB17	Span 1	Start		0.000	0.0000	OK	B
SB17	Span 1	Middle		0.000	0.0000	OK	B
SB17	Span 1	End		0.000	0.0000	OK	B
SB17	Span 2	Start		0.000	0.0000	OK	B
SB17	Span 2	Middle		0.000	0.0000	OK	B
SB17	Span 2	End		0.000	0.0000	OK	B
SB17	Span 3	Start		0.000	0.0000	OK	B
SB17	Span 3	Middle		0.000	0.0000	OK	B
SB17	Span 3	End		0.000	0.0000	OK	B
SB17	Span 4	Start		0.000	0.0000	OK	B
SB17	Span 4	Middle		0.000	0.0000	OK	B
SB17	Span 4	End		0.000	0.0000	OK	B
SB17	Span 5	Start		0.000	0.0000	OK	B
SB17	Span 5	Middle		0.000	0.0000	OK	B
SB17	Span 5	End		0.000	0.0000	OK	B
SB17	Span 6	Start		0.000	0.0000	OK	B
SB17	Span 6	Middle		0.000	0.0000	OK	B
SB17	Span 6	End		0.000	0.0000	OK	B
SB17	Span 7	Start		0.000	0.0000	OK	B
SB17	Span 7	Middle	UDCONU2	2.877	0.0000	OK	B
SB17	Span 7	End	UDCONU2	4.119	0.0000	OK	B
SB17	Span 8	Start	UDCONU2	4.119	0.0000	OK	B
SB17	Span 8	Middle	UDCONU2	2.372	0.0000	OK	B
SB17	Span 8	End	UDCONU2	0.308	0.0000	OK	B
SB17	Span 9	Start	UDCONU2	0.308	0.0000	OK	B
SB17	Span 9	Middle	UDCONU2	2.263	0.0000	OK	B
SB17	Span 9	End	UDCONU2	4.522	0.0000	OK	B
SB17	Span 10	Start	UDCONU2	4.522	0.0000	OK	B
SB17	Span 10	Middle	UDCONU2	7.608	0.0000	OK	B
SB17	Span 10	End	UDCONU2	0.208	0.0000	OK	B
SB18	Span 1	Start		0.000	0.0000	OK	B
SB18	Span 1	Middle		0.000	0.0000	OK	B
SB18	Span 1	End		0.000	0.0000	OK	B
SB18	Span 2	Start		0.000	0.0000	OK	B
SB18	Span 2	Middle		0.000	0.0000	OK	B
SB18	Span 2	End		0.000	0.0000	OK	B
SB18	Span 3	Start		0.000	0.0000	OK	B
SB18	Span 3	Middle		0.000	0.0000	OK	B
SB18	Span 3	End		0.000	0.0000	OK	B
SB18	Span 4	Start		0.000	0.0000	OK	B
SB18	Span 4	Middle		0.000	0.0000	OK	B
SB18	Span 4	End		0.000	0.0000	OK	B
SB18	Span 5	Start		0.000	0.0000	OK	B
SB18	Span 5	Middle		0.000	0.0000	OK	B

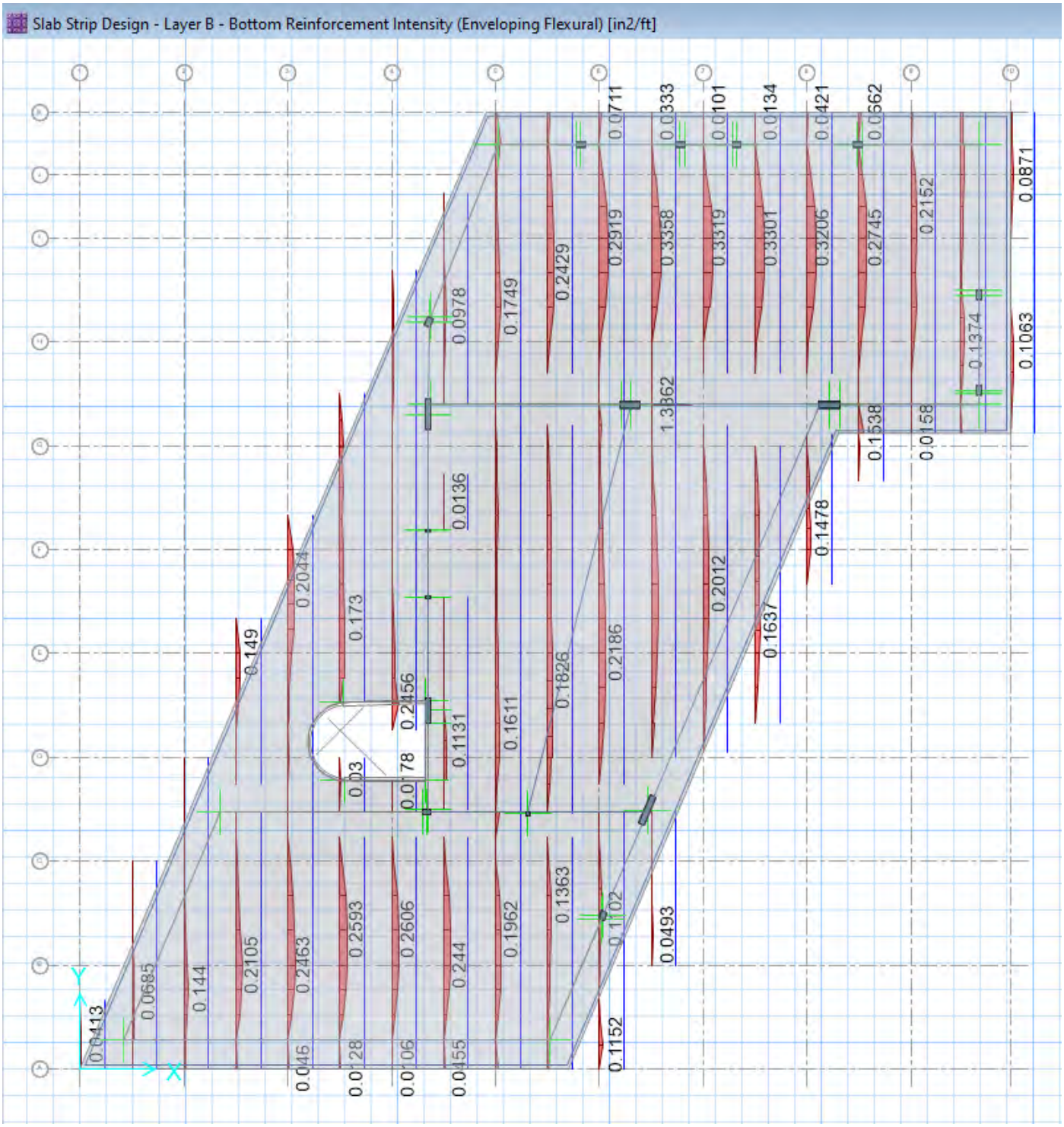
Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

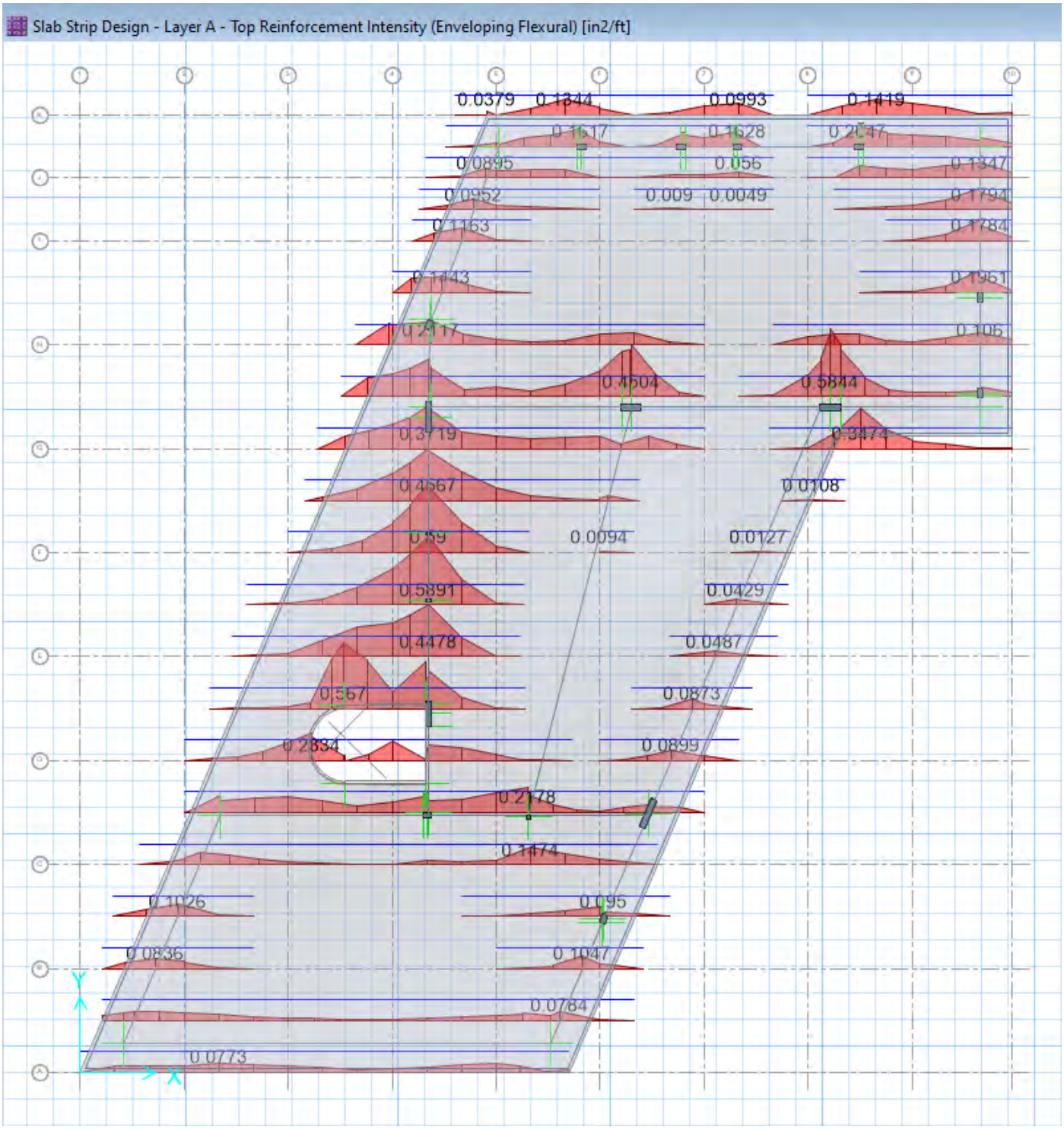
Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB18	Span 5	End		0.000	0.0000	OK	B
SB18	Span 6	Start		0.000	0.0000	OK	B
SB18	Span 6	Middle		0.000	0.0000	OK	B
SB18	Span 6	End		0.000	0.0000	OK	B
SB18	Span 7	Start		0.000	0.0000	OK	B
SB18	Span 7	Middle		0.000	0.0000	OK	B
SB18	Span 7	End	UDCONU2	2.944	0.0000	OK	B
SB18	Span 8	Start	UDCONU2	2.944	0.0000	OK	B
SB18	Span 8	Middle		0.000	0.0000	OK	B
SB18	Span 8	End	UDCONU1	0.901	0.0000	OK	B
SB18	Span 9	Start	UDCONU1	0.901	0.0000	OK	B
SB18	Span 9	Middle		0.000	0.0000	OK	B
SB18	Span 9	End	UDCONU1	0.520	0.0000	OK	B
SB18	Span 10	Start	UDCONU1	0.520	0.0000	OK	B
SB18	Span 10	Middle	UDCONU2	3.196	0.0000	OK	B
SB18	Span 10	End	UDCONU2	0.716	0.0000	OK	B
SB18	Span 11	Start	UDCONU2	0.716	0.0000	OK	B
SB18	Span 11	Middle	UDCONU2	0.716	0.0000	OK	B
SB18	Span 11	End	UDCONU2	4.376	0.0000	OK	B
SB18	Span 12	Start	UDCONU2	4.376	0.0000	OK	B
SB18	Span 12	Middle		0.000	0.0000	OK	B
SB18	Span 12	End	UDCONU2	0.729	0.0000	OK	B
SB18	Span 13	Start	UDCONU2	0.729	0.0000	OK	B
SB18	Span 13	Middle	UDCONU2	1.680	0.0000	OK	B
SB18	Span 13	End	UDCONU2	0.688	0.0000	OK	B
SB18	Span 14	Start	UDCONU2	0.688	0.0000	OK	B
SB18	Span 14	Middle	UDCONU2	0.658	0.0000	OK	B
SB18	Span 14	End	UDCONU2	0.480	0.0000	OK	B
SB18	Span 15	Start	UDCONU2	0.480	0.0000	OK	B
SB18	Span 15	Middle		0.000	0.0000	OK	B
SB18	Span 15	End	UDCONU2	0.202	0.0000	OK	B
SB18	Span 16	Start	UDCONU2	0.202	0.0000	OK	B
SB18	Span 16	Middle		0.000	0.0000	OK	B
SB18	Span 16	End	UDCONU2	0.208	0.0000	OK	B
SB19	Span 1	Start		0.000	0.0000	OK	B
SB19	Span 1	Middle		0.000	0.0000	OK	B
SB19	Span 1	End		0.000	0.0000	OK	B
SB19	Span 2	Start		0.000	0.0000	OK	B
SB19	Span 2	Middle		0.000	0.0000	OK	B
SB19	Span 2	End		0.000	0.0000	OK	B
SB19	Span 3	Start		0.000	0.0000	OK	B
SB19	Span 3	Middle		0.000	0.0000	OK	B
SB19	Span 3	End		0.000	0.0000	OK	B
SB19	Span 4	Start		0.000	0.0000	OK	B
SB19	Span 4	Middle		0.000	0.0000	OK	B
SB19	Span 4	End		0.000	0.0000	OK	B
SB19	Span 5	Start		0.000	0.0000	OK	B
SB19	Span 5	Middle		0.000	0.0000	OK	B
SB19	Span 5	End		0.000	0.0000	OK	B
SB19	Span 6	Start		0.000	0.0000	OK	B
SB19	Span 6	Middle		0.000	0.0000	OK	B
SB19	Span 6	End		0.000	0.0000	OK	B
SB19	Span 7	Start		0.000	0.0000	OK	B
SB19	Span 7	Middle	UDCONU2	1.709	0.0000	OK	B

Table 40: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

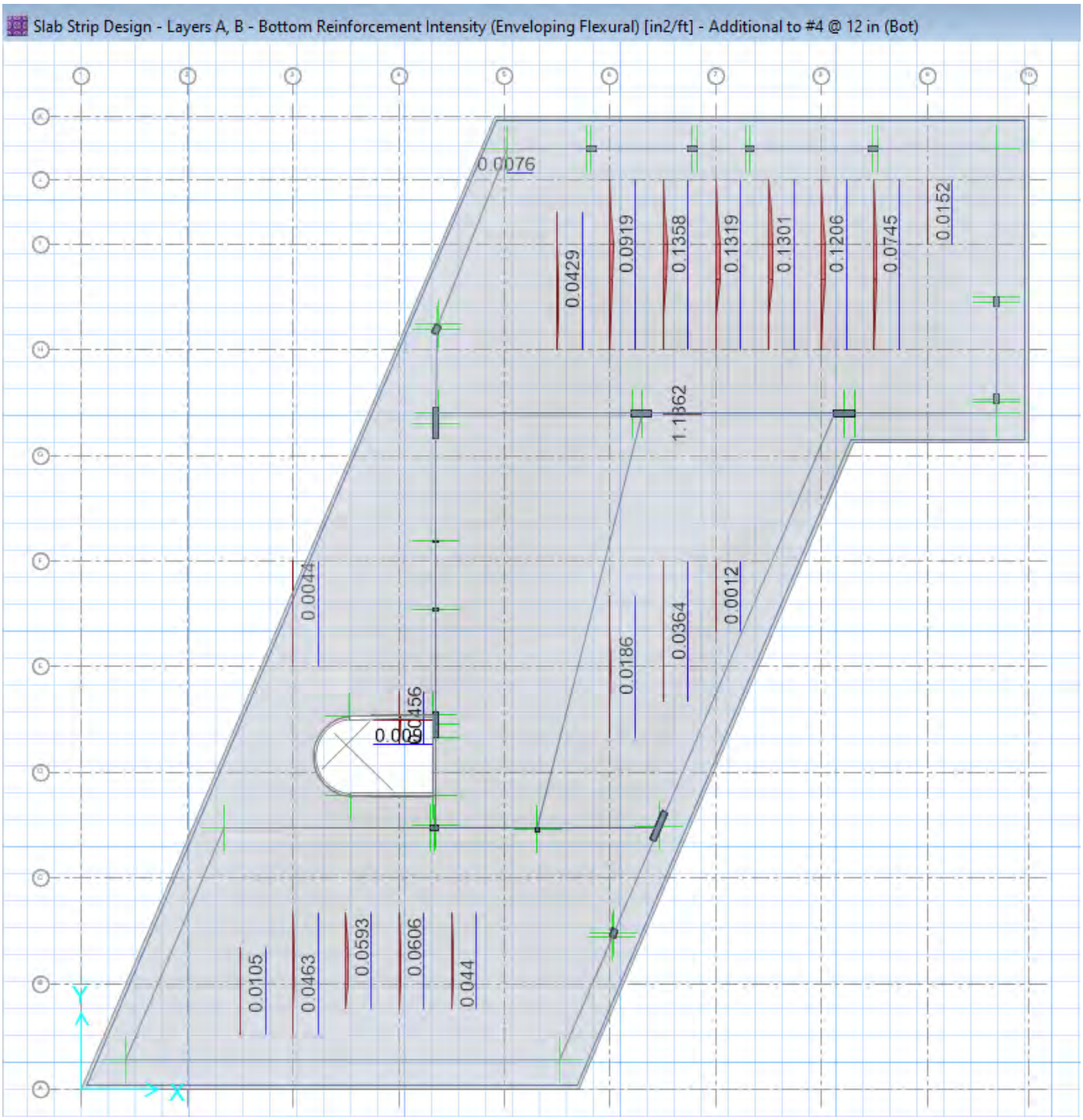
Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB19	Span 7	End	UDCONU2	0.979	0.0000	OK	B
SB19	Span 8	Start	UDCONU2	0.979	0.0000	OK	B
SB19	Span 8	Middle	UDCONU2	3.805	0.0000	OK	B
SB19	Span 8	End	UDCONU2	0.579	0.0000	OK	B
SB19	Span 9	Start	UDCONU2	0.579	0.0000	OK	B
SB19	Span 9	Middle	UDCONU2	1.029	0.0000	OK	B
SB19	Span 9	End	UDCONU2	1.526	0.0000	OK	B
SB19	Span 10	Start	UDCONU2	1.526	0.0000	OK	B
SB19	Span 10	Middle	UDCONU1	1.317	0.0000	OK	B
SB19	Span 10	End	UDCONU2	0.025	0.0000	OK	B

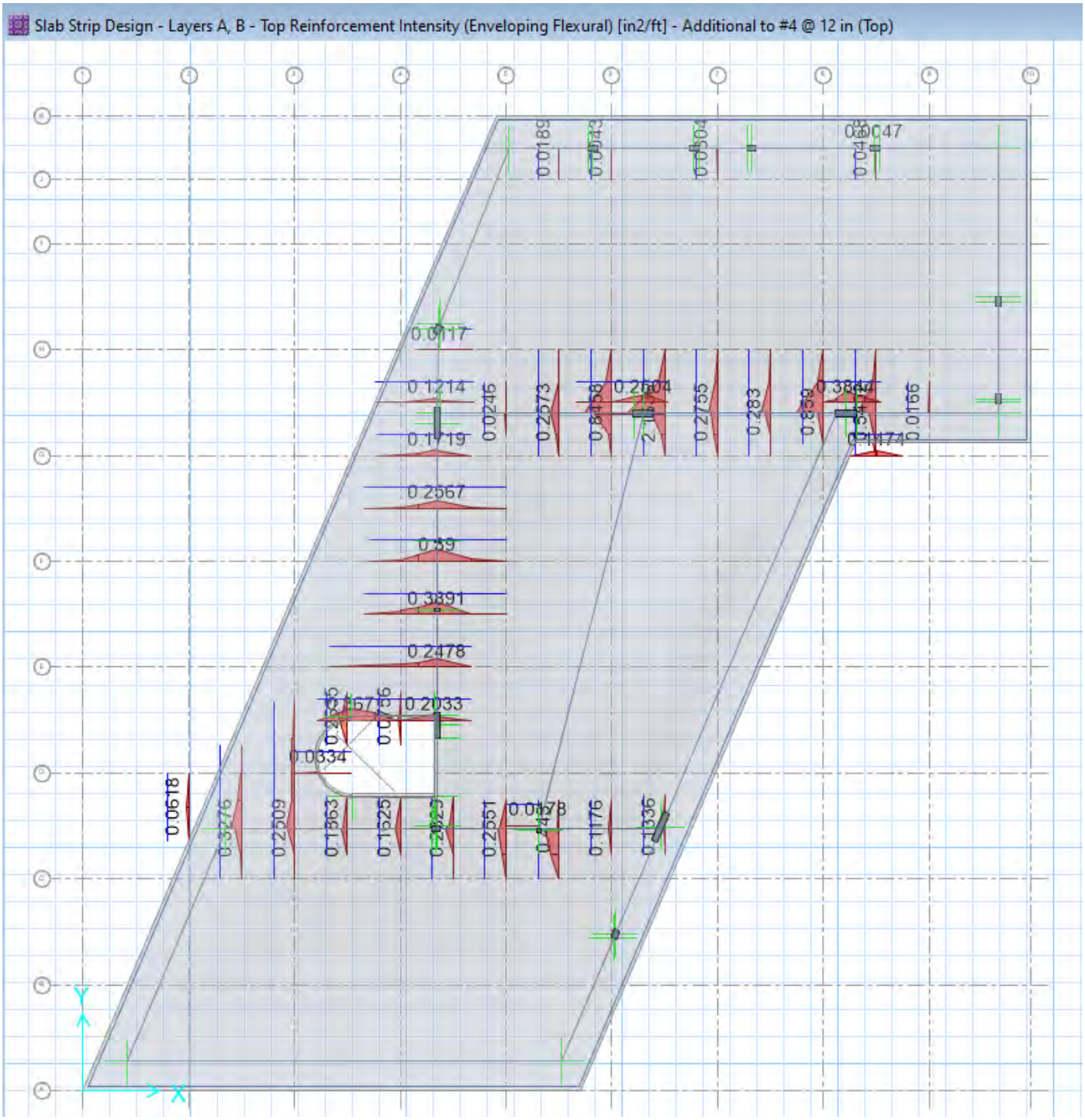

Slab Strip Design - Layer A - Bottom Reinforcement Intensity (Enveloping Flexural) [in2/ft]
0.1939
0.1189
0.2076
0.0082
0.1141
0.1423
0.1309
0.1318
0.0206
0.1271
0.0134
0.0105
0.1018
0.1356
0.0159
0.0797
0.1149
0.0155
0.1096
0.0055
0.1442
0.0611
0.1677
0.1002
0.1848
0.198
0.1959
0.209
0.1185
0.1701
0.1144
0.084
0.0119
0.0973
0.0062
0.1062
0.0096
0.1106
0.0662
0.1423


Slab Strip Design - Layer B - Bottom Reinforcement Intensity (Enveloping Flexural) [in2/ft]
0.0711
0.0333
0.0101
0.0134
0.0421
0.0662
0.0871
0.2152
0.2745
0.3206
0.3301
0.3319
0.3358
0.2919
0.2429
0.1749
0.0978
1.3862
0.1374
0.1063
0.1538
0.0158
0.1478
0.0136
0.2044
0.173
0.149
0.2012
0.1637
0.2186
0.1826
0.1611
0.2456
0.1131
0.03
0.0078
0.0413
0.0685
0.144
0.2105
0.2463
0.2593
0.2606
0.244
0.1962
0.1363
0.1102
0.0493
0.1152
0.046
0.0455


Slab Strip Design - Layer A - Top Reinforcement Intensity (Enveloping Flexural) [in2/ft]
0.0379
0.1344
0.0993
0.1419
0.1617
0.1628
0.2147
0.0895
0.056
0.1347
0.0952
0.009
0.0049
0.1794
0.1163
0.1784
0.1443
0.1961
0.2117
0.105
0.4504
0.5844
0.3719
0.3474
0.4567
0.0108
0.159
0.0094
0.0127
0.5891
0.0429
0.4478
0.0487
0.367
0.0873
0.2834
0.0899
0.2178
0.1474
0.1026
0.095
0.0836
0.1047
0.0784
0.0773
Y
X


Slab Strip Design - Layer B - Top Reinforcement Intensity (Enveloping Flexural) [in2/ft]


Slab Strip Design - Layers A, B - Bottom Reinforcement Intensity (Enveloping Flexural) [in2/ft] - Additional to #4 @ 12 in (Bot)
0.0076
0.0429
0.0919
0.1358
0.1319
0.1301
0.1206
0.0745
0.0152
1.1862
0.0044
0.0186
0.0364
0.0012
0.0456
0.009
0.0105
0.0463
0.0593
0.0606
0.044
Y
X


Slab Strip Design - Layers A, B - Top Reinforcement Intensity (Enveloping Flexural) [in2/ft] - Additional to #4 @ 12 in (Top)
0.0189
0.0504
0.0047
0.0117
0.1214
0.0246
0.2573
0.2755
0.283
0.0155
0.2567
0.3891
0.2478
0.2033
0.0334
0.0618
0.3276
0.2509
0.1363
0.1525
0.2551
0.1176
0.1336
Y
X

6.4. Punching check/design

Table 42: Concrete Slab Design 02 - Punching Shear Data, Part 1 of 3

Table 42: Concrete Slab Design 02 - Punching Shear Data, Part 1 of 3

Point	GlobalX	GlobalY	Location	Perimeter	Depth	Status	Ratio
	ft	ft		in	in		
4	57.8596	106.8464				Not Calculated	
6	69.3557	106.8464				Not Calculated	
8	75.8596	106.8464				Not Calculated	
10	89.8557	106.8464				Not Calculated	
12	103.8592	78.4317				Not Calculated	
14	103.8592	89.4279				Not Calculated	
20	86.5981	76.7633				Not Calculated	
28	63.5981	76.7633				Not Calculated	
32	40.1910	75.6515				Not Calculated	
34	40.1918	41.4310				Not Calculated	
36	40.0262	29.6798				Not Calculated	
42	60.3873	17.7265				Not Calculated	
46	65.5475	29.8831				Not Calculated	
48	40.3085	86.3523				Not Calculated	
109	51.7560	29.5131				Not Calculated	
111	40.1941	54.4730				Not Calculated	
113	40.1910	62.2224				Not Calculated	

Table 42: Concrete Slab Design 02 - Punching Shear Data, Part 2 of 3

Table 42: Concrete Slab Design 02 - Punching Shear Data, Part 2 of 3

Point	Combo	Vu	Gamma_v2	Gamma_v3	ShrStrMax	Mu2	ShrStrCap
		kip			kip/in2	kip-ft	kip/in2
4							
6							
8							
10							
12							
14							
20							
28							
32							
34							

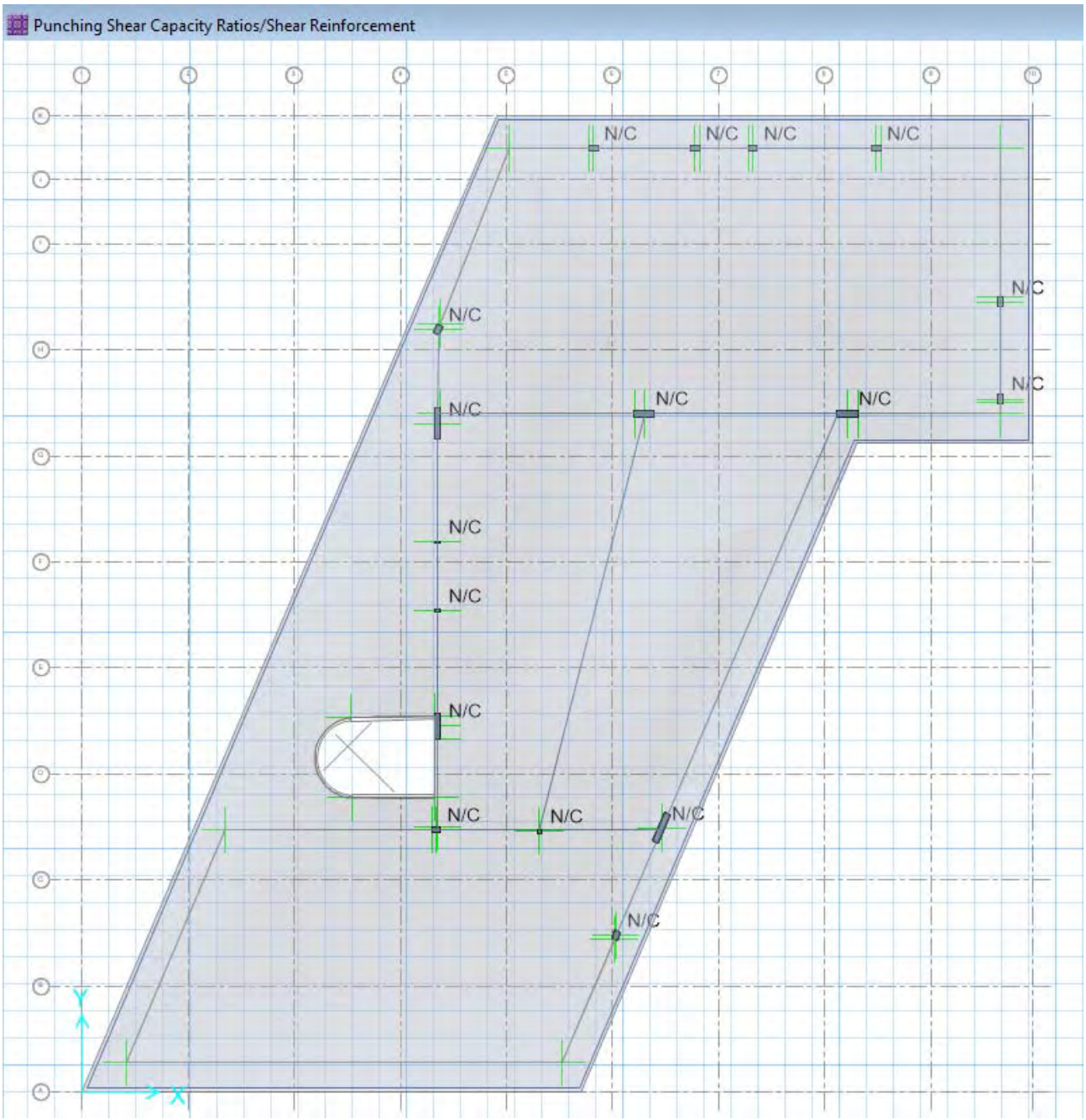
Table 42: Concrete Slab Design 02 - Punching Shear Data, Part 2 of 3

Point	Combo	Vu	Gamma_v2	Gamma_v3	ShrStrMax	Mu2	ShrStrCap
		kip			kip/in2	kip-ft	kip/in2
36							
42							
46							
48							
109							
111							
113							

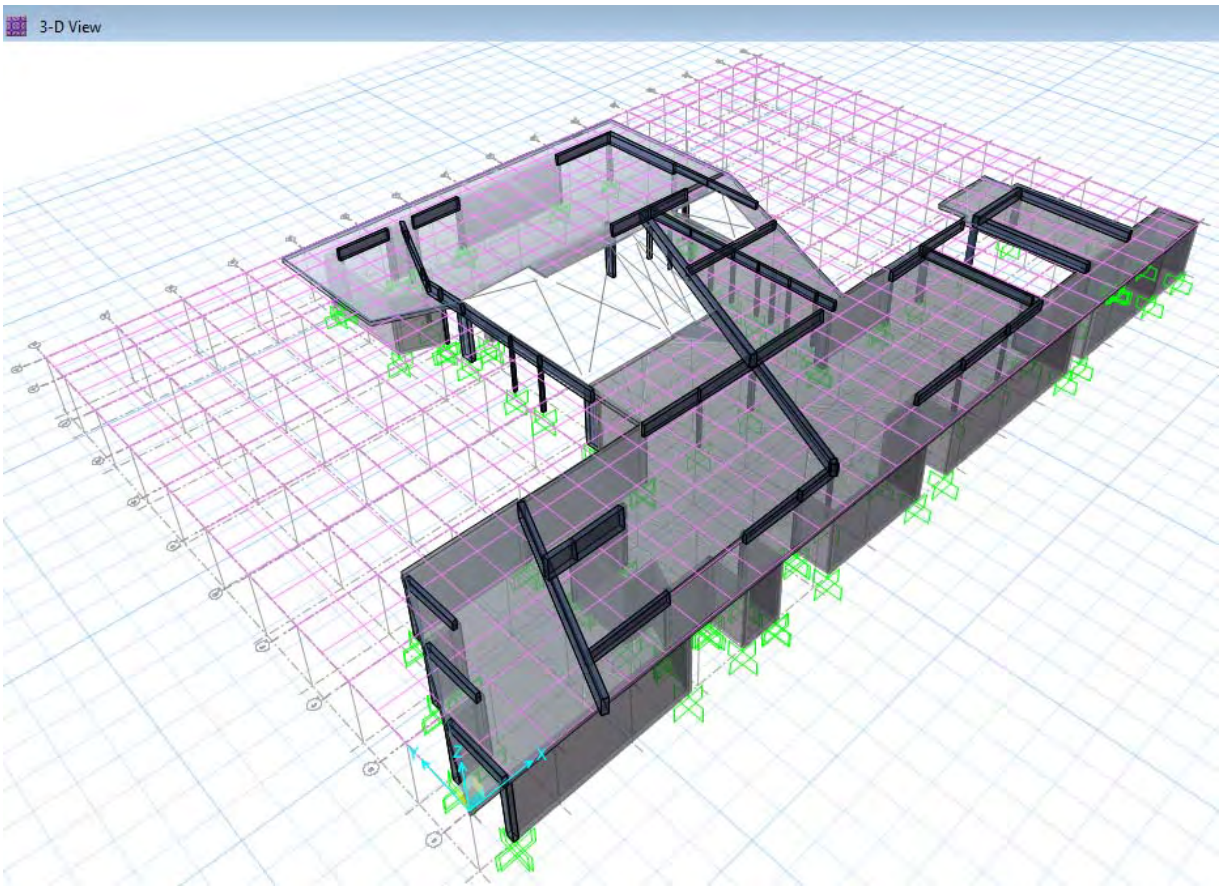
Table 42: Concrete Slab Design 02 - Punching Shear Data, Part 3 of 3

Table 42: Concrete Slab Design 02 - Punching Shear Data, Part 3 of 3

Point	Mu3	ReinfType	UnbalMu2	UnbalMu3	NumRails	StudPerRail
	kip-ft		kip-ft	kip-ft		
4		None				
6		None				
8		None				
10		None				
12		None				
14		None				
20		None				
28		None				
32		None				
34		None				
36		None				
42		None				
46		None				
48		None				
109		None				
111		None				
113		None				


Punching Shear Capacity Ratios/Shear Reinforcement
N/C
N/C
N/C
N/C
N/C
N/C
N/C
N/C
N/C
N/C
N/C
N/C
N/C
N/C
N/C
N/C
N/C
N/C
Y
X

SAFE®



SAFE Analysis & Design Report

Model Name: SECOND SLAB.fdb

14 October 2022

Contents

Model Definition 4
1. Model geometry 5
1.1. Connectivity 5
2. Model properties 23
2.1. Material properties 23
2.2. Section properties 24
2.3. Support properties 26
3. Model assignments 28
3.1. Slab assignments 28
3.2. Beam assignments 29
3.3. Column assignments 30
3.4. Wall assignments 31
3.5. Support assignments 32
4. Model loading 35
4.1. Load patterns 35
4.2. Load cases 37
4.3. Load combinations 37
Analysis Results 41
5. Analysis results 43
5.1. Support results 43
5.2. Structure results 79
Design 98
6. Design summary 99
6.1. Preferences 99
6.2. Overwrites 99
6.3. Slab design 105
6.4. Punching check/design 268

List of Figures

Figure 1: Finite element model 4
Figure 2: Deformed shape 42
Figure 3: Finite element model 98

List of Tables

Table 1: Concrete Slab Design Summary 02 - Span Definition Data 5
Table 2: Material Properties 03 - Concrete 23
Table 3: Material Properties 04 - Rebar 23
Table 4: Material Properties 05 - Tendon 23
Table 5: Material Properties 02 - Steel 23
Table 6: Material Properties 06 - Other 23
Table 7: Slab Properties 02 - Solid Slabs, Part 1 of 2 24
Table 7: Slab Properties 02 - Solid Slabs, Part 2 of 2 24
Table 8: Beam Properties 02 - Rectangular Beam 24
Table 9: Beam Properties 06 - Design Data 24
Table 10: Tendon Properties 25
Table 11: Column Properties 02 - Rectangular 25
Table 12: Column Properties 03 - Circular 26
Table 13: Wall Properties 26

Table 14: Soil Properties 26
Table 15: Spring Properties - Point 26
Table 16: Spring Properties - Line 27
Table 17: Slab Property Assignments 28
Table 18: Slab Edge Releases 28
Table 19: Beam Property Assignments 29
Table 20: Column Property Assignments 30
Table 21: Wall Property Assignments 31
Table 22: Point Restraint Assignments 32
Table 23: Load Patterns 35
Table 24: Load Assignments - Surface Loads 35
Table 25: Load Assignments - Line Objects - Distributed Loads, Part 1 of 2 35
Table 25: Load Assignments - Line Objects - Distributed Loads, Part 2 of 2 36
Table 26: Load Assignments - Line Objects - Point Loads 36
Table 27: Load Cases 02 - Static 37
Table 28: Load Cases 06 - Loads Applied 37
Table 29: Load Combinations 37
Table 30: Nodal Reactions 43
Table 31: Integrated Wall Forces 64
Table 32: Sum Of Reactions, Part 1 of 2 79
Table 32: Sum Of Reactions, Part 2 of 2 79
Table 33: Nodal Displacements - Summary, Part 1 of 2 80
Table 33: Nodal Displacements - Summary, Part 2 of 2 88
Table 34: Design Preferences 01 - Resistance Factors 99
Table 35: Design Preferences 02 - Rebar Cover - Slabs 99
Table 36: Design Preferences 03 - Rebar Cover - Beams 99
Table 37: Design Preferences 04 - Prestress Data 99
Table 38: Slab Design Overwrites 01 - Strip Based 99
Table 39: Slab Design Overwrites 02 - Finite Element Based, Part 1 of 2 101
Table 39: Slab Design Overwrites 02 - Finite Element Based, Part 2 of 2 101
Table 40: Beam Design Overwrites 101
Table 41: Punching Shear Design Overwrites 01 - General 103
Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3 105
Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3 158
Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3 212
Table 44: Concrete Slab Design 02 - Punching Shear Data, Part 1 of 3 268
Table 44: Concrete Slab Design 02 - Punching Shear Data, Part 2 of 3 270
Table 44: Concrete Slab Design 02 - Punching Shear Data, Part 3 of 3 271

Model Definition

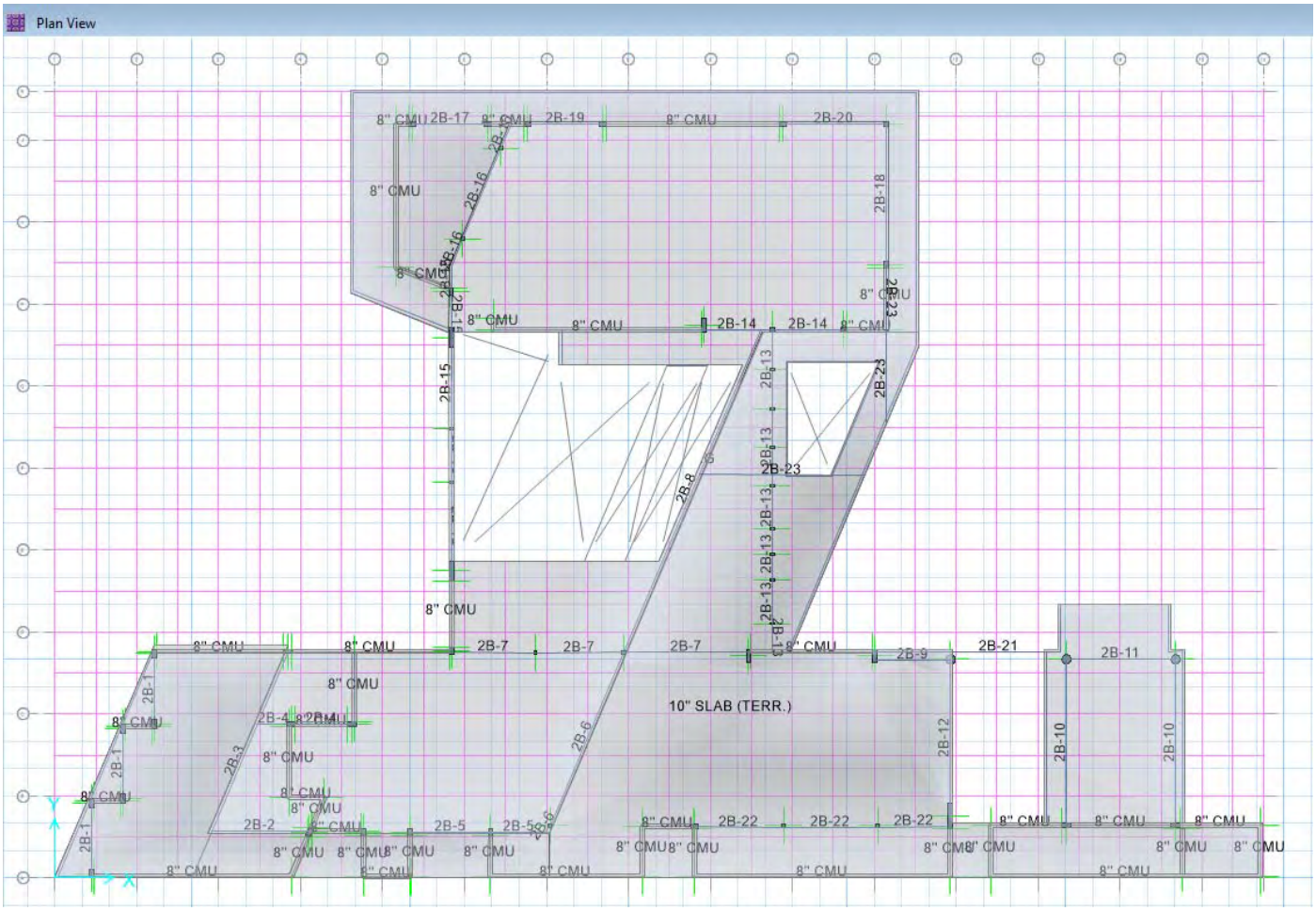


Figure 1: Finite element model

1. Model geometry

This section provides model geometry information, including items such as joint coordinates, joint restraints, and element connectivity.

1.1. Connectivity

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SA1	Span 1	5.3260	0.0000	0.0000	0.0000	5.3260	0.0000
SA1	Span 2	0.3333	5.3260	5.3260	0.0000	5.6594	0.0000
SA1	Span 3	6.3406	5.6594	5.6594	0.0000	12.0000	0.0000
SA1	Span 4	8.1421	12.0000	12.0000	0.0000	20.1421	0.0000
SA1	Span 5	3.8579	20.1421	20.1421	0.0000	24.0000	0.0000
SA1	Span 6	10.6084	24.0000	24.0000	0.0000	34.6084	0.0000
SA1	Span 7	1.3916	34.6084	34.6084	0.0000	36.0000	0.0000
SA1	Span 8	9.1594	36.0000	36.0000	0.0000	45.1594	0.0000
SA1	Span 9	2.8406	45.1594	45.1594	0.0000	48.0000	0.0000
SA1	Span 10	3.9437	48.0000	48.0000	0.0000	51.9437	0.0000
SA1	Span 11	0.2157	51.9437	51.9437	0.0000	52.1594	0.0000
SA1	Span 12	7.8406	52.1594	52.1594	0.0000	60.0000	0.0000
SA1	Span 13	3.7933	60.0000	60.0000	0.0000	63.7933	0.0000
SA1	Span 14	8.2067	63.7933	63.7933	0.0000	72.0000	0.0000
SA1	Span 15	12.0000	72.0000	72.0000	0.0000	84.0000	0.0000
SA1	Span 16	1.9847	84.0000	84.0000	0.0000	85.9847	0.0000
SA1	Span 17	7.6362	85.9847	85.9847	0.0000	93.6210	0.0000
SA1	Span 18	2.3790	93.6210	93.6210	0.0000	96.0000	0.0000
SA1	Span 19	12.0000	96.0000	96.0000	0.0000	108.0000	0.0000
SA1	Span 20	12.0000	108.0000	108.0000	0.0000	120.0000	0.0000
SA1	Span 21	11.1594	120.0000	120.0000	0.0000	131.1594	0.0000
SA1	Span 22	0.8406	131.1594	131.1594	0.0000	132.0000	0.0000
SA1	Span 23	4.9924	132.0000	132.0000	0.0000	136.9924	0.0000
SA1	Span 24	7.0076	136.9924	136.9924	0.0000	144.0000	0.0000
SA1	Span 25	12.0000	144.0000	144.0000	0.0000	156.0000	0.0000
SA1	Span 26	9.0761	156.0000	156.0000	0.0000	165.0761	0.0000
SA1	Span 27	2.9239	165.0761	165.0761	0.0000	168.0000	0.0000
SA1	Span 28	8.5730	168.0000	168.0000	0.0000	176.5730	0.0000
SA2	Cantilever Start	2.7792	0.0000	2.5468	6.0000	5.3260	6.0000
SA2	Span 1	6.6740	2.7792	5.3260	6.0000	12.0000	6.0000
SA2	Span 2	10.5498	9.4532	12.0000	6.0000	22.5498	6.0000
SA2	Span 3	0.3651	20.0030	22.5498	6.0000	22.9149	6.0000
SA2	Span 4	1.0851	20.3681	22.9149	6.0000	24.0000	6.0000
SA2	Span 5	12.0000	21.4532	24.0000	6.0000	36.0000	6.0000
SA2	Span 6	0.9858	33.4532	36.0000	6.0000	36.9858	6.0000
SA2	Span 7	0.2613	34.4390	36.9858	6.0000	37.2471	6.0000
SA2	Span 8	0.2593	34.7003	37.2471	6.0000	37.5064	6.0000
SA2	Span 9	0.1683	34.9596	37.5064	6.0000	37.6747	6.0000
SA2	Span 10	7.1513	35.1279	37.6747	6.0000	44.8260	6.0000
SA2	Span 11	0.3333	42.2792	44.8260	6.0000	45.1594	6.0000
SA2	Span 12	2.8406	42.6125	45.1594	6.0000	48.0000	6.0000
SA2	Span 13	3.9927	45.4532	48.0000	6.0000	51.9927	6.0000
SA2	Span 14	8.0073	49.4459	51.9927	6.0000	60.0000	6.0000

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SA2	Span 15	3.8260	57.4532	60.0000	6.0000	63.8260	6.0000
SA2	Span 16	8.0000	61.2792	63.8260	6.0000	71.8260	6.0000
SA2	Span 17	0.1740	69.2792	71.8260	6.0000	72.0000	6.0000
SA2	Span 18	0.3260	69.4532	72.0000	6.0000	72.3260	6.0000
SA2	Span 19	0.1916	69.7792	72.3260	6.0000	72.5177	6.0000
SA2	Span 20	11.4823	69.9708	72.5177	6.0000	84.0000	6.0000
SA2	Span 21	1.9927	81.4532	84.0000	6.0000	85.9927	6.0000
SA2	Span 22	7.5000	83.4459	85.9927	6.0000	93.4927	6.0000
SA2	Span 23	0.1667	90.9459	93.4927	6.0000	93.6594	6.0000
SA2	Span 24	2.3406	91.1126	93.6594	6.0000	96.0000	6.0000
SA2	Span 25	10.7603	93.4532	96.0000	6.0000	106.7603	6.0000
SA2	Span 26	1.2397	104.2134	106.7603	6.0000	108.0000	6.0000
SA2	Span 27	12.0000	105.4532	108.0000	6.0000	120.0000	6.0000
SA2	Span 28	0.4927	117.4532	120.0000	6.0000	120.4927	6.0000
SA2	Span 29	10.6666	117.9459	120.4927	6.0000	131.1593	6.0000
SA2	Span 30	0.8407	128.6125	131.1593	6.0000	132.0000	6.0000
SA2	Span 31	4.9927	129.4532	132.0000	6.0000	136.9927	6.0000
SA2	Span 32	7.0073	134.4459	136.9927	6.0000	144.0000	6.0000
SA2	Span 33	3.4927	141.4532	144.0000	6.0000	147.4927	6.0000
SA2	Span 34	0.6584	144.9459	147.4927	6.0000	148.1512	6.0000
SA2	Span 35	7.8488	145.6043	148.1512	6.0000	156.0000	6.0000
SA2	Span 36	8.1136	153.4532	156.0000	6.0000	164.1136	6.0000
SA2	Span 37	0.6667	161.5668	164.1136	6.0000	164.7803	6.0000
SA2	Span 38	0.2958	162.2335	164.7803	6.0000	165.0761	6.0000
SA2	Span 39	2.9239	162.5293	165.0761	6.0000	168.0000	6.0000
SA2	Span 40	8.5730	165.4532	168.0000	6.0000	176.5730	6.0000
SA3	Span 1	5.3260	0.0000	0.0000	12.0000	5.3260	12.0000
SA3	Span 2	0.3333	5.3260	5.3260	12.0000	5.6594	12.0000
SA3	Span 3	4.0000	5.6594	5.6594	12.0000	9.6594	12.0000
SA3	Span 4	0.3333	9.6594	9.6594	12.0000	9.9927	12.0000
SA3	Span 5	2.0073	9.9927	9.9927	12.0000	12.0000	12.0000
SA3	Span 6	12.0000	12.0000	12.0000	12.0000	24.0000	12.0000
SA3	Span 7	1.0992	24.0000	24.0000	12.0000	25.0992	12.0000
SA3	Span 8	9.1435	25.0992	25.0992	12.0000	34.2427	12.0000
SA3	Span 9	1.7573	34.2427	34.2427	12.0000	36.0000	12.0000
SA3	Span 10	3.4231	36.0000	36.0000	12.0000	39.4231	12.0000
SA3	Span 11	8.5769	39.4231	39.4231	12.0000	48.0000	12.0000
SA3	Span 12	12.0000	48.0000	48.0000	12.0000	60.0000	12.0000
SA3	Span 13	12.0000	60.0000	60.0000	12.0000	72.0000	12.0000
SA3	Span 14	2.3575	72.0000	72.0000	12.0000	74.3575	12.0000
SA3	Span 15	9.6425	74.3575	74.3575	12.0000	84.0000	12.0000
SA3	Span 16	12.0000	84.0000	84.0000	12.0000	96.0000	12.0000
SA3	Span 17	12.0000	96.0000	96.0000	12.0000	108.0000	12.0000
SA3	Span 18	12.0000	108.0000	108.0000	12.0000	120.0000	12.0000
SA3	Span 19	11.1594	120.0000	120.0000	12.0000	131.1594	12.0000
SA3	Span 20	0.8406	131.1594	131.1594	12.0000	132.0000	12.0000
SA3	Span 21	12.0000	132.0000	132.0000	12.0000	144.0000	12.0000
SA3	Span 22	4.1512	144.0000	144.0000	12.0000	148.1512	12.0000
SA3	Span 23	7.8488	148.1512	148.1512	12.0000	156.0000	12.0000
SA3	Span 24	8.1203	156.0000	156.0000	12.0000	164.1203	12.0000
SA3	Span 25	3.8797	164.1203	164.1203	12.0000	168.0000	12.0000
SA3	Span 26	9.0000	168.0000	168.0000	12.0000	177.0000	12.0000
SA4	Span 1	9.9927	0.0000	0.0000	18.0000	9.9927	18.0000
SA4	Span 2	2.0073	9.9927	9.9927	18.0000	12.0000	18.0000

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SA4	Span 3	12.0000	12.0000	12.0000	18.0000	24.0000	18.0000
SA4	Span 4	3.6485	24.0000	24.0000	18.0000	27.6485	18.0000
SA4	Span 5	8.3515	27.6485	27.6485	18.0000	36.0000	18.0000
SA4	Span 6	12.0000	36.0000	36.0000	18.0000	48.0000	18.0000
SA4	Span 7	12.0000	48.0000	48.0000	18.0000	60.0000	18.0000
SA4	Span 8	12.0000	60.0000	60.0000	18.0000	72.0000	18.0000
SA4	Span 9	4.9046	72.0000	72.0000	18.0000	76.9046	18.0000
SA4	Span 10	7.0954	76.9046	76.9046	18.0000	84.0000	18.0000
SA4	Span 11	12.0000	84.0000	84.0000	18.0000	96.0000	18.0000
SA4	Span 12	12.0000	96.0000	96.0000	18.0000	108.0000	18.0000
SA4	Span 13	12.0000	108.0000	108.0000	18.0000	120.0000	18.0000
SA4	Span 14	11.1617	120.0000	120.0000	18.0000	131.1617	18.0000
SA4	Span 15	0.8383	131.1617	131.1617	18.0000	132.0000	18.0000
SA4	Span 16	12.0000	132.0000	132.0000	18.0000	144.0000	18.0000
SA4	Span 17	4.1512	144.0000	144.0000	18.0000	148.1512	18.0000
SA4	Span 18	7.8488	148.1512	148.1512	18.0000	156.0000	18.0000
SA4	Span 19	8.1296	156.0000	156.0000	18.0000	164.1296	18.0000
SA4	Span 20	3.8704	164.1296	164.1296	18.0000	168.0000	18.0000
SA4	Span 21	9.0000	168.0000	168.0000	18.0000	177.0000	18.0000
SA5	Span 1	9.9927	0.0000	0.0000	24.0000	9.9927	24.0000
SA5	Span 2	0.3333	9.9927	9.9927	24.0000	10.3260	24.0000
SA5	Span 3	1.6740	10.3260	10.3260	24.0000	12.0000	24.0000
SA5	Span 4	2.2427	12.0000	12.0000	24.0000	14.2427	24.0000
SA5	Span 5	0.3333	14.2427	14.2427	24.0000	14.5760	24.0000
SA5	Span 6	9.4240	14.5760	14.5760	24.0000	24.0000	24.0000
SA5	Span 7	5.5697	24.0000	24.0000	24.0000	29.5697	24.0000
SA5	Span 8	0.6282	29.5697	29.5697	24.0000	30.1979	24.0000
SA5	Span 9	4.0448	30.1979	30.1979	24.0000	34.2427	24.0000
SA5	Span 10	0.3314	34.2427	34.2427	24.0000	34.5741	24.0000
SA5	Span 11	0.6647	34.5741	34.5741	24.0000	35.2389	24.0000
SA5	Span 12	0.7611	35.2389	35.2389	24.0000	36.0000	24.0000
SA5	Span 13	6.8360	36.0000	36.0000	24.0000	42.8360	24.0000
SA5	Span 14	0.6647	42.8360	42.8360	24.0000	43.5007	24.0000
SA5	Span 15	0.3314	43.5007	43.5007	24.0000	43.8321	24.0000
SA5	Span 16	4.1679	43.8321	43.8321	24.0000	48.0000	24.0000
SA5	Span 17	12.0000	48.0000	48.0000	24.0000	60.0000	24.0000
SA5	Span 18	12.0000	60.0000	60.0000	24.0000	72.0000	24.0000
SA5	Span 19	7.4517	72.0000	72.0000	24.0000	79.4517	24.0000
SA5	Span 20	4.5483	79.4517	79.4517	24.0000	84.0000	24.0000
SA5	Span 21	12.0000	84.0000	84.0000	24.0000	96.0000	24.0000
SA5	Span 22	12.0000	96.0000	96.0000	24.0000	108.0000	24.0000
SA5	Span 23	12.0000	108.0000	108.0000	24.0000	120.0000	24.0000
SA5	Span 24	11.1633	120.0000	120.0000	24.0000	131.1633	24.0000
SA5	Span 25	0.8367	131.1633	131.1633	24.0000	132.0000	24.0000
SA5	Span 26	12.0000	132.0000	132.0000	24.0000	144.0000	24.0000
SA5	Span 27	4.1512	144.0000	144.0000	24.0000	148.1512	24.0000
SA5	Span 28	7.8488	148.1512	148.1512	24.0000	156.0000	24.0000
SA5	Span 29	8.1388	156.0000	156.0000	24.0000	164.1388	24.0000
SA5	Span 30	3.8612	164.1388	164.1388	24.0000	168.0000	24.0000
SA5	Span 31	9.0000	168.0000	168.0000	24.0000	177.0000	24.0000
SA6	Span 1	12.0000	0.0000	0.0000	30.0000	12.0000	30.0000
SA6	Span 2	2.5760	12.0000	12.0000	30.0000	14.5760	30.0000
SA6	Span 3	9.4240	14.5760	14.5760	30.0000	24.0000	30.0000
SA6	Span 4	8.7473	24.0000	24.0000	30.0000	32.7473	30.0000

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SA6	Span 5	3.2527	32.7473	32.7473	30.0000	36.0000	30.0000
SA6	Span 6	7.8321	36.0000	36.0000	30.0000	43.8321	30.0000
SA6	Span 7	4.1679	43.8321	43.8321	30.0000	48.0000	30.0000
SA6	Span 8	12.0000	48.0000	48.0000	30.0000	60.0000	30.0000
SA6	Span 9	10.3808	60.0000	60.0000	30.0000	70.3808	30.0000
SA6	Span 10	1.6192	70.3808	70.3808	30.0000	72.0000	30.0000
SA6	Span 11	9.9988	72.0000	72.0000	30.0000	81.9988	30.0000
SA6	Span 12	2.0012	81.9988	81.9988	30.0000	84.0000	30.0000
SA6	Span 13	12.0000	84.0000	84.0000	30.0000	96.0000	30.0000
SA6	Span 14	5.5960	96.0000	96.0000	30.0000	101.5960	30.0000
SA6	Span 15	6.4040	101.5960	101.5960	30.0000	108.0000	30.0000
SA6	Span 16	12.0000	108.0000	108.0000	30.0000	120.0000	30.0000
SA6	Span 17	0.0760	120.0000	120.0000	30.0000	120.0760	30.0000
SA6	Span 18	11.0833	120.0760	120.0760	30.0000	131.1594	30.0000
SA6	Span 19	0.8406	131.1594	131.1594	30.0000	132.0000	30.0000
SA6	Span 20	12.0000	132.0000	132.0000	30.0000	144.0000	30.0000
SA6	Span 21	4.1512	144.0000	144.0000	30.0000	148.1512	30.0000
SA6	Span 22	7.8488	148.1512	148.1512	30.0000	156.0000	30.0000
SA6	Span 23	8.1512	156.0000	156.0000	30.0000	164.1512	30.0000
SA6	Span 24	3.8488	164.1512	164.1512	30.0000	168.0000	30.0000
SA6	Span 25	9.0000	168.0000	168.0000	30.0000	177.0000	30.0000
SA7	Span 1	12.0000	0.0000	0.0000	36.0000	12.0000	36.0000
SA7	Span 2	2.9094	12.0000	12.0000	36.0000	14.9094	36.0000
SA7	Span 3	9.0906	14.9094	14.9094	36.0000	24.0000	36.0000
SA7	Span 4	9.4651	24.0000	24.0000	36.0000	33.4651	36.0000
SA7	Span 5	0.6276	33.4651	33.4651	36.0000	34.0927	36.0000
SA7	Span 6	0.6292	34.0927	34.0927	36.0000	34.7219	36.0000
SA7	Span 7	1.2781	34.7219	34.7219	36.0000	36.0000	36.0000
SA7	Span 8	12.0000	36.0000	36.0000	36.0000	48.0000	36.0000
SA7	Span 9	9.7396	48.0000	48.0000	36.0000	57.7396	36.0000
SA7	Span 10	0.3333	57.7396	57.7396	36.0000	58.0729	36.0000
SA7	Span 11	1.9271	58.0729	58.0729	36.0000	60.0000	36.0000
SA7	Span 12	12.0000	60.0000	60.0000	36.0000	72.0000	36.0000
SA7	Span 13	11.2741	72.0000	72.0000	36.0000	83.2741	36.0000
SA7	Span 14	0.7259	83.2741	83.2741	36.0000	84.0000	36.0000
SA7	Span 15	0.5458	84.0000	84.0000	36.0000	84.5458	36.0000
SA7	Span 16	11.4542	84.5458	84.5458	36.0000	96.0000	36.0000
SA7	Span 17	5.5960	96.0000	96.0000	36.0000	101.5960	36.0000
SA7	Span 18	0.3333	101.5960	101.5960	36.0000	101.9293	36.0000
SA7	Span 19	3.1468	101.9293	101.9293	36.0000	105.0760	36.0000
SA7	Span 20	2.9240	105.0760	105.0760	36.0000	108.0000	36.0000
SA7	Span 21	11.7427	108.0000	108.0000	36.0000	119.7427	36.0000
SA7	Span 22	0.2573	119.7427	119.7427	36.0000	120.0000	36.0000
SA7	Span 23	11.1655	120.0000	120.0000	36.0000	131.1655	36.0000
SA7	Span 24	0.8345	131.1655	131.1655	36.0000	132.0000	36.0000
SA7	Span 25	12.0000	132.0000	132.0000	36.0000	144.0000	36.0000
SA7	Span 26	1.2467	144.0000	144.0000	36.0000	145.2467	36.0000
SA7	Span 27	10.7533	145.2467	145.2467	36.0000	156.0000	36.0000
SA7	Span 28	12.0000	156.0000	156.0000	36.0000	168.0000	36.0000
SA7	Span 29	9.0000	168.0000	168.0000	36.0000	177.0000	36.0000
SA8	Span 1	12.0000	0.0000	0.0000	42.0000	12.0000	42.0000
SA8	Span 2	12.0000	12.0000	12.0000	42.0000	24.0000	42.0000
SA8	Span 3	12.0000	24.0000	24.0000	42.0000	36.0000	42.0000
SA8	Span 4	12.0000	36.0000	36.0000	42.0000	48.0000	42.0000

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SA8	Span 5	10.0729	48.0000	48.0000	42.0000	58.0729	42.0000
SA8	Span 6	0.0417	58.0729	58.0729	42.0000	58.1146	42.0000
SA8	Span 7	1.8854	58.1146	58.1146	42.0000	60.0000	42.0000
SA8	Span 8	12.0000	60.0000	60.0000	42.0000	72.0000	42.0000
SA8	Span 9	12.0000	72.0000	72.0000	42.0000	84.0000	42.0000
SA8	Span 10	3.0927	84.0000	84.0000	42.0000	87.0927	42.0000
SA8	Span 11	8.9073	87.0927	87.0927	42.0000	96.0000	42.0000
SA8	Span 12	9.0760	96.0000	96.0000	42.0000	105.0760	42.0000
SA8	Span 13	2.9240	105.0760	105.0760	42.0000	108.0000	42.0000
SA8	Span 14	12.0000	108.0000	108.0000	42.0000	120.0000	42.0000
SA8	Span 15	12.0000	120.0000	120.0000	42.0000	132.0000	42.0000
SA8	Span 16	12.0000	132.0000	132.0000	42.0000	144.0000	42.0000
SA8	Span 17	12.0000	144.0000	144.0000	42.0000	156.0000	42.0000
SA8	Span 18	12.0000	156.0000	156.0000	42.0000	168.0000	42.0000
SA8	Span 19	9.0000	168.0000	168.0000	42.0000	177.0000	42.0000
SA9	Span 1	12.0000	0.0000	0.0000	48.0000	12.0000	48.0000
SA9	Span 2	12.0000	12.0000	12.0000	48.0000	24.0000	48.0000
SA9	Span 3	12.0000	24.0000	24.0000	48.0000	36.0000	48.0000
SA9	Span 4	12.0000	36.0000	36.0000	48.0000	48.0000	48.0000
SA9	Span 5	10.1055	48.0000	48.0000	48.0000	58.1055	48.0000
SA9	Span 6	1.8945	58.1055	58.1055	48.0000	60.0000	48.0000
SA9	Span 7	12.0000	60.0000	60.0000	48.0000	72.0000	48.0000
SA9	Span 8	12.0000	72.0000	72.0000	48.0000	84.0000	48.0000
SA9	Span 9	5.6395	84.0000	84.0000	48.0000	89.6395	48.0000
SA9	Span 10	6.3605	89.6395	89.6395	48.0000	96.0000	48.0000
SA9	Span 11	9.0760	96.0000	96.0000	48.0000	105.0760	48.0000
SA9	Span 12	2.9240	105.0760	105.0760	48.0000	108.0000	48.0000
SA9	Span 13	12.0000	108.0000	108.0000	48.0000	120.0000	48.0000
SA9	Span 14	12.0000	120.0000	120.0000	48.0000	132.0000	48.0000
SA9	Span 15	12.0000	132.0000	132.0000	48.0000	144.0000	48.0000
SA9	Span 16	12.0000	144.0000	144.0000	48.0000	156.0000	48.0000
SA9	Span 17	12.0000	156.0000	156.0000	48.0000	168.0000	48.0000
SA9	Span 18	9.0000	168.0000	168.0000	48.0000	177.0000	48.0000
SB1	Span 1	12.0000	0.0000	0.0000	0.0000	0.0000	12.0000
SB1	Span 2	12.0000	12.0000	0.0000	12.0000	0.0000	24.0000
SB1	Span 3	12.0000	24.0000	0.0000	24.0000	0.0000	36.0000
SB1	Span 4	12.0000	36.0000	0.0000	36.0000	0.0000	48.0000
SB1	Span 5	12.0000	48.0000	0.0000	48.0000	0.0000	60.0000
SB1	Span 6	12.0000	60.0000	0.0000	60.0000	0.0000	72.0000
SB1	Span 7	12.0000	72.0000	0.0000	72.0000	0.0000	84.0000
SB1	Span 8	12.0000	84.0000	0.0000	84.0000	0.0000	96.0000
SB1	Span 9	12.0000	96.0000	0.0000	96.0000	0.0000	108.0000
SB1	Span 10	7.1700	108.0000	0.0000	108.0000	0.0000	115.1700
SB2	Span 1	0.3333	0.0000	6.0000	0.0000	6.0000	0.3333
SB2	Span 2	0.3333	0.3333	6.0000	0.3333	6.0000	0.6667
SB2	Span 3	10.3354	0.6667	6.0000	0.6667	6.0000	11.0020
SB2	Span 4	0.3333	11.0020	6.0000	11.0020	6.0000	11.3354
SB2	Span 5	0.6646	11.3354	6.0000	11.3354	6.0000	12.0000
SB2	Span 6	12.0000	12.0000	6.0000	12.0000	6.0000	24.0000
SB2	Span 7	12.0000	24.0000	6.0000	24.0000	6.0000	36.0000
SB2	Span 8	12.0000	36.0000	6.0000	36.0000	6.0000	48.0000
SB2	Span 9	12.0000	48.0000	6.0000	48.0000	6.0000	60.0000
SB2	Span 10	12.0000	60.0000	6.0000	60.0000	6.0000	72.0000
SB2	Span 11	12.0000	72.0000	6.0000	72.0000	6.0000	84.0000

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SB2	Span 12	12.0000	84.0000	6.0000	84.0000	6.0000	96.0000
SB2	Span 13	12.0000	96.0000	6.0000	96.0000	6.0000	108.0000
SB2	Span 14	7.1700	108.0000	6.0000	108.0000	6.0000	115.1700
SB3	Span 1	11.3354	0.0000	12.0000	0.0000	12.0000	11.3354
SB3	Span 2	0.2917	11.3354	12.0000	11.3354	12.0000	11.6270
SB3	Span 3	0.3730	11.6270	12.0000	11.6270	12.0000	12.0000
SB3	Span 4	9.9187	12.0000	12.0000	12.0000	12.0000	21.9187
SB3	Span 5	0.3333	21.9187	12.0000	21.9187	12.0000	22.2520
SB3	Span 6	0.2917	22.2520	12.0000	22.2520	12.0000	22.5437
SB3	Span 7	1.4563	22.5437	12.0000	22.5437	12.0000	24.0000
SB3	Span 8	8.8333	24.0000	12.0000	24.0000	12.0000	32.8333
SB3	Span 9	0.3333	32.8333	12.0000	32.8333	12.0000	33.1667
SB3	Span 10	2.8333	33.1667	12.0000	33.1667	12.0000	36.0000
SB3	Span 11	12.0000	36.0000	12.0000	36.0000	12.0000	48.0000
SB3	Span 12	12.0000	48.0000	12.0000	48.0000	12.0000	60.0000
SB3	Span 13	12.0000	60.0000	12.0000	60.0000	12.0000	72.0000
SB3	Span 14	12.0000	72.0000	12.0000	72.0000	12.0000	84.0000
SB3	Span 15	12.0000	84.0000	12.0000	84.0000	12.0000	96.0000
SB3	Span 16	12.0000	96.0000	12.0000	96.0000	12.0000	108.0000
SB3	Span 17	7.1700	108.0000	12.0000	108.0000	12.0000	115.1700
SB4	Span 1	0.3333	0.0000	18.0000	0.0000	18.0000	0.3333
SB4	Span 2	11.6667	0.3333	18.0000	0.3333	18.0000	12.0000
SB4	Span 3	12.0000	12.0000	18.0000	12.0000	18.0000	24.0000
SB4	Span 4	12.0000	24.0000	18.0000	24.0000	18.0000	36.0000
SB4	Span 5	12.0000	36.0000	18.0000	36.0000	18.0000	48.0000
SB4	Span 6	12.0000	48.0000	18.0000	48.0000	18.0000	60.0000
SB4	Span 7	12.0000	60.0000	18.0000	60.0000	18.0000	72.0000
SB4	Span 8	12.0000	72.0000	18.0000	72.0000	18.0000	84.0000
SB4	Span 9	12.0000	84.0000	18.0000	84.0000	18.0000	96.0000
SB4	Span 10	12.0000	96.0000	18.0000	96.0000	18.0000	108.0000
SB4	Span 11	7.1700	108.0000	18.0000	108.0000	18.0000	115.1700
SB5	Span 1	6.8333	0.0000	24.0000	0.0000	24.0000	6.8333
SB5	Span 2	2.5797	6.8333	24.0000	6.8333	24.0000	9.4130
SB5	Span 3	2.5870	9.4130	24.0000	9.4130	24.0000	12.0000
SB5	Span 4	12.0000	12.0000	24.0000	12.0000	24.0000	24.0000
SB5	Span 5	12.0000	24.0000	24.0000	24.0000	24.0000	36.0000
SB5	Span 6	12.0000	36.0000	24.0000	36.0000	24.0000	48.0000
SB5	Span 7	12.0000	48.0000	24.0000	48.0000	24.0000	60.0000
SB5	Span 8	12.0000	60.0000	24.0000	60.0000	24.0000	72.0000
SB5	Span 9	12.0000	72.0000	24.0000	72.0000	24.0000	84.0000
SB5	Span 10	12.0000	84.0000	24.0000	84.0000	24.0000	96.0000
SB5	Span 11	12.0000	96.0000	24.0000	96.0000	24.0000	108.0000
SB5	Span 12	7.1700	108.0000	24.0000	108.0000	24.0000	115.1700
SB6	Span 1	6.8333	0.0000	30.0000	0.0000	30.0000	6.8333
SB6	Span 2	5.1667	6.8333	30.0000	6.8333	30.0000	12.0000
SB6	Span 3	10.5111	12.0000	30.0000	12.0000	30.0000	22.5111
SB6	Span 4	1.0231	22.5111	30.0000	22.5111	30.0000	23.5342
SB6	Span 5	0.4658	23.5342	30.0000	23.5342	30.0000	24.0000
SB6	Span 6	12.0000	24.0000	30.0000	24.0000	30.0000	36.0000
SB6	Span 7	12.0000	36.0000	30.0000	36.0000	30.0000	48.0000
SB6	Span 8	12.0000	48.0000	30.0000	48.0000	30.0000	60.0000
SB6	Span 9	12.0000	60.0000	30.0000	60.0000	30.0000	72.0000
SB6	Span 10	12.0000	72.0000	30.0000	72.0000	30.0000	84.0000
SB6	Span 11	12.0000	84.0000	30.0000	84.0000	30.0000	96.0000

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SB6	Span 12	12.0000	96.0000	30.0000	96.0000	30.0000	108.0000
SB6	Span 13	7.1700	108.0000	30.0000	108.0000	30.0000	115.1700
SB7	Span 1	0.3333	0.0000	36.0000	0.0000	36.0000	0.3333
SB7	Span 2	5.7590	0.3333	36.0000	0.3333	36.0000	6.0923
SB7	Span 3	0.6159	6.0923	36.0000	6.0923	36.0000	6.7082
SB7	Span 4	0.1251	6.7082	36.0000	6.7082	36.0000	6.8333
SB7	Span 5	0.0278	6.8333	36.0000	6.8333	36.0000	6.8612
SB7	Span 6	0.4612	6.8612	36.0000	6.8612	36.0000	7.3224
SB7	Span 7	4.5150	7.3224	36.0000	7.3224	36.0000	11.8374
SB7	Span 8	0.1626	11.8374	36.0000	11.8374	36.0000	12.0000
SB7	Span 9	10.1707	12.0000	36.0000	12.0000	36.0000	22.1707
SB7	Span 10	0.3333	22.1707	36.0000	22.1707	36.0000	22.5041
SB7	Span 11	1.4959	22.5041	36.0000	22.5041	36.0000	24.0000
SB7	Span 12	9.1667	24.0000	36.0000	24.0000	36.0000	33.1667
SB7	Span 13	2.8333	33.1667	36.0000	33.1667	36.0000	36.0000
SB7	Span 14	12.0000	36.0000	36.0000	36.0000	36.0000	48.0000
SB7	Span 15	12.0000	48.0000	36.0000	48.0000	36.0000	60.0000
SB7	Span 16	12.0000	60.0000	36.0000	60.0000	36.0000	72.0000
SB7	Span 17	12.0000	72.0000	36.0000	72.0000	36.0000	84.0000
SB7	Span 18	12.0000	84.0000	36.0000	84.0000	36.0000	96.0000
SB7	Span 19	12.0000	96.0000	36.0000	96.0000	36.0000	108.0000
SB7	Span 20	7.1700	108.0000	36.0000	108.0000	36.0000	115.1700
SB8	Span 1	6.8612	0.0000	42.0000	0.0000	42.0000	6.8612
SB8	Span 2	4.9762	6.8612	42.0000	6.8612	42.0000	11.8374
SB8	Span 3	0.1626	11.8374	42.0000	11.8374	42.0000	12.0000
SB8	Span 4	10.5041	12.0000	42.0000	12.0000	42.0000	22.5041
SB8	Span 5	0.3333	22.5041	42.0000	22.5041	42.0000	22.8374
SB8	Span 6	1.1626	22.8374	42.0000	22.8374	42.0000	24.0000
SB8	Span 7	8.8333	24.0000	42.0000	24.0000	42.0000	32.8333
SB8	Span 8	3.1667	32.8333	42.0000	32.8333	42.0000	36.0000
SB8	Span 9	12.0000	36.0000	42.0000	36.0000	42.0000	48.0000
SB8	Span 10	12.0000	48.0000	42.0000	48.0000	42.0000	60.0000
SB8	Span 11	12.0000	60.0000	42.0000	60.0000	42.0000	72.0000
SB8	Span 12	12.0000	72.0000	42.0000	72.0000	42.0000	84.0000
SB8	Span 13	12.0000	84.0000	42.0000	84.0000	42.0000	96.0000
SB8	Span 14	12.0000	96.0000	42.0000	96.0000	42.0000	108.0000
SB8	Span 15	7.1700	108.0000	42.0000	108.0000	42.0000	115.1700
SB9	Span 1	0.1945	0.0000	48.0000	0.0000	48.0000	0.1945
SB9	Span 2	6.0000	0.1945	48.0000	0.1945	48.0000	6.1945
SB9	Span 3	0.5000	6.1945	48.0000	6.1945	48.0000	6.6945
SB9	Span 4	5.3055	6.6945	48.0000	6.6945	48.0000	12.0000
SB9	Span 5	12.0000	12.0000	48.0000	12.0000	48.0000	24.0000
SB9	Span 6	12.0000	24.0000	48.0000	24.0000	48.0000	36.0000
SB9	Span 7	12.0000	36.0000	48.0000	36.0000	48.0000	48.0000
SB9	Span 8	12.0000	48.0000	48.0000	48.0000	48.0000	60.0000
SB9	Span 9	12.0000	60.0000	48.0000	60.0000	48.0000	72.0000
SB9	Span 10	12.0000	72.0000	48.0000	72.0000	48.0000	84.0000
SB9	Span 11	5.3759	84.0000	48.0000	84.0000	48.0000	89.3759
SB9	Span 12	6.6241	89.3759	48.0000	89.3759	48.0000	96.0000
SB9	Span 13	12.0000	96.0000	48.0000	96.0000	48.0000	108.0000
SB9	Span 14	2.3493	108.0000	48.0000	108.0000	48.0000	110.3493
SB9	Span 15	4.8207	110.3493	48.0000	110.3493	48.0000	115.1700
SA10	Span 1	12.0000	0.0000	0.0000	54.0000	12.0000	54.0000
SA10	Span 2	12.0000	12.0000	12.0000	54.0000	24.0000	54.0000

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SA10	Span 3	12.0000	24.0000	24.0000	54.0000	36.0000	54.0000
SA10	Span 4	12.0000	36.0000	36.0000	54.0000	48.0000	54.0000
SA10	Span 5	10.0877	48.0000	48.0000	54.0000	58.0877	54.0000
SA10	Span 6	1.9123	58.0877	58.0877	54.0000	60.0000	54.0000
SA10	Span 7	12.0000	60.0000	60.0000	54.0000	72.0000	54.0000
SA10	Span 8	12.0000	72.0000	72.0000	54.0000	84.0000	54.0000
SA10	Span 9	8.1864	84.0000	84.0000	54.0000	92.1864	54.0000
SA10	Span 10	3.8136	92.1864	92.1864	54.0000	96.0000	54.0000
SA10	Span 11	9.0760	96.0000	96.0000	54.0000	105.0760	54.0000
SA10	Span 12	2.9240	105.0760	105.0760	54.0000	108.0000	54.0000
SA10	Span 13	12.0000	108.0000	108.0000	54.0000	120.0000	54.0000
SA10	Span 14	12.0000	120.0000	120.0000	54.0000	132.0000	54.0000
SA10	Span 15	12.0000	132.0000	132.0000	54.0000	144.0000	54.0000
SA10	Span 16	12.0000	144.0000	144.0000	54.0000	156.0000	54.0000
SA10	Span 17	12.0000	156.0000	156.0000	54.0000	168.0000	54.0000
SA10	Span 18	9.0000	168.0000	168.0000	54.0000	177.0000	54.0000
SA11	Span 1	12.0000	0.0000	0.0000	60.0000	12.0000	60.0000
SA11	Span 2	12.0000	12.0000	12.0000	60.0000	24.0000	60.0000
SA11	Span 3	12.0000	24.0000	24.0000	60.0000	36.0000	60.0000
SA11	Span 4	12.0000	36.0000	36.0000	60.0000	48.0000	60.0000
SA11	Span 5	10.0760	48.0000	48.0000	60.0000	58.0760	60.0000
SA11	Span 6	1.9240	58.0760	58.0760	60.0000	60.0000	60.0000
SA11	Span 7	12.0000	60.0000	60.0000	60.0000	72.0000	60.0000
SA11	Span 8	12.0000	72.0000	72.0000	60.0000	84.0000	60.0000
SA11	Span 9	10.3420	84.0000	84.0000	60.0000	94.3420	60.0000
SA11	Span 10	0.3912	94.3420	94.3420	60.0000	94.7332	60.0000
SA11	Span 11	1.2668	94.7332	94.7332	60.0000	96.0000	60.0000
SA11	Span 12	9.0760	96.0000	96.0000	60.0000	105.0760	60.0000
SA11	Span 13	2.9240	105.0760	105.0760	60.0000	108.0000	60.0000
SA11	Span 14	10.5142	108.0000	108.0000	60.0000	118.5142	60.0000
SA11	Span 15	1.4858	118.5142	118.5142	60.0000	120.0000	60.0000
SA11	Span 16	12.0000	120.0000	120.0000	60.0000	132.0000	60.0000
SA11	Span 17	12.0000	132.0000	132.0000	60.0000	144.0000	60.0000
SA11	Span 18	12.0000	144.0000	144.0000	60.0000	156.0000	60.0000
SA11	Span 19	12.0000	156.0000	156.0000	60.0000	168.0000	60.0000
SA11	Span 20	9.0000	168.0000	168.0000	60.0000	177.0000	60.0000
SA12	Span 1	12.0000	0.0000	0.0000	66.0000	12.0000	66.0000
SA12	Span 2	12.0000	12.0000	12.0000	66.0000	24.0000	66.0000
SA12	Span 3	12.0000	24.0000	24.0000	66.0000	36.0000	66.0000
SA12	Span 4	12.0000	36.0000	36.0000	66.0000	48.0000	66.0000
SA12	Span 5	10.0729	48.0000	48.0000	66.0000	58.0729	66.0000
SA12	Span 6	1.9271	58.0729	58.0729	66.0000	60.0000	66.0000
SA12	Span 7	12.0000	60.0000	60.0000	66.0000	72.0000	66.0000
SA12	Span 8	12.0000	72.0000	72.0000	66.0000	84.0000	66.0000
SA12	Span 9	12.0000	84.0000	84.0000	66.0000	96.0000	66.0000
SA12	Span 10	1.2801	96.0000	96.0000	66.0000	97.2801	66.0000
SA12	Span 11	7.7960	97.2801	97.2801	66.0000	105.0760	66.0000
SA12	Span 12	2.9240	105.0760	105.0760	66.0000	108.0000	66.0000
SA12	Span 13	12.0000	108.0000	108.0000	66.0000	120.0000	66.0000
SA12	Span 14	1.6979	120.0000	120.0000	66.0000	121.6979	66.0000
SA12	Span 15	10.3021	121.6979	121.6979	66.0000	132.0000	66.0000
SA12	Span 16	12.0000	132.0000	132.0000	66.0000	144.0000	66.0000
SA12	Span 17	12.0000	144.0000	144.0000	66.0000	156.0000	66.0000
SA12	Span 18	12.0000	156.0000	156.0000	66.0000	168.0000	66.0000

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SA12	Span 19	9.0000	168.0000	168.0000	66.0000	177.0000	66.0000
SA13	Span 1	12.0000	0.0000	0.0000	72.0000	12.0000	72.0000
SA13	Span 2	12.0000	12.0000	12.0000	72.0000	24.0000	72.0000
SA13	Span 3	12.0000	24.0000	24.0000	72.0000	36.0000	72.0000
SA13	Span 4	12.0000	36.0000	36.0000	72.0000	48.0000	72.0000
SA13	Span 5	10.0729	48.0000	48.0000	72.0000	58.0729	72.0000
SA13	Span 6	1.9271	58.0729	58.0729	72.0000	60.0000	72.0000
SA13	Span 7	12.0000	60.0000	60.0000	72.0000	72.0000	72.0000
SA13	Span 8	12.0000	72.0000	72.0000	72.0000	84.0000	72.0000
SA13	Span 9	12.0000	84.0000	84.0000	72.0000	96.0000	72.0000
SA13	Span 10	3.8269	96.0000	96.0000	72.0000	99.8269	72.0000
SA13	Span 11	5.2491	99.8269	99.8269	72.0000	105.0760	72.0000
SA13	Span 12	2.9240	105.0760	105.0760	72.0000	108.0000	72.0000
SA13	Span 13	12.0000	108.0000	108.0000	72.0000	120.0000	72.0000
SA13	Span 14	1.7249	120.0000	120.0000	72.0000	121.7249	72.0000
SA13	Span 15	10.2751	121.7249	121.7249	72.0000	132.0000	72.0000
SA13	Span 16	12.0000	132.0000	132.0000	72.0000	144.0000	72.0000
SA13	Span 17	12.0000	144.0000	144.0000	72.0000	156.0000	72.0000
SA13	Span 18	12.0000	156.0000	156.0000	72.0000	168.0000	72.0000
SA13	Span 19	9.0000	168.0000	168.0000	72.0000	177.0000	72.0000
SA14	Span 1	12.0000	0.0000	0.0000	78.0000	12.0000	78.0000
SA14	Span 2	12.0000	12.0000	12.0000	78.0000	24.0000	78.0000
SA14	Span 3	12.0000	24.0000	24.0000	78.0000	36.0000	78.0000
SA14	Span 4	12.0000	36.0000	36.0000	78.0000	48.0000	78.0000
SA14	Span 5	10.0729	48.0000	48.0000	78.0000	58.0729	78.0000
SA14	Span 6	1.9271	58.0729	58.0729	78.0000	60.0000	78.0000
SA14	Span 7	4.3260	60.0000	60.0000	78.0000	64.3260	78.0000
SA14	Span 8	7.6740	64.3260	64.3260	78.0000	72.0000	78.0000
SA14	Span 9	12.0000	72.0000	72.0000	78.0000	84.0000	78.0000
SA14	Span 10	10.6589	84.0000	84.0000	78.0000	94.6589	78.0000
SA14	Span 11	0.3338	94.6589	94.6589	78.0000	94.9927	78.0000
SA14	Span 12	1.0073	94.9927	94.9927	78.0000	96.0000	78.0000
SA14	Span 13	6.3738	96.0000	96.0000	78.0000	102.3738	78.0000
SA14	Span 14	0.9551	102.3738	102.3738	78.0000	103.3288	78.0000
SA14	Span 15	1.7472	103.3288	103.3288	78.0000	105.0760	78.0000
SA14	Span 16	2.9240	105.0760	105.0760	78.0000	108.0000	78.0000
SA14	Span 17	7.5394	108.0000	108.0000	78.0000	115.5394	78.0000
SA14	Span 18	0.4981	115.5394	115.5394	78.0000	116.0374	78.0000
SA14	Span 19	3.9626	116.0374	116.0374	78.0000	120.0000	78.0000
SA14	Span 20	1.7427	120.0000	120.0000	78.0000	121.7427	78.0000
SA14	Span 21	0.0202	121.7427	121.7427	78.0000	121.7629	78.0000
SA14	Span 22	10.2371	121.7629	121.7629	78.0000	132.0000	78.0000
SA14	Span 23	12.0000	132.0000	132.0000	78.0000	144.0000	78.0000
SA14	Span 24	12.0000	144.0000	144.0000	78.0000	156.0000	78.0000
SA14	Span 25	12.0000	156.0000	156.0000	78.0000	168.0000	78.0000
SA14	Span 26	9.0000	168.0000	168.0000	78.0000	177.0000	78.0000
SA15	Span 1	12.0000	0.0000	0.0000	84.0000	12.0000	84.0000
SA15	Span 2	12.0000	12.0000	12.0000	84.0000	24.0000	84.0000
SA15	Span 3	12.0000	24.0000	24.0000	84.0000	36.0000	84.0000
SA15	Span 4	12.0000	36.0000	36.0000	84.0000	48.0000	84.0000
SA15	Span 5	9.7396	48.0000	48.0000	84.0000	57.7396	84.0000
SA15	Span 6	0.3128	57.7396	57.7396	84.0000	58.0524	84.0000
SA15	Span 7	1.9476	58.0524	58.0524	84.0000	60.0000	84.0000
SA15	Span 8	4.3260	60.0000	60.0000	84.0000	64.3260	84.0000

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SA15	Span 9	7.6740	64.3260	64.3260	84.0000	72.0000	84.0000
SA15	Span 10	12.0000	72.0000	72.0000	84.0000	84.0000	84.0000
SA15	Span 11	12.0000	84.0000	84.0000	84.0000	96.0000	84.0000
SA15	Span 12	12.0000	96.0000	96.0000	84.0000	108.0000	84.0000
SA15	Span 13	12.0000	108.0000	108.0000	84.0000	120.0000	84.0000
SA15	Span 14	1.7545	120.0000	120.0000	84.0000	121.7545	84.0000
SA15	Span 15	10.2455	121.7545	121.7545	84.0000	132.0000	84.0000
SA15	Span 16	12.0000	132.0000	132.0000	84.0000	144.0000	84.0000
SA15	Span 17	12.0000	144.0000	144.0000	84.0000	156.0000	84.0000
SA15	Span 18	12.0000	156.0000	156.0000	84.0000	168.0000	84.0000
SA15	Span 19	9.0000	168.0000	168.0000	84.0000	177.0000	84.0000
SA16	Span 1	12.0000	0.0000	0.0000	90.0000	12.0000	90.0000
SA16	Span 2	12.0000	12.0000	12.0000	90.0000	24.0000	90.0000
SA16	Span 3	12.0000	24.0000	24.0000	90.0000	36.0000	90.0000
SA16	Span 4	12.0000	36.0000	36.0000	90.0000	48.0000	90.0000
SA16	Span 5	1.9094	48.0000	48.0000	90.0000	49.9094	90.0000
SA16	Span 6	8.1635	49.9094	49.9094	90.0000	58.0729	90.0000
SA16	Span 7	0.1807	58.0729	58.0729	90.0000	58.2536	90.0000
SA16	Span 8	1.7464	58.2536	58.2536	90.0000	60.0000	90.0000
SA16	Span 9	12.0000	60.0000	60.0000	90.0000	72.0000	90.0000
SA16	Span 10	12.0000	72.0000	72.0000	90.0000	84.0000	90.0000
SA16	Span 11	12.0000	84.0000	84.0000	90.0000	96.0000	90.0000
SA16	Span 12	12.0000	96.0000	96.0000	90.0000	108.0000	90.0000
SA16	Span 13	12.0000	108.0000	108.0000	90.0000	120.0000	90.0000
SA16	Span 14	1.7427	120.0000	120.0000	90.0000	121.7427	90.0000
SA16	Span 15	10.2573	121.7427	121.7427	90.0000	132.0000	90.0000
SA16	Span 16	12.0000	132.0000	132.0000	90.0000	144.0000	90.0000
SA16	Span 17	12.0000	144.0000	144.0000	90.0000	156.0000	90.0000
SA16	Span 18	12.0000	156.0000	156.0000	90.0000	168.0000	90.0000
SA16	Span 19	9.0000	168.0000	168.0000	90.0000	177.0000	90.0000
SA17	Span 1	12.0000	0.0000	0.0000	96.0000	12.0000	96.0000
SA17	Span 2	12.0000	12.0000	12.0000	96.0000	24.0000	96.0000
SA17	Span 3	12.0000	24.0000	24.0000	96.0000	36.0000	96.0000
SA17	Span 4	12.0000	36.0000	36.0000	96.0000	48.0000	96.0000
SA17	Span 5	11.7080	48.0000	48.0000	96.0000	59.7080	96.0000
SA17	Span 6	0.2920	59.7080	59.7080	96.0000	60.0000	96.0000
SA17	Span 7	0.7575	60.0000	60.0000	96.0000	60.7575	96.0000
SA17	Span 8	11.2425	60.7575	60.7575	96.0000	72.0000	96.0000
SA17	Span 9	12.0000	72.0000	72.0000	96.0000	84.0000	96.0000
SA17	Span 10	12.0000	84.0000	84.0000	96.0000	96.0000	96.0000
SA17	Span 11	12.0000	96.0000	96.0000	96.0000	108.0000	96.0000
SA17	Span 12	12.0000	108.0000	108.0000	96.0000	120.0000	96.0000
SA17	Span 13	1.7427	120.0000	120.0000	96.0000	121.7427	96.0000
SA17	Span 14	10.2573	121.7427	121.7427	96.0000	132.0000	96.0000
SA17	Span 15	12.0000	132.0000	132.0000	96.0000	144.0000	96.0000
SA17	Span 16	12.0000	144.0000	144.0000	96.0000	156.0000	96.0000
SA17	Span 17	12.0000	156.0000	156.0000	96.0000	168.0000	96.0000
SA17	Span 18	9.0000	168.0000	168.0000	96.0000	177.0000	96.0000
SA18	Span 1	12.0000	0.0000	0.0000	102.0000	12.0000	102.0000
SA18	Span 2	12.0000	12.0000	12.0000	102.0000	24.0000	102.0000
SA18	Span 3	12.0000	24.0000	24.0000	102.0000	36.0000	102.0000
SA18	Span 4	12.0000	36.0000	36.0000	102.0000	48.0000	102.0000
SA18	Span 5	12.0000	48.0000	48.0000	102.0000	60.0000	102.0000
SA18	Span 6	3.3005	60.0000	60.0000	102.0000	63.3005	102.0000

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SA18	Span 7	8.6995	63.3005	63.3005	102.0000	72.0000	102.0000
SA18	Span 8	12.0000	72.0000	72.0000	102.0000	84.0000	102.0000
SA18	Span 9	12.0000	84.0000	84.0000	102.0000	96.0000	102.0000
SA18	Span 10	12.0000	96.0000	96.0000	102.0000	108.0000	102.0000
SA18	Span 11	12.0000	108.0000	108.0000	102.0000	120.0000	102.0000
SA18	Span 12	1.7427	120.0000	120.0000	102.0000	121.7427	102.0000
SA18	Span 13	10.2573	121.7427	121.7427	102.0000	132.0000	102.0000
SA18	Span 14	12.0000	132.0000	132.0000	102.0000	144.0000	102.0000
SA18	Span 15	12.0000	144.0000	144.0000	102.0000	156.0000	102.0000
SA18	Span 16	12.0000	156.0000	156.0000	102.0000	168.0000	102.0000
SA18	Span 17	9.0000	168.0000	168.0000	102.0000	177.0000	102.0000
SA19	Span 1	12.0000	0.0000	0.0000	108.0000	12.0000	108.0000
SA19	Span 2	12.0000	12.0000	12.0000	108.0000	24.0000	108.0000
SA19	Span 3	12.0000	24.0000	24.0000	108.0000	36.0000	108.0000
SA19	Span 4	12.0000	36.0000	36.0000	108.0000	48.0000	108.0000
SA19	Span 5	1.9094	48.0000	48.0000	108.0000	49.9094	108.0000
SA19	Span 6	2.0038	49.9094	49.9094	108.0000	51.9132	108.0000
SA19	Span 7	0.4981	51.9132	51.9132	108.0000	52.4113	108.0000
SA19	Span 8	7.5887	52.4113	52.4113	108.0000	60.0000	108.0000
SA19	Span 9	3.4075	60.0000	60.0000	108.0000	63.4075	108.0000
SA19	Span 10	0.4981	63.4075	63.4075	108.0000	63.9055	108.0000
SA19	Span 11	1.4205	63.9055	63.9055	108.0000	65.3260	108.0000
SA19	Span 12	0.5371	65.3260	65.3260	108.0000	65.8631	108.0000
SA19	Span 13	1.0266	65.8631	65.8631	108.0000	66.8897	108.0000
SA19	Span 14	1.8568	66.8897	66.8897	108.0000	68.7466	108.0000
SA19	Span 15	0.4981	68.7466	68.7466	108.0000	69.2446	108.0000
SA19	Span 16	2.7554	69.2446	69.2446	108.0000	72.0000	108.0000
SA19	Span 17	8.2408	72.0000	72.0000	108.0000	80.2408	108.0000
SA19	Span 18	0.4981	80.2408	80.2408	108.0000	80.7389	108.0000
SA19	Span 19	3.2611	80.7389	80.7389	108.0000	84.0000	108.0000
SA19	Span 20	12.0000	84.0000	84.0000	108.0000	96.0000	108.0000
SA19	Span 21	10.0799	96.0000	96.0000	108.0000	106.0799	108.0000
SA19	Span 22	0.4981	106.0799	106.0799	108.0000	106.5780	108.0000
SA19	Span 23	1.4220	106.5780	106.5780	108.0000	108.0000	108.0000
SA19	Span 24	12.0000	108.0000	108.0000	108.0000	120.0000	108.0000
SA19	Span 25	1.7427	120.0000	120.0000	108.0000	121.7427	108.0000
SA19	Span 26	10.2573	121.7427	121.7427	108.0000	132.0000	108.0000
SA19	Span 27	12.0000	132.0000	132.0000	108.0000	144.0000	108.0000
SA19	Span 28	12.0000	144.0000	144.0000	108.0000	156.0000	108.0000
SA19	Span 29	12.0000	156.0000	156.0000	108.0000	168.0000	108.0000
SA19	Span 30	9.0000	168.0000	168.0000	108.0000	177.0000	108.0000
SA20	Span 1	12.0000	0.0000	0.0000	111.5850	12.0000	111.5850
SA20	Span 2	12.0000	12.0000	12.0000	111.5850	24.0000	111.5850
SA20	Span 3	12.0000	24.0000	24.0000	111.5850	36.0000	111.5850
SA20	Span 4	12.0000	36.0000	36.0000	111.5850	48.0000	111.5850
SA20	Span 5	1.9094	48.0000	48.0000	111.5850	49.9094	111.5850
SA20	Span 6	2.0038	49.9094	49.9094	111.5850	51.9132	111.5850
SA20	Span 7	0.4981	51.9132	51.9132	111.5850	52.4113	111.5850
SA20	Span 8	7.5887	52.4113	52.4113	111.5850	60.0000	111.5850
SA20	Span 9	3.4075	60.0000	60.0000	111.5850	63.4075	111.5850
SA20	Span 10	0.4981	63.4075	63.4075	111.5850	63.9055	111.5850
SA20	Span 11	2.9842	63.9055	63.9055	111.5850	66.8897	111.5850
SA20	Span 12	1.8568	66.8897	66.8897	111.5850	68.7466	111.5850
SA20	Span 13	0.4981	68.7466	68.7466	111.5850	69.2446	111.5850

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SA20	Span 14	2.7554	69.2446	69.2446	111.5850	72.0000	111.5850
SA20	Span 15	8.2408	72.0000	72.0000	111.5850	80.2408	111.5850
SA20	Span 16	0.4981	80.2408	80.2408	111.5850	80.7389	111.5850
SA20	Span 17	3.2611	80.7389	80.7389	111.5850	84.0000	111.5850
SA20	Span 18	12.0000	84.0000	84.0000	111.5850	96.0000	111.5850
SA20	Span 19	10.0799	96.0000	96.0000	111.5850	106.0799	111.5850
SA20	Span 20	0.4981	106.0799	106.0799	111.5850	106.5780	111.5850
SA20	Span 21	1.4220	106.5780	106.5780	111.5850	108.0000	111.5850
SA20	Span 22	12.0000	108.0000	108.0000	111.5850	120.0000	111.5850
SA20	Span 23	1.7427	120.0000	120.0000	111.5850	121.7427	111.5850
SA20	Span 24	10.2573	121.7427	121.7427	111.5850	132.0000	111.5850
SA20	Span 25	12.0000	132.0000	132.0000	111.5850	144.0000	111.5850
SA20	Span 26	12.0000	144.0000	144.0000	111.5850	156.0000	111.5850
SA20	Span 27	12.0000	156.0000	156.0000	111.5850	168.0000	111.5850
SA20	Span 28	9.0000	168.0000	168.0000	111.5850	177.0000	111.5850
SA21	Span 1	12.0000	0.0000	0.0000	115.1700	12.0000	115.1700
SA21	Span 2	12.0000	12.0000	12.0000	115.1700	24.0000	115.1700
SA21	Span 3	12.0000	24.0000	24.0000	115.1700	36.0000	115.1700
SA21	Span 4	12.0000	36.0000	36.0000	115.1700	48.0000	115.1700
SA21	Span 5	12.0000	48.0000	48.0000	115.1700	60.0000	115.1700
SA21	Span 6	12.0000	60.0000	60.0000	115.1700	72.0000	115.1700
SA21	Span 7	12.0000	72.0000	72.0000	115.1700	84.0000	115.1700
SA21	Span 8	12.0000	84.0000	84.0000	115.1700	96.0000	115.1700
SA21	Span 9	12.0000	96.0000	96.0000	115.1700	108.0000	115.1700
SA21	Span 10	12.0000	108.0000	108.0000	115.1700	120.0000	115.1700
SA21	Span 11	12.0000	120.0000	120.0000	115.1700	132.0000	115.1700
SA21	Span 12	12.0000	132.0000	132.0000	115.1700	144.0000	115.1700
SA21	Span 13	12.0000	144.0000	144.0000	115.1700	156.0000	115.1700
SA21	Span 14	12.0000	156.0000	156.0000	115.1700	168.0000	115.1700
SA21	Span 15	9.0000	168.0000	168.0000	115.1700	177.0000	115.1700
SB10	Span 1	0.1945	0.0000	54.0000	0.0000	54.0000	0.1945
SB10	Span 2	0.1388	0.1945	54.0000	0.1945	54.0000	0.3333
SB10	Span 3	5.8333	0.3333	54.0000	0.3333	54.0000	6.1667
SB10	Span 4	0.5000	6.1667	54.0000	6.1667	54.0000	6.6667
SB10	Span 5	5.3333	6.6667	54.0000	6.6667	54.0000	12.0000
SB10	Span 6	12.0000	12.0000	54.0000	12.0000	54.0000	24.0000
SB10	Span 7	12.0000	24.0000	54.0000	24.0000	54.0000	36.0000
SB10	Span 8	12.0000	36.0000	54.0000	36.0000	54.0000	48.0000
SB10	Span 9	12.0000	48.0000	54.0000	48.0000	54.0000	60.0000
SB10	Span 10	12.0000	60.0000	54.0000	60.0000	54.0000	72.0000
SB10	Span 11	12.0000	72.0000	54.0000	72.0000	54.0000	84.0000
SB10	Span 12	12.0000	84.0000	54.0000	84.0000	54.0000	96.0000
SB10	Span 13	12.0000	96.0000	54.0000	96.0000	54.0000	108.0000
SB10	Span 14	2.3612	108.0000	54.0000	108.0000	54.0000	110.3612
SB10	Span 15	4.8088	110.3612	54.0000	110.3612	54.0000	115.1700
SB11	Span 1	6.6667	0.0000	60.0000	0.0000	60.0000	6.6667
SB11	Span 2	5.3333	6.6667	60.0000	6.6667	60.0000	12.0000
SB11	Span 3	12.0000	12.0000	60.0000	12.0000	60.0000	24.0000
SB11	Span 4	9.1406	24.0000	60.0000	24.0000	60.0000	33.1406
SB11	Span 5	0.0261	33.1406	60.0000	33.1406	60.0000	33.1667
SB11	Span 6	0.1647	33.1667	60.0000	33.1667	60.0000	33.3314
SB11	Span 7	0.4981	33.3314	60.0000	33.3314	60.0000	33.8295
SB11	Span 8	2.1705	33.8295	60.0000	33.8295	60.0000	36.0000
SB11	Span 9	7.5000	36.0000	60.0000	36.0000	60.0000	43.5000

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SB11	Span 10	1.4179	43.5000	60.0000	43.5000	60.0000	44.9179
SB11	Span 11	3.0821	44.9179	60.0000	44.9179	60.0000	48.0000
SB11	Span 12	9.9599	48.0000	60.0000	48.0000	60.0000	57.9599
SB11	Span 13	2.0401	57.9599	60.0000	57.9599	60.0000	60.0000
SB11	Span 14	5.7093	60.0000	60.0000	60.0000	60.0000	65.7093
SB11	Span 15	6.2907	65.7093	60.0000	65.7093	60.0000	72.0000
SB11	Span 16	7.1010	72.0000	60.0000	72.0000	60.0000	79.1010
SB11	Span 17	4.8990	79.1010	60.0000	79.1010	60.0000	84.0000
SB11	Span 18	1.7861	84.0000	60.0000	84.0000	60.0000	85.7861
SB11	Span 19	0.4766	85.7861	60.0000	85.7861	60.0000	86.2627
SB11	Span 20	3.2996	86.2627	60.0000	86.2627	60.0000	89.5622
SB11	Span 21	3.9615	89.5622	60.0000	89.5622	60.0000	93.5237
SB11	Span 22	0.6891	93.5237	60.0000	93.5237	60.0000	94.2128
SB11	Span 23	1.7872	94.2128	60.0000	94.2128	60.0000	96.0000
SB11	Span 24	12.0000	96.0000	60.0000	96.0000	60.0000	108.0000
SB11	Span 25	2.3579	108.0000	60.0000	108.0000	60.0000	110.3579
SB11	Span 26	4.8121	110.3579	60.0000	110.3579	60.0000	115.1700
SB12	Span 1	0.3333	0.0000	66.0000	0.0000	66.0000	0.3333
SB12	Span 2	5.8333	0.3333	66.0000	0.3333	66.0000	6.1667
SB12	Span 3	0.5000	6.1667	66.0000	6.1667	66.0000	6.6667
SB12	Span 4	5.3333	6.6667	66.0000	6.6667	66.0000	12.0000
SB12	Span 5	12.0000	12.0000	66.0000	12.0000	66.0000	24.0000
SB12	Span 6	9.0593	24.0000	66.0000	24.0000	66.0000	33.0593
SB12	Span 7	2.9407	33.0593	66.0000	33.0593	66.0000	36.0000
SB12	Span 8	12.0000	36.0000	66.0000	36.0000	66.0000	48.0000
SB12	Span 9	12.0000	48.0000	66.0000	48.0000	66.0000	60.0000
SB12	Span 10	12.0000	60.0000	66.0000	60.0000	66.0000	72.0000
SB12	Span 11	8.2778	72.0000	66.0000	72.0000	66.0000	80.2778
SB12	Span 12	1.6667	80.2778	66.0000	80.2778	66.0000	81.9445
SB12	Span 13	2.0555	81.9445	66.0000	81.9445	66.0000	84.0000
SB12	Span 14	12.0000	84.0000	66.0000	84.0000	66.0000	96.0000
SB12	Span 15	10.7793	96.0000	66.0000	96.0000	66.0000	106.7793
SB12	Span 16	1.2207	106.7793	66.0000	106.7793	66.0000	108.0000
SB12	Span 17	0.3111	108.0000	66.0000	108.0000	66.0000	108.3111
SB12	Span 18	2.0222	108.3111	66.0000	108.3111	66.0000	110.3333
SB12	Span 19	0.0176	110.3333	66.0000	110.3333	66.0000	110.3510
SB12	Span 20	4.8190	110.3510	66.0000	110.3510	66.0000	115.1700
SB13	Span 1	6.6667	0.0000	72.0000	0.0000	72.0000	6.6667
SB13	Span 2	0.2513	6.6667	72.0000	6.6667	72.0000	6.9180
SB13	Span 3	0.7480	6.9180	72.0000	6.9180	72.0000	7.6660
SB13	Span 4	4.3340	7.6660	72.0000	7.6660	72.0000	12.0000
SB13	Span 5	12.0000	12.0000	72.0000	12.0000	72.0000	24.0000
SB13	Span 6	9.0000	24.0000	72.0000	24.0000	72.0000	33.0000
SB13	Span 7	3.0000	33.0000	72.0000	33.0000	72.0000	36.0000
SB13	Span 8	12.0000	36.0000	72.0000	36.0000	72.0000	48.0000
SB13	Span 9	12.0000	48.0000	72.0000	48.0000	72.0000	60.0000
SB13	Span 10	12.0000	60.0000	72.0000	60.0000	72.0000	72.0000
SB13	Span 11	12.0000	72.0000	72.0000	72.0000	72.0000	84.0000
SB13	Span 12	12.0000	84.0000	72.0000	84.0000	72.0000	96.0000
SB13	Span 13	12.0000	96.0000	72.0000	96.0000	72.0000	108.0000
SB13	Span 14	2.3510	108.0000	72.0000	108.0000	72.0000	110.3510
SB13	Span 15	4.8190	110.3510	72.0000	110.3510	72.0000	115.1700
SB14	Span 1	12.0000	0.0000	78.0000	0.0000	78.0000	12.0000
SB14	Span 2	8.5803	12.0000	78.0000	12.0000	78.0000	20.5803

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SB14	Span 3	3.4197	20.5803	78.0000	20.5803	78.0000	24.0000
SB14	Span 4	9.0024	24.0000	78.0000	24.0000	78.0000	33.0024
SB14	Span 5	2.9976	33.0024	78.0000	33.0024	78.0000	36.0000
SB14	Span 6	12.0000	36.0000	78.0000	36.0000	78.0000	48.0000
SB14	Span 7	12.0000	48.0000	78.0000	48.0000	78.0000	60.0000
SB14	Span 8	12.0000	60.0000	78.0000	60.0000	78.0000	72.0000
SB14	Span 9	12.0000	72.0000	78.0000	72.0000	78.0000	84.0000
SB14	Span 10	12.0000	84.0000	78.0000	84.0000	78.0000	96.0000
SB14	Span 11	12.0000	96.0000	78.0000	96.0000	78.0000	108.0000
SB14	Span 12	2.3510	108.0000	78.0000	108.0000	78.0000	110.3510
SB14	Span 13	4.8190	110.3510	78.0000	110.3510	78.0000	115.1700
SB15	Span 1	0.3333	0.0000	84.0000	0.0000	84.0000	0.3333
SB15	Span 2	7.1707	0.3333	84.0000	0.3333	84.0000	7.5041
SB15	Span 3	4.4959	7.5041	84.0000	7.5041	84.0000	12.0000
SB15	Span 4	12.0000	12.0000	84.0000	12.0000	84.0000	24.0000
SB15	Span 5	9.0041	24.0000	84.0000	24.0000	84.0000	33.0041
SB15	Span 6	1.7101	33.0041	84.0000	33.0041	84.0000	34.7142
SB15	Span 7	1.2858	34.7142	84.0000	34.7142	84.0000	36.0000
SB15	Span 8	12.0000	36.0000	84.0000	36.0000	84.0000	48.0000
SB15	Span 9	12.0000	48.0000	84.0000	48.0000	84.0000	60.0000
SB15	Span 10	12.0000	60.0000	84.0000	60.0000	84.0000	72.0000
SB15	Span 11	12.0000	72.0000	84.0000	72.0000	84.0000	84.0000
SB15	Span 12	12.0000	84.0000	84.0000	84.0000	84.0000	96.0000
SB15	Span 13	12.0000	96.0000	84.0000	96.0000	84.0000	108.0000
SB15	Span 14	7.1700	108.0000	84.0000	108.0000	84.0000	115.1700
SB16	Span 1	12.0000	0.0000	90.0000	0.0000	90.0000	12.0000
SB16	Span 2	12.0000	12.0000	90.0000	12.0000	90.0000	24.0000
SB16	Span 3	9.0652	24.0000	90.0000	24.0000	90.0000	33.0652
SB16	Span 4	2.9348	33.0652	90.0000	33.0652	90.0000	36.0000
SB16	Span 5	12.0000	36.0000	90.0000	36.0000	90.0000	48.0000
SB16	Span 6	0.8493	48.0000	90.0000	48.0000	90.0000	48.8493
SB16	Span 7	11.1507	48.8493	90.0000	48.8493	90.0000	60.0000
SB16	Span 8	12.0000	60.0000	90.0000	60.0000	90.0000	72.0000
SB16	Span 9	12.0000	72.0000	90.0000	72.0000	90.0000	84.0000
SB16	Span 10	12.0000	84.0000	90.0000	84.0000	90.0000	96.0000
SB16	Span 11	12.0000	96.0000	90.0000	96.0000	90.0000	108.0000
SB16	Span 12	7.1700	108.0000	90.0000	108.0000	90.0000	115.1700
SB17	Span 1	0.3333	0.0000	96.0000	0.0000	96.0000	0.3333
SB17	Span 2	6.8374	0.3333	96.0000	0.3333	96.0000	7.1707
SB17	Span 3	0.3333	7.1707	96.0000	7.1707	96.0000	7.5041
SB17	Span 4	4.4959	7.5041	96.0000	7.5041	96.0000	12.0000
SB17	Span 5	12.0000	12.0000	96.0000	12.0000	96.0000	24.0000
SB17	Span 6	9.1198	24.0000	96.0000	24.0000	96.0000	33.1198
SB17	Span 7	2.8802	33.1198	96.0000	33.1198	96.0000	36.0000
SB17	Span 8	12.0000	36.0000	96.0000	36.0000	96.0000	48.0000
SB17	Span 9	11.0784	48.0000	96.0000	48.0000	96.0000	59.0784
SB17	Span 10	0.9216	59.0784	96.0000	59.0784	96.0000	60.0000
SB17	Span 11	2.9844	60.0000	96.0000	60.0000	96.0000	62.9844
SB17	Span 12	9.0156	62.9844	96.0000	62.9844	96.0000	72.0000
SB17	Span 13	8.2559	72.0000	96.0000	72.0000	96.0000	80.2559
SB17	Span 14	0.6775	80.2559	96.0000	80.2559	96.0000	80.9335
SB17	Span 15	3.0665	80.9335	96.0000	80.9335	96.0000	84.0000
SB17	Span 16	12.0000	84.0000	96.0000	84.0000	96.0000	96.0000
SB17	Span 17	12.0000	96.0000	96.0000	96.0000	96.0000	108.0000

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SB17	Span 18	7.1700	108.0000	96.0000	108.0000	96.0000	115.1700
SB18	Span 1	7.4674	0.0000	102.0000	0.0000	102.0000	7.4674
SB18	Span 2	4.5326	7.4674	102.0000	7.4674	102.0000	12.0000
SB18	Span 3	12.0000	12.0000	102.0000	12.0000	102.0000	24.0000
SB18	Span 4	8.5041	24.0000	102.0000	24.0000	102.0000	32.5041
SB18	Span 5	0.6667	32.5041	102.0000	32.5041	102.0000	33.1707
SB18	Span 6	2.8293	33.1707	102.0000	33.1707	102.0000	36.0000
SB18	Span 7	12.0000	36.0000	102.0000	36.0000	102.0000	48.0000
SB18	Span 8	11.0114	48.0000	102.0000	48.0000	102.0000	59.0114
SB18	Span 9	0.9886	59.0114	102.0000	59.0114	102.0000	60.0000
SB18	Span 10	12.0000	60.0000	102.0000	60.0000	102.0000	72.0000
SB18	Span 11	5.1195	72.0000	102.0000	72.0000	102.0000	77.1195
SB18	Span 12	3.1305	77.1195	102.0000	77.1195	102.0000	80.2500
SB18	Span 13	3.7500	80.2500	102.0000	80.2500	102.0000	84.0000
SB18	Span 14	12.0000	84.0000	102.0000	84.0000	102.0000	96.0000
SB18	Span 15	12.0000	96.0000	102.0000	96.0000	102.0000	108.0000
SB18	Span 16	7.1700	108.0000	102.0000	108.0000	102.0000	115.1700
SB19	Span 1	7.4465	0.0000	108.0000	0.0000	108.0000	7.4465
SB19	Span 2	0.2480	7.4465	108.0000	7.4465	108.0000	7.6945
SB19	Span 3	4.3055	7.6945	108.0000	7.6945	108.0000	12.0000
SB19	Span 4	12.0000	12.0000	108.0000	12.0000	108.0000	24.0000
SB19	Span 5	9.1707	24.0000	108.0000	24.0000	108.0000	33.1707
SB19	Span 6	2.8293	33.1707	108.0000	33.1707	108.0000	36.0000
SB19	Span 7	1.2541	36.0000	108.0000	36.0000	108.0000	37.2541
SB19	Span 8	6.3333	37.2541	108.0000	37.2541	108.0000	43.5874
SB19	Span 9	3.7500	43.5874	108.0000	43.5874	108.0000	47.3374
SB19	Span 10	0.6626	47.3374	108.0000	47.3374	108.0000	48.0000
SB19	Span 11	3.0874	48.0000	108.0000	48.0000	108.0000	51.0874
SB19	Span 12	6.3571	51.0874	108.0000	51.0874	108.0000	57.4445
SB19	Span 13	1.5143	57.4445	108.0000	57.4445	108.0000	58.9588
SB19	Span 14	1.0412	58.9588	108.0000	58.9588	108.0000	60.0000
SB19	Span 15	3.0576	60.0000	108.0000	60.0000	108.0000	63.0576
SB19	Span 16	5.6131	63.0576	108.0000	63.0576	108.0000	68.6707
SB19	Span 17	3.3293	68.6707	108.0000	68.6707	108.0000	72.0000
SB19	Span 18	2.4617	72.0000	108.0000	72.0000	108.0000	74.4617
SB19	Span 19	5.7910	74.4617	108.0000	74.4617	108.0000	80.2526
SB19	Span 20	3.7474	80.2526	108.0000	80.2526	108.0000	84.0000
SB19	Span 21	12.0000	84.0000	108.0000	84.0000	108.0000	96.0000
SB19	Span 22	12.0000	96.0000	108.0000	96.0000	108.0000	108.0000
SB19	Span 23	2.3374	108.0000	108.0000	108.0000	108.0000	110.3374
SB19	Span 24	4.8326	110.3374	108.0000	110.3374	108.0000	115.1700
SB20	Span 1	7.4465	0.0000	114.0000	0.0000	114.0000	7.4465
SB20	Span 2	4.5535	7.4465	114.0000	7.4465	114.0000	12.0000
SB20	Span 3	12.0000	12.0000	114.0000	12.0000	114.0000	24.0000
SB20	Span 4	12.0000	24.0000	114.0000	24.0000	114.0000	36.0000
SB20	Span 5	12.0000	36.0000	114.0000	36.0000	114.0000	48.0000
SB20	Span 6	10.9063	48.0000	114.0000	48.0000	114.0000	58.9063
SB20	Span 7	1.0937	58.9063	114.0000	58.9063	114.0000	60.0000
SB20	Span 8	12.0000	60.0000	114.0000	60.0000	114.0000	72.0000
SB20	Span 9	8.2500	72.0000	114.0000	72.0000	114.0000	80.2500
SB20	Span 10	3.7500	80.2500	114.0000	80.2500	114.0000	84.0000
SB20	Span 11	12.0000	84.0000	114.0000	84.0000	114.0000	96.0000
SB20	Span 12	12.0000	96.0000	114.0000	96.0000	114.0000	108.0000
SB20	Span 13	2.3374	108.0000	114.0000	108.0000	114.0000	110.3374

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SB20	Span 14	4.8326	110.3374	114.0000	110.3374	114.0000	115.1700
SB21	Span 1	7.4465	0.0000	120.0000	0.0000	120.0000	7.4465
SB21	Span 2	0.2480	7.4465	120.0000	7.4465	120.0000	7.6945
SB21	Span 3	4.3055	7.6945	120.0000	7.6945	120.0000	12.0000
SB21	Span 4	12.0000	12.0000	120.0000	12.0000	120.0000	24.0000
SB21	Span 5	7.8374	24.0000	120.0000	24.0000	120.0000	31.8374
SB21	Span 6	0.6667	31.8374	120.0000	31.8374	120.0000	32.5041
SB21	Span 7	0.6646	32.5041	120.0000	32.5041	120.0000	33.1687
SB21	Span 8	2.8313	33.1687	120.0000	33.1687	120.0000	36.0000
SB21	Span 9	12.0000	36.0000	120.0000	36.0000	120.0000	48.0000
SB21	Span 10	10.8668	48.0000	120.0000	48.0000	120.0000	58.8668
SB21	Span 11	1.1332	58.8668	120.0000	58.8668	120.0000	60.0000
SB21	Span 12	6.3669	60.0000	120.0000	60.0000	120.0000	66.3669
SB21	Span 13	5.6331	66.3669	120.0000	66.3669	120.0000	72.0000
SB21	Span 14	7.9167	72.0000	120.0000	72.0000	120.0000	79.9167
SB21	Span 15	0.3333	79.9167	120.0000	79.9167	120.0000	80.2500
SB21	Span 16	3.7500	80.2500	120.0000	80.2500	120.0000	84.0000
SB21	Span 17	5.2538	84.0000	120.0000	84.0000	120.0000	89.2538
SB21	Span 18	0.4981	89.2538	120.0000	89.2538	120.0000	89.7519
SB21	Span 19	6.2481	89.7519	120.0000	89.7519	120.0000	96.0000
SB21	Span 20	12.0000	96.0000	120.0000	96.0000	120.0000	108.0000
SB21	Span 21	2.3374	108.0000	120.0000	108.0000	120.0000	110.3374
SB21	Span 22	4.8326	110.3374	120.0000	110.3374	120.0000	115.1700
SB22	Span 1	7.5094	0.0000	126.0000	0.0000	126.0000	7.5094
SB22	Span 2	4.4906	7.5094	126.0000	7.5094	126.0000	12.0000
SB22	Span 3	12.0000	12.0000	126.0000	12.0000	126.0000	24.0000
SB22	Span 4	7.8374	24.0000	126.0000	24.0000	126.0000	31.8374
SB22	Span 5	4.1626	31.8374	126.0000	31.8374	126.0000	36.0000
SB22	Span 6	12.0000	36.0000	126.0000	36.0000	126.0000	48.0000
SB22	Span 7	12.0000	48.0000	126.0000	48.0000	126.0000	60.0000
SB22	Span 8	12.0000	60.0000	126.0000	60.0000	126.0000	72.0000
SB22	Span 9	12.0000	72.0000	126.0000	72.0000	126.0000	84.0000
SB22	Span 10	12.0000	84.0000	126.0000	84.0000	126.0000	96.0000
SB22	Span 11	12.0000	96.0000	126.0000	96.0000	126.0000	108.0000
SB22	Span 12	7.1700	108.0000	126.0000	108.0000	126.0000	115.1700
SB23	Span 1	0.3333	0.0000	132.0000	0.0000	132.0000	0.3333
SB23	Span 2	6.9616	0.3333	132.0000	0.3333	132.0000	7.2949
SB23	Span 3	0.2735	7.2949	132.0000	7.2949	132.0000	7.5684
SB23	Span 4	1.5697	7.5684	132.0000	7.5684	132.0000	9.1381
SB23	Span 5	2.8619	9.1381	132.0000	9.1381	132.0000	12.0000
SB23	Span 6	12.0000	12.0000	132.0000	12.0000	132.0000	24.0000
SB23	Span 7	7.8374	24.0000	132.0000	24.0000	132.0000	31.8374
SB23	Span 8	0.1667	31.8374	132.0000	31.8374	132.0000	32.0041
SB23	Span 9	1.1626	32.0041	132.0000	32.0041	132.0000	33.1667
SB23	Span 10	2.8333	33.1667	132.0000	33.1667	132.0000	36.0000
SB23	Span 11	12.0000	36.0000	132.0000	36.0000	132.0000	48.0000
SB23	Span 12	12.0000	48.0000	132.0000	48.0000	132.0000	60.0000
SB23	Span 13	12.0000	60.0000	132.0000	60.0000	132.0000	72.0000
SB23	Span 14	12.0000	72.0000	132.0000	72.0000	132.0000	84.0000
SB23	Span 15	12.0000	84.0000	132.0000	84.0000	132.0000	96.0000
SB23	Span 16	12.0000	96.0000	132.0000	96.0000	132.0000	108.0000
SB23	Span 17	7.1700	108.0000	132.0000	108.0000	132.0000	115.1700
SB24	Span 1	0.3333	0.0000	138.0000	0.0000	138.0000	0.3333
SB24	Span 2	7.3374	0.3333	138.0000	0.3333	138.0000	7.6707

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SB24	Span 3	4.3293	7.6707	138.0000	7.6707	138.0000	12.0000
SB24	Span 4	12.0000	12.0000	138.0000	12.0000	138.0000	24.0000
SB24	Span 5	9.1343	24.0000	138.0000	24.0000	138.0000	33.1343
SB24	Span 6	2.8657	33.1343	138.0000	33.1343	138.0000	36.0000
SB24	Span 7	12.0000	36.0000	138.0000	36.0000	138.0000	48.0000
SB24	Span 8	12.0000	48.0000	138.0000	48.0000	138.0000	60.0000
SB24	Span 9	12.0000	60.0000	138.0000	60.0000	138.0000	72.0000
SB24	Span 10	12.0000	72.0000	138.0000	72.0000	138.0000	84.0000
SB24	Span 11	12.0000	84.0000	138.0000	84.0000	138.0000	96.0000
SB24	Span 12	12.0000	96.0000	138.0000	96.0000	138.0000	108.0000
SB24	Span 13	7.1700	108.0000	138.0000	108.0000	138.0000	115.1700
SB25	Span 1	12.0000	0.0000	144.0000	0.0000	144.0000	12.0000
SB25	Span 2	12.0000	12.0000	144.0000	12.0000	144.0000	24.0000
SB25	Span 3	9.0999	24.0000	144.0000	24.0000	144.0000	33.0999
SB25	Span 4	2.9001	33.0999	144.0000	33.0999	144.0000	36.0000
SB25	Span 5	12.0000	36.0000	144.0000	36.0000	144.0000	48.0000
SB25	Span 6	12.0000	48.0000	144.0000	48.0000	144.0000	60.0000
SB25	Span 7	12.0000	60.0000	144.0000	60.0000	144.0000	72.0000
SB25	Span 8	12.0000	72.0000	144.0000	72.0000	144.0000	84.0000
SB25	Span 9	12.0000	84.0000	144.0000	84.0000	144.0000	96.0000
SB25	Span 10	12.0000	96.0000	144.0000	96.0000	144.0000	108.0000
SB25	Span 11	7.1700	108.0000	144.0000	108.0000	144.0000	115.1700
SB26	Span 1	7.6707	0.0000	150.0000	0.0000	150.0000	7.6707
SB26	Span 2	4.3293	7.6707	150.0000	7.6707	150.0000	12.0000
SB26	Span 3	12.0000	12.0000	150.0000	12.0000	150.0000	24.0000
SB26	Span 4	8.0278	24.0000	150.0000	24.0000	150.0000	32.0278
SB26	Span 5	3.9722	32.0278	150.0000	32.0278	150.0000	36.0000
SB26	Span 6	12.0000	36.0000	150.0000	36.0000	150.0000	48.0000
SB26	Span 7	12.0000	48.0000	150.0000	48.0000	150.0000	60.0000
SB26	Span 8	12.0000	60.0000	150.0000	60.0000	150.0000	72.0000
SB26	Span 9	12.0000	72.0000	150.0000	72.0000	150.0000	84.0000
SB26	Span 10	12.0000	84.0000	150.0000	84.0000	150.0000	96.0000
SB26	Span 11	12.0000	96.0000	150.0000	96.0000	150.0000	108.0000
SB26	Span 12	7.1700	108.0000	150.0000	108.0000	150.0000	115.1700
SB27	Span 1	12.0000	0.0000	156.0000	0.0000	156.0000	12.0000
SB27	Span 2	12.0000	12.0000	156.0000	12.0000	156.0000	24.0000
SB27	Span 3	8.0278	24.0000	156.0000	24.0000	156.0000	32.0278
SB27	Span 4	3.9722	32.0278	156.0000	32.0278	156.0000	36.0000
SB27	Span 5	12.0000	36.0000	156.0000	36.0000	156.0000	48.0000
SB27	Span 6	12.0000	48.0000	156.0000	48.0000	156.0000	60.0000
SB27	Span 7	12.0000	60.0000	156.0000	60.0000	156.0000	72.0000
SB27	Span 8	12.0000	72.0000	156.0000	72.0000	156.0000	84.0000
SB27	Span 9	12.0000	84.0000	156.0000	84.0000	156.0000	96.0000
SB27	Span 10	12.0000	96.0000	156.0000	96.0000	156.0000	108.0000
SB27	Span 11	7.1700	108.0000	156.0000	108.0000	156.0000	115.1700
SB28	Span 1	7.6683	0.0000	162.0000	0.0000	162.0000	7.6683
SB28	Span 2	4.3317	7.6683	162.0000	7.6683	162.0000	12.0000
SB28	Span 3	12.0000	12.0000	162.0000	12.0000	162.0000	24.0000
SB28	Span 4	8.0278	24.0000	162.0000	24.0000	162.0000	32.0278
SB28	Span 5	3.9722	32.0278	162.0000	32.0278	162.0000	36.0000
SB28	Span 6	12.0000	36.0000	162.0000	36.0000	162.0000	48.0000
SB28	Span 7	12.0000	48.0000	162.0000	48.0000	162.0000	60.0000
SB28	Span 8	12.0000	60.0000	162.0000	60.0000	162.0000	72.0000
SB28	Span 9	12.0000	72.0000	162.0000	72.0000	162.0000	84.0000

Table 1: Concrete Slab Design Summary 02 - Span Definition Data

Strip	SpanID	SpanLength	StartDist	GlobalX1	GlobalY1	GlobalX2	GlobalY2
		ft	ft	ft	ft	ft	ft
SB28	Span 10	12.0000	84.0000	162.0000	84.0000	162.0000	96.0000
SB28	Span 11	12.0000	96.0000	162.0000	96.0000	162.0000	108.0000
SB28	Span 12	7.1700	108.0000	162.0000	108.0000	162.0000	115.1700
SB29	Span 1	0.6536	0.0000	168.0000	0.0000	168.0000	0.6536
SB29	Span 2	6.6335	0.6536	168.0000	0.6536	168.0000	7.2871
SB29	Span 3	4.7129	7.2871	168.0000	7.2871	168.0000	12.0000
SB29	Span 4	12.0000	12.0000	168.0000	12.0000	168.0000	24.0000
SB29	Span 5	12.0000	24.0000	168.0000	24.0000	168.0000	36.0000
SB29	Span 6	12.0000	36.0000	168.0000	36.0000	168.0000	48.0000
SB29	Span 7	12.0000	48.0000	168.0000	48.0000	168.0000	60.0000
SB29	Span 8	12.0000	60.0000	168.0000	60.0000	168.0000	72.0000
SB29	Span 9	12.0000	72.0000	168.0000	72.0000	168.0000	84.0000
SB29	Span 10	12.0000	84.0000	168.0000	84.0000	168.0000	96.0000
SB29	Span 11	12.0000	96.0000	168.0000	96.0000	168.0000	108.0000
SB29	Span 12	7.1700	108.0000	168.0000	108.0000	168.0000	115.1700
SB30	Span 1	12.0000	0.0000	172.5000	0.0000	172.5000	12.0000
SB30	Span 2	12.0000	12.0000	172.5000	12.0000	172.5000	24.0000
SB30	Span 3	12.0000	24.0000	172.5000	24.0000	172.5000	36.0000
SB30	Span 4	12.0000	36.0000	172.5000	36.0000	172.5000	48.0000
SB30	Span 5	12.0000	48.0000	172.5000	48.0000	172.5000	60.0000
SB30	Span 6	12.0000	60.0000	172.5000	60.0000	172.5000	72.0000
SB30	Span 7	12.0000	72.0000	172.5000	72.0000	172.5000	84.0000
SB30	Span 8	12.0000	84.0000	172.5000	84.0000	172.5000	96.0000
SB30	Span 9	12.0000	96.0000	172.5000	96.0000	172.5000	108.0000
SB30	Span 10	7.1700	108.0000	172.5000	108.0000	172.5000	115.1700
SB31	Span 1	7.3325	0.3333	177.0000	0.3333	177.0000	7.6658
SB31	Span 2	4.3342	7.6658	177.0000	7.6658	177.0000	12.0000
SB31	Span 3	12.0000	12.0000	177.0000	12.0000	177.0000	24.0000
SB31	Span 4	12.0000	24.0000	177.0000	24.0000	177.0000	36.0000
SB31	Span 5	12.0000	36.0000	177.0000	36.0000	177.0000	48.0000
SB31	Span 6	12.0000	48.0000	177.0000	48.0000	177.0000	60.0000
SB31	Span 7	12.0000	60.0000	177.0000	60.0000	177.0000	72.0000
SB31	Span 8	12.0000	72.0000	177.0000	72.0000	177.0000	84.0000
SB31	Span 9	12.0000	84.0000	177.0000	84.0000	177.0000	96.0000
SB31	Span 10	12.0000	96.0000	177.0000	96.0000	177.0000	108.0000
SB31	Span 11	7.1700	108.0000	177.0000	108.0000	177.0000	115.1700

2. Model properties

This section provides model properties, including items such as material properties, section properties, and support properties.

2.1. Material properties

Table 2: Material Properties 03 - Concrete

Table 2: Material Properties 03 - Concrete

Material	E	U	A	UnitWt	Fc	LtWtConc	UserModRup
	kip/in2		1/F	lb/ft3	kip/in2		
4000Psi	3604.997	0.200000	5.5000E-06	1.5000E+02	4.000	No	No
5000Psi	4030.509	0.200000	5.5000E-06	1.5000E+02	5.000	No	No

Table 3: Material Properties 04 - Rebar

Table 3: Material Properties 04 - Rebar

Material	E	UnitWt	Fy	Fu
	kip/in2	lb/ft3	kip/in2	kip/in2
A615Gr60	29000.000	4.9000E+02	60.000	90.000

Table 4: Material Properties 05 - Tendon

Table 4: Material Properties 05 - Tendon

Material	E	UnitWt	Fy	Fu
	kip/in2	lb/ft3	kip/in2	kip/in2
A416Gr270	28500.000	4.9000E+02	245.100	270.000

Table 5: Material Properties 02 - Steel

Table 5: Material Properties 02 - Steel

Material	E	U	A	UnitWt	Fy	Fu
	kip/in2		1/F	lb/ft3	kip/in2	kip/in2
A500GrB46	29000.000	0.300000	6.5000E-06	4.9000E+02	46.000	58.000

Table 6: Material Properties 06 - Other

Table 6: Material Properties 06 - Other

Material	E	U	A	UnitWt
	kip/in2		1/F	lb/ft3
CMU	1500.000	0.200000	4.5000E-06	1.3000E+02

2.2. Section properties

Table 7: Slab Properties 02 - Solid Slabs

Table 7: Slab Properties 02 - Solid Slabs

Slab	Type	MatProp	Thickness	Ortho
			in	
10" SLAB	Slab	5000Psi	10.0000	No
10" SLAB (TERR.)	Slab	5000Psi	10.0000	No

Table 8: Beam Properties 02 - Rectangular Beam

Table 8: Beam Properties 02 - Rectangular Beam

Beam	MatProp	Depth	WidthTop	WidthBot
		in	in	in
2B-1	5000Psi	20.0000	8.0000	8.0000
2B-10	5000Psi	24.0000	16.0000	16.0000
2B-11	5000Psi	24.0000	16.0000	16.0000
2B-12	5000Psi	24.0000	8.0000	8.0000
2B-13	5000Psi	24.0000	8.0000	8.0000
2B-14	5000Psi	30.0000	8.0000	8.0000
2B-15	5000Psi	30.0000	8.0000	8.0000
2B-16	5000Psi	30.0000	8.0000	8.0000
2B-17	5000Psi	30.0000	8.0000	8.0000
2B-18	5000Psi	30.0000	12.0000	12.0000
2B-19	5000Psi	30.0000	8.0000	8.0000
2B-2	5000Psi	28.0000	8.0000	8.0000
2B-20	5000Psi	30.0000	8.0000	8.0000
2B-21	5000Psi	26.0000	8.0000	8.0000
2B-22	5000Psi	24.0000	8.0000	8.0000
2B-23	5000Psi	20.0000	8.0000	8.0000
2B-3	5000Psi	36.0000	12.0000	12.0000
2B-4	5000Psi	48.0000	12.0000	12.0000
2B-5	5000Psi	24.0000	8.0000	8.0000
2B-6	5000Psi	30.0000	12.0000	12.0000
2B-7	5000Psi	30.0000	8.0000	8.0000
2B-8	5000Psi	30.0000	16.0000	16.0000
2B-9	5000Psi	24.0000	8.0000	8.0000

Table 9: Beam Properties 06 - Design Data

Table 9: Beam Properties 06 - Design Data

Beam	MatRebarL	MatRebarS	FlngWOpt	CoverTop	CoverBot	NoDesign
				in	in	
2B-1	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
2B-10	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
2B-11	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
2B-12	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No

Table 9: Beam Properties 06 - Design Data

Beam	MatRebarL	MatRebarS	FlngWOpt	CoverTop	CoverBot	NoDesign
				in	in	
2B-13	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
2B-14	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
2B-15	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
2B-16	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
2B-17	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
2B-18	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
2B-19	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
2B-2	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
2B-20	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
2B-21	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
2B-22	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
2B-23	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
2B-3	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
2B-4	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
2B-5	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
2B-6	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
2B-7	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
2B-8	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No
2B-9	A615Gr60	A615Gr60	Analysis Property	3.0000	3.0000	No

Table 10: Tendon Properties

Table 10: Tendon Properties

TendonProp	MatProp	StrandArea
		in2
TENDON1	A416Gr270	0.15

Table 11: Column Properties 02 - Rectangular

Table 11: Column Properties 02 - Rectangular

Column	MatProp	SecDim2	SecDim3	AutoRigid	AutoDrop	IncludeCap
		in	in			
HSS-1	A500GrB46	8.0000	8.0000	No	No	No
HSS-2	A500GrB46	6.0000	6.0000	No	No	No
HSS-3	A500GrB46	8.0000	4.0000	No	No	No

Table 11: Column Properties 02 - Rectangular

Column	MatProp	SecDim2	SecDim3	AutoRigid	AutoDrop	IncludeCap
		in	in			
TC-1	5000Psi	8.0000	12.0000	No	No	No
TC-2	5000Psi	8.0000	16.0000	No	No	No
TC-3	5000Psi	8.0000	24.0000	No	No	No
TC-4	5000Psi	8.0000	44.0000	No	No	No
TC-5	5000Psi	8.0000	34.0000	No	No	No

Table 12: Column Properties 03 - Circular

Table 12: Column Properties 03 - Circular

Column	MatProp	Diameter	AutoRigid	AutoDrop	IncludeCap
		in			
COL1	5000Psi	16.0000	No	No	No

Table 13: Wall Properties

Table 13: Wall Properties

Wall	MatProp	Thickness	AutoRigid	OutOfPlane
		in		
8" CMU	CMU	8.0000	No	No

2.3. Support properties

Table 14: Soil Properties

Table 14: Soil Properties

Soil	Subgrade	NonlinOpt
	lb/ft3	
SOIL1	1.7280E+05	Compression Only

Table 15: Spring Properties - Point

Table 15: Spring Properties - Point

Spring	Ux	Uy	Uz	Rx	Ry	Rz	NonlinOpt
	kip/in	kip/in	kip/in	kip-in/rad	kip-in/rad	kip-in/rad	
PSPR1	0.0000	0.0000	0.0010	0.000	0.000	0.000	None (Linear)

Table 16: Spring Properties - Line

Table 16: Spring Properties - Line

Spring	VertStiff kip/in/in	RotStiff kip/rad	NonlinOpt
LSPR1	0.00100	0.0010	None (Linear)

3. Model assignments

This section provides model assignments, including assignments to slabs, beams, and joints.

3.1. Slab assignments

Table 17: Slab Property Assignments

Table 17: Slab Property Assignments

Area	SlabProp	OpeningType
4	None	None
26	None	None
44	10" SLAB	None
45	None	Unloaded
46	None	Unloaded
47	None	Unloaded
48	None	None
51	10" SLAB (TERR.)	None
52	None	Unloaded

Table 18: Slab Edge Releases

Table 18: Slab Edge Releases

Area	Edge	Shear	Moment
44	1	No	Yes
44	2	No	Yes
44	3	No	Yes
44	4	No	Yes
44	5	No	Yes
44	6	No	Yes
44	7	No	Yes
44	8	No	Yes
44	9	No	Yes
44	10	No	Yes
51	1	No	Yes
51	2	No	Yes
51	3	No	Yes
51	4	No	Yes
51	5	No	Yes
51	6	No	Yes
51	7	No	Yes
51	8	No	Yes
51	9	No	Yes
51	10	No	Yes
51	11	No	Yes
51	12	No	Yes
51	13	No	Yes
51	14	No	Yes
51	15	No	Yes
51	16	No	Yes
51	17	No	Yes

Table 18: Slab Edge Releases

Area	Edge	Shear	Moment
51	18	No	Yes
51	19	No	Yes
51	20	No	Yes
51	21	No	Yes

3.2. Beam assignments

Table 19: Beam Property Assignments

Table 19: Beam Property Assignments

Line	BeamProp
32	2B-21
63	2B-1
64	2B-1
65	2B-1
66	2B-2
67	2B-3
68	2B-4
69	2B-4
70	2B-5
71	2B-5
72	2B-6
73	2B-6
74	2B-7
75	2B-7
76	2B-7
77	2B-8
78	2B-9
79	2B-10
80	2B-10
81	2B-11
82	2B-12
83	2B-13
84	2B-13
85	2B-13
86	2B-13
87	2B-13
88	2B-13
89	2B-14
90	2B-14
91	2B-15
92	2B-15
93	2B-15
94	2B-15
95	2B-15
96	2B-16
97	2B-16
98	2B-16
99	2B-17

Table 19: Beam Property Assignments

Line	BeamProp
100	2B-19
101	2B-18
102	2B-20
103	2B-22
104	2B-22
105	2B-22
119	2B-23
120	2B-23
121	2B-23

3.3. Column assignments

Table 20: Column Property Assignments

Table 20: Column Property Assignments

Line	ColProp
1	COL1
2	COL1
3	COL1
8	TC-2
9	TC-2
10	TC-4
12	TC-3
13	TC-3
15	TC-1
16	TC-1
17	TC-1
18	TC-1
21	TC-2
22	TC-2
23	TC-2
24	TC-2
25	TC-2
26	TC-2
27	TC-2
28	TC-2
29	TC-2
30	TC-2
31	TC-1
34	TC-5
35	TC-5
36	TC-1
37	TC-1
38	TC-1
39	TC-1
40	TC-1
41	TC-1
42	HSS-1

Table 20: Column Property Assignments

Line	ColProp
43	TC-1
44	TC-1
47	TC-3
48	HSS-1
49	HSS-2
50	HSS-2
51	HSS-3
52	HSS-3
53	HSS-3
54	HSS-3
55	HSS-3
56	HSS-3
57	HSS-3
58	HSS-2
59	HSS-1
60	HSS-2
61	HSS-2
62	HSS-2
109	HSS-3
110	HSS-3
111	HSS-3

3.4. Wall assignments

Table 21: Wall Property Assignments

Table 21: Wall Property Assignments

Area	WallProp
1	8" CMU
2	8" CMU
3	8" CMU
5	8" CMU
6	8" CMU
7	8" CMU
8	8" CMU
9	8" CMU
10	8" CMU
11	8" CMU
12	8" CMU
13	8" CMU
14	8" CMU
15	8" CMU
16	8" CMU
17	8" CMU
18	8" CMU
19	8" CMU
20	8" CMU
21	8" CMU

Table 21: Wall Property Assignments

Area	WallProp
22	8" CMU
23	8" CMU
24	8" CMU
25	8" CMU
27	8" CMU
28	8" CMU
29	8" CMU
30	8" CMU
31	8" CMU
32	8" CMU
33	8" CMU
34	8" CMU
35	8" CMU
36	8" CMU
37	8" CMU
38	8" CMU
39	8" CMU
40	8" CMU
41	8" CMU
42	8" CMU

3.5. Support assignments

Table 22: Point Restraint Assignments

Table 22: Point Restraint Assignments

Point	Ux	Uy	Uz	Rx	Ry	Rz
1	Yes	Yes	Yes	Yes	Yes	Yes
3	Yes	Yes	Yes	Yes	Yes	Yes
5	Yes	Yes	Yes	Yes	Yes	Yes
15	Yes	Yes	Yes	Yes	Yes	Yes
17	Yes	Yes	Yes	Yes	Yes	Yes
19	Yes	Yes	Yes	Yes	Yes	Yes
23	Yes	Yes	Yes	Yes	Yes	Yes
25	Yes	Yes	Yes	Yes	Yes	Yes
29	Yes	Yes	Yes	Yes	Yes	Yes
31	Yes	Yes	Yes	Yes	Yes	Yes
33	Yes	Yes	Yes	Yes	Yes	Yes
35	Yes	Yes	Yes	Yes	Yes	Yes
39	Yes	Yes	Yes	Yes	Yes	Yes
41	Yes	Yes	Yes	Yes	Yes	Yes
43	Yes	Yes	Yes	Yes	Yes	Yes
45	Yes	Yes	Yes	Yes	Yes	Yes
47	Yes	Yes	Yes	Yes	Yes	Yes
49	Yes	Yes	Yes	Yes	Yes	Yes
51	Yes	Yes	Yes	Yes	Yes	Yes
53	Yes	Yes	Yes	Yes	Yes	Yes
55	Yes	Yes	Yes	Yes	Yes	Yes
57	Yes	Yes	Yes	Yes	Yes	Yes

Table 22: Point Restraint Assignments

Point	Ux	Uy	Uz	Rx	Ry	Rz
59	Yes	Yes	Yes	Yes	Yes	Yes
65	Yes	Yes	Yes	Yes	Yes	Yes
67	Yes	Yes	Yes	Yes	Yes	Yes
69	Yes	Yes	Yes	Yes	Yes	Yes
71	Yes	Yes	Yes	Yes	Yes	Yes
73	Yes	Yes	Yes	Yes	Yes	Yes
75	Yes	Yes	Yes	Yes	Yes	Yes
77	Yes	Yes	Yes	Yes	Yes	Yes
79	Yes	Yes	Yes	Yes	Yes	Yes
81	Yes	Yes	Yes	Yes	Yes	Yes
83	Yes	Yes	Yes	Yes	Yes	Yes
85	Yes	Yes	Yes	Yes	Yes	Yes
91	Yes	Yes	Yes	Yes	Yes	Yes
93	Yes	Yes	Yes	Yes	Yes	Yes
95	Yes	Yes	Yes	Yes	Yes	Yes
97	Yes	Yes	Yes	Yes	Yes	Yes
99	Yes	Yes	Yes	Yes	Yes	Yes
101	Yes	Yes	Yes	Yes	Yes	Yes
103	Yes	Yes	Yes	Yes	Yes	Yes
105	Yes	Yes	Yes	Yes	Yes	Yes
107	Yes	Yes	Yes	Yes	Yes	Yes
109	Yes	Yes	Yes	Yes	Yes	Yes
111	Yes	Yes	Yes	Yes	Yes	Yes
113	Yes	Yes	Yes	Yes	Yes	Yes
115	Yes	Yes	Yes	Yes	Yes	Yes
117	Yes	Yes	Yes	Yes	Yes	Yes
119	Yes	Yes	Yes	Yes	Yes	Yes
121	Yes	Yes	Yes	Yes	Yes	Yes
123	Yes	Yes	Yes	Yes	Yes	Yes
124	Yes	Yes	Yes	Yes	Yes	Yes
127	Yes	Yes	Yes	Yes	Yes	Yes
128	Yes	Yes	Yes	Yes	Yes	Yes
131	Yes	Yes	Yes	Yes	Yes	Yes
132	Yes	Yes	Yes	Yes	Yes	Yes
139	Yes	Yes	Yes	Yes	Yes	Yes
140	Yes	Yes	Yes	Yes	Yes	Yes
143	Yes	Yes	Yes	Yes	Yes	Yes
144	Yes	Yes	Yes	Yes	Yes	Yes
147	Yes	Yes	Yes	Yes	Yes	Yes
148	Yes	Yes	Yes	Yes	Yes	Yes
151	Yes	Yes	Yes	Yes	Yes	Yes
153	Yes	Yes	Yes	Yes	Yes	Yes
154	Yes	Yes	Yes	Yes	Yes	Yes
157	Yes	Yes	Yes	Yes	Yes	Yes
159	Yes	Yes	Yes	Yes	Yes	Yes
161	Yes	Yes	Yes	Yes	Yes	Yes
162	Yes	Yes	Yes	Yes	Yes	Yes
165	Yes	Yes	Yes	Yes	Yes	Yes
166	Yes	Yes	Yes	Yes	Yes	Yes
169	Yes	Yes	Yes	Yes	Yes	Yes
170	Yes	Yes	Yes	Yes	Yes	Yes
173	Yes	Yes	Yes	Yes	Yes	Yes
174	Yes	Yes	Yes	Yes	Yes	Yes
177	Yes	Yes	Yes	Yes	Yes	Yes

Table 22: Point Restraint Assignments

Point	Ux	Uy	Uz	Rx	Ry	Rz
179	Yes	Yes	Yes	Yes	Yes	Yes
180	Yes	Yes	Yes	Yes	Yes	Yes
183	Yes	Yes	Yes	Yes	Yes	Yes
184	Yes	Yes	Yes	Yes	Yes	Yes
187	Yes	Yes	Yes	Yes	Yes	Yes
189	Yes	Yes	Yes	Yes	Yes	Yes
191	Yes	Yes	Yes	Yes	Yes	Yes
193	Yes	Yes	Yes	Yes	Yes	Yes
194	Yes	Yes	Yes	Yes	Yes	Yes
197	Yes	Yes	Yes	Yes	Yes	Yes
199	Yes	Yes	Yes	Yes	Yes	Yes
201	Yes	Yes	Yes	Yes	Yes	Yes
202	Yes	Yes	Yes	Yes	Yes	Yes
209	Yes	Yes	Yes	Yes	Yes	Yes
211	Yes	Yes	Yes	Yes	Yes	Yes
213	Yes	Yes	Yes	Yes	Yes	Yes
215	Yes	Yes	Yes	Yes	Yes	Yes
217	Yes	Yes	Yes	Yes	Yes	Yes
218	Yes	Yes	Yes	Yes	Yes	Yes
221	Yes	Yes	Yes	Yes	Yes	Yes
223	Yes	Yes	Yes	Yes	Yes	Yes
224	Yes	Yes	Yes	Yes	Yes	Yes
227	Yes	Yes	Yes	Yes	Yes	Yes
229	Yes	Yes	Yes	Yes	Yes	Yes
231	Yes	Yes	Yes	Yes	Yes	Yes
232	Yes	Yes	Yes	Yes	Yes	Yes
235	Yes	Yes	Yes	Yes	Yes	Yes
236	Yes	Yes	Yes	Yes	Yes	Yes
239	Yes	Yes	Yes	Yes	Yes	Yes
240	Yes	Yes	Yes	Yes	Yes	Yes
243	Yes	Yes	Yes	Yes	Yes	Yes
245	Yes	Yes	Yes	Yes	Yes	Yes
246	Yes	Yes	Yes	Yes	Yes	Yes
249	Yes	Yes	Yes	Yes	Yes	Yes
250	Yes	Yes	Yes	Yes	Yes	Yes
253	Yes	Yes	Yes	Yes	Yes	Yes
308	Yes	Yes	Yes	Yes	Yes	Yes
310	Yes	Yes	Yes	Yes	Yes	Yes
316	Yes	Yes	Yes	Yes	Yes	Yes

4. Model loading

This section provides model loading information, including load patterns, load cases, and load combinations.

4.1. Load patterns

Table 23: Load Patterns

Table 23: Load Patterns

LoadPat	Type	SelfWtMult
DEAD	DEAD	1.000000
LIVE	LIVE	0.000000
SDL	SUPER DEAD	0.000000

Table 24: Load Assignments - Surface Loads

Table 24: Load Assignments - Surface Loads

Area	LoadPat	Dir	UnifLoad	A	B	C
			lb/ft2	lb/ft3	lb/ft3	lb/ft2
4	LIVE	Gravity	20.00	0.0000E+00	0.0000E+00	0.00
26	LIVE	Gravity	20.00	0.0000E+00	0.0000E+00	0.00
48	LIVE	Gravity	60.00	0.0000E+00	0.0000E+00	0.00
44	SDL	Gravity	25.00	0.0000E+00	0.0000E+00	0.00
44	LIVE	Gravity	40.00	0.0000E+00	0.0000E+00	0.00
51	LIVE	Gravity	40.00	0.0000E+00	0.0000E+00	0.00
51	SDL	Gravity	25.00	0.0000E+00	0.0000E+00	0.00

Table 25: Load Assignments - Line Objects - Distributed Loads, Part 1 of 2

Table 25: Load Assignments - Line Objects - Distributed Loads, Part 1 of 2

Line	LoadPat	Type	Dir	RelDistA	RelDistB	AbsDistA	AbsDistB
						ft	ft
4	DEAD	Force	Gravity	0.0000	1.0000	0.0000	15.1135
5	DEAD	Force	Gravity	0.0000	1.0000	0.0000	28.8874
6	DEAD	Force	Gravity	0.0000	1.0000	0.0000	82.5122
7	DEAD	Force	Gravity	0.0000	1.0000	0.0000	36.9285
11	DEAD	Force	Gravity	0.0000	1.0000	0.0000	48.2515
14	DEAD	Force	Gravity	0.0000	1.0000	0.0000	4.8224
19	DEAD	Force	Gravity	0.0000	1.0000	0.0000	35.6688
20	DEAD	Force	Gravity	0.0000	1.0000	0.0000	11.0894
33	DEAD	Force	Gravity	0.0000	1.0000	0.0000	2.0011
45	DEAD	Force	Gravity	0.0000	1.0000	0.0000	6.8333
46	DEAD	Force	Gravity	0.0000	1.0000	0.0000	25.1007
106	DEAD	Force	Gravity	0.0000	1.0000	0.0000	6.8333
107	DEAD	Force	Gravity	0.0000	1.0000	0.0000	2.0000
108	DEAD	Force	Gravity	0.0000	1.0000	0.0000	25.1750
112	DEAD	Force	Gravity	0.0000	1.0000	0.0000	5.4500
112	LIVE	Force	Gravity	0.0000	1.0000	0.0000	5.4500
113	LIVE	Force	Gravity	0.0000	1.0000	0.0000	5.4318
113	DEAD	Force	Gravity	0.0000	1.0000	0.0000	5.4318

Table 25: Load Assignments - Line Objects - Distributed Loads, Part 1 of 2

Line	LoadPat	Type	Dir	RelDistA	RelDistB	AbsDistA	AbsDistB
						ft	ft
114	DEAD	Force	Gravity	0.0000	1.0000	0.0000	5.2248
114	LIVE	Force	Gravity	0.0000	1.0000	0.0000	5.2248
115	DEAD	Force	Gravity	0.0000	1.0000	0.0000	13.7500
116	DEAD	Force	Gravity	0.0000	1.0000	0.0000	18.3333
117	DEAD	Force	Gravity	0.0000	1.0000	0.0000	6.5833
118	DEAD	Force	Gravity	0.0000	1.0000	0.0000	16.8759

Table 25: Load Assignments - Line Objects - Distributed Loads, Part 2 of 2

Table 25: Load Assignments - Line Objects - Distributed Loads, Part 2 of 2

Line	LoadPat	FOverLA	FOverLB
		kip/ft	kip/ft
4	DEAD	0.151	0.151
5	DEAD	0.151	0.151
6	DEAD	0.151	0.151
7	DEAD	0.151	0.151
11	DEAD	0.151	0.151
14	DEAD	0.151	0.151
19	DEAD	0.151	0.151
20	DEAD	0.151	0.151
33	DEAD	0.151	0.151
45	DEAD	0.151	0.151
46	DEAD	0.151	0.151
106	DEAD	0.151	0.151
107	DEAD	0.151	0.151
108	DEAD	0.151	0.151
112	DEAD	0.775	0.775
112	LIVE	1.550	1.550
113	LIVE	1.550	1.550
113	DEAD	0.775	0.775
114	DEAD	0.200	0.200
114	LIVE	0.300	0.300
115	DEAD	0.151	0.151
116	DEAD	0.151	0.151
117	DEAD	0.151	0.151
118	DEAD	0.151	0.151

Table 26: Load Assignments - Line Objects - Point Loads

Table 26: Load Assignments - Line Objects - Point Loads

Line	LoadPat	Type	Dir	RelDist	AbsDist	Force
					ft	kip
112	DEAD	Force	Gravity	0.0000	0.0000	1.938
112	DEAD	Force	Gravity	1.0000	5.4500	1.938
112	LIVE	Force	Gravity	1.0000	5.4500	3.875
112	LIVE	Force	Gravity	0.0000	0.0000	3.875
113	LIVE	Force	Gravity	0.0000	0.0000	3.875
113	LIVE	Force	Gravity	0.9941	5.4000	3.875
113	DEAD	Force	Gravity	0.9941	5.4000	1.938
113	DEAD	Force	Gravity	0.0000	0.0000	1.938

4.2. Load cases

Table 27: Load Cases 02 - Static

Table 27: Load Cases 02 - Static

LoadCase	InitialCond	AType	RelForceTol	CreepCoeff	ShrinkStrn
DEAD	Zero	Linear			
LIVE	Zero	Linear			
SDL	Zero	Linear			
CASE-1	Zero	Cracked	0.000010		
CASE-2	Zero	Cracked	0.000010		
CASE-3	Zero	Long Term Cracked	0.000010	2.000000	0.000500
CASE-4	Zero	Long Term Cracked	0.000010	1.000000	0.000500
CASE-5	Zero	Long Term Cracked	0.000010	2.000000	0.000500

Table 28: Load Cases 06 - Loads Applied

Table 28: Load Cases 06 - Loads Applied

LoadCase	LoadPat	SF
DEAD	DEAD	1.000000
LIVE	LIVE	1.000000
SDL	SDL	1.000000
CASE-1	DEAD	1.000000
CASE-1	SDL	1.000000
CASE-1	LIVE	1.000000
CASE-2	DEAD	1.000000
CASE-2	SDL	1.000000
CASE-2	LIVE	0.250000
CASE-3	DEAD	1.000000
CASE-3	SDL	1.000000
CASE-3	LIVE	0.250000
CASE-4	DEAD	1.000000
CASE-4	SDL	0.300000
CASE-5	DEAD	1.000000
CASE-5	SDL	1.000000

4.3. Load combinations

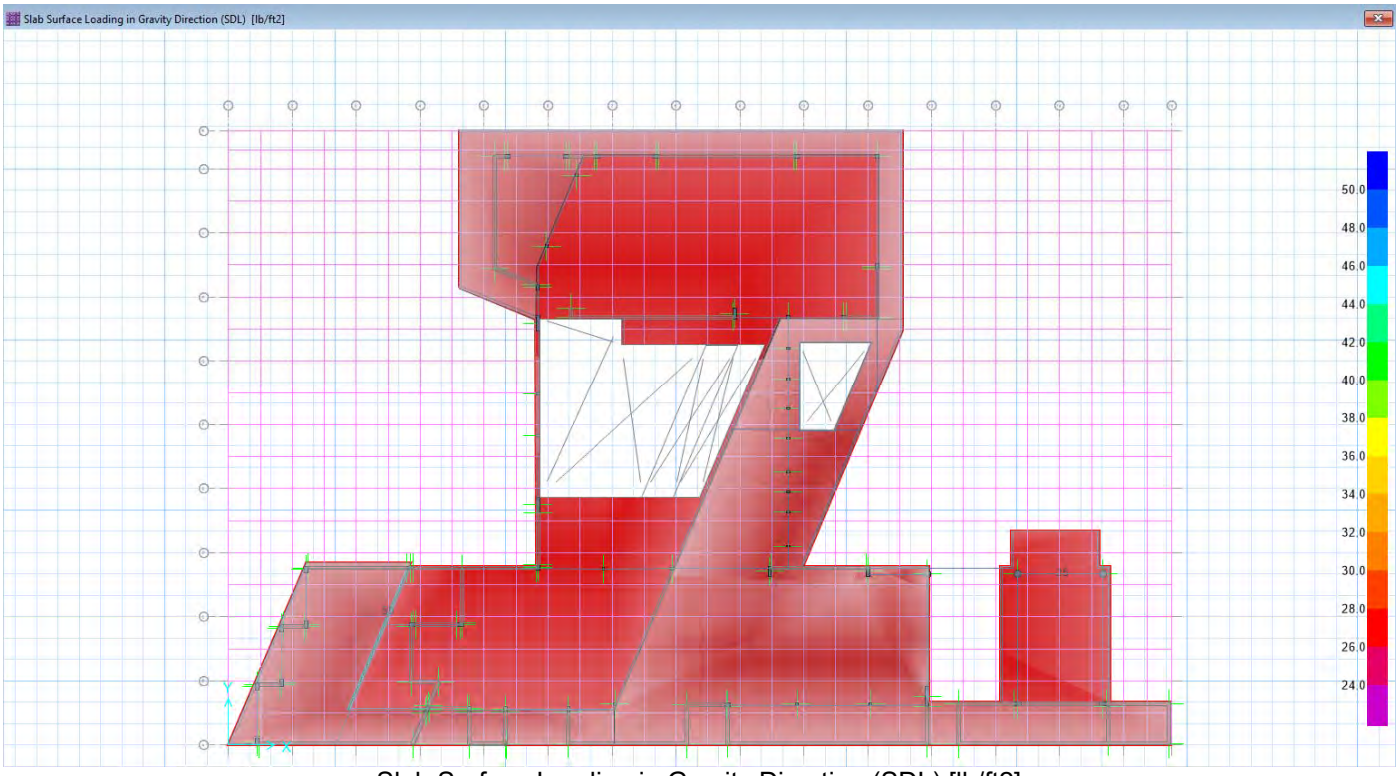
Table 29: Load Combinations

Table 29: Load Combinations

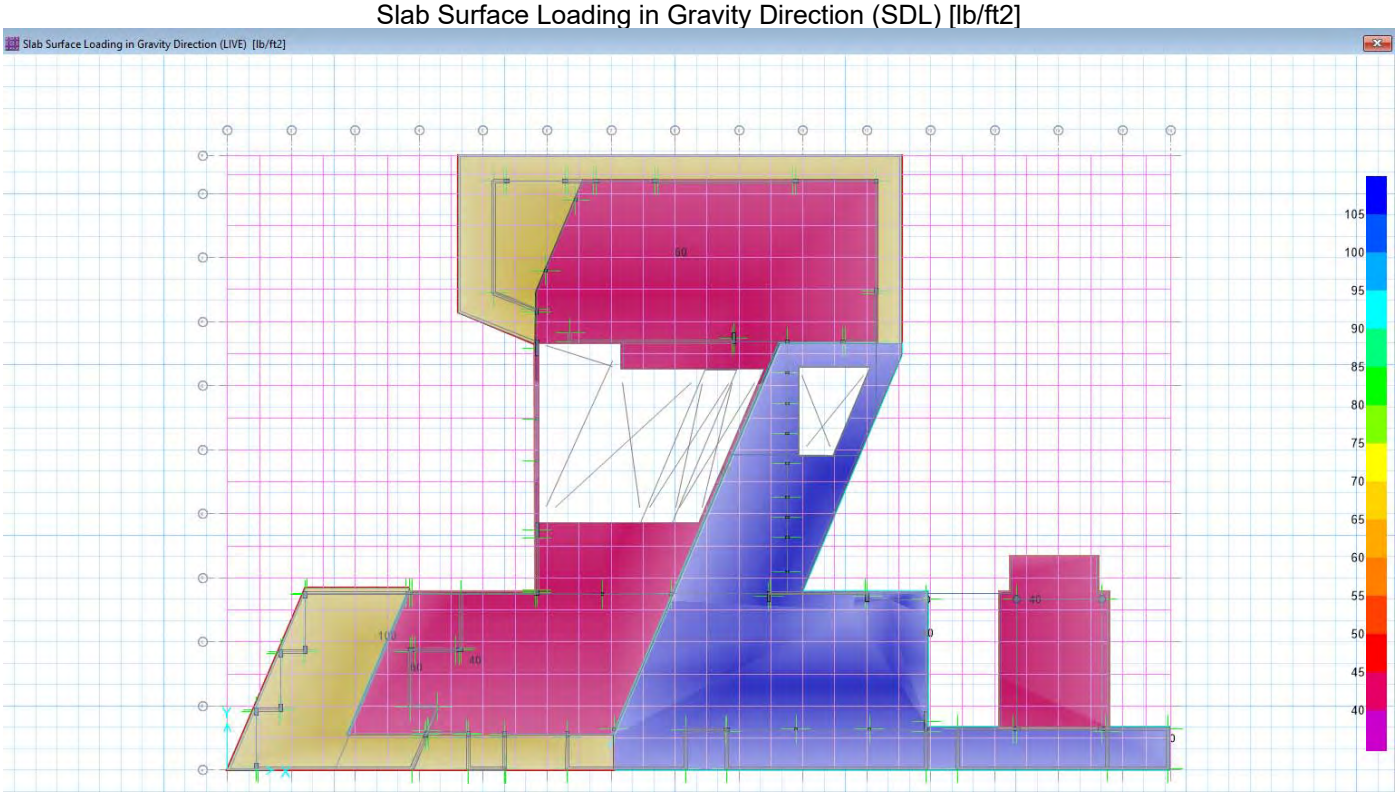
Combo	Load	SF	Type	DSStrength	DSServInit	DSServNorm	DSServLong
LONG TERM (ACI L/240 LIMIT)	CASE-3	1.000000	Linear Add	No	No	No	Yes

Table 29: Load Combinations

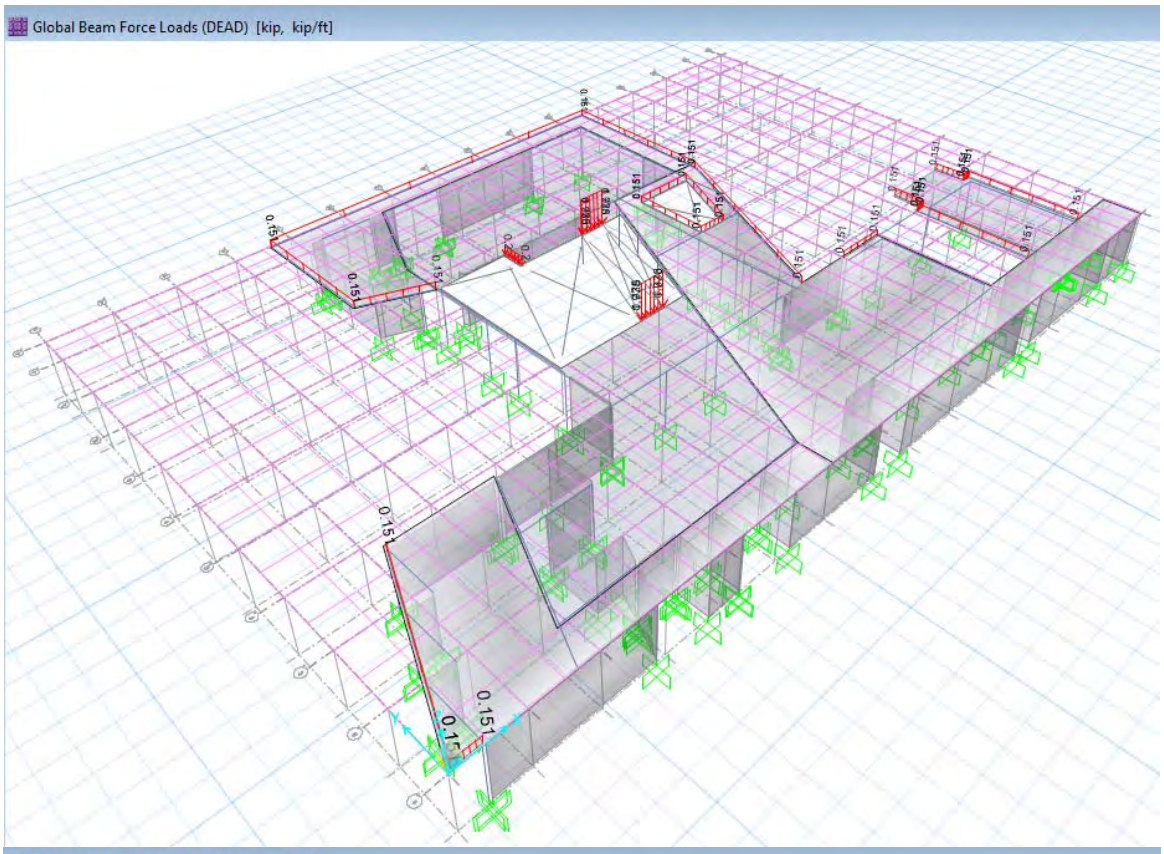
Combo	Load	SF	Type	DSStrength	DSServInit	DSServNorm	DSServLong
LONG TERM (ACI L/240 LIMIT)	CASE-1	1.000000					
LONG TERM (ACI L/240 LIMIT)	CASE-2	-1.000000					
LONG TERM (ACI L/240 LIMIT)	CASE-4	-1.000000					
LONG TERM (ACI LIVE L/360 LIMIT)	CASE-3	1.000000	Linear Add	No	No	No	Yes
LONG TERM (ACI LIVE L/360 LIMIT)	CASE-1	1.000000					
LONG TERM (ACI LIVE L/360 LIMIT)	CASE-2	-1.000000					
LONG TERM (ACI LIVE L/360 LIMIT)	CASE-5	-1.000000					
UDCONN1	DEAD	1.000000	Linear Add	No	No	Yes	No
UDCONN1	SDL	1.000000					
UDCONN2	DEAD	1.000000	Linear Add	No	No	Yes	No
UDCONN2	LIVE	1.000000					
UDCONN2	SDL	1.000000					
UDCONU1	DEAD	1.400000	Linear Add	Yes	No	No	No
UDCONU1	SDL	1.400000					
UDCONU2	DEAD	1.200000	Linear Add	Yes	No	No	No
UDCONU2	LIVE	1.600000					
UDCONU2	SDL	1.200000					

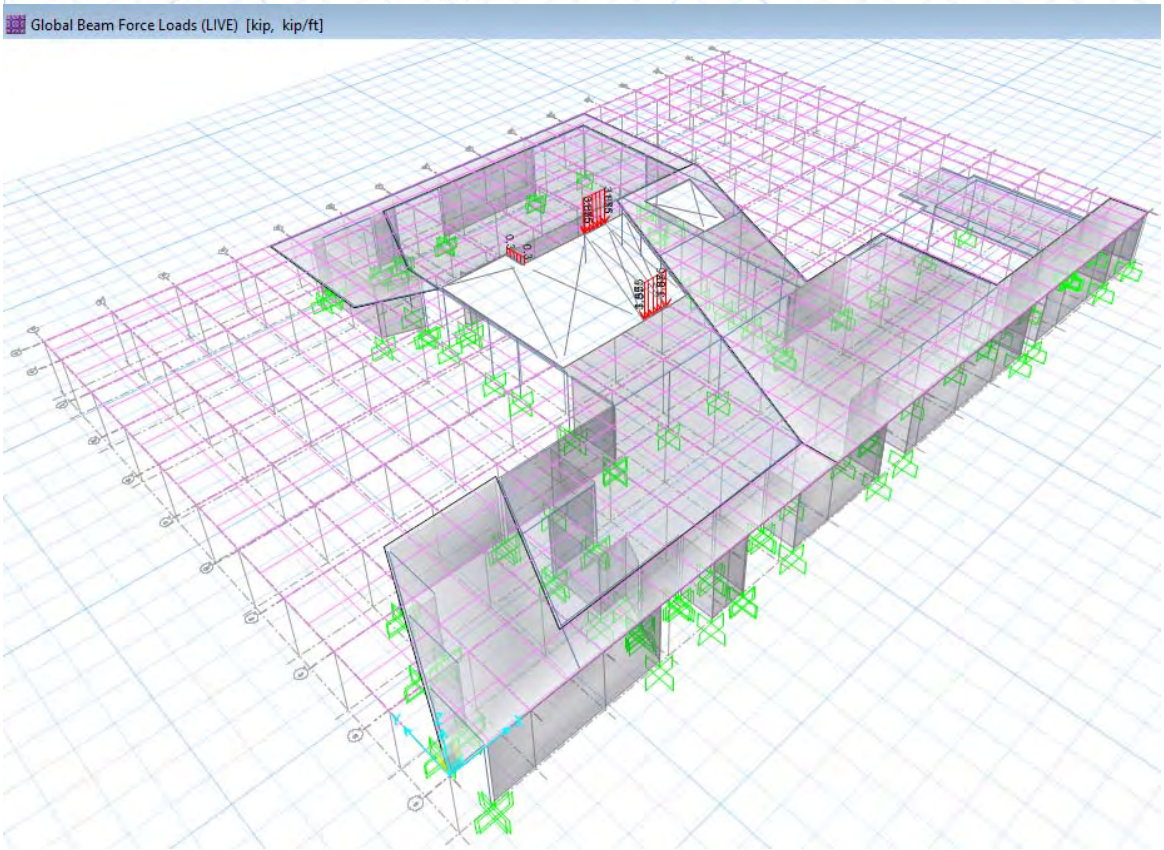


Slab Surface Loading in Gravity Direction (SDL) [lb/ft2]

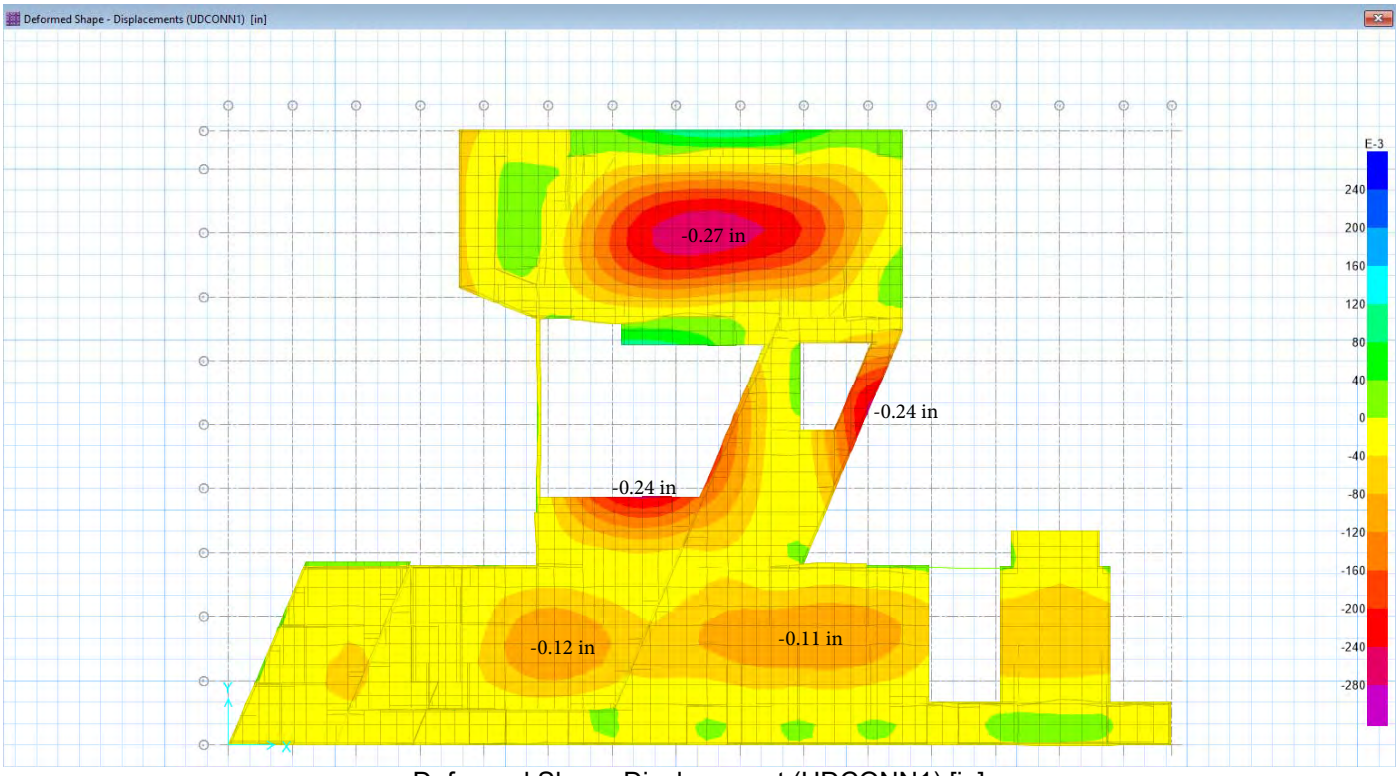


Slab Surface Loading in Gravity Direction (LIVE) [lb/ft2]

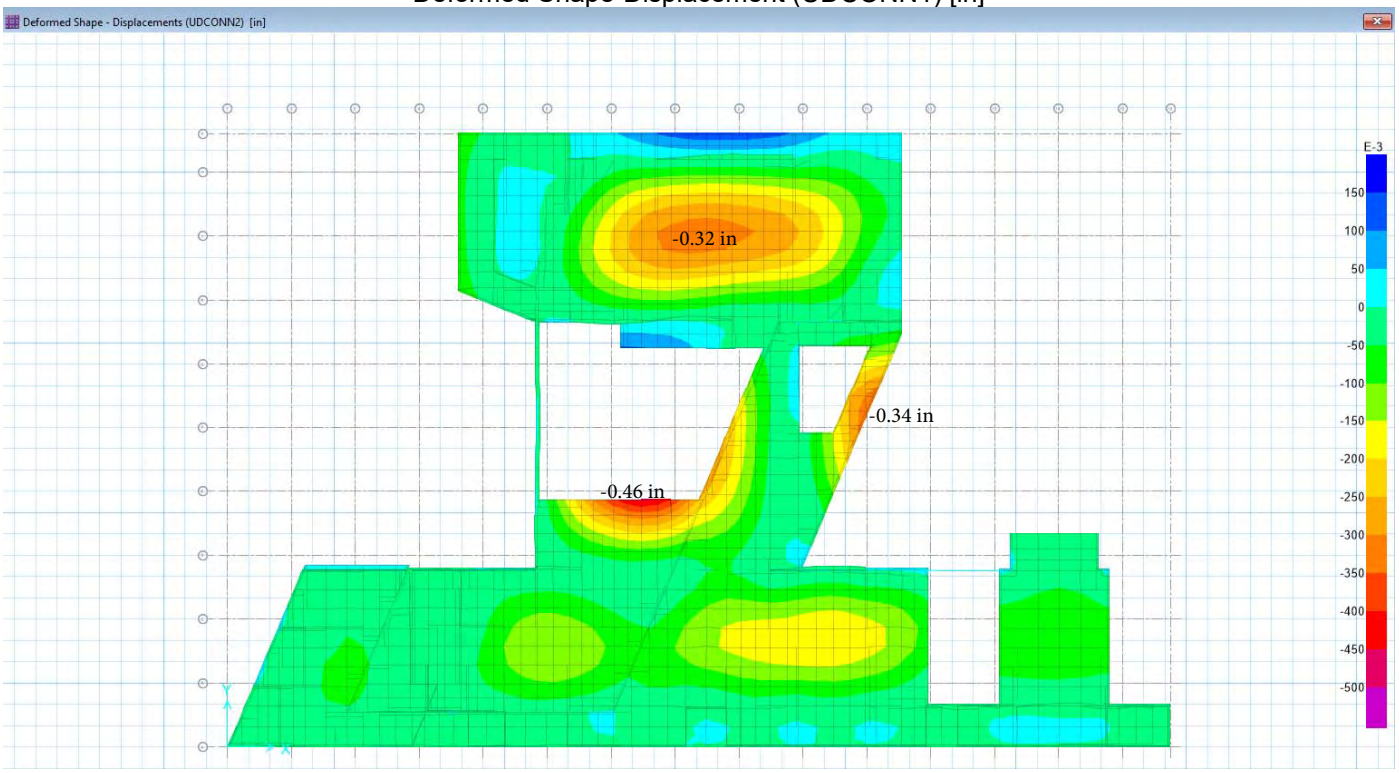

Global Beam Force Loads (DEAD) [kip, kip/ft]


Global Beam Force Loads (LIVE) [kip, kip/ft]

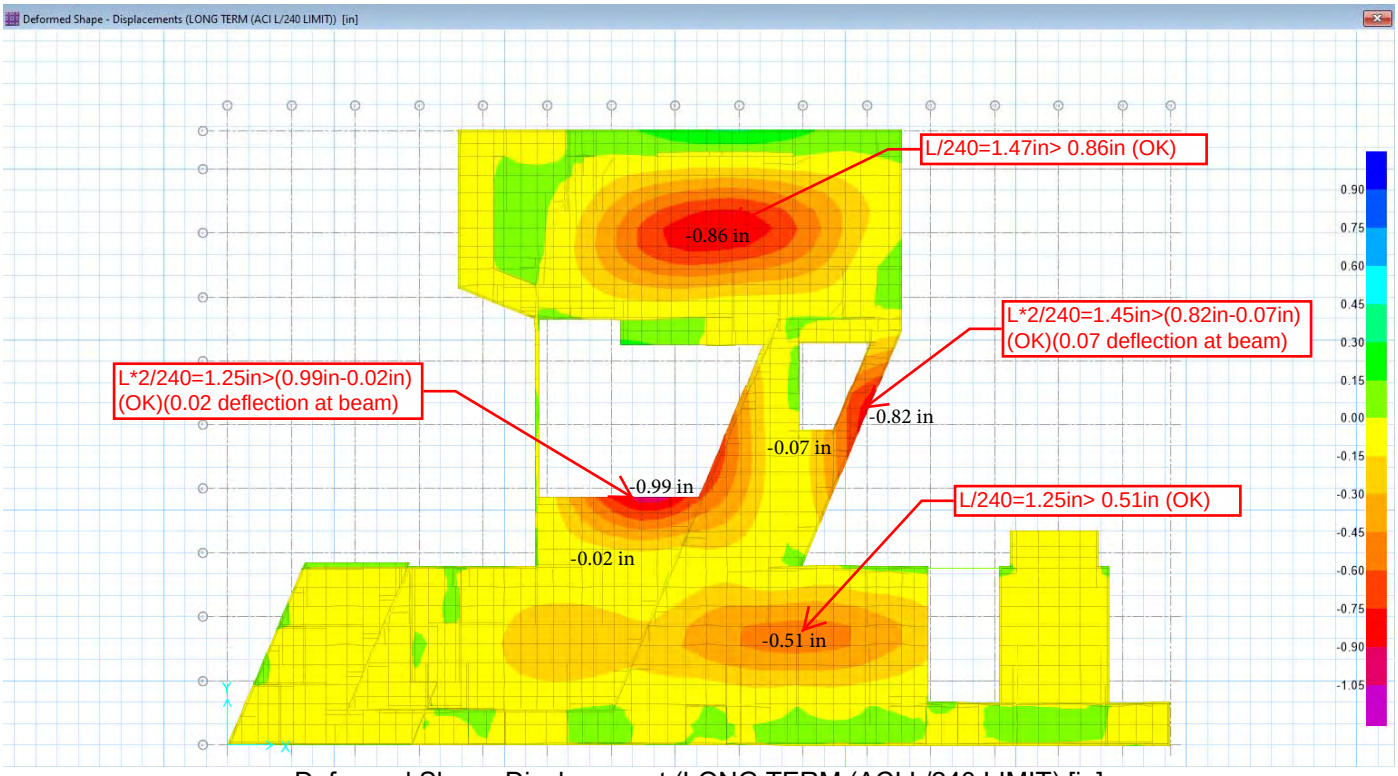
Analysis Results



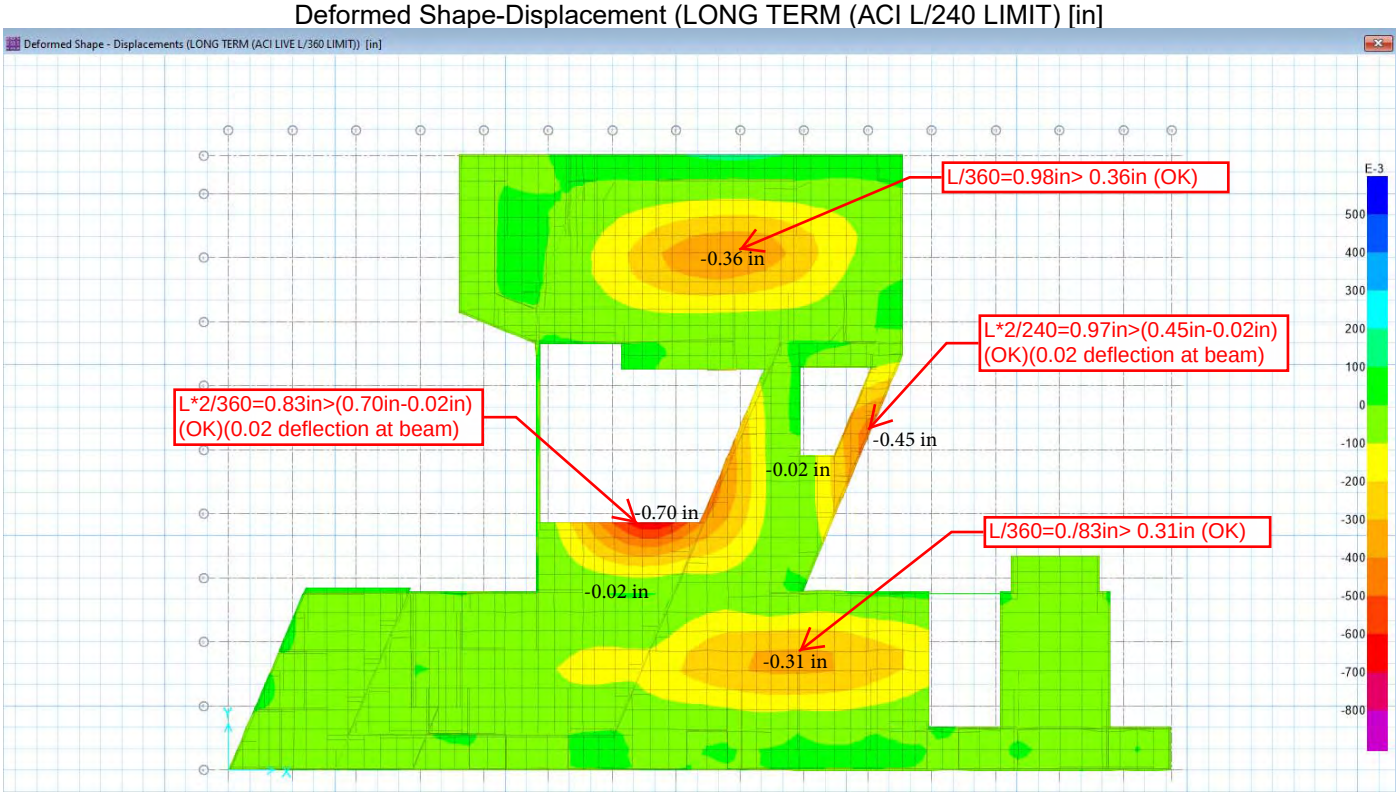
Deformed Shape-Displacement (UDCONN1) [in]



Deformed Shape-Displacement (UDCONN2) [in]



Deformed Shape-Displacement (LONG TERM (ACI L/240 LIMIT) [in]



Deformed Shape-Displacement (LONG TERM (ACI LIVE L/360 LIMIT) [in]

Figure 2: Deformed shape

5. Analysis results

5.1. Support results

This section provides support results, including items such as column, support, and spring reactions, .

Table 30: Nodal Reactions

Table 30: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
1	CASE-1	-0.849	-6.114	25.108	24.1672	-3.3432	0.0305
1	CASE-1	-0.849	-6.114	25.108	24.1672	-3.3432	0.0305
3	CASE-1	1.540	-2.917	56.867	11.4570	6.0791	0.0019
3	CASE-1	1.540	-2.917	56.867	11.4570	6.0791	0.0019
5	CASE-1	-1.886	-2.768	45.388	10.8599	-7.4494	0.0052
5	CASE-1	-1.886	-2.768	45.388	10.8599	-7.4494	0.0052
15	CASE-1	-0.093	0.611	13.024	-2.4469	-0.3515	0.0020
15	CASE-1	-0.093	0.611	13.024	-2.4469	-0.3515	0.0020
17	CASE-1	2.047	0.639	25.229	-2.5553	0.4175	0.0049
17	CASE-1	2.047	0.639	25.229	-2.5553	0.4175	0.0049
19	CASE-1	-1.080	8.451	35.850	-27.0564	-4.2907	-0.0323
19	CASE-1	-1.080	8.451	35.850	-27.0564	-4.2907	-0.0323
23	CASE-1	0.017	-12.417	47.544	47.6236	0.0719	-2.349E-004
23	CASE-1	0.017	-12.417	47.544	47.6236	0.0719	-2.349E-004
25	CASE-1	-0.907	-6.183	58.994	24.0309	-3.6018	0.0133
25	CASE-1	-0.907	-6.183	58.994	24.0309	-3.6018	0.0133
29	CASE-1	0.509	0.700	23.512	-2.7555	2.0224	0.0052
29	CASE-1	0.509	0.700	23.512	-2.7555	2.0224	0.0052
31	CASE-1	-0.118	2.369	19.112	-9.3130	-0.4698	-0.0014
31	CASE-1	-0.118	2.369	19.112	-9.3130	-0.4698	-0.0014
33	CASE-1	0.108	1.661	18.726	-6.5165	0.4315	-0.0012
33	CASE-1	0.108	1.661	18.726	-6.5165	0.4315	-0.0012
35	CASE-1	0.049	0.659	5.048	-2.5559	0.1954	-0.0013
35	CASE-1	0.049	0.659	5.048	-2.5559	0.1954	-0.0013
39	CASE-1	-0.240	-0.113	7.591	0.5521	-0.9848	-0.0014
39	CASE-1	-0.240	-0.113	7.591	0.5521	-0.9848	-0.0014
41	CASE-1	0.064	-0.271	11.544	1.1088	0.2598	5.960E-004
41	CASE-1	0.064	-0.271	11.544	1.1088	0.2598	5.960E-004
43	CASE-1	-0.706	0.067	25.628	-0.2348	-2.7622	-6.335E-004
43	CASE-1	-0.706	0.067	25.628	-0.2348	-2.7622	-6.335E-004
45	CASE-1	0.040	-0.566	5.787	2.3316	0.1615	-7.950E-004
45	CASE-1	0.040	-0.566	5.787	2.3316	0.1615	-7.950E-004
47	CASE-1	0.291	-0.245	27.744	1.0710	1.1563	-0.0031
47	CASE-1	0.291	-0.245	27.744	1.0710	1.1563	-0.0031
49	CASE-1	0.095	-0.407	2.802	1.7053	0.3779	5.906E-004
49	CASE-1	0.095	-0.407	2.802	1.7053	0.3779	5.906E-004
51	CASE-1	0.271	-0.083	24.105	0.4316	1.0768	-0.0016
51	CASE-1	0.271	-0.083	24.105	0.4316	1.0768	-0.0016
53	CASE-1	0.033	-0.527	5.806	2.1745	0.1265	7.170E-004
53	CASE-1	0.033	-0.527	5.806	2.1745	0.1265	7.170E-004
55	CASE-1	-0.078	0.481	10.297	-1.7809	-0.3139	0.0021
55	CASE-1	-0.078	0.481	10.297	-1.7809	-0.3139	0.0021
57	CASE-1	-0.128	-0.216	5.790	0.8907	-0.4911	-0.0010
57	CASE-1	-0.128	-0.216	5.790	0.8907	-0.4911	-0.0010

Table 30: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
59	CASE-1	0.236	-1.082	10.996	4.3443	0.9384	-0.0021
59	CASE-1	0.236	-1.082	10.996	4.3443	0.9384	-0.0021
65	CASE-1	3.869	1.105	22.658	-2.4257	15.3880	0.0259
65	CASE-1	3.869	1.105	22.658	-2.4257	15.3880	0.0259
67	CASE-1	-0.234	0.114	9.006	0.7949	-0.9644	-0.1156
67	CASE-1	-0.234	0.114	9.006	0.7949	-0.9644	-0.1156
69	CASE-1	0.125	-0.035	8.166	0.1887	0.4887	-0.0011
69	CASE-1	0.125	-0.035	8.166	0.1887	0.4887	-0.0011
71	CASE-1	-0.033	0.134	6.310	-0.5085	-0.1388	4.068E-004
71	CASE-1	-0.033	0.134	6.310	-0.5085	-0.1388	4.068E-004
73	CASE-1	-0.093	-0.024	-1.937	0.1195	-0.3772	7.080E-005
73	CASE-1	-0.093	-0.024	-1.937	0.1195	-0.3772	7.080E-005
75	CASE-1	0.323	-0.626	9.721	2.5165	1.2656	-9.630E-004
75	CASE-1	0.323	-0.626	9.721	2.5165	1.2656	-9.630E-004
77	CASE-1	-0.211	-1.954	20.700	7.8047	-0.8439	3.042E-004
77	CASE-1	-0.211	-1.954	20.700	7.8047	-0.8439	3.042E-004
79	CASE-1	0.416	-2.012	25.109	8.0316	1.6195	3.428E-004
79	CASE-1	0.416	-2.012	25.109	8.0316	1.6195	3.428E-004
81	CASE-1	-1.023	-4.324	47.365	17.2936	-4.1231	0.0029
81	CASE-1	-1.023	-4.324	47.365	17.2936	-4.1231	0.0029
83	CASE-1	-0.882	1.661	45.063	-6.5282	-3.5231	7.753E-004
83	CASE-1	-0.882	1.661	45.063	-6.5282	-3.5231	7.753E-004
85	CASE-1	-0.080	0.721	5.335	-2.8506	-0.3367	0.0022
85	CASE-1	-0.080	0.721	5.335	-2.8506	-0.3367	0.0022
91	CASE-1	0.145	5.581	47.189	-20.8714	0.5574	0.0044
91	CASE-1	0.145	5.581	47.189	-20.8714	0.5574	0.0044
93	CASE-1	-0.647	1.011	49.757	-3.9173	-2.6142	0.0013
93	CASE-1	-0.647	1.011	49.757	-3.9173	-2.6142	0.0013
95	CASE-1	0.824	-0.082	53.715	0.3606	3.2793	-9.253E-004
95	CASE-1	0.824	-0.082	53.715	0.3606	3.2793	-9.253E-004
97	CASE-1	0.488	-0.511	66.660	2.0732	1.9397	7.065E-006
97	CASE-1	0.488	-0.511	66.660	2.0732	1.9397	7.065E-006
99	CASE-1	0.010	-3.066E-003	3.894	0.0287	-0.6179	-0.0815
99	CASE-1	0.010	-3.066E-003	3.894	0.0287	-0.6179	-0.0815
101	CASE-1	0.577	-0.026	7.876	0.1223	1.5766	0.0880
101	CASE-1	0.577	-0.026	7.876	0.1223	1.5766	0.0880
103	CASE-1	-1.794	-3.927E-003	18.287	0.0288	-7.1444	0.0020
103	CASE-1	-1.794	-3.927E-003	18.287	0.0288	-7.1444	0.0020
105	CASE-1	-0.512	-4.818E-003	50.745	0.0323	-2.0391	8.108E-004
105	CASE-1	-0.512	-4.818E-003	50.745	0.0323	-2.0391	8.108E-004
107	CASE-1	-0.384	0.025	33.062	-0.0859	-1.5255	0.0011
107	CASE-1	-0.384	0.025	33.062	-0.0859	-1.5255	0.0011
109	CASE-1	-0.525	8.011E-004	23.973	0.0093	-2.0842	6.099E-004
109	CASE-1	-0.525	8.011E-004	23.973	0.0093	-2.0842	6.099E-004
111	CASE-1	-0.400	0.049	-3.046	-0.1856	-1.5822	0.0012
111	CASE-1	-0.400	0.049	-3.046	-0.1856	-1.5822	0.0012
113	CASE-1	-1.328E-003	0.567	72.520	-2.2179	-0.0035	-7.244E-004
113	CASE-1	-1.328E-003	0.567	72.520	-2.2179	-0.0035	-7.244E-004
115	CASE-1	0.541	3.622	148.894	-14.2776	2.1611	0.0033
115	CASE-1	0.541	3.622	148.894	-14.2776	2.1611	0.0033
117	CASE-1	-0.049	1.477	80.117	-5.8579	-0.1895	0.0019
117	CASE-1	-0.049	1.477	80.117	-5.8579	-0.1895	0.0019
119	CASE-1	-0.117	1.286	71.270	-5.1048	-0.4592	-4.956E-004
119	CASE-1	-0.117	1.286	71.270	-5.1048	-0.4592	-4.956E-004

Table 30: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
121	CASE-1	-0.167	1.417	60.105	-5.6056	-0.6602	-0.0016
121	CASE-1	-0.167	1.417	60.105	-5.6056	-0.6602	-0.0016
123	CASE-1	1.814	-6.745E-005	9.794	2.648E-004	-0.0787	-3.233E-005
123	CASE-1	1.814	-6.745E-005	9.794	2.648E-004	-0.0787	-3.233E-005
124	CASE-1	0.981	-5.441E-005	5.221	2.565E-004	1.2543	3.090E-005
124	CASE-1	0.981	-5.441E-005	5.221	2.565E-004	1.2543	3.090E-005
127	CASE-1	2.374	-6.259E-005	11.059	3.037E-004	0.1845	-3.723E-005
127	CASE-1	2.374	-6.259E-005	11.059	3.037E-004	0.1845	-3.723E-005
128	CASE-1	1.290	-8.408E-005	4.853	3.171E-004	1.3771	3.927E-005
128	CASE-1	1.290	-8.408E-005	4.853	3.171E-004	1.3771	3.927E-005
131	CASE-1	1.098	-1.020E-004	4.434	5.880E-004	0.2079	-7.267E-005
131	CASE-1	1.098	-1.020E-004	4.434	5.880E-004	0.2079	-7.267E-005
132	CASE-1	-0.820	-1.171E-004	3.714	5.932E-004	-0.0968	8.301E-005
132	CASE-1	-0.820	-1.171E-004	3.714	5.932E-004	-0.0968	8.301E-005
136	CASE-1	0.000	0.000	0.000	0.0000	0.0000	0.0000
136	CASE-1	0.000	0.000	0.000	0.0000	0.0000	0.0000
139	CASE-1	0.597	-4.809E-005	2.356	1.966E-004	0.0703	-2.686E-005
139	CASE-1	0.597	-4.809E-005	2.356	1.966E-004	0.0703	-2.686E-005
140	CASE-1	-0.299	-3.241E-004	2.287	9.892E-004	0.1562	2.377E-004
140	CASE-1	-0.299	-3.241E-004	2.287	9.892E-004	0.1562	2.377E-004
143	CASE-1	1.405E-004	-0.116	0.337	0.1791	0.0015	-1.839E-004
143	CASE-1	1.405E-004	-0.116	0.337	0.1791	0.0015	-1.839E-004
144	CASE-1	9.678E-004	-1.176	4.392	0.2749	0.0029	3.860E-004
144	CASE-1	9.678E-004	-1.176	4.392	0.2749	0.0029	3.860E-004
147	CASE-1	0.703	-1.321E-006	2.433	-2.266E-004	0.0163	4.770E-005
147	CASE-1	0.703	-1.321E-006	2.433	-2.266E-004	0.0163	4.770E-005
148	CASE-1	-0.769	0.934	8.203	-0.2247	-0.1050	3.269E-006
148	CASE-1	-0.769	0.934	8.203	-0.2247	-0.1050	3.269E-006
151	CASE-1	-0.276	-0.668	3.664	-0.0445	0.0183	-7.627E-006
151	CASE-1	-0.276	-0.668	3.664	-0.0445	0.0183	-7.627E-006
153	CASE-1	0.271	0.639	3.124	-0.1120	0.0475	1.143E-005
153	CASE-1	0.271	0.639	3.124	-0.1120	0.0475	1.143E-005
154	CASE-1	-1.426	-0.327	9.409	-0.2206	0.0137	-1.355E-005
154	CASE-1	-1.426	-0.327	9.409	-0.2206	0.0137	-1.355E-005
157	CASE-1	0.349	1.856	13.520	0.0888	-0.0915	1.193E-005
157	CASE-1	0.349	1.856	13.520	0.0888	-0.0915	1.193E-005
159	CASE-1	-1.518E-004	-1.968	11.127	-0.1032	-4.068E-004	-6.099E-005
159	CASE-1	-1.518E-004	-1.968	11.127	-0.1032	-4.068E-004	-6.099E-005
161	CASE-1	0.727	4.193E-005	4.730	1.118E-005	-0.0660	-2.128E-006
161	CASE-1	0.727	4.193E-005	4.730	1.118E-005	-0.0660	-2.128E-006
162	CASE-1	-1.683	-6.868E-005	5.387	1.354E-004	-0.1953	2.694E-005
162	CASE-1	-1.683	-6.868E-005	5.387	1.354E-004	-0.1953	2.694E-005
165	CASE-1	2.296E-004	-0.273	3.713	0.3484	6.447E-004	-9.060E-005
165	CASE-1	2.296E-004	-0.273	3.713	0.3484	6.447E-004	-9.060E-005
166	CASE-1	-1.619E-006	-2.367	7.787	0.4646	2.478E-004	3.211E-005
166	CASE-1	-1.619E-006	-2.367	7.787	0.4646	2.478E-004	3.211E-005
169	CASE-1	0.678	-1.439E-005	2.999	-1.729E-004	0.0422	2.297E-005
169	CASE-1	0.678	-1.439E-005	2.999	-1.729E-004	0.0422	2.297E-005
170	CASE-1	-0.338	8.419E-005	0.674	-1.463E-004	0.0269	-5.529E-005
170	CASE-1	-0.338	8.419E-005	0.674	-1.463E-004	0.0269	-5.529E-005
173	CASE-1	2.355E-006	-0.921	3.173	0.1325	1.131E-005	1.473E-006
173	CASE-1	2.355E-006	-0.921	3.173	0.1325	1.131E-005	1.473E-006
174	CASE-1	0.667	-0.282	4.095	0.2307	0.1159	-1.347E-005
174	CASE-1	0.667	-0.282	4.095	0.2307	0.1159	-1.347E-005

Table 30: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
177	CASE-1	-0.216	9.248E-006	2.633	1.477E-005	-0.0170	1.487E-006
177	CASE-1	-0.216	9.248E-006	2.633	1.477E-005	-0.0170	1.487E-006
179	CASE-1	-6.756E-003	-0.808	4.354	-0.0057	1.470E-004	1.839E-005
179	CASE-1	-6.756E-003	-0.808	4.354	-0.0057	1.470E-004	1.839E-005
180	CASE-1	9.307E-003	1.106	5.555	-0.1162	0.0011	-1.228E-005
180	CASE-1	9.307E-003	1.106	5.555	-0.1162	0.0011	-1.228E-005
183	CASE-1	-4.657E-003	-0.818	4.123	0.0392	-3.078E-004	-1.608E-005
183	CASE-1	-4.657E-003	-0.818	4.123	0.0392	-3.078E-004	-1.608E-005
184	CASE-1	0.185	0.671	6.040	-0.0099	-0.1901	-3.528E-006
184	CASE-1	0.185	0.671	6.040	-0.0099	-0.1901	-3.528E-006
187	CASE-1	-0.276	3.863	11.667	-0.5338	0.0704	-6.181E-005
187	CASE-1	-0.276	3.863	11.667	-0.5338	0.0704	-6.181E-005
189	CASE-1	1.580	1.500	21.058	-0.2161	-0.1364	-1.644E-004
189	CASE-1	1.580	1.500	21.058	-0.2161	-0.1364	-1.644E-004
191	CASE-1	-3.151	4.827E-004	15.318	-0.0011	-0.7383	-1.327E-004
191	CASE-1	-3.151	4.827E-004	15.318	-0.0011	-0.7383	-1.327E-004
193	CASE-1	4.235E-003	0.732	3.458	-1.1137	0.0065	3.558E-005
193	CASE-1	4.235E-003	0.732	3.458	-1.1137	0.0065	3.558E-005
194	CASE-1	0.401	1.578	5.296	-1.2273	0.0354	6.107E-006
194	CASE-1	0.401	1.578	5.296	-1.2273	0.0354	6.107E-006
197	CASE-1	-0.113	0.483	3.468	-0.1339	0.0576	-1.599E-006
197	CASE-1	-0.113	0.483	3.468	-0.1339	0.0576	-1.599E-006
199	CASE-1	-4.560E-005	-0.444	3.685	-0.0568	-9.886E-005	-1.303E-005
199	CASE-1	-4.560E-005	-0.444	3.685	-0.0568	-9.886E-005	-1.303E-005
201	CASE-1	1.812	-0.823	10.545	0.1272	0.4384	-1.177E-005
201	CASE-1	1.812	-0.823	10.545	0.1272	0.4384	-1.177E-005
202	CASE-1	-0.672	-8.699E-005	3.977	4.612E-004	-0.0384	8.928E-005
202	CASE-1	-0.672	-8.699E-005	3.977	4.612E-004	-0.0384	8.928E-005
209	CASE-1	0.675	0.267	6.678	0.0602	0.0750	-1.385E-006
209	CASE-1	0.675	0.267	6.678	0.0602	0.0750	-1.385E-006
211	CASE-1	-0.584	0.499	5.714	-0.0384	-0.0693	1.120E-006
211	CASE-1	-0.584	0.499	5.714	-0.0384	-0.0693	1.120E-006
213	CASE-1	-1.033	-0.383	6.457	0.0068	-0.1427	-1.463E-005
213	CASE-1	-1.033	-0.383	6.457	0.0068	-0.1427	-1.463E-005
215	CASE-1	0.636	3.923E-005	3.347	-5.020E-004	0.0639	8.488E-005
215	CASE-1	0.636	3.923E-005	3.347	-5.020E-004	0.0639	8.488E-005
217	CASE-1	2.475E-005	0.628	1.124	-0.7169	8.817E-005	1.143E-005
217	CASE-1	2.475E-005	0.628	1.124	-0.7169	8.817E-005	1.143E-005
218	CASE-1	1.189E-005	0.896	2.834	-0.6028	6.832E-005	-9.197E-006
218	CASE-1	1.189E-005	0.896	2.834	-0.6028	6.832E-005	-9.197E-006
221	CASE-1	-2.035	1.482E-003	9.802	0.0020	-0.2788	1.876E-004
221	CASE-1	-2.035	1.482E-003	9.802	0.0020	-0.2788	1.876E-004
223	CASE-1	-1.664	0.662	5.070	-0.0883	-0.2225	3.433E-005
223	CASE-1	-1.664	0.662	5.070	-0.0883	-0.2225	3.433E-005
224	CASE-1	0.206	0.371	5.376	0.1056	-0.2265	7.517E-007
224	CASE-1	0.206	0.371	5.376	0.1056	-0.2265	7.517E-007
227	CASE-1	0.861	-1.785	14.413	-0.1639	-0.8111	-4.733E-005
227	CASE-1	0.861	-1.785	14.413	-0.1639	-0.8111	-4.733E-005
229	CASE-1	-2.429	-0.015	9.054	-8.686E-004	0.1644	4.320E-005
229	CASE-1	-2.429	-0.015	9.054	-8.686E-004	0.1644	4.320E-005
231	CASE-1	0.082	-7.459E-005	1.090	4.271E-004	0.0199	-5.790E-005
231	CASE-1	0.082	-7.459E-005	1.090	4.271E-004	0.0199	-5.790E-005
232	CASE-1	1.019	-1.349E-003	-1.430	6.712E-004	0.2228	9.165E-005
232	CASE-1	1.019	-1.349E-003	-1.430	6.712E-004	0.2228	9.165E-005

Table 30: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
235	CASE-1	1.510	-2.593E-004	6.995	9.287E-004	0.0198	-8.990E-005
235	CASE-1	1.510	-2.593E-004	6.995	9.287E-004	0.0198	-8.990E-005
236	CASE-1	-1.742	-2.525E-005	7.816	0.0044	0.0104	7.989E-004
236	CASE-1	-1.742	-2.525E-005	7.816	0.0044	0.0104	7.989E-004
239	CASE-1	-7.129E-004	-2.392	9.409	-0.9203	-0.0013	-2.241E-004
239	CASE-1	-7.129E-004	-2.392	9.409	-0.9203	-0.0013	-2.241E-004
240	CASE-1	1.020	0.871	11.102	1.5818	0.4757	-6.755E-005
240	CASE-1	1.020	0.871	11.102	1.5818	0.4757	-6.755E-005
243	CASE-1	2.217	1.688E-003	10.031	-0.0027	0.2187	6.771E-004
243	CASE-1	2.217	1.688E-003	10.031	-0.0027	0.2187	6.771E-004
245	CASE-1	0.519	1.865E-004	8.495	0.0013	-0.5063	-1.446E-004
245	CASE-1	0.519	1.865E-004	8.495	0.0013	-0.5063	-1.446E-004
246	CASE-1	-1.599	-4.061E-004	3.503	0.0035	-0.2931	5.673E-004
246	CASE-1	-1.599	-4.061E-004	3.503	0.0035	-0.2931	5.673E-004
249	CASE-1	-7.780E-004	2.528	-6.552	-0.9737	-3.312E-004	-1.196E-004
249	CASE-1	-7.780E-004	2.528	-6.552	-0.9737	-3.312E-004	-1.196E-004
250	CASE-1	0.858	1.935	11.719	-0.8291	-0.3299	8.817E-005
250	CASE-1	0.858	1.935	11.719	-0.8291	-0.3299	8.817E-005
253	CASE-1	-2.059	1.728E-003	8.380	0.0014	-0.0543	2.622E-004
253	CASE-1	-2.059	1.728E-003	8.380	0.0014	-0.0543	2.622E-004
308	CASE-1	-0.404	2.125E-003	22.681	0.0042	-1.6048	8.766E-004
308	CASE-1	-0.404	2.125E-003	22.681	0.0042	-1.6048	8.766E-004
310	CASE-1	-1.010	-0.016	25.013	0.0772	-4.0317	0.0025
310	CASE-1	-1.010	-0.016	25.013	0.0772	-4.0317	0.0025
316	CASE-1	-1.223	-0.044	30.207	0.1908	-4.8688	3.308E-004
316	CASE-1	-1.223	-0.044	30.207	0.1908	-4.8688	3.308E-004
1	CASE-2	-0.540	-3.188	19.618	12.6796	-2.1246	0.0134
1	CASE-2	-0.540	-3.188	19.618	12.6796	-2.1246	0.0134
3	CASE-2	1.236	-2.735	49.910	10.7481	4.8777	6.155E-004
3	CASE-2	1.236	-2.735	49.910	10.7481	4.8777	6.155E-004
5	CASE-2	-1.623	-2.652	39.769	10.4145	-6.4089	0.0046
5	CASE-2	-1.623	-2.652	39.769	10.4145	-6.4089	0.0046
15	CASE-2	-0.073	0.555	11.900	-2.2203	-0.2757	0.0015
15	CASE-2	-0.073	0.555	11.900	-2.2203	-0.2757	0.0015
17	CASE-2	1.880	0.546	22.688	-2.1846	0.4196	0.0046
17	CASE-2	1.880	0.546	22.688	-2.1846	0.4196	0.0046
19	CASE-2	-0.673	5.573	27.124	-17.0605	-2.6783	-0.0188
19	CASE-2	-0.673	5.573	27.124	-17.0605	-2.6783	-0.0188
23	CASE-2	0.032	-7.802	32.154	30.1690	0.1313	0.0077
23	CASE-2	0.032	-7.802	32.154	30.1690	0.1313	0.0077
25	CASE-2	-0.588	-3.527	41.799	13.9261	-2.3321	0.0027
25	CASE-2	-0.588	-3.527	41.799	13.9261	-2.3321	0.0027
29	CASE-2	0.288	0.387	17.194	-1.5132	1.1441	0.0025
29	CASE-2	0.288	0.387	17.194	-1.5132	1.1441	0.0025
31	CASE-2	-0.083	1.776	16.658	-6.9761	-0.3318	-0.0013
31	CASE-2	-0.083	1.776	16.658	-6.9761	-0.3318	-0.0013
33	CASE-2	0.086	1.328	15.640	-5.2073	0.3422	-0.0012
33	CASE-2	0.086	1.328	15.640	-5.2073	0.3422	-0.0012
35	CASE-2	0.041	0.533	4.487	-2.0642	0.1601	-0.0011
35	CASE-2	0.041	0.533	4.487	-2.0642	0.1601	-0.0011
39	CASE-2	-0.208	-0.099	6.840	0.4748	-0.8524	-0.0013
39	CASE-2	-0.208	-0.099	6.840	0.4748	-0.8524	-0.0013
41	CASE-2	0.060	-0.202	10.343	0.8297	0.2408	5.921E-004
41	CASE-2	0.060	-0.202	10.343	0.8297	0.2408	5.921E-004

Table 30: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
43	CASE-2	-0.557	0.052	22.644	-0.1803	-2.1791	-5.854E-004
43	CASE-2	-0.557	0.052	22.644	-0.1803	-2.1791	-5.854E-004
45	CASE-2	0.030	-0.450	5.151	1.8523	0.1224	-8.308E-004
45	CASE-2	0.030	-0.450	5.151	1.8523	0.1224	-8.308E-004
47	CASE-2	0.223	-0.215	22.635	0.9328	0.8864	-0.0025
47	CASE-2	0.223	-0.215	22.635	0.9328	0.8864	-0.0025
49	CASE-2	0.076	-0.319	2.813	1.3357	0.2997	2.898E-004
49	CASE-2	0.076	-0.319	2.813	1.3357	0.2997	2.898E-004
51	CASE-2	0.208	-0.084	20.040	0.4129	0.8258	-0.0013
51	CASE-2	0.208	-0.084	20.040	0.4129	0.8258	-0.0013
53	CASE-2	0.024	-0.431	5.188	1.7764	0.0921	4.384E-004
53	CASE-2	0.024	-0.431	5.188	1.7764	0.0921	4.384E-004
55	CASE-2	-0.070	0.396	8.964	-1.4680	-0.2800	0.0018
55	CASE-2	-0.070	0.396	8.964	-1.4680	-0.2800	0.0018
57	CASE-2	-0.091	-0.167	5.105	0.6919	-0.3454	-9.169E-004
57	CASE-2	-0.091	-0.167	5.105	0.6919	-0.3454	-9.169E-004
59	CASE-2	0.175	-0.819	9.896	3.2921	0.6968	-0.0029
59	CASE-2	0.175	-0.819	9.896	3.2921	0.6968	-0.0029
65	CASE-2	2.384	0.758	16.795	-1.5277	9.4919	0.0082
65	CASE-2	2.384	0.758	16.795	-1.5277	9.4919	0.0082
67	CASE-2	-0.190	0.051	8.908	0.8878	-0.7787	-0.0708
67	CASE-2	-0.190	0.051	8.908	0.8878	-0.7787	-0.0708
69	CASE-2	0.106	-0.032	7.212	0.1732	0.4188	-0.0015
69	CASE-2	0.106	-0.032	7.212	0.1732	0.4188	-0.0015
71	CASE-2	-0.030	0.115	5.312	-0.4380	-0.1178	-2.926E-004
71	CASE-2	-0.030	0.115	5.312	-0.4380	-0.1178	-2.926E-004
73	CASE-2	-0.061	-2.927E-003	-1.138	0.0327	-0.2423	-6.526E-004
73	CASE-2	-0.061	-2.927E-003	-1.138	0.0327	-0.2423	-6.526E-004
75	CASE-2	0.255	-0.450	8.831	1.8121	1.0096	-0.0014
75	CASE-2	0.255	-0.450	8.831	1.8121	1.0096	-0.0014
77	CASE-2	-0.165	-1.400	17.249	5.6000	-0.6520	-4.686E-004
77	CASE-2	-0.165	-1.400	17.249	5.6000	-0.6520	-4.686E-004
79	CASE-2	0.343	-1.446	20.801	5.7801	1.3448	-1.027E-004
79	CASE-2	0.343	-1.446	20.801	5.7801	1.3448	-1.027E-004
81	CASE-2	-0.849	-3.195	40.520	12.8218	-3.4055	0.0019
81	CASE-2	-0.849	-3.195	40.520	12.8218	-3.4055	0.0019
83	CASE-2	-0.665	1.109	38.014	-4.3330	-2.6523	2.616E-004
83	CASE-2	-0.665	1.109	38.014	-4.3330	-2.6523	2.616E-004
85	CASE-2	-0.061	0.539	5.089	-2.1218	-0.2502	0.0013
85	CASE-2	-0.061	0.539	5.089	-2.1218	-0.2502	0.0013
91	CASE-2	0.070	5.175	41.801	-19.3220	0.2679	0.0034
91	CASE-2	0.070	5.175	41.801	-19.3220	0.2679	0.0034
93	CASE-2	-0.393	0.860	46.464	-3.3061	-1.5826	2.547E-005
93	CASE-2	-0.393	0.860	46.464	-3.3061	-1.5826	2.547E-005
95	CASE-2	0.602	-0.062	45.072	0.2794	2.4007	-0.0013
95	CASE-2	0.602	-0.062	45.072	0.2794	2.4007	-0.0013
97	CASE-2	0.383	-0.412	54.877	1.6778	1.5280	-5.671E-004
97	CASE-2	0.383	-0.412	54.877	1.6778	1.5280	-5.671E-004
99	CASE-2	1.057E-003	-2.127E-003	4.076	0.0221	-0.3887	-0.0483
99	CASE-2	1.057E-003	-2.127E-003	4.076	0.0221	-0.3887	-0.0483
101	CASE-2	0.352	-0.020	6.724	0.0932	0.9743	0.0536
101	CASE-2	0.352	-0.020	6.724	0.0932	0.9743	0.0536
103	CASE-2	-1.407	-7.136E-003	14.183	0.0428	-5.5945	0.0016
103	CASE-2	-1.407	-7.136E-003	14.183	0.0428	-5.5945	0.0016

Table 30: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
105	CASE-2	-0.610	-5.833E-003	37.123	0.0377	-2.4204	3.313E-004
105	CASE-2	-0.610	-5.833E-003	37.123	0.0377	-2.4204	3.313E-004
107	CASE-2	-0.614	0.013	25.084	-0.0365	-2.4360	3.589E-004
107	CASE-2	-0.614	0.013	25.084	-0.0365	-2.4360	3.589E-004
109	CASE-2	-0.265	-6.747E-003	17.541	0.0410	-1.0462	1.477E-005
109	CASE-2	-0.265	-6.747E-003	17.541	0.0410	-1.0462	1.477E-005
111	CASE-2	-0.261	0.020	2.454	-0.0673	-1.0295	3.603E-004
111	CASE-2	-0.261	0.020	2.454	-0.0673	-1.0295	3.603E-004
113	CASE-2	0.018	0.221	64.305	-0.8445	0.0745	-8.974E-004
113	CASE-2	0.018	0.221	64.305	-0.8445	0.0745	-8.974E-004
115	CASE-2	0.295	2.164	110.355	-8.4911	1.1867	-1.771E-004
115	CASE-2	0.295	2.164	110.355	-8.4911	1.1867	-1.771E-004
117	CASE-2	-1.362E-003	0.724	59.072	-2.8542	-0.0015	9.094E-004
117	CASE-2	-1.362E-003	0.724	59.072	-2.8542	-0.0015	9.094E-004
119	CASE-2	-0.082	0.608	51.525	-2.3937	-0.3234	2.156E-004
119	CASE-2	-0.082	0.608	51.525	-2.3937	-0.3234	2.156E-004
121	CASE-2	-0.153	0.901	44.656	-3.5539	-0.6069	-0.0011
121	CASE-2	-0.153	0.901	44.656	-3.5539	-0.6069	-0.0011
123	CASE-2	1.494	-5.391E-005	8.318	2.083E-004	-0.0922	-2.545E-005
123	CASE-2	1.494	-5.391E-005	8.318	2.083E-004	-0.0922	-2.545E-005
124	CASE-2	0.785	-4.158E-005	4.592	2.004E-004	1.0508	2.413E-005
124	CASE-2	0.785	-4.158E-005	4.592	2.004E-004	1.0508	2.413E-005
127	CASE-2	1.942	-4.787E-005	9.241	2.229E-004	0.1582	-2.726E-005
127	CASE-2	1.942	-4.787E-005	9.241	2.229E-004	0.1582	-2.726E-005
128	CASE-2	0.964	-5.854E-005	4.319	2.295E-004	1.0801	2.854E-005
128	CASE-2	0.964	-5.854E-005	4.319	2.295E-004	1.0801	2.854E-005
131	CASE-2	0.941	-8.033E-005	3.913	4.594E-004	0.1716	-5.682E-005
131	CASE-2	0.941	-8.033E-005	3.913	4.594E-004	0.1716	-5.682E-005
132	CASE-2	-0.712	-8.969E-005	3.265	4.575E-004	-0.0831	6.399E-005
132	CASE-2	-0.712	-8.969E-005	3.265	4.575E-004	-0.0831	6.399E-005
136	CASE-2	0.000	0.000	0.000	0.0000	0.0000	0.0000
136	CASE-2	0.000	0.000	0.000	0.0000	0.0000	0.0000
139	CASE-2	0.574	-3.712E-005	2.287	1.551E-004	0.0686	-2.122E-005
139	CASE-2	0.574	-3.712E-005	2.287	1.551E-004	0.0686	-2.122E-005
140	CASE-2	-0.339	-2.462E-004	2.173	7.626E-004	0.1180	1.836E-004
140	CASE-2	-0.339	-2.462E-004	2.173	7.626E-004	0.1180	1.836E-004
143	CASE-2	9.708E-005	-0.201	0.708	0.2286	9.141E-004	-1.131E-004
143	CASE-2	9.708E-005	-0.201	0.708	0.2286	9.141E-004	-1.131E-004
144	CASE-2	5.762E-004	-1.181	4.083	0.2578	0.0017	2.304E-004
144	CASE-2	5.762E-004	-1.181	4.083	0.2578	0.0017	2.304E-004
147	CASE-2	0.605	-2.289E-006	2.134	-1.829E-004	0.0120	3.830E-005
147	CASE-2	0.605	-2.289E-006	2.134	-1.829E-004	0.0120	3.830E-005
148	CASE-2	-0.671	0.849	7.448	-0.2042	-0.0931	2.515E-006
148	CASE-2	-0.671	0.849	7.448	-0.2042	-0.0931	2.515E-006
151	CASE-2	-0.236	-0.573	3.204	-0.0414	0.0170	-7.300E-006
151	CASE-2	-0.236	-0.573	3.204	-0.0414	0.0170	-7.300E-006
153	CASE-2	0.241	0.569	2.801	-0.0909	0.0385	9.904E-006
153	CASE-2	0.241	0.569	2.801	-0.0909	0.0385	9.904E-006
154	CASE-2	-1.292	-0.323	8.371	-0.1765	-0.0066	-1.188E-005
154	CASE-2	-1.292	-0.323	8.371	-0.1765	-0.0066	-1.188E-005
157	CASE-2	0.295	1.689	11.937	0.0440	-0.0913	1.130E-005
157	CASE-2	0.295	1.689	11.937	0.0440	-0.0913	1.130E-005
159	CASE-2	-1.177E-004	-1.690	9.650	-0.0814	-3.169E-004	-4.753E-005
159	CASE-2	-1.177E-004	-1.690	9.650	-0.0814	-3.169E-004	-4.753E-005

Table 30: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
161	CASE-2	0.650	3.223E-005	4.256	8.872E-006	-0.0602	-1.710E-006
161	CASE-2	0.650	3.223E-005	4.256	8.872E-006	-0.0602	-1.710E-006
162	CASE-2	-1.517	-5.225E-005	4.866	1.037E-004	-0.1760	2.067E-005
162	CASE-2	-1.517	-5.225E-005	4.866	1.037E-004	-0.1760	2.067E-005
165	CASE-2	1.901E-004	-0.169	3.428	0.2831	5.317E-004	-7.484E-005
165	CASE-2	1.901E-004	-0.169	3.428	0.2831	5.317E-004	-7.484E-005
166	CASE-2	-3.248E-006	-2.046	6.763	0.4032	1.999E-004	2.584E-005
166	CASE-2	-3.248E-006	-2.046	6.763	0.4032	1.999E-004	2.584E-005
169	CASE-2	0.580	-1.080E-005	2.699	-1.418E-004	0.0266	1.888E-005
169	CASE-2	0.580	-1.080E-005	2.699	-1.418E-004	0.0266	1.888E-005
170	CASE-2	-0.329	6.776E-005	0.671	-1.185E-004	0.0202	-4.478E-005
170	CASE-2	-0.329	6.776E-005	0.671	-1.185E-004	0.0202	-4.478E-005
173	CASE-2	2.864E-006	-0.782	2.774	0.1083	1.050E-005	1.437E-006
173	CASE-2	2.864E-006	-0.782	2.774	0.1083	1.050E-005	1.437E-006
174	CASE-2	0.574	-0.172	3.911	0.1793	0.0952	-1.050E-005
174	CASE-2	0.574	-0.172	3.911	0.1793	0.0952	-1.050E-005
177	CASE-2	-0.237	7.177E-006	2.491	1.211E-005	-0.0206	1.290E-006
177	CASE-2	-0.237	7.177E-006	2.491	1.211E-005	-0.0206	1.290E-006
179	CASE-2	-5.758E-003	-0.689	3.746	-0.0064	1.318E-004	1.450E-005
179	CASE-2	-5.758E-003	-0.689	3.746	-0.0064	1.318E-004	1.450E-005
180	CASE-2	8.091E-003	0.962	4.872	-0.1027	9.431E-004	-9.533E-006
180	CASE-2	8.091E-003	0.962	4.872	-0.1027	9.431E-004	-9.533E-006
183	CASE-2	-3.617E-003	-0.636	3.579	0.0033	-8.036E-005	-1.151E-005
183	CASE-2	-3.617E-003	-0.636	3.579	0.0033	-8.036E-005	-1.151E-005
184	CASE-2	0.213	0.804	6.043	-0.0766	-0.1461	-4.691E-007
184	CASE-2	0.213	0.804	6.043	-0.0766	-0.1461	-4.691E-007
187	CASE-2	-0.391	2.590	8.969	-0.3300	0.0111	-3.023E-005
187	CASE-2	-0.391	2.590	8.969	-0.3300	0.0111	-3.023E-005
189	CASE-2	1.306	0.798	16.264	-0.1021	0.0301	-1.126E-004
189	CASE-2	1.306	0.798	16.264	-0.1021	0.0301	-1.126E-004
191	CASE-2	-2.191	2.754E-004	11.309	-6.202E-004	-0.4115	-7.401E-005
191	CASE-2	-2.191	2.754E-004	11.309	-6.202E-004	-0.4115	-7.401E-005
193	CASE-2	2.136E-003	0.366	3.271	-0.6739	0.0040	2.331E-005
193	CASE-2	2.136E-003	0.366	3.271	-0.6739	0.0040	2.331E-005
194	CASE-2	0.351	1.209	4.591	-0.7806	0.0308	5.031E-006
194	CASE-2	0.351	1.209	4.591	-0.7806	0.0308	5.031E-006
197	CASE-2	-0.166	0.367	3.195	-0.0581	0.0267	-1.370E-007
197	CASE-2	-0.166	0.367	3.195	-0.0581	0.0267	-1.370E-007
199	CASE-2	-3.469E-005	-0.513	3.589	0.0155	-7.380E-005	-9.813E-006
199	CASE-2	-3.469E-005	-0.513	3.589	0.0155	-7.380E-005	-9.813E-006
201	CASE-2	1.604	-0.740	9.353	0.1209	0.3858	-9.095E-006
201	CASE-2	1.604	-0.740	9.353	0.1209	0.3858	-9.095E-006
202	CASE-2	-0.631	-7.250E-005	3.640	3.812E-004	-0.0449	7.385E-005
202	CASE-2	-0.631	-7.250E-005	3.640	3.812E-004	-0.0449	7.385E-005
209	CASE-2	0.484	0.195	5.410	0.0579	0.0451	-1.354E-006
209	CASE-2	0.484	0.195	5.410	0.0579	0.0451	-1.354E-006
211	CASE-2	-0.487	0.436	5.013	-0.0346	-0.0547	7.604E-007
211	CASE-2	-0.487	0.436	5.013	-0.0346	-0.0547	7.604E-007
213	CASE-2	-0.958	-0.351	5.879	0.0089	-0.1315	-1.215E-005
213	CASE-2	-0.958	-0.351	5.879	0.0089	-0.1315	-1.215E-005
215	CASE-2	0.575	3.873E-005	3.037	-4.475E-004	0.0674	7.681E-005
215	CASE-2	0.575	3.873E-005	3.037	-4.475E-004	0.0674	7.681E-005
217	CASE-2	2.496E-005	0.812	0.485	-0.8116	9.000E-005	1.160E-005
217	CASE-2	2.496E-005	0.812	0.485	-0.8116	9.000E-005	1.160E-005

Table 30: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
218	CASE-2	1.285E-005	0.784	2.127	-0.6612	7.094E-005	-9.537E-006
218	CASE-2	1.285E-005	0.784	2.127	-0.6612	7.094E-005	-9.537E-006
221	CASE-2	-1.839	1.360E-003	9.029	0.0018	-0.2279	1.587E-004
221	CASE-2	-1.839	1.360E-003	9.029	0.0018	-0.2279	1.587E-004
223	CASE-2	-1.338	0.532	4.225	-0.0680	-0.1715	2.937E-005
223	CASE-2	-1.338	0.532	4.225	-0.0680	-0.1715	2.937E-005
224	CASE-2	0.260	0.259	4.755	0.1106	-0.1645	-5.720E-007
224	CASE-2	0.260	0.259	4.755	0.1106	-0.1645	-5.720E-007
227	CASE-2	0.753	-1.517	12.394	-0.1361	-0.6427	-3.877E-005
227	CASE-2	0.753	-1.517	12.394	-0.1361	-0.6427	-3.877E-005
229	CASE-2	-2.029	-0.012	7.589	-6.071E-004	0.1175	3.646E-005
229	CASE-2	-2.029	-0.012	7.589	-6.071E-004	0.1175	3.646E-005
231	CASE-2	0.359	-3.912E-004	1.856	3.974E-004	0.0892	-4.235E-005
231	CASE-2	0.359	-3.912E-004	1.856	3.974E-004	0.0892	-4.235E-005
232	CASE-2	0.772	-1.021E-003	-0.964	5.307E-004	0.1939	6.806E-005
232	CASE-2	0.772	-1.021E-003	-0.964	5.307E-004	0.1939	6.806E-005
235	CASE-2	1.431	-1.690E-004	6.368	2.180E-004	0.0419	2.981E-005
235	CASE-2	1.431	-1.690E-004	6.368	2.180E-004	0.0419	2.981E-005
236	CASE-2	-1.426	6.491E-005	6.490	0.0032	0.0287	5.749E-004
236	CASE-2	-1.426	6.491E-005	6.490	0.0032	0.0287	5.749E-004
239	CASE-2	-5.528E-004	-1.813	8.206	-0.7023	-0.0010	-1.712E-004
239	CASE-2	-5.528E-004	-1.813	8.206	-0.7023	-0.0010	-1.712E-004
240	CASE-2	0.419	1.165	9.663	0.9429	0.2161	-7.037E-005
240	CASE-2	0.419	1.165	9.663	0.9429	0.2161	-7.037E-005
243	CASE-2	1.587	1.424E-003	7.728	-0.0023	0.0695	5.567E-004
243	CASE-2	1.587	1.424E-003	7.728	-0.0023	0.0695	5.567E-004
245	CASE-2	0.516	1.266E-004	6.578	6.149E-004	-0.3052	-6.413E-005
245	CASE-2	0.516	1.266E-004	6.578	6.149E-004	-0.3052	-6.413E-005
246	CASE-2	-1.183	-2.278E-004	3.187	0.0021	-0.1701	3.370E-004
246	CASE-2	-1.183	-2.278E-004	3.187	0.0021	-0.1701	3.370E-004
249	CASE-2	-6.599E-004	1.890	-4.790	-0.7576	-2.798E-004	-1.007E-004
249	CASE-2	-6.599E-004	1.890	-4.790	-0.7576	-2.798E-004	-1.007E-004
250	CASE-2	0.774	1.605	9.950	-0.6540	-0.2504	6.883E-005
250	CASE-2	0.774	1.605	9.950	-0.6540	-0.2504	6.883E-005
253	CASE-2	-1.544	1.418E-003	6.212	5.611E-004	-0.0596	1.034E-004
253	CASE-2	-1.544	1.418E-003	6.212	5.611E-004	-0.0596	1.034E-004
308	CASE-2	-0.479	1.021E-003	17.053	0.0101	-1.8994	1.153E-004
308	CASE-2	-0.479	1.021E-003	17.053	0.0101	-1.8994	1.153E-004
310	CASE-2	-0.704	-0.016	15.167	0.0788	-2.8037	0.0019
310	CASE-2	-0.704	-0.016	15.167	0.0788	-2.8037	0.0019
316	CASE-2	-1.085	-0.035	23.469	0.1530	-4.3125	4.600E-004
316	CASE-2	-1.085	-0.035	23.469	0.1530	-4.3125	4.600E-004
1	CASE-3	-1.083	-4.939	20.118	19.3507	-4.2046	0.0606
1	CASE-3	-1.083	-4.939	20.118	19.3507	-4.2046	0.0606
3	CASE-3	2.988	-4.937	51.055	18.9681	11.9106	-0.0251
3	CASE-3	2.988	-4.937	51.055	18.9681	11.9106	-0.0251
5	CASE-3	-3.856	-4.633	41.108	17.8331	-15.0509	-0.0182
5	CASE-3	-3.856	-4.633	41.108	17.8331	-15.0509	-0.0182
15	CASE-3	-0.133	1.265	12.284	-5.1146	-0.4828	-0.0042
15	CASE-3	-0.133	1.265	12.284	-5.1146	-0.4828	-0.0042
17	CASE-3	1.830	1.388	21.348	-5.6222	0.1174	0.0062
17	CASE-3	1.830	1.388	21.348	-5.6222	0.1174	0.0062
19	CASE-3	-1.476	5.440	24.997	-17.7486	-5.8597	-0.0482
19	CASE-3	-1.476	5.440	24.997	-17.7486	-5.8597	-0.0482

Table 30: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
23	CASE-3	0.101	-9.320	37.977	35.1690	0.4242	0.0247
23	CASE-3	0.101	-9.320	37.977	35.1690	0.4242	0.0247
25	CASE-3	-1.425	-4.374	45.052	16.9197	-5.6503	0.0183
25	CASE-3	-1.425	-4.374	45.052	16.9197	-5.6503	0.0183
29	CASE-3	0.793	0.978	21.894	-3.8654	3.1342	0.0125
29	CASE-3	0.793	0.978	21.894	-3.8654	3.1342	0.0125
31	CASE-3	-0.270	4.264	22.180	-16.7983	-1.0745	-0.0047
31	CASE-3	-0.270	4.264	22.180	-16.7983	-1.0745	-0.0047
33	CASE-3	0.241	3.106	20.598	-12.2262	0.9564	-0.0017
33	CASE-3	0.241	3.106	20.598	-12.2262	0.9564	-0.0017
35	CASE-3	0.073	1.175	4.121	-4.6021	0.2816	-0.0091
35	CASE-3	0.073	1.175	4.121	-4.6021	0.2816	-0.0091
39	CASE-3	-0.463	-0.128	7.954	0.5886	-1.8721	-0.0022
39	CASE-3	-0.463	-0.128	7.954	0.5886	-1.8721	-0.0022
41	CASE-3	0.217	-0.423	9.912	1.7085	0.8436	0.0098
41	CASE-3	0.217	-0.423	9.912	1.7085	0.8436	0.0098
43	CASE-3	-1.373	0.146	25.600	-0.5459	-5.3830	-0.0024
43	CASE-3	-1.373	0.146	25.600	-0.5459	-5.3830	-0.0024
45	CASE-3	0.050	-0.956	6.888	3.8037	0.1967	-0.0021
45	CASE-3	0.050	-0.956	6.888	3.8037	0.1967	-0.0021
47	CASE-3	0.519	-0.389	23.894	1.5952	2.0492	-0.0110
47	CASE-3	0.519	-0.389	23.894	1.5952	2.0492	-0.0110
49	CASE-3	9.568E-003	-0.552	2.981	2.2242	0.0228	6.679E-004
49	CASE-3	9.568E-003	-0.552	2.981	2.2242	0.0228	6.679E-004
51	CASE-3	0.510	-6.840E-004	22.053	0.0803	2.0127	-0.0057
51	CASE-3	0.510	-6.840E-004	22.053	0.0803	2.0127	-0.0057
53	CASE-3	-0.060	-0.852	5.137	3.4289	-0.2547	0.0067
53	CASE-3	-0.060	-0.852	5.137	3.4289	-0.2547	0.0067
55	CASE-3	-0.184	1.054	11.474	-4.0295	-0.7374	0.0071
55	CASE-3	-0.184	1.054	11.474	-4.0295	-0.7374	0.0071
57	CASE-3	-0.285	-0.439	4.485	1.7755	-1.1140	-0.0020
57	CASE-3	-0.285	-0.439	4.485	1.7755	-1.1140	-0.0020
59	CASE-3	0.463	-1.992	13.893	7.9420	1.8312	-0.0085
59	CASE-3	0.463	-1.992	13.893	7.9420	1.8312	-0.0085
65	CASE-3	4.402	1.212	19.946	-2.3644	17.4378	0.0398
65	CASE-3	4.402	1.212	19.946	-2.3644	17.4378	0.0398
67	CASE-3	-0.424	-0.160	8.594	1.9291	-1.7124	-0.0784
67	CASE-3	-0.424	-0.160	8.594	1.9291	-1.7124	-0.0784
69	CASE-3	0.181	-0.123	7.504	0.5382	0.7217	-0.0064
69	CASE-3	0.181	-0.123	7.504	0.5382	0.7217	-0.0064
71	CASE-3	0.177	0.371	3.247	-1.4425	0.7286	0.0062
71	CASE-3	0.177	0.371	3.247	-1.4425	0.7286	0.0062
73	CASE-3	-0.314	0.262	-0.255	-1.0189	-1.2286	0.0018
73	CASE-3	-0.314	0.262	-0.255	-1.0189	-1.2286	0.0018
75	CASE-3	0.707	-1.140	11.894	4.5619	2.8024	-0.0010
75	CASE-3	0.707	-1.140	11.894	4.5619	2.8024	-0.0010
77	CASE-3	-0.524	-3.950	20.211	15.7445	-2.0614	0.0098
77	CASE-3	-0.524	-3.950	20.211	15.7445	-2.0614	0.0098
79	CASE-3	1.026	-3.909	24.769	15.5657	4.0253	-0.0044
79	CASE-3	1.026	-3.909	24.769	15.5657	4.0253	-0.0044
81	CASE-3	-2.074	-5.631	42.394	22.3607	-8.2990	0.0096
81	CASE-3	-2.074	-5.631	42.394	22.3607	-8.2990	0.0096
83	CASE-3	-1.808	2.295	40.711	-9.0681	-7.2002	0.0037
83	CASE-3	-1.808	2.295	40.711	-9.0681	-7.2002	0.0037

Table 30: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
85	CASE-3	-0.147	1.460	7.113	-5.8061	-0.5731	0.0068
85	CASE-3	-0.147	1.460	7.113	-5.8061	-0.5731	0.0068
91	CASE-3	0.222	8.316	49.197	-31.0338	0.8897	0.0058
91	CASE-3	0.222	8.316	49.197	-31.0338	0.8897	0.0058
93	CASE-3	-0.865	2.280	43.271	-8.9646	-3.4161	0.0183
93	CASE-3	-0.865	2.280	43.271	-8.9646	-3.4161	0.0183
95	CASE-3	1.757	-0.238	42.168	0.9841	7.0205	-0.0025
95	CASE-3	1.757	-0.238	42.168	0.9841	7.0205	-0.0025
97	CASE-3	1.007	-0.900	58.616	3.6259	4.0245	0.0030
97	CASE-3	1.007	-0.900	58.616	3.6259	4.0245	0.0030
99	CASE-3	-0.046	0.031	4.538	-0.1052	-0.5285	-0.0437
99	CASE-3	-0.046	0.031	4.538	-0.1052	-0.5285	-0.0437
101	CASE-3	0.272	-0.065	5.629	0.2820	0.6677	0.0393
101	CASE-3	0.272	-0.065	5.629	0.2820	0.6677	0.0393
103	CASE-3	-2.620	0.016	13.614	-0.0518	-10.3698	0.0047
103	CASE-3	-2.620	0.016	13.614	-0.0518	-10.3698	0.0047
105	CASE-3	-0.121	0.026	40.522	-0.0916	-0.4580	-0.0017
105	CASE-3	-0.121	0.026	40.522	-0.0916	-0.4580	-0.0017
107	CASE-3	0.058	0.049	24.351	-0.1827	0.2523	2.214E-004
107	CASE-3	0.058	0.049	24.351	-0.1827	0.2523	2.214E-004
109	CASE-3	-0.345	-0.024	21.529	0.1046	-1.3541	-2.966E-004
109	CASE-3	-0.345	-0.024	21.529	0.1046	-1.3541	-2.966E-004
111	CASE-3	-0.566	0.096	-1.468	-0.3744	-2.2325	0.0016
111	CASE-3	-0.566	0.096	-1.468	-0.3744	-2.2325	0.0016
113	CASE-3	-0.185	0.458	58.673	-1.7814	-0.7444	-0.0017
113	CASE-3	-0.185	0.458	58.673	-1.7814	-0.7444	-0.0017
115	CASE-3	1.311	5.090	112.558	-20.1015	5.2104	0.0056
115	CASE-3	1.311	5.090	112.558	-20.1015	5.2104	0.0056
117	CASE-3	-0.019	1.773	57.414	-7.0504	-0.0671	0.0032
117	CASE-3	-0.019	1.773	57.414	-7.0504	-0.0671	0.0032
119	CASE-3	-0.239	1.585	49.983	-6.3152	-0.9438	-0.0019
119	CASE-3	-0.239	1.585	49.983	-6.3152	-0.9438	-0.0019
121	CASE-3	-0.191	2.384	44.669	-9.4541	-0.7588	-7.601E-004
121	CASE-3	-0.191	2.384	44.669	-9.4541	-0.7588	-7.601E-004
123	CASE-3	0.702	-1.135E-004	7.215	4.931E-004	-1.1611	-5.934E-005
123	CASE-3	0.702	-1.135E-004	7.215	4.931E-004	-1.1611	-5.934E-005
124	CASE-3	1.766	-1.278E-004	3.471	5.022E-004	2.1677	6.073E-005
124	CASE-3	1.766	-1.278E-004	3.471	5.022E-004	2.1677	6.073E-005
127	CASE-3	1.134	-9.626E-005	8.318	5.093E-004	-0.9378	-6.129E-005
127	CASE-3	1.134	-9.626E-005	8.318	5.093E-004	-0.9378	-6.129E-005
128	CASE-3	1.868	-1.623E-004	3.498	5.502E-004	2.1881	6.856E-005
128	CASE-3	1.868	-1.623E-004	3.498	5.502E-004	2.1881	6.856E-005
131	CASE-3	0.891	-2.330E-004	3.468	0.0014	0.1876	-1.782E-004
131	CASE-3	0.891	-2.330E-004	3.468	0.0014	0.1876	-1.782E-004
132	CASE-3	-0.744	-2.584E-004	3.332	0.0014	-0.0948	1.935E-004
132	CASE-3	-0.744	-2.584E-004	3.332	0.0014	-0.0948	1.935E-004
136	CASE-3	0.000	0.000	0.000	0.0000	0.0000	0.0000
136	CASE-3	0.000	0.000	0.000	0.0000	0.0000	0.0000
139	CASE-3	0.506	-1.022E-004	2.065	3.172E-004	0.0336	-4.209E-005
139	CASE-3	0.506	-1.022E-004	2.065	3.172E-004	0.0336	-4.209E-005
140	CASE-3	-0.242	-6.992E-004	2.647	0.0022	0.2537	5.401E-004
140	CASE-3	-0.242	-6.992E-004	2.647	0.0022	0.2537	5.401E-004
143	CASE-3	2.986E-004	-0.655	-1.176	0.5934	0.0023	-2.938E-004
143	CASE-3	2.986E-004	-0.655	-1.176	0.5934	0.0023	-2.938E-004

Table 30: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
144	CASE-3	1.306E-003	-0.712	3.348	-0.0954	0.0041	5.390E-004
144	CASE-3	1.306E-003	-0.712	3.348	-0.0954	0.0041	5.390E-004
147	CASE-3	0.361	-6.536E-005	1.598	-6.069E-004	-0.1341	1.307E-004
147	CASE-3	0.361	-6.536E-005	1.598	-6.069E-004	-0.1341	1.307E-004
148	CASE-3	-0.588	1.230	8.041	-0.6315	0.0548	-3.010E-006
148	CASE-3	-0.588	1.230	8.041	-0.6315	0.0548	-3.010E-006
151	CASE-3	-0.205	-0.497	3.364	-0.2394	0.0987	-1.324E-005
151	CASE-3	-0.205	-0.497	3.364	-0.2394	0.0987	-1.324E-005
153	CASE-3	0.169	0.398	2.301	-0.1384	0.0586	2.175E-005
153	CASE-3	0.169	0.398	2.301	-0.1384	0.0586	2.175E-005
154	CASE-3	-1.184	-0.104	8.151	-0.3002	0.1247	-1.616E-005
154	CASE-3	-1.184	-0.104	8.151	-0.3002	0.1247	-1.616E-005
157	CASE-3	0.059	1.645	11.755	0.5538	-0.0591	1.958E-005
157	CASE-3	0.059	1.645	11.755	0.5538	-0.0591	1.958E-005
159	CASE-3	-3.537E-004	-1.350	9.975	-0.5824	-9.707E-004	-1.459E-004
159	CASE-3	-3.537E-004	-1.350	9.975	-0.5824	-9.707E-004	-1.459E-004
161	CASE-3	0.352	8.444E-005	3.015	-3.181E-005	-0.1030	6.114E-006
161	CASE-3	0.352	8.444E-005	3.015	-3.181E-005	-0.1030	6.114E-006
162	CASE-3	-1.333	-1.162E-004	4.059	1.987E-004	-0.1686	3.900E-005
162	CASE-3	-1.333	-1.162E-004	4.059	1.987E-004	-0.1686	3.900E-005
165	CASE-3	5.069E-004	-0.318	3.635	0.3640	0.0014	-2.010E-004
165	CASE-3	5.069E-004	-0.318	3.635	0.3640	0.0014	-2.010E-004
166	CASE-3	5.855E-006	-2.398	7.077	0.5746	5.736E-004	7.464E-005
166	CASE-3	5.855E-006	-2.398	7.077	0.5746	5.736E-004	7.464E-005
169	CASE-3	0.648	-3.789E-005	2.965	-3.293E-004	-0.1463	4.302E-005
169	CASE-3	0.648	-3.789E-005	2.965	-3.293E-004	-0.1463	4.302E-005
170	CASE-3	-0.362	1.584E-004	0.689	-2.860E-004	0.0507	-1.044E-004
170	CASE-3	-0.362	1.584E-004	0.689	-2.860E-004	0.0507	-1.044E-004
173	CASE-3	3.106E-006	-0.935	2.751	0.1701	1.086E-007	-4.798E-007
173	CASE-3	3.106E-006	-0.935	2.751	0.1701	1.086E-007	-4.798E-007
174	CASE-3	0.319	-0.573	1.999	0.3035	0.0010	-1.315E-005
174	CASE-3	0.319	-0.573	1.999	0.3035	0.0010	-1.315E-005
177	CASE-3	-0.479	-6.385E-006	3.022	5.594E-005	-0.1186	6.917E-006
177	CASE-3	-0.479	-6.385E-006	3.022	5.594E-005	-0.1186	6.917E-006
179	CASE-3	-7.754E-003	-0.934	4.054	0.1110	-7.101E-004	4.143E-005
179	CASE-3	-7.754E-003	-0.934	4.054	0.1110	-7.101E-004	4.143E-005
180	CASE-3	3.685E-003	0.437	3.440	0.1090	-7.139E-004	-2.336E-005
180	CASE-3	3.685E-003	0.437	3.440	0.1090	-7.139E-004	-2.336E-005
183	CASE-3	-6.153E-003	-1.070	4.067	0.1532	-0.0011	-3.491E-005
183	CASE-3	-6.153E-003	-1.070	4.067	0.1532	-0.0011	-3.491E-005
184	CASE-3	0.106	-0.078	3.912	0.2299	-0.4109	-2.067E-005
184	CASE-3	0.106	-0.078	3.912	0.2299	-0.4109	-2.067E-005
187	CASE-3	-0.351	1.908	7.963	0.1672	0.1597	-2.306E-006
187	CASE-3	-0.351	1.908	7.963	0.1672	0.1597	-2.306E-006
189	CASE-3	1.506	0.227	17.164	0.4001	0.1398	-1.132E-004
189	CASE-3	1.506	0.227	17.164	0.4001	0.1398	-1.132E-004
191	CASE-3	-1.912	7.367E-004	10.870	-0.0016	-0.2676	-2.080E-004
191	CASE-3	-1.912	7.367E-004	10.870	-0.0016	-0.2676	-2.080E-004
193	CASE-3	6.270E-003	1.078	2.745	-2.0519	0.0120	6.163E-005
193	CASE-3	6.270E-003	1.078	2.745	-2.0519	0.0120	6.163E-005
194	CASE-3	0.491	1.528	3.778	-2.1663	0.0536	-1.062E-005
194	CASE-3	0.491	1.528	3.778	-2.1663	0.0536	-1.062E-005
197	CASE-3	-0.127	0.550	3.728	-0.1743	0.0809	-2.420E-006
197	CASE-3	-0.127	0.550	3.728	-0.1743	0.0809	-2.420E-006

Table 30: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
199	CASE-3	-8.732E-005	-0.199	2.521	-0.1177	-2.004E-004	-2.683E-005
199	CASE-3	-8.732E-005	-0.199	2.521	-0.1177	-2.004E-004	-2.683E-005
201	CASE-3	1.269	-0.320	6.588	0.1407	0.3553	-6.307E-007
201	CASE-3	1.269	-0.320	6.588	0.1407	0.3553	-6.307E-007
202	CASE-3	-0.784	2.657E-004	3.940	-4.661E-004	-0.0730	-5.220E-005
202	CASE-3	-0.784	2.657E-004	3.940	-4.661E-004	-0.0730	-5.220E-005
209	CASE-3	0.481	0.636	8.064	0.0663	-0.0606	4.855E-006
209	CASE-3	0.481	0.636	8.064	0.0663	-0.0606	4.855E-006
211	CASE-3	-0.735	0.692	6.621	-0.1204	-0.1072	-2.187E-006
211	CASE-3	-0.735	0.692	6.621	-0.1204	-0.1072	-2.187E-006
213	CASE-3	-0.701	-0.058	4.147	-0.1089	-0.0871	-7.254E-006
213	CASE-3	-0.701	-0.058	4.147	-0.1089	-0.0871	-7.254E-006
215	CASE-3	0.738	1.323E-004	3.461	-6.460E-004	0.1018	1.127E-004
215	CASE-3	0.738	1.323E-004	3.461	-6.460E-004	0.1018	1.127E-004
217	CASE-3	1.317E-004	1.921	-2.404	-1.7435	4.507E-004	6.067E-005
217	CASE-3	1.317E-004	1.921	-2.404	-1.7435	4.507E-004	6.067E-005
218	CASE-3	5.591E-005	1.562	3.610	-1.4306	3.390E-004	-4.431E-005
218	CASE-3	5.591E-005	1.562	3.610	-1.4306	3.390E-004	-4.431E-005
221	CASE-3	-1.854	1.799E-003	8.578	-2.027E-004	-0.3428	-4.797E-005
221	CASE-3	-1.854	1.799E-003	8.578	-2.027E-004	-0.3428	-4.797E-005
223	CASE-3	-1.636	0.650	3.890	0.0282	0.0694	9.787E-005
223	CASE-3	-1.636	0.650	3.890	0.0282	0.0694	9.787E-005
224	CASE-3	-0.344	-0.117	4.196	0.4271	-0.6512	1.338E-006
224	CASE-3	-0.344	-0.117	4.196	0.4271	-0.6512	1.338E-006
227	CASE-3	0.413	-0.872	11.995	-1.0247	-1.6501	-6.214E-005
227	CASE-3	0.413	-0.872	11.995	-1.0247	-1.6501	-6.214E-005
229	CASE-3	-2.704	-0.016	9.966	-0.0040	0.7097	1.037E-004
229	CASE-3	-2.704	-0.016	9.966	-0.0040	0.7097	1.037E-004
231	CASE-3	-0.314	5.690E-004	0.235	2.643E-004	0.0414	-2.889E-005
231	CASE-3	-0.314	5.690E-004	0.235	2.643E-004	0.0414	-2.889E-005
232	CASE-3	1.443	-2.001E-003	-2.262	8.982E-004	0.3256	1.107E-004
232	CASE-3	1.443	-2.001E-003	-2.262	8.982E-004	0.3256	1.107E-004
235	CASE-3	1.157	-1.682E-003	5.931	0.0077	-0.1560	-0.0013
235	CASE-3	1.157	-1.682E-003	5.931	0.0077	-0.1560	-0.0013
236	CASE-3	-1.167	-1.297E-003	6.176	0.0098	0.2413	0.0017
236	CASE-3	-1.167	-1.297E-003	6.176	0.0098	0.2413	0.0017
239	CASE-3	-1.445E-003	-1.363	5.664	-2.2114	-0.0030	-4.837E-004
239	CASE-3	-1.445E-003	-1.363	5.664	-2.2114	-0.0030	-4.837E-004
240	CASE-3	0.915	-1.010	8.627	3.4084	0.2804	-4.597E-005
240	CASE-3	0.915	-1.010	8.627	3.4084	0.2804	-4.597E-005
243	CASE-3	2.002	1.936E-003	9.920	-0.0031	0.0629	6.941E-004
243	CASE-3	2.002	1.936E-003	9.920	-0.0031	0.0629	6.941E-004
245	CASE-3	0.364	4.043E-004	6.759	0.0013	-0.5531	-1.397E-004
245	CASE-3	0.364	4.043E-004	6.759	0.0013	-0.5531	-1.397E-004
246	CASE-3	-1.106	-4.657E-004	1.967	0.0041	-0.2596	6.619E-004
246	CASE-3	-1.106	-4.657E-004	1.967	0.0041	-0.2596	6.619E-004
249	CASE-3	-1.684E-003	4.068	-10.806	-1.8309	-7.563E-004	-2.768E-004
249	CASE-3	-1.684E-003	4.068	-10.806	-1.8309	-7.563E-004	-2.768E-004
250	CASE-3	0.996	2.673	15.350	-0.8941	-0.4004	1.593E-004
250	CASE-3	0.996	2.673	15.350	-0.8941	-0.4004	1.593E-004
253	CASE-3	-1.022	8.368E-004	4.747	-0.0011	0.1437	-2.171E-004
253	CASE-3	-1.022	8.368E-004	4.747	-0.0011	0.1437	-2.171E-004
308	CASE-3	-0.142	-6.119E-003	12.753	0.0351	-0.5449	2.881E-004
308	CASE-3	-0.142	-6.119E-003	12.753	0.0351	-0.5449	2.881E-004

Table 30: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
310	CASE-3	-1.170	-0.055	17.370	0.2347	-4.6217	0.0067
310	CASE-3	-1.170	-0.055	17.370	0.2347	-4.6217	0.0067
316	CASE-3	-1.848	-0.076	22.507	0.3154	-7.3103	-0.0057
316	CASE-3	-1.848	-0.076	22.507	0.3154	-7.3103	-0.0057
1	CASE-4	-0.740	-2.932	16.863	11.5902	-2.8823	0.0186
1	CASE-4	-0.740	-2.932	16.863	11.5902	-2.8823	0.0186
3	CASE-4	2.007	-3.545	44.827	13.6928	7.9751	-0.0095
3	CASE-4	2.007	-3.545	44.827	13.6928	7.9751	-0.0095
5	CASE-4	-2.601	-3.358	35.972	12.9817	-10.1969	0.0015
5	CASE-4	-2.601	-3.358	35.972	12.9817	-10.1969	0.0015
15	CASE-4	-0.061	0.859	10.579	-3.4695	-0.2130	-0.0019
15	CASE-4	-0.061	0.859	10.579	-3.4695	-0.2130	-0.0019
17	CASE-4	1.593	0.911	19.129	-3.6844	0.1858	0.0056
17	CASE-4	1.593	0.911	19.129	-3.6844	0.1858	0.0056
19	CASE-4	-0.819	4.493	20.301	-13.7937	-3.2582	-0.0430
19	CASE-4	-0.819	4.493	20.301	-13.7937	-3.2582	-0.0430
23	CASE-4	0.110	-6.425	27.387	24.5977	0.4507	0.0106
23	CASE-4	0.110	-6.425	27.387	24.5977	0.4507	0.0106
25	CASE-4	-0.967	-3.322	37.244	13.0186	-3.8339	0.0015
25	CASE-4	-0.967	-3.322	37.244	13.0186	-3.8339	0.0015
29	CASE-4	0.421	0.473	15.490	-1.8554	1.6661	0.0070
29	CASE-4	0.421	0.473	15.490	-1.8554	1.6661	0.0070
31	CASE-4	-0.166	2.700	17.554	-10.6289	-0.6601	-0.0019
31	CASE-4	-0.166	2.700	17.554	-10.6289	-0.6601	-0.0019
33	CASE-4	0.174	2.045	15.691	-8.0396	0.6908	-9.533E-004
33	CASE-4	0.174	2.045	15.691	-8.0396	0.6908	-9.533E-004
35	CASE-4	0.077	0.733	4.370	-2.8591	0.3022	-0.0059
35	CASE-4	0.077	0.733	4.370	-2.8591	0.3022	-0.0059
39	CASE-4	-0.313	-0.108	6.739	0.5065	-1.2689	-0.0011
39	CASE-4	-0.313	-0.108	6.739	0.5065	-1.2689	-0.0011
41	CASE-4	0.131	-0.317	7.875	1.2876	0.5173	0.0056
41	CASE-4	0.131	-0.317	7.875	1.2876	0.5173	0.0056
43	CASE-4	-1.039	0.121	23.081	-0.4479	-4.0691	-0.0013
43	CASE-4	-1.039	0.121	23.081	-0.4479	-4.0691	-0.0013
45	CASE-4	0.024	-0.715	5.966	2.8730	0.0964	-0.0042
45	CASE-4	0.024	-0.715	5.966	2.8730	0.0964	-0.0042
47	CASE-4	0.336	-0.291	19.698	1.2234	1.3291	-0.0065
47	CASE-4	0.336	-0.291	19.698	1.2234	1.3291	-0.0065
49	CASE-4	0.016	-0.421	2.981	1.7257	0.0571	2.805E-004
49	CASE-4	0.016	-0.421	2.981	1.7257	0.0571	2.805E-004
51	CASE-4	0.330	-0.050	17.731	0.2773	1.3018	-0.0030
51	CASE-4	0.330	-0.050	17.731	0.2773	1.3018	-0.0030
53	CASE-4	-0.030	-0.680	5.054	2.7548	-0.1279	0.0018
53	CASE-4	-0.030	-0.680	5.054	2.7548	-0.1279	0.0018
55	CASE-4	-0.148	0.789	9.911	-2.9991	-0.5938	0.0048
55	CASE-4	-0.148	0.789	9.911	-2.9991	-0.5938	0.0048
57	CASE-4	-0.172	-0.315	4.242	1.2830	-0.6647	-0.0016
57	CASE-4	-0.172	-0.315	4.242	1.2830	-0.6647	-0.0016
59	CASE-4	0.315	-1.294	11.147	5.1667	1.2487	-0.0077
59	CASE-4	0.315	-1.294	11.147	5.1667	1.2487	-0.0077
65	CASE-4	3.131	1.011	16.008	-2.1883	12.4348	0.0295
65	CASE-4	3.131	1.011	16.008	-2.1883	12.4348	0.0295
67	CASE-4	-0.301	-0.107	8.678	1.4336	-1.2188	-0.0692
67	CASE-4	-0.301	-0.107	8.678	1.4336	-1.2188	-0.0692

Table 30: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
69	CASE-4	0.112	-0.075	6.867	0.3383	0.4438	-0.0036
69	CASE-4	0.112	-0.075	6.867	0.3383	0.4438	-0.0036
71	CASE-4	0.078	0.255	3.240	-0.9915	0.3262	0.0028
71	CASE-4	0.078	0.255	3.240	-0.9915	0.3262	0.0028
73	CASE-4	-0.184	0.176	0.504	-0.6787	-0.7162	4.586E-004
73	CASE-4	-0.184	0.176	0.504	-0.6787	-0.7162	4.586E-004
75	CASE-4	0.442	-0.708	10.270	2.8378	1.7559	-0.0019
75	CASE-4	0.442	-0.708	10.270	2.8378	1.7559	-0.0019
77	CASE-4	-0.324	-2.492	16.502	9.9440	-1.2723	0.0019
77	CASE-4	-0.324	-2.492	16.502	9.9440	-1.2723	0.0019
79	CASE-4	0.633	-2.384	20.142	9.5085	2.4801	-2.495E-004
79	CASE-4	0.633	-2.384	20.142	9.5085	2.4801	-2.495E-004
81	CASE-4	-1.285	-3.446	36.474	13.7403	-5.1514	0.0087
81	CASE-4	-1.285	-3.446	36.474	13.7403	-5.1514	0.0087
83	CASE-4	-1.152	1.395	33.234	-5.4907	-4.5886	0.0026
83	CASE-4	-1.152	1.395	33.234	-5.4907	-4.5886	0.0026
85	CASE-4	-0.155	0.925	6.217	-3.6702	-0.6098	0.0060
85	CASE-4	-0.155	0.925	6.217	-3.6702	-0.6098	0.0060
91	CASE-4	0.134	5.904	39.649	-22.0126	0.5344	0.0098
91	CASE-4	0.134	5.904	39.649	-22.0126	0.5344	0.0098
93	CASE-4	-0.503	1.117	36.904	-4.3413	-1.9914	0.0093
93	CASE-4	-0.503	1.117	36.904	-4.3413	-1.9914	0.0093
95	CASE-4	1.076	-0.140	35.980	0.5895	4.2996	-0.0025
95	CASE-4	1.076	-0.140	35.980	0.5895	4.2996	-0.0025
97	CASE-4	0.641	-0.604	46.021	2.4410	2.5651	0.0010
97	CASE-4	0.641	-0.604	46.021	2.4410	2.5651	0.0010
99	CASE-4	-0.036	0.022	4.380	-0.0722	-0.4787	-0.0416
99	CASE-4	-0.036	0.022	4.380	-0.0722	-0.4787	-0.0416
101	CASE-4	0.274	-0.044	5.371	0.1929	0.7046	0.0422
101	CASE-4	0.274	-0.044	5.371	0.1929	0.7046	0.0422
103	CASE-4	-2.096	-9.957E-004	11.929	0.0170	-8.3059	0.0031
103	CASE-4	-2.096	-9.957E-004	11.929	0.0170	-8.3059	0.0031
105	CASE-4	-0.172	8.575E-003	30.980	-0.0213	-0.6604	-8.273E-004
105	CASE-4	-0.172	8.575E-003	30.980	-0.0213	-0.6604	-8.273E-004
107	CASE-4	-0.706	0.025	20.647	-0.0871	-2.7876	-9.102E-005
107	CASE-4	-0.706	0.025	20.647	-0.0871	-2.7876	-9.102E-005
109	CASE-4	-0.142	-5.813E-003	15.134	0.0348	-0.5477	-5.272E-004
109	CASE-4	-0.142	-5.813E-003	15.134	0.0348	-0.5477	-5.272E-004
111	CASE-4	-0.325	0.049	-0.343	-0.1849	-1.2753	5.395E-004
111	CASE-4	-0.325	0.049	-0.343	-0.1849	-1.2753	5.395E-004
113	CASE-4	-0.068	0.227	51.159	-0.8672	-0.2711	-9.976E-004
113	CASE-4	-0.068	0.227	51.159	-0.8672	-0.2711	-9.976E-004
115	CASE-4	0.488	3.691	90.338	-14.5615	1.9498	0.0017
115	CASE-4	0.488	3.691	90.338	-14.5615	1.9498	0.0017
117	CASE-4	2.417E-003	0.985	44.610	-3.9024	0.0162	0.0021
117	CASE-4	2.417E-003	0.985	44.610	-3.9024	0.0162	0.0021
119	CASE-4	-0.127	0.825	39.162	-3.2716	-0.4992	-0.0011
119	CASE-4	-0.127	0.825	39.162	-3.2716	-0.4992	-0.0011
121	CASE-4	-0.160	1.383	35.387	-5.4754	-0.6365	-0.0012
121	CASE-4	-0.160	1.383	35.387	-5.4754	-0.6365	-0.0012
123	CASE-4	0.770	-7.724E-005	6.331	2.950E-004	-0.6866	-3.568E-005
123	CASE-4	0.770	-7.724E-005	6.331	2.950E-004	-0.6866	-3.568E-005
124	CASE-4	1.166	-6.070E-005	3.302	2.843E-004	1.4899	3.444E-005
124	CASE-4	1.166	-6.070E-005	3.302	2.843E-004	1.4899	3.444E-005

Table 30: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
127	CASE-4	1.133	-6.444E-005	7.341	3.231E-004	-0.5384	-3.917E-005
127	CASE-4	1.133	-6.444E-005	7.341	3.231E-004	-0.5384	-3.917E-005
128	CASE-4	1.317	-9.548E-005	3.309	3.423E-004	1.5716	4.260E-005
128	CASE-4	1.317	-9.548E-005	3.309	3.423E-004	1.5716	4.260E-005
131	CASE-4	0.814	-1.520E-004	3.315	9.432E-004	0.1573	-1.164E-004
131	CASE-4	0.814	-1.520E-004	3.315	9.432E-004	0.1573	-1.164E-004
132	CASE-4	-0.638	-1.727E-004	2.943	9.291E-004	-0.0781	1.291E-004
132	CASE-4	-0.638	-1.727E-004	2.943	9.291E-004	-0.0781	1.291E-004
136	CASE-4	0.000	0.000	0.000	0.0000	0.0000	0.0000
136	CASE-4	0.000	0.000	0.000	0.0000	0.0000	0.0000
139	CASE-4	0.537	-7.992E-005	2.157	2.585E-004	0.0544	-3.502E-005
139	CASE-4	0.537	-7.992E-005	2.157	2.585E-004	0.0544	-3.502E-005
140	CASE-4	-0.255	-4.371E-004	2.205	0.0014	0.1742	3.435E-004
140	CASE-4	-0.255	-4.371E-004	2.205	0.0014	0.1742	3.435E-004
143	CASE-4	1.769E-004	-0.451	-0.059	0.4171	0.0014	-1.778E-004
143	CASE-4	1.769E-004	-0.451	-0.059	0.4171	0.0014	-1.778E-004
144	CASE-4	8.319E-004	-0.957	3.535	0.1055	0.0026	3.381E-004
144	CASE-4	8.319E-004	-0.957	3.535	0.1055	0.0026	3.381E-004
147	CASE-4	0.393	-1.328E-005	1.591	-3.832E-004	-0.0815	8.178E-005
147	CASE-4	0.393	-1.328E-005	1.591	-3.832E-004	-0.0815	8.178E-005
148	CASE-4	-0.579	0.963	7.102	-0.3962	0.0029	-1.413E-007
148	CASE-4	-0.579	0.963	7.102	-0.3962	0.0029	-1.413E-007
151	CASE-4	-0.201	-0.486	2.989	-0.1340	0.0552	-1.280E-005
151	CASE-4	-0.201	-0.486	2.989	-0.1340	0.0552	-1.280E-005
153	CASE-4	0.156	0.369	2.139	-0.0994	0.0421	1.471E-005
153	CASE-4	0.156	0.369	2.139	-0.0994	0.0421	1.471E-005
154	CASE-4	-0.949	-0.192	7.016	-0.2073	0.0983	-9.795E-006
154	CASE-4	-0.949	-0.192	7.016	-0.2073	0.0983	-9.795E-006
157	CASE-4	0.246	1.533	10.401	0.3125	-0.0223	1.323E-005
157	CASE-4	0.246	1.533	10.401	0.3125	-0.0223	1.323E-005
159	CASE-4	-2.424E-004	-1.210	8.782	-0.3988	-6.541E-004	-9.822E-005
159	CASE-4	-2.424E-004	-1.210	8.782	-0.3988	-6.541E-004	-9.822E-005
161	CASE-4	0.316	6.314E-005	2.941	-2.712E-005	-0.1034	5.246E-006
161	CASE-4	0.316	6.314E-005	2.941	-2.712E-005	-0.1034	5.246E-006
162	CASE-4	-1.291	-8.303E-005	3.889	1.413E-004	-0.1701	2.747E-005
162	CASE-4	-1.291	-8.303E-005	3.889	1.413E-004	-0.1701	2.747E-005
165	CASE-4	3.464E-004	-0.105	3.091	0.1941	9.848E-004	-1.382E-004
165	CASE-4	3.464E-004	-0.105	3.091	0.1941	9.848E-004	-1.382E-004
166	CASE-4	5.638E-006	-1.944	6.104	0.4818	3.995E-004	5.215E-005
166	CASE-4	5.638E-006	-1.944	6.104	0.4818	3.995E-004	5.215E-005
169	CASE-4	0.521	-2.817E-005	2.556	-1.788E-004	-0.1123	2.286E-005
169	CASE-4	0.521	-2.817E-005	2.556	-1.788E-004	-0.1123	2.286E-005
170	CASE-4	-0.288	9.836E-005	0.559	-1.693E-004	0.0430	-6.208E-005
170	CASE-4	-0.288	9.836E-005	0.559	-1.693E-004	0.0430	-6.208E-005
173	CASE-4	-1.263E-006	-0.793	2.657	0.1352	-7.472E-007	-8.201E-007
173	CASE-4	-1.263E-006	-0.793	2.657	0.1352	-7.472E-007	-8.201E-007
174	CASE-4	0.338	-0.282	3.034	0.2326	-0.0262	-1.022E-005
174	CASE-4	0.338	-0.282	3.034	0.2326	-0.0262	-1.022E-005
177	CASE-4	-0.395	-2.808E-006	2.585	3.537E-005	-0.1249	4.729E-006
177	CASE-4	-0.395	-2.808E-006	2.585	3.537E-005	-0.1249	4.729E-006
179	CASE-4	-6.320E-003	-0.760	3.495	0.0694	-4.188E-004	3.045E-005
179	CASE-4	-6.320E-003	-0.760	3.495	0.0694	-4.188E-004	3.045E-005
180	CASE-4	4.261E-003	0.506	3.386	0.0446	-2.197E-004	-1.825E-005
180	CASE-4	4.261E-003	0.506	3.386	0.0446	-2.197E-004	-1.825E-005

Table 30: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
183	CASE-4	-4.397E-003	-0.766	3.484	0.0555	-4.381E-004	-2.335E-005
183	CASE-4	-4.397E-003	-0.766	3.484	0.0555	-4.381E-004	-2.335E-005
184	CASE-4	0.172	0.303	4.684	0.0521	-0.2852	-1.015E-005
184	CASE-4	0.172	0.303	4.684	0.0521	-0.2852	-1.015E-005
187	CASE-4	-0.370	1.725	7.061	0.0273	0.1045	2.007E-005
187	CASE-4	-0.370	1.725	7.061	0.0273	0.1045	2.007E-005
189	CASE-4	1.076	0.294	13.748	0.2341	0.0682	-1.128E-004
189	CASE-4	1.076	0.294	13.748	0.2341	0.0682	-1.128E-004
191	CASE-4	-1.687	3.642E-004	9.084	-7.766E-004	-0.2543	-9.798E-005
191	CASE-4	-1.687	3.642E-004	9.084	-7.766E-004	-0.2543	-9.798E-005
193	CASE-4	3.199E-003	0.548	2.994	-1.2996	0.0076	3.594E-005
193	CASE-4	3.199E-003	0.548	2.994	-1.2996	0.0076	3.594E-005
194	CASE-4	0.446	1.181	3.987	-1.4093	0.0543	-4.625E-006
194	CASE-4	0.446	1.181	3.987	-1.4093	0.0543	-4.625E-006
197	CASE-4	-0.193	0.420	3.405	-0.0788	0.0300	-9.633E-007
197	CASE-4	-0.193	0.420	3.405	-0.0788	0.0300	-9.633E-007
199	CASE-4	-5.400E-005	-0.383	2.982	-0.0115	-1.226E-004	-1.678E-005
199	CASE-4	-5.400E-005	-0.383	2.982	-0.0115	-1.226E-004	-1.678E-005
201	CASE-4	1.180	-0.441	6.873	0.1045	0.2993	-1.692E-007
201	CASE-4	1.180	-0.441	6.873	0.1045	0.2993	-1.692E-007
202	CASE-4	-0.700	1.296E-004	3.507	-1.611E-004	-0.0726	-9.885E-006
202	CASE-4	-0.700	1.296E-004	3.507	-1.611E-004	-0.0726	-9.885E-006
209	CASE-4	0.460	0.426	6.492	0.0387	-0.0073	1.892E-006
209	CASE-4	0.460	0.426	6.492	0.0387	-0.0073	1.892E-006
211	CASE-4	-0.595	0.538	5.585	-0.0678	-0.0844	-9.478E-007
211	CASE-4	-0.595	0.538	5.585	-0.0678	-0.0844	-9.478E-007
213	CASE-4	-0.713	-0.180	4.441	-0.0391	-0.0852	-7.097E-006
213	CASE-4	-0.713	-0.180	4.441	-0.0391	-0.0852	-7.097E-006
215	CASE-4	0.627	7.974E-005	2.996	-4.960E-004	0.0968	8.655E-005
215	CASE-4	0.627	7.974E-005	2.996	-4.960E-004	0.0968	8.655E-005
217	CASE-4	7.866E-005	1.343	-1.032	-1.3073	2.708E-004	3.601E-005
217	CASE-4	7.866E-005	1.343	-1.032	-1.3073	2.708E-004	3.601E-005
218	CASE-4	3.417E-005	1.226	2.952	-1.0928	2.047E-004	-2.679E-005
218	CASE-4	3.417E-005	1.226	2.952	-1.0928	2.047E-004	-2.679E-005
221	CASE-4	-1.567	1.396E-003	7.633	4.016E-004	-0.2037	1.773E-005
221	CASE-4	-1.567	1.396E-003	7.633	4.016E-004	-0.2037	1.773E-005
223	CASE-4	-1.378	0.548	3.672	-0.0229	-0.0586	6.433E-005
223	CASE-4	-1.378	0.548	3.672	-0.0229	-0.0586	6.433E-005
224	CASE-4	-0.048	-3.888E-003	3.937	0.2635	-0.3741	3.797E-008
224	CASE-4	-0.048	-3.888E-003	3.937	0.2635	-0.3741	3.797E-008
227	CASE-4	0.487	-0.889	10.657	-0.6635	-1.0927	-4.709E-005
227	CASE-4	0.487	-0.889	10.657	-0.6635	-1.0927	-4.709E-005
229	CASE-4	-2.172	-0.013	8.050	-0.0022	0.3984	6.817E-005
229	CASE-4	-2.172	-0.013	8.050	-0.0022	0.3984	6.817E-005
231	CASE-4	0.025	1.042E-004	1.303	2.167E-004	0.0658	-1.906E-005
231	CASE-4	0.025	1.042E-004	1.303	2.167E-004	0.0658	-1.906E-005
232	CASE-4	0.979	-1.342E-003	-1.247	6.241E-004	0.2529	7.075E-005
232	CASE-4	0.979	-1.342E-003	-1.247	6.241E-004	0.2529	7.075E-005
235	CASE-4	1.110	-9.230E-004	5.324	0.0038	-0.0650	-6.240E-004
235	CASE-4	1.110	-9.230E-004	5.324	0.0038	-0.0650	-6.240E-004
236	CASE-4	-1.064	-5.928E-004	5.452	0.0057	0.1819	0.0010
236	CASE-4	-1.064	-5.928E-004	5.452	0.0057	0.1819	0.0010
239	CASE-4	-9.263E-004	-1.513	5.868	-1.1272	-0.0019	-3.069E-004
239	CASE-4	-9.263E-004	-1.513	5.868	-1.1272	-0.0019	-3.069E-004

Table 30: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
240	CASE-4	0.696	0.084	7.762	1.7530	0.2279	-2.104E-005
240	CASE-4	0.696	0.084	7.762	1.7530	0.2279	-2.104E-005
243	CASE-4	1.691	1.422E-003	8.165	-0.0023	0.0714	5.329E-004
243	CASE-4	1.691	1.422E-003	8.165	-0.0023	0.0714	5.329E-004
245	CASE-4	0.443	1.792E-004	5.569	7.292E-004	-0.3623	-7.795E-005
245	CASE-4	0.443	1.792E-004	5.569	7.292E-004	-0.3623	-7.795E-005
246	CASE-4	-0.887	-2.437E-004	2.266	0.0023	-0.1591	3.714E-004
246	CASE-4	-0.887	-2.437E-004	2.266	0.0023	-0.1591	3.714E-004
249	CASE-4	-1.048E-003	2.592	-6.761	-1.1390	-4.607E-004	-1.707E-004
249	CASE-4	-1.048E-003	2.592	-6.761	-1.1390	-4.607E-004	-1.707E-004
250	CASE-4	0.879	1.872	11.263	-0.6666	-0.2277	1.015E-004
250	CASE-4	0.879	1.872	11.263	-0.6666	-0.2277	1.015E-004
253	CASE-4	-1.040	1.005E-003	4.435	-7.149E-004	0.1035	-1.463E-004
253	CASE-4	-1.040	1.005E-003	4.435	-7.149E-004	0.1035	-1.463E-004
308	CASE-4	-0.192	-2.348E-003	12.763	0.0212	-0.7436	-3.479E-004
308	CASE-4	-0.192	-2.348E-003	12.763	0.0212	-0.7436	-3.479E-004
310	CASE-4	-0.877	-0.021	15.142	0.0948	-3.4689	0.0046
310	CASE-4	-0.877	-0.021	15.142	0.0948	-3.4689	0.0046
316	CASE-4	-1.378	-0.047	18.772	0.2022	-5.4533	-0.0023
316	CASE-4	-1.378	-0.047	18.772	0.2022	-5.4533	-0.0023
1	CASE-5	-1.026	-3.941	18.240	15.4748	-3.9864	0.0408
1	CASE-5	-1.026	-3.941	18.240	15.4748	-3.9864	0.0408
3	CASE-5	2.761	-4.612	48.921	17.7145	11.0043	-0.0201
3	CASE-5	2.761	-4.612	48.921	17.7145	11.0043	-0.0201
5	CASE-5	-3.564	-4.442	39.321	17.0926	-13.9145	-0.0068
5	CASE-5	-3.564	-4.442	39.321	17.0926	-13.9145	-0.0068
15	CASE-5	-0.118	1.243	11.901	-5.0218	-0.4247	-0.0042
15	CASE-5	-0.118	1.243	11.901	-5.0218	-0.4247	-0.0042
17	CASE-5	1.716	1.307	20.435	-5.2955	0.0963	0.0068
17	CASE-5	1.716	1.307	20.435	-5.2955	0.0963	0.0068
19	CASE-5	-1.176	4.466	21.942	-14.1758	-4.6692	-0.0457
19	CASE-5	-1.176	4.466	21.942	-14.1758	-4.6692	-0.0457
23	CASE-5	0.115	-7.802	32.983	29.5101	0.4803	0.0197
23	CASE-5	0.115	-7.802	32.983	29.5101	0.4803	0.0197
25	CASE-5	-1.316	-3.855	41.332	14.9878	-5.2160	0.0030
25	CASE-5	-1.316	-3.855	41.332	14.9878	-5.2160	0.0030
29	CASE-5	0.627	0.811	18.769	-3.2013	2.4805	0.0105
29	CASE-5	0.627	0.811	18.769	-3.2013	2.4805	0.0105
31	CASE-5	-0.263	3.874	20.941	-15.2582	-1.0491	-0.0041
31	CASE-5	-0.263	3.874	20.941	-15.2582	-1.0491	-0.0041
33	CASE-5	0.251	2.901	19.103	-11.4168	0.9961	-0.0013
33	CASE-5	0.251	2.901	19.103	-11.4168	0.9961	-0.0013
35	CASE-5	0.066	1.070	4.607	-4.1877	0.2567	-0.0030
35	CASE-5	0.066	1.070	4.607	-4.1877	0.2567	-0.0030
39	CASE-5	-0.464	-0.156	8.017	0.6985	-1.8745	-0.0023
39	CASE-5	-0.464	-0.156	8.017	0.6985	-1.8745	-0.0023
41	CASE-5	0.202	-0.384	8.986	1.5512	0.7921	0.0084
41	CASE-5	0.202	-0.384	8.986	1.5512	0.7921	0.0084
43	CASE-5	-1.271	0.121	24.264	-0.4480	-4.9803	-0.0016
43	CASE-5	-1.271	0.121	24.264	-0.4480	-4.9803	-0.0016
45	CASE-5	0.042	-0.940	6.717	3.7414	0.1641	-0.0011
45	CASE-5	0.042	-0.940	6.717	3.7414	0.1641	-0.0011
47	CASE-5	0.468	-0.335	21.961	1.3869	1.8480	-0.0101
47	CASE-5	0.468	-0.335	21.961	1.3869	1.8480	-0.0101

Table 30: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
49	CASE-5	0.040	-0.556	2.881	2.2424	0.1450	0.0045
49	CASE-5	0.040	-0.556	2.881	2.2424	0.1450	0.0045
51	CASE-5	0.491	0.020	20.796	-5.647E-005	1.9394	-0.0053
51	CASE-5	0.491	0.020	20.796	-5.647E-005	1.9394	-0.0053
53	CASE-5	-0.076	-0.765	5.011	3.0822	-0.3160	0.0057
53	CASE-5	-0.076	-0.765	5.011	3.0822	-0.3160	0.0057
55	CASE-5	-0.191	0.998	11.079	-3.8184	-0.7667	0.0077
55	CASE-5	-0.191	0.998	11.079	-3.8184	-0.7667	0.0077
57	CASE-5	-0.281	-0.383	4.384	1.5532	-1.0957	-0.0016
57	CASE-5	-0.281	-0.383	4.384	1.5532	-1.0957	-0.0016
59	CASE-5	0.422	-1.878	13.372	7.4877	1.6660	-0.0070
59	CASE-5	0.422	-1.878	13.372	7.4877	1.6660	-0.0070
65	CASE-5	3.631	1.064	17.401	-2.0680	14.3901	0.0478
65	CASE-5	3.631	1.064	17.401	-2.0680	14.3901	0.0478
67	CASE-5	-0.372	-0.256	8.454	2.1549	-1.5003	-0.0791
67	CASE-5	-0.372	-0.256	8.454	2.1549	-1.5003	-0.0791
69	CASE-5	0.166	-0.103	6.834	0.4564	0.6613	-0.0067
69	CASE-5	0.166	-0.103	6.834	0.4564	0.6613	-0.0067
71	CASE-5	0.165	0.352	2.919	-1.3703	0.6835	0.0057
71	CASE-5	0.165	0.352	2.919	-1.3703	0.6835	0.0057
73	CASE-5	-0.281	0.268	0.251	-1.0429	-1.0953	0.0017
73	CASE-5	-0.281	0.268	0.251	-1.0429	-1.0953	0.0017
75	CASE-5	0.676	-1.050	11.534	4.2034	2.6851	-0.0014
75	CASE-5	0.676	-1.050	11.534	4.2034	2.6851	-0.0014
77	CASE-5	-0.519	-3.830	19.700	15.2670	-2.0396	0.0066
77	CASE-5	-0.519	-3.830	19.700	15.2670	-2.0396	0.0066
79	CASE-5	0.929	-3.620	23.425	14.4195	3.6492	-0.0014
79	CASE-5	0.929	-3.620	23.425	14.4195	3.6492	-0.0014
81	CASE-5	-1.936	-4.880	40.075	19.3893	-7.7375	0.0094
81	CASE-5	-1.936	-4.880	40.075	19.3893	-7.7375	0.0094
83	CASE-5	-1.684	2.048	37.405	-8.0830	-6.7055	0.0040
83	CASE-5	-1.684	2.048	37.405	-8.0830	-6.7055	0.0040
85	CASE-5	-0.176	1.328	6.885	-5.2806	-0.6855	0.0071
85	CASE-5	-0.176	1.328	6.885	-5.2806	-0.6855	0.0071
91	CASE-5	0.258	7.818	47.321	-29.1587	1.0362	0.0129
91	CASE-5	0.258	7.818	47.321	-29.1587	1.0362	0.0129
93	CASE-5	-0.753	2.002	42.553	-7.8542	-2.9634	0.0174
93	CASE-5	-0.753	2.002	42.553	-7.8542	-2.9634	0.0174
95	CASE-5	1.603	-0.219	39.711	0.9073	6.4099	-0.0026
95	CASE-5	1.603	-0.219	39.711	0.9073	6.4099	-0.0026
97	CASE-5	0.935	-0.852	53.999	3.4307	3.7408	0.0025
97	CASE-5	0.935	-0.852	53.999	3.4307	3.7408	0.0025
99	CASE-5	-0.051	0.032	4.588	-0.1088	-0.5376	-0.0425
99	CASE-5	-0.051	0.032	4.588	-0.1088	-0.5376	-0.0425
101	CASE-5	0.267	-0.057	5.308	0.2472	0.6646	0.0389
101	CASE-5	0.267	-0.057	5.308	0.2472	0.6646	0.0389
103	CASE-5	-2.435	0.015	12.430	-0.0440	-9.6364	0.0040
103	CASE-5	-2.435	0.015	12.430	-0.0440	-9.6364	0.0040
105	CASE-5	0.017	0.031	35.772	-0.1115	0.0935	-0.0016
105	CASE-5	0.017	0.031	35.772	-0.1115	0.0935	-0.0016
107	CASE-5	0.156	0.030	21.065	-0.1053	0.6444	6.089E-005
107	CASE-5	0.156	0.030	21.065	-0.1053	0.6444	6.089E-005
109	CASE-5	-0.367	-4.375E-004	16.661	0.0127	-1.4393	-4.740E-004
109	CASE-5	-0.367	-4.375E-004	16.661	0.0127	-1.4393	-4.740E-004

Table 30: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
111	CASE-5	-0.367	0.082	-1.256	-0.3189	-1.4381	9.744E-004
111	CASE-5	-0.367	0.082	-1.256	-0.3189	-1.4381	9.744E-004
113	CASE-5	-0.031	0.478	55.925	-1.8584	-0.1297	-0.0026
113	CASE-5	-0.031	0.478	55.925	-1.8584	-0.1297	-0.0026
115	CASE-5	0.808	4.444	99.769	-17.5286	3.2121	0.0033
115	CASE-5	0.808	4.444	99.769	-17.5286	3.2121	0.0033
117	CASE-5	-0.028	1.604	50.065	-6.3751	-0.1048	0.0018
117	CASE-5	-0.028	1.604	50.065	-6.3751	-0.1048	0.0018
119	CASE-5	-0.198	1.302	43.919	-5.1806	-0.7795	-0.0013
119	CASE-5	-0.198	1.302	43.919	-5.1806	-0.7795	-0.0013
121	CASE-5	-0.180	2.128	40.231	-8.4344	-0.7128	-0.0014
121	CASE-5	-0.180	2.128	40.231	-8.4344	-0.7128	-0.0014
123	CASE-5	0.523	-8.979E-005	6.549	3.697E-004	-1.1940	-4.512E-005
123	CASE-5	0.523	-8.979E-005	6.549	3.697E-004	-1.1940	-4.512E-005
124	CASE-5	1.665	-8.794E-005	3.213	3.688E-004	2.0874	4.396E-005
124	CASE-5	1.665	-8.794E-005	3.213	3.688E-004	2.0874	4.396E-005
127	CASE-5	1.172	-8.697E-005	7.832	4.607E-004	-0.6776	-5.544E-005
127	CASE-5	1.172	-8.697E-005	7.832	4.607E-004	-0.6776	-5.544E-005
128	CASE-5	1.544	-1.463E-004	3.445	4.974E-004	1.8179	6.192E-005
128	CASE-5	1.544	-1.463E-004	3.445	4.974E-004	1.8179	6.192E-005
131	CASE-5	0.830	-2.328E-004	3.334	0.0014	0.1417	-1.697E-004
131	CASE-5	0.830	-2.328E-004	3.334	0.0014	0.1417	-1.697E-004
132	CASE-5	-0.709	-2.131E-004	3.215	0.0012	-0.0852	1.685E-004
132	CASE-5	-0.709	-2.131E-004	3.215	0.0012	-0.0852	1.685E-004
136	CASE-5	0.000	0.000	0.000	0.0000	0.0000	0.0000
136	CASE-5	0.000	0.000	0.000	0.0000	0.0000	0.0000
139	CASE-5	0.496	-6.375E-005	2.042	2.309E-004	0.0424	-2.848E-005
139	CASE-5	0.496	-6.375E-005	2.042	2.309E-004	0.0424	-2.848E-005
140	CASE-5	-0.254	-6.273E-004	2.585	0.0021	0.2363	5.145E-004
140	CASE-5	-0.254	-6.273E-004	2.585	0.0021	0.2363	5.145E-004
143	CASE-5	2.667E-004	-0.618	-0.766	0.5739	0.0020	-2.514E-004
143	CASE-5	2.667E-004	-0.618	-0.766	0.5739	0.0020	-2.514E-004
144	CASE-5	1.095E-003	-0.732	3.361	-0.0848	0.0034	4.537E-004
144	CASE-5	1.095E-003	-0.732	3.361	-0.0848	0.0034	4.537E-004
147	CASE-5	0.343	-6.922E-005	1.556	-4.877E-004	-0.1065	1.017E-004
147	CASE-5	0.343	-6.922E-005	1.556	-4.877E-004	-0.1065	1.017E-004
148	CASE-5	-0.513	1.265	7.706	-0.6597	0.0894	2.370E-007
148	CASE-5	-0.513	1.265	7.706	-0.6597	0.0894	2.370E-007
151	CASE-5	-0.153	-0.370	2.992	-0.2700	0.1113	-2.050E-005
151	CASE-5	-0.153	-0.370	2.992	-0.2700	0.1113	-2.050E-005
153	CASE-5	0.147	0.345	2.122	-0.1159	0.0491	2.162E-005
153	CASE-5	0.147	0.345	2.122	-0.1159	0.0491	2.162E-005
154	CASE-5	-1.146	-0.155	7.788	-0.2583	0.0928	-1.437E-005
154	CASE-5	-1.146	-0.155	7.788	-0.2583	0.0928	-1.437E-005
157	CASE-5	0.080	1.529	11.134	0.5370	-0.0670	1.816E-005
157	CASE-5	0.080	1.529	11.134	0.5370	-0.0670	1.816E-005
159	CASE-5	-3.217E-004	-1.289	9.502	-0.5501	-8.854E-004	-1.336E-004
159	CASE-5	-3.217E-004	-1.289	9.502	-0.5501	-8.854E-004	-1.336E-004
161	CASE-5	0.335	7.017E-005	2.914	-9.388E-006	-0.0999	1.988E-006
161	CASE-5	0.335	7.017E-005	2.914	-9.388E-006	-0.0999	1.988E-006
162	CASE-5	-1.252	-1.056E-004	3.815	1.907E-004	-0.1584	3.773E-005
162	CASE-5	-1.252	-1.056E-004	3.815	1.907E-004	-0.1584	3.773E-005
165	CASE-5	4.746E-004	-0.132	3.688	0.2225	0.0014	-1.950E-004
165	CASE-5	4.746E-004	-0.132	3.688	0.2225	0.0014	-1.950E-004

Table 30: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
166	CASE-5	2.644E-005	-2.284	6.840	0.6072	6.157E-004	8.179E-005
166	CASE-5	2.644E-005	-2.284	6.840	0.6072	6.157E-004	8.179E-005
169	CASE-5	0.671	-4.111E-005	2.955	-2.424E-004	-0.1186	3.340E-005
169	CASE-5	0.671	-4.111E-005	2.955	-2.424E-004	-0.1186	3.340E-005
170	CASE-5	-0.324	1.338E-004	0.616	-2.351E-004	0.0488	-8.392E-005
170	CASE-5	-0.324	1.338E-004	0.616	-2.351E-004	0.0488	-8.392E-005
173	CASE-5	-3.569E-006	-0.893	2.737	0.1679	-8.567E-006	-1.996E-006
173	CASE-5	-3.569E-006	-0.893	2.737	0.1679	-8.567E-006	-1.996E-006
174	CASE-5	0.260	-0.482	2.291	0.2944	-0.0588	-1.149E-005
174	CASE-5	0.260	-0.482	2.291	0.2944	-0.0588	-1.149E-005
177	CASE-5	-0.508	-8.526E-006	2.889	5.562E-005	-0.1721	7.143E-006
177	CASE-5	-0.508	-8.526E-006	2.889	5.562E-005	-0.1721	7.143E-006
179	CASE-5	-7.516E-003	-0.906	3.852	0.1127	-7.053E-004	4.558E-005
179	CASE-5	-7.516E-003	-0.906	3.852	0.1127	-7.053E-004	4.558E-005
180	CASE-5	2.977E-003	0.353	3.030	0.1096	-7.011E-004	-2.562E-005
180	CASE-5	2.977E-003	0.353	3.030	0.1096	-7.011E-004	-2.562E-005
183	CASE-5	-5.887E-003	-1.025	3.982	0.1163	-8.249E-004	-3.042E-005
183	CASE-5	-5.887E-003	-1.025	3.982	0.1163	-8.249E-004	-3.042E-005
184	CASE-5	0.149	-0.032	3.904	0.1614	-0.3437	-1.919E-005
184	CASE-5	0.149	-0.032	3.904	0.1614	-0.3437	-1.919E-005
187	CASE-5	-0.350	1.814	7.289	0.1425	0.1146	1.562E-005
187	CASE-5	-0.350	1.814	7.289	0.1425	0.1146	1.562E-005
189	CASE-5	1.239	0.265	15.547	0.3723	0.0871	-1.276E-004
189	CASE-5	1.239	0.265	15.547	0.3723	0.0871	-1.276E-004
191	CASE-5	-1.867	6.382E-004	10.105	-0.0014	-0.2800	-1.783E-004
191	CASE-5	-1.867	6.382E-004	10.105	-0.0014	-0.2800	-1.783E-004
193	CASE-5	4.626E-003	0.790	2.836	-1.7388	0.0102	5.530E-005
193	CASE-5	4.626E-003	0.790	2.836	-1.7388	0.0102	5.530E-005
194	CASE-5	0.489	1.251	3.533	-1.8354	0.0698	-1.050E-005
194	CASE-5	0.489	1.251	3.533	-1.8354	0.0698	-1.050E-005
197	CASE-5	-0.140	0.508	3.569	-0.1390	0.0777	-1.146E-006
197	CASE-5	-0.140	0.508	3.569	-0.1390	0.0777	-1.146E-006
199	CASE-5	-7.416E-005	-0.242	2.602	-0.0817	-1.746E-004	-2.357E-005
199	CASE-5	-7.416E-005	-0.242	2.602	-0.0817	-1.746E-004	-2.357E-005
201	CASE-5	1.159	-0.296	6.145	0.1345	0.3048	6.308E-007
201	CASE-5	1.159	-0.296	6.145	0.1345	0.3048	6.308E-007
202	CASE-5	-0.764	2.615E-004	3.798	-4.776E-004	-0.0617	-5.516E-005
202	CASE-5	-0.764	2.615E-004	3.798	-4.776E-004	-0.0617	-5.516E-005
209	CASE-5	0.483	0.614	7.677	0.0569	-0.0306	4.720E-006
209	CASE-5	0.483	0.614	7.677	0.0569	-0.0306	4.720E-006
211	CASE-5	-0.692	0.617	6.302	-0.0595	-0.1074	-2.620E-006
211	CASE-5	-0.692	0.617	6.302	-0.0595	-0.1074	-2.620E-006
213	CASE-5	-0.674	-0.092	4.057	-0.0570	-0.0842	-5.559E-006
213	CASE-5	-0.674	-0.092	4.057	-0.0570	-0.0842	-5.559E-006
215	CASE-5	0.682	1.450E-004	3.274	-6.238E-004	0.0817	1.073E-004
215	CASE-5	0.682	1.450E-004	3.274	-6.238E-004	0.0817	1.073E-004
217	CASE-5	1.209E-004	1.908	-2.380	-1.7830	4.111E-004	5.489E-005
217	CASE-5	1.209E-004	1.908	-2.380	-1.7830	4.111E-004	5.489E-005
218	CASE-5	4.932E-005	1.525	3.400	-1.4615	3.063E-004	-3.980E-005
218	CASE-5	4.932E-005	1.525	3.400	-1.4615	3.063E-004	-3.980E-005
221	CASE-5	-1.734	1.715E-003	8.174	-3.077E-004	-0.2999	-6.089E-005
221	CASE-5	-1.734	1.715E-003	8.174	-3.077E-004	-0.2999	-6.089E-005
223	CASE-5	-1.536	0.610	3.621	0.0129	0.0311	9.274E-005
223	CASE-5	-1.536	0.610	3.621	0.0129	0.0311	9.274E-005

Table 30: Nodal Reactions

Point	OutputCase	Fx	Fy	Fz	Mx	My	Mz
		kip	kip	kip	kip-ft	kip-ft	kip-ft
224	CASE-5	-0.286	-0.110	4.048	0.4020	-0.5878	9.416E-007
224	CASE-5	-0.286	-0.110	4.048	0.4020	-0.5878	9.416E-007
227	CASE-5	0.386	-0.795	11.303	-0.9912	-1.5570	-5.825E-005
227	CASE-5	0.386	-0.795	11.303	-0.9912	-1.5570	-5.825E-005
229	CASE-5	-2.540	-0.015	9.370	-0.0038	0.6734	9.843E-005
229	CASE-5	-2.540	-0.015	9.370	-0.0038	0.6734	9.843E-005
231	CASE-5	-0.318	5.701E-004	0.293	2.714E-004	0.0887	-2.224E-005
231	CASE-5	-0.318	5.701E-004	0.293	2.714E-004	0.0887	-2.224E-005
232	CASE-5	1.368	-1.893E-003	-2.065	8.635E-004	0.3392	9.972E-005
232	CASE-5	1.368	-1.893E-003	-2.065	8.635E-004	0.3392	9.972E-005
235	CASE-5	1.108	-1.716E-003	5.707	0.0073	-0.1545	-0.0013
235	CASE-5	1.108	-1.716E-003	5.707	0.0073	-0.1545	-0.0013
236	CASE-5	-1.039	-1.195E-003	5.754	0.0089	0.2907	0.0016
236	CASE-5	-1.039	-1.195E-003	5.754	0.0089	0.2907	0.0016
239	CASE-5	-1.371E-003	-1.320	5.613	-2.0318	-0.0028	-4.650E-004
239	CASE-5	-1.371E-003	-1.320	5.613	-2.0318	-0.0028	-4.650E-004
240	CASE-5	0.932	-0.899	7.938	3.1353	0.3077	-2.183E-005
240	CASE-5	0.932	-0.899	7.938	3.1353	0.3077	-2.183E-005
243	CASE-5	1.906	1.672E-003	9.242	-0.0027	0.0985	6.026E-004
243	CASE-5	1.906	1.672E-003	9.242	-0.0027	0.0985	6.026E-004
245	CASE-5	0.405	2.945E-004	6.168	0.0010	-0.4420	-1.118E-004
245	CASE-5	0.405	2.945E-004	6.168	0.0010	-0.4420	-1.118E-004
246	CASE-5	-0.887	-3.985E-004	1.844	0.0034	-0.1036	5.430E-004
246	CASE-5	-0.887	-3.985E-004	1.844	0.0034	-0.1036	5.430E-004
249	CASE-5	-1.580E-003	3.729	-9.860	-1.7019	-7.091E-004	-2.592E-004
249	CASE-5	-1.580E-003	3.729	-9.860	-1.7019	-7.091E-004	-2.592E-004
250	CASE-5	0.979	2.517	14.566	-0.8227	-0.3722	1.481E-004
250	CASE-5	0.979	2.517	14.566	-0.8227	-0.3722	1.481E-004
253	CASE-5	-0.886	8.940E-004	3.937	-0.0017	0.2030	-3.397E-004
253	CASE-5	-0.886	8.940E-004	3.937	-0.0017	0.2030	-3.397E-004
308	CASE-5	-0.332	-1.073E-003	13.492	0.0161	-1.2986	2.815E-004
308	CASE-5	-0.332	-1.073E-003	13.492	0.0161	-1.2986	2.815E-004
310	CASE-5	-0.886	-0.038	14.252	0.1662	-3.4895	0.0061
310	CASE-5	-0.886	-0.038	14.252	0.1662	-3.4895	0.0061
316	CASE-5	-1.683	-0.082	20.524	0.3421	-6.6526	-0.0053
316	CASE-5	-1.683	-0.082	20.524	0.3421	-6.6526	-0.0053

Table 31: Integrated Wall Forces

Table 31: Integrated Wall Forces

Area	OutputCase	CaseType	Fx	Fy	Fz	Mx	My	Mz
			kip	kip	kip	kip-ft	kip-ft	kip-ft
1	CASE-1	NonStatic	2.795	-1.219E-004	15.015	5.213E-004	10.3222	2.465E-005
1	CASE-1	NonStatic	2.795	-1.219E-004	15.015	5.213E-004	10.3222	2.465E-005
2	CASE-1	NonStatic	3.663	-1.467E-004	15.912	6.208E-004	13.7158	-4.005E-005
2	CASE-1	NonStatic	3.663	-1.467E-004	15.912	6.208E-004	13.7158	-4.005E-005
3	CASE-1	NonStatic	1.452	-1.780E-003	40.631	0.0073	2.1983	-4.804E-004
3	CASE-1	NonStatic	1.452	-1.780E-003	40.631	0.0073	2.1983	-4.804E-004
5	CASE-1	NonStatic	6.212	-2.719E-003	43.724	0.0112	-64.7091	-0.0143
5	CASE-1	NonStatic	6.212	-2.719E-003	43.724	0.0112	-64.7091	-0.0143
6	CASE-1	NonStatic	3.033E-003	-1.577	12.729	24.1098	0.0122	-0.0042
6	CASE-1	NonStatic	3.033E-003	-1.577	12.729	24.1098	0.0122	-0.0042

Table 31: Integrated Wall Forces

Area	OutputCase	CaseType	Fx	Fy	Fz	Mx	My	Mz
			kip	kip	kip	kip-ft	kip-ft	kip-ft
7	CASE-1	NonStatic	-1.584	2.809E-003	71.153	-0.0111	-0.3742	-0.0029
7	CASE-1	NonStatic	-1.584	2.809E-003	71.153	-0.0111	-0.3742	-0.0029
8	CASE-1	NonStatic	0.176	0.426	16.241	2.6004	-1.0737	3.497E-006
8	CASE-1	NonStatic	0.176	0.426	16.241	2.6004	-1.0737	3.497E-006
9	CASE-1	NonStatic	0.326	0.768	14.237	1.2344	-0.5243	-2.545E-005
9	CASE-1	NonStatic	0.326	0.768	14.237	1.2344	-0.5243	-2.545E-005
10	CASE-1	NonStatic	-2.050	9.004E-005	19.876	-1.710E-004	-3.6059	2.105E-005
10	CASE-1	NonStatic	-2.050	9.004E-005	19.876	-1.710E-004	-3.6059	2.105E-005
11	CASE-1	NonStatic	-3.181E-004	1.145	57.293	22.6644	-0.0014	0.0012
11	CASE-1	NonStatic	-3.181E-004	1.145	57.293	22.6644	-0.0014	0.0012
12	CASE-1	NonStatic	-3.314	-1.105E-004	31.195	5.166E-004	-6.9052	-4.674E-004
12	CASE-1	NonStatic	-3.314	-1.105E-004	31.195	5.166E-004	-6.9052	-4.674E-004
13	CASE-1	NonStatic	6.742E-004	-6.870	33.232	26.1217	0.0027	0.0013
13	CASE-1	NonStatic	6.742E-004	-6.870	33.232	26.1217	0.0027	0.0013
14	CASE-1	NonStatic	1.286	3.443E-004	11.592	-0.0013	2.7575	6.354E-004
14	CASE-1	NonStatic	1.286	3.443E-004	11.592	-0.0013	2.7575	6.354E-004
15	CASE-1	NonStatic	3.146E-005	-2.040	10.024	8.6235	8.526E-005	4.264E-005
15	CASE-1	NonStatic	3.146E-005	-2.040	10.024	8.6235	8.526E-005	4.264E-005
16	CASE-1	NonStatic	1.158	-3.190E-005	9.602	1.439E-004	0.0653	1.252E-004
16	CASE-1	NonStatic	1.158	-3.190E-005	9.602	1.439E-004	0.0653	1.252E-004
17	CASE-1	NonStatic	7.294E-003	0.856	19.421	1.8468	-0.0151	-1.017E-004
17	CASE-1	NonStatic	7.294E-003	0.856	19.421	1.8468	-0.0151	-1.017E-004
18	CASE-1	NonStatic	1.160E-003	0.214	15.758	5.4502	-0.0308	2.152E-004
18	CASE-1	NonStatic	1.160E-003	0.214	15.758	5.4502	-0.0308	2.152E-004
19	CASE-1	NonStatic	-1.123	3.015E-004	30.168	-4.821E-004	-0.7510	0.0029
19	CASE-1	NonStatic	-1.123	3.015E-004	30.168	-4.821E-004	-0.7510	0.0029
20	CASE-1	NonStatic	0.015	13.167	30.709	-9.1380	0.0107	-1.589E-004
20	CASE-1	NonStatic	0.015	13.167	30.709	-9.1380	0.0107	-1.589E-004
21	CASE-1	NonStatic	-1.895	6.319E-004	54.631	-0.0026	-11.6806	0.0021
21	CASE-1	NonStatic	-1.895	6.319E-004	54.631	-0.0026	-11.6806	0.0021
22	CASE-1	NonStatic	0.014	2.507	12.945	-0.5326	0.0037	-5.160E-004
22	CASE-1	NonStatic	0.014	2.507	12.945	-0.5326	0.0037	-5.160E-004
23	CASE-1	NonStatic	6.369	-9.604E-004	33.250	0.0045	-14.6596	-0.0026
23	CASE-1	NonStatic	6.369	-9.604E-004	33.250	0.0045	-14.6596	-0.0026
24	CASE-1	NonStatic	-6.316E-005	0.098	11.681	4.6313	-2.247E-004	2.392E-004
24	CASE-1	NonStatic	-6.316E-005	0.098	11.681	4.6313	-2.247E-004	2.392E-004
25	CASE-1	NonStatic	3.962	-1.053E-003	30.151	0.0034	-0.3885	-0.0017
25	CASE-1	NonStatic	3.962	-1.053E-003	30.151	0.0034	-0.3885	-0.0017
27	CASE-1	NonStatic	-3.819E-005	-0.869	14.845	4.8089	-2.230E-004	-1.434E-004
27	CASE-1	NonStatic	-3.819E-005	-0.869	14.845	4.8089	-2.230E-004	-1.434E-004
28	CASE-1	NonStatic	2.312	-4.292E-004	57.244	0.0016	-16.5479	0.0024
28	CASE-1	NonStatic	2.312	-4.292E-004	57.244	0.0016	-16.5479	0.0024
29	CASE-1	NonStatic	-1.191E-004	0.249	12.276	-0.4559	-2.934E-004	9.299E-005
29	CASE-1	NonStatic	-1.191E-004	0.249	12.276	-0.4559	-2.934E-004	9.299E-005
30	CASE-1	NonStatic	-3.032	1.413E-003	26.770	-0.0016	6.4068	-0.0024
30	CASE-1	NonStatic	-3.032	1.413E-003	26.770	-0.0016	6.4068	-0.0024
31	CASE-1	NonStatic	7.747E-005	1.469	7.493	-6.6821	3.129E-004	-3.481E-005
31	CASE-1	NonStatic	7.747E-005	1.469	7.493	-6.6821	3.129E-004	-3.481E-005
32	CASE-1	NonStatic	-0.733	-6.441E-003	73.411	0.0260	5.3089	-0.0123
32	CASE-1	NonStatic	-0.733	-6.441E-003	73.411	0.0260	5.3089	-0.0123
33	CASE-1	NonStatic	-3.397	1.350	21.357	-1.7207	-4.3286	-3.624E-004
33	CASE-1	NonStatic	-3.397	1.350	21.357	-1.7207	-4.3286	-3.624E-004
34	CASE-1	NonStatic	-1.593E-003	-4.665	83.604	90.7152	-0.0065	0.0034
34	CASE-1	NonStatic	-1.593E-003	-4.665	83.604	90.7152	-0.0065	0.0034

Table 31: Integrated Wall Forces

Area	OutputCase	CaseType	Fx	Fy	Fz	Mx	My	Mz
			kip	kip	kip	kip-ft	kip-ft	kip-ft
35	CASE-1	NonStatic	-1.567	-9.352E-003	14.063	0.0280	-4.6984	-1.542E-004
35	CASE-1	NonStatic	-1.567	-9.352E-003	14.063	0.0280	-4.6984	-1.542E-004
36	CASE-1	NonStatic	2.068	-2.766E-003	-0.509	0.0096	6.9278	-6.030E-004
36	CASE-1	NonStatic	2.068	-2.766E-003	-0.509	0.0096	6.9278	-6.030E-004
37	CASE-1	NonStatic	-3.196	-0.017	132.240	0.0596	-34.4530	-0.0552
37	CASE-1	NonStatic	-3.196	-0.017	132.240	0.0596	-34.4530	-0.0552
38	CASE-1	NonStatic	-1.098E-004	-1.476	47.350	11.8134	-0.0024	0.0052
38	CASE-1	NonStatic	-1.098E-004	-1.476	47.350	11.8134	-0.0024	0.0052
39	CASE-1	NonStatic	6.130	1.137E-003	26.705	-0.0054	20.2496	-0.0070
39	CASE-1	NonStatic	6.130	1.137E-003	26.705	-0.0054	20.2496	-0.0070
40	CASE-1	NonStatic	-13.785	-8.857E-003	60.381	0.0518	104.1981	-0.0170
40	CASE-1	NonStatic	-13.785	-8.857E-003	60.381	0.0518	104.1981	-0.0170
41	CASE-1	NonStatic	-2.038E-004	4.464	-0.237	-12.5243	-4.330E-004	8.580E-004
41	CASE-1	NonStatic	-2.038E-004	4.464	-0.237	-12.5243	-4.330E-004	8.580E-004
42	CASE-1	NonStatic	3.013	-6.909E-003	146.501	-0.1364	-202.8580	-0.1346
42	CASE-1	NonStatic	3.013	-6.909E-003	146.501	-0.1364	-202.8580	-0.1346
1	CASE-2	NonStatic	2.280	-9.549E-005	12.910	4.087E-004	8.4114	2.334E-005
1	CASE-2	NonStatic	2.280	-9.549E-005	12.910	4.087E-004	8.4114	2.334E-005
2	CASE-2	NonStatic	2.906	-1.064E-004	13.560	4.524E-004	10.8776	-1.963E-005
2	CASE-2	NonStatic	2.906	-1.064E-004	13.560	4.524E-004	10.8776	-1.963E-005
3	CASE-2	NonStatic	1.177	-1.380E-003	35.934	0.0056	2.3759	-3.349E-004
3	CASE-2	NonStatic	1.177	-1.380E-003	35.934	0.0056	2.3759	-3.349E-004
5	CASE-2	NonStatic	5.463	-2.124E-003	41.609	0.0087	-57.5684	-0.0111
5	CASE-2	NonStatic	5.463	-2.124E-003	41.609	0.0087	-57.5684	-0.0111
6	CASE-2	NonStatic	1.826E-003	-2.325	12.883	20.0583	0.0074	-0.0024
6	CASE-2	NonStatic	1.826E-003	-2.325	12.883	20.0583	0.0074	-0.0024
7	CASE-2	NonStatic	-1.471	2.354E-003	62.997	-0.0093	-4.3931	-0.0021
7	CASE-2	NonStatic	-1.471	2.354E-003	62.997	-0.0093	-4.3931	-0.0021
8	CASE-2	NonStatic	0.185	0.448	14.483	1.8557	-0.7663	5.991E-006
8	CASE-2	NonStatic	0.185	0.448	14.483	1.8557	-0.7663	5.991E-006
9	CASE-2	NonStatic	0.266	0.627	12.737	1.1122	-0.4724	-1.807E-005
9	CASE-2	NonStatic	0.266	0.627	12.737	1.1122	-0.4724	-1.807E-005
10	CASE-2	NonStatic	-1.862	7.834E-005	17.547	-1.512E-004	-3.3048	3.314E-006
10	CASE-2	NonStatic	-1.862	7.834E-005	17.547	-1.512E-004	-3.3048	3.314E-006
11	CASE-2	NonStatic	-2.499E-004	1.285	50.182	18.6642	-0.0011	9.560E-004
11	CASE-2	NonStatic	-2.499E-004	1.285	50.182	18.6642	-0.0011	9.560E-004
12	CASE-2	NonStatic	-3.005	-8.433E-005	28.150	3.973E-004	-6.3277	-3.569E-004
12	CASE-2	NonStatic	-3.005	-8.433E-005	28.150	3.973E-004	-6.3277	-3.569E-004
13	CASE-2	NonStatic	5.527E-004	-5.794	29.500	21.3656	0.0022	0.0011
13	CASE-2	NonStatic	5.527E-004	-5.794	29.500	21.3656	0.0022	0.0011
14	CASE-2	NonStatic	0.988	2.801E-004	10.739	-0.0011	1.9669	5.105E-004
14	CASE-2	NonStatic	0.988	2.801E-004	10.739	-0.0011	1.9669	5.105E-004
15	CASE-2	NonStatic	2.690E-005	-1.607	9.135	6.9323	7.261E-005	3.099E-005
15	CASE-2	NonStatic	2.690E-005	-1.607	9.135	6.9323	7.261E-005	3.099E-005
16	CASE-2	NonStatic	0.884	-2.421E-005	9.017	1.132E-004	-0.2165	9.533E-005
16	CASE-2	NonStatic	0.884	-2.421E-005	9.017	1.132E-004	-0.2165	9.533E-005
17	CASE-2	NonStatic	6.419E-003	0.754	16.878	1.3538	-0.0111	-8.228E-005
17	CASE-2	NonStatic	6.419E-003	0.754	16.878	1.3538	-0.0111	-8.228E-005
18	CASE-2	NonStatic	4.046E-003	0.726	14.621	3.2419	-0.0184	1.518E-004
18	CASE-2	NonStatic	4.046E-003	0.726	14.621	3.2419	-0.0184	1.518E-004
19	CASE-2	NonStatic	-1.678	2.584E-004	29.129	-4.965E-004	5.2677	0.0017
19	CASE-2	NonStatic	-1.678	2.584E-004	29.129	-4.965E-004	5.2677	0.0017
20	CASE-2	NonStatic	9.568E-003	8.497	23.492	-4.3018	0.0051	-1.766E-004
20	CASE-2	NonStatic	9.568E-003	8.497	23.492	-4.3018	0.0051	-1.766E-004

Table 31: Integrated Wall Forces

Area	OutputCase	CaseType	Fx	Fy	Fz	Mx	My	Mz
			kip	kip	kip	kip-ft	kip-ft	kip-ft
21	CASE-2	NonStatic	-1.179	3.580E-004	40.848	-0.0015	-7.7473	0.0012
21	CASE-2	NonStatic	-1.179	3.580E-004	40.848	-0.0015	-7.7473	0.0012
22	CASE-2	NonStatic	0.012	2.033	11.833	0.5963	-0.0029	-3.255E-004
22	CASE-2	NonStatic	0.012	2.033	11.833	0.5963	-0.0029	-3.255E-004
23	CASE-2	NonStatic	4.276	-3.145E-004	33.307	0.0018	-15.0189	-0.0020
23	CASE-2	NonStatic	4.276	-3.145E-004	33.307	0.0018	-15.0189	-0.0020
24	CASE-2	NonStatic	-4.634E-005	-0.121	11.045	5.2886	-1.693E-004	1.691E-004
24	CASE-2	NonStatic	-4.634E-005	-0.121	11.045	5.2886	-1.693E-004	1.691E-004
25	CASE-2	NonStatic	3.525	-8.897E-004	27.230	0.0028	-1.2482	-0.0014
25	CASE-2	NonStatic	3.525	-8.897E-004	27.230	0.0028	-1.2482	-0.0014
27	CASE-2	NonStatic	-3.165E-005	-0.843	12.745	4.9748	-2.176E-004	-1.416E-004
27	CASE-2	NonStatic	-3.165E-005	-0.843	12.745	4.9748	-2.176E-004	-1.416E-004
28	CASE-2	NonStatic	2.116	-5.914E-004	46.025	0.0023	-37.2669	0.0025
28	CASE-2	NonStatic	2.116	-5.914E-004	46.025	0.0023	-37.2669	0.0025
29	CASE-2	NonStatic	-1.053E-004	0.195	10.990	-0.1358	-2.576E-004	9.078E-005
29	CASE-2	NonStatic	-1.053E-004	0.195	10.990	-0.1358	-2.576E-004	9.078E-005
30	CASE-2	NonStatic	-2.964	1.299E-003	24.467	-0.0013	5.7466	-0.0021
30	CASE-2	NonStatic	-2.964	1.299E-003	24.467	-0.0013	5.7466	-0.0021
31	CASE-2	NonStatic	7.998E-005	1.443	4.823	-6.2603	3.218E-004	-3.231E-005
31	CASE-2	NonStatic	7.998E-005	1.443	4.823	-6.2603	3.218E-004	-3.231E-005
32	CASE-2	NonStatic	-0.584	-5.744E-003	67.219	0.0220	3.4758	-0.0107
32	CASE-2	NonStatic	-0.584	-5.744E-003	67.219	0.0220	3.4758	-0.0107
33	CASE-2	NonStatic	-2.445	0.972	18.503	-0.8904	-2.2402	-3.164E-004
33	CASE-2	NonStatic	-2.445	0.972	18.503	-0.8904	-2.2402	-3.164E-004
34	CASE-2	NonStatic	-1.320E-003	-4.301	71.544	77.6209	-0.0054	0.0028
34	CASE-2	NonStatic	-1.320E-003	-4.301	71.544	77.6209	-0.0054	0.0028
35	CASE-2	NonStatic	-1.275	-7.612E-003	11.900	0.0227	-3.8081	-1.292E-004
35	CASE-2	NonStatic	-1.275	-7.612E-003	11.900	0.0227	-3.8081	-1.292E-004
36	CASE-2	NonStatic	1.993	-2.572E-003	1.721	0.0093	7.0718	-4.580E-004
36	CASE-2	NonStatic	1.993	-2.572E-003	1.721	0.0093	7.0718	-4.580E-004
37	CASE-2	NonStatic	-0.678	-0.012	114.987	0.0463	-13.2611	-0.0461
37	CASE-2	NonStatic	-0.678	-0.012	114.987	0.0463	-13.2611	-0.0461
38	CASE-2	NonStatic	-2.505E-005	-0.071	40.860	11.3233	-0.0017	0.0039
38	CASE-2	NonStatic	-2.505E-005	-0.071	40.860	11.3233	-0.0017	0.0039
39	CASE-2	NonStatic	3.907	9.811E-004	21.584	-0.0047	13.8444	-0.0058
39	CASE-2	NonStatic	3.907	9.811E-004	21.584	-0.0047	13.8444	-0.0058
40	CASE-2	NonStatic	-9.236	-5.024E-003	49.670	0.0309	69.1543	-0.0112
40	CASE-2	NonStatic	-9.236	-5.024E-003	49.670	0.0309	69.1543	-0.0112
41	CASE-2	NonStatic	-1.701E-004	3.496	0.490	-9.8023	-3.643E-004	7.328E-004
41	CASE-2	NonStatic	-1.701E-004	3.496	0.490	-9.8023	-3.643E-004	7.328E-004
42	CASE-2	NonStatic	1.649	-2.772E-003	118.228	-0.0771	-106.1855	-0.0915
42	CASE-2	NonStatic	1.649	-2.772E-003	118.228	-0.0771	-106.1855	-0.0915
1	CASE-3	NonStatic	2.469	-2.413E-004	10.686	9.953E-004	8.4960	-2.718E-005
1	CASE-3	NonStatic	2.469	-2.413E-004	10.686	9.953E-004	8.4960	-2.718E-005
2	CASE-3	NonStatic	3.002	-2.586E-004	11.816	0.0011	10.6884	-1.221E-004
2	CASE-3	NonStatic	3.002	-2.586E-004	11.816	0.0011	10.6884	-1.221E-004
3	CASE-3	NonStatic	0.897	-4.494E-003	34.403	0.0180	-6.6148	-7.391E-004
3	CASE-3	NonStatic	0.897	-4.494E-003	34.403	0.0180	-6.6148	-7.391E-004
5	CASE-3	NonStatic	5.853	-5.990E-003	44.856	0.0243	-91.6069	-0.0347
5	CASE-3	NonStatic	5.853	-5.990E-003	44.856	0.0243	-91.6069	-0.0347
6	CASE-3	NonStatic	4.465E-003	-1.674	5.975	26.9258	0.0179	-0.0051
6	CASE-3	NonStatic	4.465E-003	-1.674	5.975	26.9258	0.0179	-0.0051
7	CASE-3	NonStatic	-2.225	8.637E-003	60.112	-0.0348	-40.3027	-0.0082
7	CASE-3	NonStatic	-2.225	8.637E-003	60.112	-0.0348	-40.3027	-0.0082

Table 31: Integrated Wall Forces

Area	OutputCase	CaseType	Fx	Fy	Fz	Mx	My	Mz
			kip	kip	kip	kip-ft	kip-ft	kip-ft
8	CASE-3	NonStatic	0.273	0.660	15.679	1.4828	-0.6123	1.956E-004
8	CASE-3	NonStatic	0.273	0.660	15.679	1.4828	-0.6123	1.956E-004
9	CASE-3	NonStatic	0.272	0.640	11.329	1.7085	-0.7257	-1.971E-004
9	CASE-3	NonStatic	0.272	0.640	11.329	1.7085	-0.7257	-1.971E-004
10	CASE-3	NonStatic	-2.440	9.544E-005	16.932	-2.224E-004	-4.5575	1.110E-004
10	CASE-3	NonStatic	-2.440	9.544E-005	16.932	-2.224E-004	-4.5575	1.110E-004
11	CASE-3	NonStatic	-7.709E-004	2.736	52.238	19.8801	-0.0034	0.0030
11	CASE-3	NonStatic	-7.709E-004	2.736	52.238	19.8801	-0.0034	0.0030
12	CASE-3	NonStatic	-3.367	-1.324E-004	21.519	6.016E-004	-7.7887	-8.381E-004
12	CASE-3	NonStatic	-3.367	-1.324E-004	21.519	6.016E-004	-7.7887	-8.381E-004
13	CASE-3	NonStatic	1.524E-003	-6.508	30.992	22.0794	0.0061	0.0027
13	CASE-3	NonStatic	1.524E-003	-6.508	30.992	22.0794	0.0061	0.0027
14	CASE-3	NonStatic	1.735	6.504E-004	11.935	-0.0026	2.0970	0.0013
14	CASE-3	NonStatic	1.735	6.504E-004	11.935	-0.0026	2.0970	0.0013
15	CASE-3	NonStatic	9.601E-006	-2.616	7.087	10.0827	1.176E-006	4.037E-006
15	CASE-3	NonStatic	9.601E-006	-2.616	7.087	10.0827	1.176E-006	4.037E-006
16	CASE-3	NonStatic	0.201	-6.785E-005	8.484	2.736E-004	-5.0344	9.371E-005
16	CASE-3	NonStatic	0.201	-6.785E-005	8.484	2.736E-004	-5.0344	9.371E-005
17	CASE-3	NonStatic	-3.019E-003	-0.385	14.910	6.3457	-0.0525	-2.942E-004
17	CASE-3	NonStatic	-3.019E-003	-0.385	14.910	6.3457	-0.0525	-2.942E-004
18	CASE-3	NonStatic	-9.618E-003	-1.701	12.681	10.1714	-0.0576	4.890E-004
18	CASE-3	NonStatic	-9.618E-003	-1.701	12.681	10.1714	-0.0576	4.890E-004
19	CASE-3	NonStatic	-0.060	-4.098E-004	27.479	0.0022	-2.8576	0.0023
19	CASE-3	NonStatic	-0.060	-4.098E-004	27.479	0.0022	-2.8576	0.0023
20	CASE-3	NonStatic	8.334E-003	7.213	22.787	0.4506	1.586E-004	-2.094E-004
20	CASE-3	NonStatic	8.334E-003	7.213	22.787	0.4506	1.586E-004	-2.094E-004
21	CASE-3	NonStatic	-0.478	7.375E-004	40.483	-0.0035	-4.1154	0.0037
21	CASE-3	NonStatic	-0.478	7.375E-004	40.483	-0.0035	-4.1154	0.0037
22	CASE-3	NonStatic	1.819E-003	0.277	9.058	2.1997	-0.0113	-9.290E-004
22	CASE-3	NonStatic	1.819E-003	0.277	9.058	2.1997	-0.0113	-9.290E-004
23	CASE-3	NonStatic	6.809	-2.475E-003	36.005	0.0107	-36.1109	-0.0034
23	CASE-3	NonStatic	6.809	-2.475E-003	36.005	0.0107	-36.1109	-0.0034
24	CASE-3	NonStatic	-1.411E-004	0.430	9.620	0.0461	-4.899E-004	4.468E-004
24	CASE-3	NonStatic	-1.411E-004	0.430	9.620	0.0461	-4.899E-004	4.468E-004
25	CASE-3	NonStatic	2.281	-6.172E-005	24.437	-7.853E-004	-12.1458	0.0017
25	CASE-3	NonStatic	2.281	-6.172E-005	24.437	-7.853E-004	-12.1458	0.0017
27	CASE-3	NonStatic	1.206E-004	0.939	12.991	-5.8892	4.605E-004	-1.329E-004
27	CASE-3	NonStatic	1.206E-004	0.939	12.991	-5.8892	4.605E-004	-1.329E-004
28	CASE-3	NonStatic	0.268	-1.633E-003	54.102	0.0057	-37.1556	0.0122
28	CASE-3	NonStatic	0.268	-1.633E-003	54.102	0.0057	-37.1556	0.0122
29	CASE-3	NonStatic	-4.838E-005	0.938	10.890	-6.6772	-1.194E-004	1.708E-006
29	CASE-3	NonStatic	-4.838E-005	0.938	10.890	-6.6772	-1.194E-004	1.708E-006
30	CASE-3	NonStatic	-1.666	1.123E-003	22.953	6.142E-004	17.2970	-0.0029
30	CASE-3	NonStatic	-1.666	1.123E-003	22.953	6.142E-004	17.2970	-0.0029
31	CASE-3	NonStatic	3.944E-004	2.617	2.215	-21.1704	0.0016	-2.049E-004
31	CASE-3	NonStatic	3.944E-004	2.617	2.215	-21.1704	0.0016	-2.049E-004
32	CASE-3	NonStatic	9.458E-003	-2.311E-003	65.033	0.0077	5.1144	-0.0108
32	CASE-3	NonStatic	9.458E-003	-2.311E-003	65.033	0.0077	5.1144	-0.0108
33	CASE-3	NonStatic	-3.763	1.496	16.916	-0.6232	-1.5700	-8.678E-004
33	CASE-3	NonStatic	-3.763	1.496	16.916	-0.6232	-1.5700	-8.678E-004
34	CASE-3	NonStatic	-4.278E-003	-4.971	71.231	77.0933	-0.0170	0.0063
34	CASE-3	NonStatic	-4.278E-003	-4.971	71.231	77.0933	-0.0170	0.0063
35	CASE-3	NonStatic	-2.291	-0.014	14.097	0.0406	-6.7866	-5.634E-004
35	CASE-3	NonStatic	-2.291	-0.014	14.097	0.0406	-6.7866	-5.634E-004

Table 31: Integrated Wall Forces

Area	OutputCase	CaseType	Fx	Fy	Fz	Mx	My	Mz
			kip	kip	kip	kip-ft	kip-ft	kip-ft
36	CASE-3	NonStatic	2.035	-2.704E-003	-3.610	0.0103	7.6712	-0.0014
36	CASE-3	NonStatic	2.035	-2.704E-003	-3.610	0.0103	7.6712	-0.0014
37	CASE-3	NonStatic	-1.540	-0.051	107.409	0.2016	-14.0310	-0.0332
37	CASE-3	NonStatic	-1.540	-0.051	107.409	0.2016	-14.0310	-0.0332
38	CASE-3	NonStatic	-1.119E-003	-3.081	31.771	1.4297	-0.0071	0.0101
38	CASE-3	NonStatic	-1.119E-003	-3.081	31.771	1.4297	-0.0071	0.0101
39	CASE-3	NonStatic	5.644	8.901E-004	25.425	-0.0058	21.1622	-0.0087
39	CASE-3	NonStatic	5.644	8.901E-004	25.425	-0.0058	21.1622	-0.0087
40	CASE-3	NonStatic	-9.159	-0.011	43.844	0.0582	96.0365	-0.0206
40	CASE-3	NonStatic	-9.159	-0.011	43.844	0.0582	96.0365	-0.0206
41	CASE-3	NonStatic	-4.109E-004	6.743	-2.145	-18.9453	-9.841E-004	0.0020
41	CASE-3	NonStatic	-4.109E-004	6.743	-2.145	-18.9453	-9.841E-004	0.0020
42	CASE-3	NonStatic	2.455	0.022	116.740	-0.0380	87.5960	-0.0736
42	CASE-3	NonStatic	2.455	0.022	116.740	-0.0380	87.5960	-0.0736
1	CASE-4	NonStatic	1.936	-1.379E-004	9.633	5.793E-004	6.8606	3.184E-005
1	CASE-4	NonStatic	1.936	-1.379E-004	9.633	5.793E-004	6.8606	3.184E-005
2	CASE-4	NonStatic	2.451	-1.599E-004	10.649	6.655E-004	8.9297	-5.736E-005
2	CASE-4	NonStatic	2.451	-1.599E-004	10.649	6.655E-004	8.9297	-5.736E-005
3	CASE-4	NonStatic	0.969	-2.947E-003	31.639	0.0119	-1.5727	-5.960E-004
3	CASE-4	NonStatic	0.969	-2.947E-003	31.639	0.0119	-1.5727	-5.960E-004
5	CASE-4	NonStatic	4.908	-3.966E-003	40.662	0.0161	-62.8174	-0.0216
5	CASE-4	NonStatic	4.908	-3.966E-003	40.662	0.0161	-62.8174	-0.0216
6	CASE-4	NonStatic	2.758E-003	-2.204	9.427	21.3751	0.0111	-0.0033
6	CASE-4	NonStatic	2.758E-003	-2.204	9.427	21.3751	0.0111	-0.0033
7	CASE-4	NonStatic	-1.660	4.920E-003	55.845	-0.0197	-27.7584	-0.0051
7	CASE-4	NonStatic	-1.660	4.920E-003	55.845	-0.0197	-27.7584	-0.0051
8	CASE-4	NonStatic	0.213	0.517	13.780	1.4628	-0.6041	1.368E-004
8	CASE-4	NonStatic	0.213	0.517	13.780	1.4628	-0.6041	1.368E-004
9	CASE-4	NonStatic	0.173	0.409	10.323	1.5311	-0.6503	-1.384E-004
9	CASE-4	NonStatic	0.173	0.409	10.323	1.5311	-0.6503	-1.384E-004
10	CASE-4	NonStatic	-1.438	7.595E-005	14.729	-1.634E-004	-2.6146	3.843E-005
10	CASE-4	NonStatic	-1.438	7.595E-005	14.729	-1.634E-004	-2.6146	3.843E-005
11	CASE-4	NonStatic	-5.045E-004	2.465	45.561	18.2383	-0.0023	0.0021
11	CASE-4	NonStatic	-5.045E-004	2.465	45.561	18.2383	-0.0023	0.0021
12	CASE-4	NonStatic	-3.407	-8.707E-005	20.890	4.137E-004	-7.1950	-6.104E-004
12	CASE-4	NonStatic	-3.407	-8.707E-005	20.890	4.137E-004	-7.1950	-6.104E-004
13	CASE-4	NonStatic	1.051E-003	-4.893	26.610	19.3850	0.0042	0.0019
13	CASE-4	NonStatic	1.051E-003	-4.893	26.610	19.3850	0.0042	0.0019
14	CASE-4	NonStatic	1.237	3.777E-004	10.151	-0.0015	1.9815	7.835E-004
14	CASE-4	NonStatic	1.237	3.777E-004	10.151	-0.0015	1.9815	7.835E-004
15	CASE-4	NonStatic	7.302E-006	-1.763	8.132	7.5749	8.519E-006	1.272E-005
15	CASE-4	NonStatic	7.302E-006	-1.763	8.132	7.5749	8.519E-006	1.272E-005
16	CASE-4	NonStatic	0.344	-4.041E-005	8.291	1.777E-004	-2.3810	7.003E-005
16	CASE-4	NonStatic	0.344	-4.041E-005	8.291	1.777E-004	-2.3810	7.003E-005
17	CASE-4	NonStatic	-5.302E-004	-0.083	13.627	4.3618	-0.0360	-2.037E-004
17	CASE-4	NonStatic	-5.302E-004	-0.083	13.627	4.3618	-0.0360	-2.037E-004
18	CASE-4	NonStatic	-3.068E-003	-0.536	12.480	6.1175	-0.0347	3.107E-004
18	CASE-4	NonStatic	-3.068E-003	-0.536	12.480	6.1175	-0.0347	3.107E-004
19	CASE-4	NonStatic	-0.759	-9.566E-005	27.144	7.618E-004	5.8821	0.0011
19	CASE-4	NonStatic	-0.759	-9.566E-005	27.144	7.618E-004	5.8821	0.0011
20	CASE-4	NonStatic	7.158E-003	6.236	19.066	-1.0242	0.0015	-1.396E-004
20	CASE-4	NonStatic	7.158E-003	6.236	19.066	-1.0242	0.0015	-1.396E-004
21	CASE-4	NonStatic	-0.938	3.317E-004	33.186	-0.0017	-5.1029	0.0018
21	CASE-4	NonStatic	-0.938	3.317E-004	33.186	-0.0017	-5.1029	0.0018

Table 31: Integrated Wall Forces

Area	OutputCase	CaseType	Fx	Fy	Fz	Mx	My	Mz
			kip	kip	kip	kip-ft	kip-ft	kip-ft
22	CASE-4	NonStatic	2.629E-003	0.438	10.027	2.4587	-0.0132	-5.074E-004
22	CASE-4	NonStatic	2.629E-003	0.438	10.027	2.4587	-0.0132	-5.074E-004
23	CASE-4	NonStatic	4.994	-1.392E-003	36.335	0.0062	-18.8660	-0.0021
23	CASE-4	NonStatic	4.994	-1.392E-003	36.335	0.0062	-18.8660	-0.0021
24	CASE-4	NonStatic	-8.818E-005	0.111	10.064	2.7166	-3.033E-004	2.603E-004
24	CASE-4	NonStatic	-8.818E-005	0.111	10.064	2.7166	-3.033E-004	2.603E-004
25	CASE-4	NonStatic	2.166	-2.793E-004	23.061	3.272E-004	-7.9661	6.294E-004
25	CASE-4	NonStatic	2.166	-2.793E-004	23.061	3.272E-004	-7.9661	6.294E-004
27	CASE-4	NonStatic	5.928E-005	0.157	11.785	-1.6170	2.002E-004	-1.031E-004
27	CASE-4	NonStatic	5.928E-005	0.157	11.785	-1.6170	2.002E-004	-1.031E-004
28	CASE-4	NonStatic	0.848	-1.218E-003	47.448	0.0044	-36.4060	0.0082
28	CASE-4	NonStatic	0.848	-1.218E-003	47.448	0.0044	-36.4060	0.0082
29	CASE-4	NonStatic	-5.940E-005	0.571	10.183	-3.8383	-1.455E-004	2.177E-005
29	CASE-4	NonStatic	-5.940E-005	0.571	10.183	-3.8383	-1.455E-004	2.177E-005
30	CASE-4	NonStatic	-1.852	1.022E-003	21.429	-3.889E-005	11.4981	-0.0023
30	CASE-4	NonStatic	-1.852	1.022E-003	21.429	-3.889E-005	11.4981	-0.0023
31	CASE-4	NonStatic	2.375E-004	1.947	3.602	-14.2265	9.501E-004	-1.205E-004
31	CASE-4	NonStatic	2.375E-004	1.947	3.602	-14.2265	9.501E-004	-1.205E-004
32	CASE-4	NonStatic	-0.298	-2.763E-003	57.539	0.0102	3.6005	-0.0089
32	CASE-4	NonStatic	-0.298	-2.763E-003	57.539	0.0102	3.6005	-0.0089
33	CASE-4	NonStatic	-2.816	1.119	15.917	-0.7251	-1.8254	-6.178E-004
33	CASE-4	NonStatic	-2.816	1.119	15.917	-0.7251	-1.8254	-6.178E-004
34	CASE-4	NonStatic	-2.734E-003	-3.915	62.517	67.6579	-0.0110	0.0044
34	CASE-4	NonStatic	-2.734E-003	-3.915	62.517	67.6579	-0.0110	0.0044
35	CASE-4	NonStatic	-1.685	-0.010	11.772	0.0300	-5.0291	-3.421E-004
35	CASE-4	NonStatic	-1.685	-0.010	11.772	0.0300	-5.0291	-3.421E-004
36	CASE-4	NonStatic	1.770	-2.262E-003	0.306	0.0086	6.6721	-9.036E-004
36	CASE-4	NonStatic	1.770	-2.262E-003	0.306	0.0086	6.6721	-9.036E-004
37	CASE-4	NonStatic	-0.843	-0.028	95.533	0.1105	-11.1248	-0.0305
37	CASE-4	NonStatic	-0.843	-0.028	95.533	0.1105	-11.1248	-0.0305
38	CASE-4	NonStatic	-6.133E-004	-2.270	30.733	5.4165	-0.0043	0.0065
38	CASE-4	NonStatic	-6.133E-004	-2.270	30.733	5.4165	-0.0043	0.0065
39	CASE-4	NonStatic	4.546	7.827E-004	21.235	-0.0044	16.8718	-0.0061
39	CASE-4	NonStatic	4.546	7.827E-004	21.235	-0.0044	16.8718	-0.0061
40	CASE-4	NonStatic	-5.890	-6.220E-003	39.756	0.0337	63.7349	-0.0118
40	CASE-4	NonStatic	-5.890	-6.220E-003	39.756	0.0337	63.7349	-0.0118
41	CASE-4	NonStatic	-2.500E-004	4.466	-0.619	-12.5569	-5.946E-004	0.0012
41	CASE-4	NonStatic	-2.500E-004	4.466	-0.619	-12.5569	-5.946E-004	0.0012
42	CASE-4	NonStatic	1.330	0.011	100.106	-0.0356	24.8024	-0.0579
42	CASE-4	NonStatic	1.330	0.011	100.106	-0.0356	24.8024	-0.0579
1	CASE-5	NonStatic	2.188	-1.777E-004	9.763	7.385E-004	7.5655	2.563E-006
1	CASE-5	NonStatic	2.188	-1.777E-004	9.763	7.385E-004	7.5655	2.563E-006
2	CASE-5	NonStatic	2.715	-2.333E-004	11.278	9.581E-004	9.7308	-1.098E-004
2	CASE-5	NonStatic	2.715	-2.333E-004	11.278	9.581E-004	9.7308	-1.098E-004
3	CASE-5	NonStatic	0.988	-4.093E-003	33.144	0.0164	-6.7582	-1.709E-005
3	CASE-5	NonStatic	0.988	-4.093E-003	33.144	0.0164	-6.7582	-1.709E-005
5	CASE-5	NonStatic	5.360	-5.870E-003	43.535	0.0238	-88.0639	-0.0342
5	CASE-5	NonStatic	5.360	-5.870E-003	43.535	0.0238	-88.0639	-0.0342
6	CASE-5	NonStatic	3.782E-003	-1.935	7.068	24.5479	0.0152	-0.0042
6	CASE-5	NonStatic	3.782E-003	-1.935	7.068	24.5479	0.0152	-0.0042
7	CASE-5	NonStatic	-2.225	7.666E-003	57.565	-0.0308	-38.2286	-0.0063
7	CASE-5	NonStatic	-2.225	7.666E-003	57.565	-0.0308	-38.2286	-0.0063
8	CASE-5	NonStatic	0.377	0.914	14.567	0.5317	-0.2198	2.717E-004
8	CASE-5	NonStatic	0.377	0.914	14.567	0.5317	-0.2198	2.717E-004

Table 31: Integrated Wall Forces

Area	OutputCase	CaseType	Fx	Fy	Fz	Mx	My	Mz
			kip	kip	kip	kip-ft	kip-ft	kip-ft
9	CASE-5	NonStatic	0.193	0.454	10.752	1.9941	-0.8470	-2.051E-004
9	CASE-5	NonStatic	0.193	0.454	10.752	1.9941	-0.8470	-2.051E-004
10	CASE-5	NonStatic	-2.228	8.468E-005	16.067	-1.964E-004	-4.1851	9.663E-005
10	CASE-5	NonStatic	-2.228	8.468E-005	16.067	-1.964E-004	-4.1851	9.663E-005
11	CASE-5	NonStatic	-7.065E-004	2.512	49.606	19.3678	-0.0031	0.0027
11	CASE-5	NonStatic	-7.065E-004	2.512	49.606	19.3678	-0.0031	0.0027
12	CASE-5	NonStatic	-3.235	-1.442E-004	20.476	6.452E-004	-6.9297	-7.373E-004
12	CASE-5	NonStatic	-3.235	-1.442E-004	20.476	6.452E-004	-6.9297	-7.373E-004
13	CASE-5	NonStatic	1.529E-003	-5.785	30.395	20.2297	0.0061	0.0024
13	CASE-5	NonStatic	1.529E-003	-5.785	30.395	20.2297	0.0061	0.0024
14	CASE-5	NonStatic	1.882	5.153E-004	11.390	-0.0020	2.8970	0.0011
14	CASE-5	NonStatic	1.882	5.153E-004	11.390	-0.0020	2.8970	0.0011
15	CASE-5	NonStatic	-5.053E-007	-2.309	7.459	9.2799	-2.768E-005	1.497E-005
15	CASE-5	NonStatic	-5.053E-007	-2.309	7.459	9.2799	-2.768E-005	1.497E-005
16	CASE-5	NonStatic	0.126	-6.017E-005	8.341	2.537E-004	-4.4374	7.229E-005
16	CASE-5	NonStatic	0.126	-6.017E-005	8.341	2.537E-004	-4.4374	7.229E-005
17	CASE-5	NonStatic	-4.091E-003	-0.515	13.729	6.6586	-0.0550	-3.182E-004
17	CASE-5	NonStatic	-4.091E-003	-0.515	13.729	6.6586	-0.0550	-3.182E-004
18	CASE-5	NonStatic	-9.044E-003	-1.600	12.507	9.8153	-0.0555	4.422E-004
18	CASE-5	NonStatic	-9.044E-003	-1.600	12.507	9.8153	-0.0555	4.422E-004
19	CASE-5	NonStatic	-0.212	-4.568E-004	26.231	0.0023	2.9389	0.0016
19	CASE-5	NonStatic	-0.212	-4.568E-004	26.231	0.0023	2.9389	0.0016
20	CASE-5	NonStatic	8.006E-003	6.927	20.810	0.0299	4.589E-004	-1.814E-004
20	CASE-5	NonStatic	8.006E-003	6.927	20.810	0.0299	4.589E-004	-1.814E-004
21	CASE-5	NonStatic	-0.957	5.996E-004	37.154	-0.0030	-4.9704	0.0032
21	CASE-5	NonStatic	-0.957	5.996E-004	37.154	-0.0030	-4.9704	0.0032
22	CASE-5	NonStatic	-4.479E-004	-0.122	8.932	3.3635	-0.0179	-8.044E-004
22	CASE-5	NonStatic	-4.479E-004	-0.122	8.932	3.3635	-0.0179	-8.044E-004
23	CASE-5	NonStatic	6.251	-2.518E-003	35.661	0.0108	-30.8794	-0.0031
23	CASE-5	NonStatic	6.251	-2.518E-003	35.661	0.0108	-30.8794	-0.0031
24	CASE-5	NonStatic	-1.297E-004	0.371	9.578	0.7720	-4.372E-004	3.855E-004
24	CASE-5	NonStatic	-1.297E-004	0.371	9.578	0.7720	-4.372E-004	3.855E-004
25	CASE-5	NonStatic	1.973	-7.518E-006	23.238	-9.339E-004	-12.3902	0.0018
25	CASE-5	NonStatic	1.973	-7.518E-006	23.238	-9.339E-004	-12.3902	0.0018
27	CASE-5	NonStatic	1.232E-004	0.912	12.265	-5.8015	4.637E-004	-1.031E-004
27	CASE-5	NonStatic	1.232E-004	0.912	12.265	-5.8015	4.637E-004	-1.031E-004
28	CASE-5	NonStatic	0.463	-1.695E-003	51.078	0.0060	-35.8003	0.0122
28	CASE-5	NonStatic	0.463	-1.695E-003	51.078	0.0060	-35.8003	0.0122
29	CASE-5	NonStatic	-3.560E-005	0.877	10.507	-6.1330	-9.040E-005	-1.427E-006
29	CASE-5	NonStatic	-3.560E-005	0.877	10.507	-6.1330	-9.040E-005	-1.427E-006
30	CASE-5	NonStatic	-1.675	1.059E-003	21.988	5.925E-004	15.9413	-0.0028
30	CASE-5	NonStatic	-1.675	1.059E-003	21.988	5.925E-004	15.9413	-0.0028
31	CASE-5	NonStatic	3.580E-004	2.425	1.863	-20.3060	0.0014	-1.949E-004
31	CASE-5	NonStatic	3.580E-004	2.425	1.863	-20.3060	0.0014	-1.949E-004
32	CASE-5	NonStatic	-0.023	-1.912E-003	61.752	0.0055	3.6803	-0.0098
32	CASE-5	NonStatic	-0.023	-1.912E-003	61.752	0.0055	3.6803	-0.0098
33	CASE-5	NonStatic	-3.403	1.353	16.041	-0.3623	-0.9137	-8.515E-004
33	CASE-5	NonStatic	-3.403	1.353	16.041	-0.3623	-0.9137	-8.515E-004
34	CASE-5	NonStatic	-4.063E-003	-4.631	67.329	71.8054	-0.0162	0.0060
34	CASE-5	NonStatic	-4.063E-003	-4.631	67.329	71.8054	-0.0162	0.0060
35	CASE-5	NonStatic	-2.154	-0.013	13.260	0.0381	-6.3719	-5.363E-004
35	CASE-5	NonStatic	-2.154	-0.013	13.260	0.0381	-6.3719	-5.363E-004
36	CASE-5	NonStatic	1.810	-2.399E-003	-3.072	0.0095	7.1017	-0.0014
36	CASE-5	NonStatic	1.810	-2.399E-003	-3.072	0.0095	7.1017	-0.0014

Table 31: Integrated Wall Forces

Area	OutputCase	CaseType	Fx	Fy	Fz	Mx	My	Mz
			kip	kip	kip	kip-ft	kip-ft	kip-ft
37	CASE-5	NonStatic	-0.731	-0.047	101.855	0.1862	-8.6030	-0.0254
37	CASE-5	NonStatic	-0.731	-0.047	101.855	0.1862	-8.6030	-0.0254
38	CASE-5	NonStatic	-1.164E-003	-2.840	30.532	3.3832	-0.0070	0.0096
38	CASE-5	NonStatic	-1.164E-003	-2.840	30.532	3.3832	-0.0070	0.0096
39	CASE-5	NonStatic	5.374	7.196E-004	23.516	-0.0049	19.9849	-0.0075
39	CASE-5	NonStatic	5.374	7.196E-004	23.516	-0.0049	19.9849	-0.0075
40	CASE-5	NonStatic	-7.077	-9.028E-003	40.305	0.0477	84.2378	-0.0171
40	CASE-5	NonStatic	-7.077	-9.028E-003	40.305	0.0477	84.2378	-0.0171
41	CASE-5	NonStatic	-3.858E-004	6.249	-1.688	-17.5495	-9.225E-004	0.0019
41	CASE-5	NonStatic	-3.858E-004	6.249	-1.688	-17.5495	-9.225E-004	0.0019
42	CASE-5	NonStatic	1.196	0.023	106.598	-0.0110	127.0629	-0.0518
42	CASE-5	NonStatic	1.196	0.023	106.598	-0.0110	127.0629	-0.0518
1	UDCONN1	Combination	1.987	-8.328E-005	12.092	3.579E-004	7.3287	2.408E-005
2	UDCONN1	Combination	2.575	-9.198E-005	12.718	3.926E-004	9.6358	-1.499E-005
3	UDCONN1	Combination	0.857	-1.179E-003	34.180	0.0048	2.6749	-8.376E-005
5	UDCONN1	Combination	4.962	-1.619E-003	41.541	0.0067	-55.0051	-0.0078
6	UDCONN1	Combination	1.296E-003	-3.393	13.695	19.2435	0.0052	-0.0016
7	UDCONN1	Combination	-1.346	1.973E-003	60.816	-0.0078	-3.1810	-0.0019
8	UDCONN1	Combination	0.132	0.321	13.709	1.8897	-0.7803	-7.743E-006
9	UDCONN1	Combination	0.215	0.506	12.055	1.1207	-0.4760	-2.039E-005
10	UDCONN1	Combination	-1.600	7.166E-005	16.390	-1.389E-004	-2.7947	-1.833E-005
11	UDCONN1	Combination	-1.859E-004	1.648	47.154	17.0493	-8.342E-004	7.188E-004
12	UDCONN1	Combination	-3.081	-5.771E-005	28.247	2.843E-004	-6.6324	-2.646E-004
13	UDCONN1	Combination	3.086E-004	-5.107	26.492	19.2836	0.0012	4.166E-004
14	UDCONN1	Combination	0.809	2.378E-004	10.643	-9.131E-004	1.5217	4.257E-004
15	UDCONN1	Combination	2.376E-005	-1.376	9.042	6.1303	6.378E-005	2.808E-005
16	UDCONN1	Combination	0.747	-1.863E-005	8.953	9.339E-005	-0.2473	8.088E-005
17	UDCONN1	Combination	6.517E-003	0.767	16.039	0.9320	-0.0075	-7.456E-005
18	UDCONN1	Combination	7.265E-003	1.298	14.768	1.3922	-0.0079	1.190E-004
19	UDCONN1	Combination	-1.912	2.802E-004	29.959	-6.488E-004	9.6759	0.0011
20	UDCONN1	Combination	7.223E-003	6.418	20.579	-2.1535	0.0027	-2.000E-004
21	UDCONN1	Combination	-0.598	2.286E-004	35.159	-9.091E-004	-5.5306	7.059E-004
22	UDCONN1	Combination	0.010	1.813	11.394	0.6125	-0.0031	-2.553E-004
23	UDCONN1	Combination	2.707	-2.461E-004	35.457	0.0015	-16.0582	-0.0020
24	UDCONN1	Combination	-4.351E-005	-0.480	11.071	6.2646	-1.576E-004	1.277E-004
25	UDCONN1	Combination	3.046	-8.686E-004	25.842	0.0028	-0.8827	-0.0014
27	UDCONN1	Combination	-4.896E-005	-1.090	11.997	6.0290	-2.813E-004	-1.171E-004
28	UDCONN1	Combination	1.638	-5.028E-004	44.686	0.0020	-30.4295	0.0019
29	UDCONN1	Combination	-1.007E-004	-0.073	10.553	1.2708	-2.460E-004	8.795E-005
30	UDCONN1	Combination	-2.922	1.190E-003	23.606	-0.0014	3.7695	-0.0018
31	UDCONN1	Combination	7.110E-005	1.212	4.851	-3.9646	2.849E-004	-9.203E-006
32	UDCONN1	Combination	-0.321	-5.326E-003	62.098	0.0192	2.0917	-0.0097
33	UDCONN1	Combination	-2.088	0.830	18.356	-0.8320	-2.0933	-2.986E-004
34	UDCONN1	Combination	-1.196E-003	-3.927	68.006	71.4175	-0.0049	0.0026
35	UDCONN1	Combination	-1.141	-6.809E-003	11.123	0.0204	-3.4124	-1.183E-004
36	UDCONN1	Combination	1.739	-2.188E-003	5.128	0.0079	6.1683	-3.522E-004
37	UDCONN1	Combination	-0.732	-6.900E-003	108.571	0.0259	-11.3536	-0.0417
38	UDCONN1	Combination	2.376E-004	-1.640	39.666	13.1317	-4.242E-004	0.0027
39	UDCONN1	Combination	3.943	1.003E-003	21.906	-0.0045	13.7123	-0.0055
40	UDCONN1	Combination	-7.871	-3.924E-003	49.920	0.0256	65.5832	-0.0085
41	UDCONN1	Combination	-1.467E-004	2.742	1.106	-7.6889	-3.190E-004	6.606E-004
42	UDCONN1	Combination	0.120	-1.870E-003	106.592	-0.0483	-68.5420	-0.0751
1	UDCONN2	Combination	2.773	-1.125E-004	14.969	4.807E-004	10.2587	3.029E-005
2	UDCONN2	Combination	3.560	-1.255E-004	15.853	5.330E-004	13.3526	-2.075E-005

Table 31: Integrated Wall Forces

Area	OutputCase	CaseType	Fx	Fy	Fz	Mx	My	Mz
			kip	kip	kip	kip-ft	kip-ft	kip-ft
3	UDCONN2	Combination	1.273	-1.570E-003	39.816	0.0064	7.1932	1.233E-005
5	UDCONN2	Combination	6.234	-1.984E-003	45.057	0.0082	-57.3738	-0.0094
6	UDCONN2	Combination	2.065E-003	-3.863	14.472	24.6535	0.0083	-0.0027
7	UDCONN2	Combination	-1.564	2.576E-003	71.248	-0.0102	-0.5743	-0.0029
8	UDCONN2	Combination	0.169	0.410	16.127	2.4321	-1.0042	-8.850E-006
9	UDCONN2	Combination	0.253	0.597	13.870	1.4967	-0.6357	-2.791E-005
10	UDCONN2	Combination	-1.986	8.757E-005	19.279	-1.681E-004	-3.4376	-1.810E-005
11	UDCONN2	Combination	-2.368E-004	2.019	56.671	21.5076	-0.0011	9.034E-004
12	UDCONN2	Combination	-3.857	-7.198E-005	33.037	3.558E-004	-7.9830	-3.313E-004
13	UDCONN2	Combination	3.849E-004	-6.358	30.391	24.5235	0.0016	5.177E-004
14	UDCONN2	Combination	1.088	2.951E-004	11.878	-0.0011	2.2486	5.324E-004
15	UDCONN2	Combination	2.857E-005	-1.741	10.395	7.7986	7.858E-005	3.455E-005
16	UDCONN2	Combination	0.987	-2.408E-005	9.944	1.189E-004	-0.0751	1.073E-004
17	UDCONN2	Combination	8.017E-003	0.943	19.334	1.3317	-0.0108	-9.262E-005
18	UDCONN2	Combination	8.444E-003	1.510	17.666	2.1551	-0.0123	1.577E-004
19	UDCONN2	Combination	-1.939	5.170E-004	35.639	-0.0014	5.3027	0.0020
20	UDCONN2	Combination	0.012	10.796	28.964	-5.5815	0.0067	-3.221E-004
21	UDCONN2	Combination	-0.366	3.903E-004	51.629	-0.0016	-7.8672	0.0011
22	UDCONN2	Combination	0.020	3.544	13.995	-1.9716	0.0116	-4.432E-004
23	UDCONN2	Combination	4.290	-3.791E-004	34.104	0.0023	-23.1037	-0.0033
24	UDCONN2	Combination	-5.992E-005	-0.440	13.435	8.2784	-2.296E-004	2.253E-004
25	UDCONN2	Combination	3.612	-1.221E-003	31.417	0.0041	-0.2086	-0.0022
27	UDCONN2	Combination	-9.414E-005	-1.692	14.746	8.5070	-4.375E-004	-1.278E-004
28	UDCONN2	Combination	2.314	-4.838E-004	53.915	0.0020	-23.7136	0.0019
29	UDCONN2	Combination	-1.244E-004	-0.203	12.309	2.0541	-3.065E-004	1.045E-004
30	UDCONN2	Combination	-3.368	1.445E-003	27.958	-0.0018	4.5731	-0.0022
31	UDCONN2	Combination	8.589E-005	1.262	6.425	-3.7107	3.453E-004	-1.093E-005
32	UDCONN2	Combination	-0.458	-7.116E-003	75.550	0.0271	4.3052	-0.0124
33	UDCONN2	Combination	-3.177	1.263	22.375	-1.7897	-4.5022	-3.759E-004
34	UDCONN2	Combination	-1.506E-003	-4.638	84.142	89.7722	-0.0062	0.0032
35	UDCONN2	Combination	-1.451	-8.659E-003	13.907	0.0260	-4.3608	-1.468E-004
36	UDCONN2	Combination	1.922	-2.437E-003	6.042	0.0084	6.4296	-4.284E-004
37	UDCONN2	Combination	-2.686	-7.274E-003	131.017	0.0207	-33.0743	-0.0525
38	UDCONN2	Combination	4.101E-004	-3.600	49.444	16.1691	-2.422E-004	0.0034
39	UDCONN2	Combination	5.956	1.383E-003	28.825	-0.0062	20.0080	-0.0076
40	UDCONN2	Combination	-12.694	-6.370E-003	70.806	0.0423	113.0037	-0.0138
41	UDCONN2	Combination	-1.774E-004	3.349	0.916	-9.3867	-3.851E-004	7.875E-004
42	UDCONN2	Combination	2.588	-8.408E-003	142.571	-0.1441	-224.7975	-0.1240
1	UDCONU1	Combination	2.782	-1.166E-004	16.928	5.010E-004	10.2601	3.371E-005
2	UDCONU1	Combination	3.605	-1.288E-004	17.805	5.497E-004	13.4901	-2.099E-005
3	UDCONU1	Combination	1.200	-1.651E-003	47.852	0.0068	3.7448	-1.173E-004
5	UDCONU1	Combination	6.946	-2.267E-003	58.157	0.0093	-77.0071	-0.0109
6	UDCONU1	Combination	1.815E-003	-4.750	19.174	26.9410	0.0073	-0.0022
7	UDCONU1	Combination	-1.884	2.762E-003	85.142	-0.0109	-4.4534	-0.0027
8	UDCONU1	Combination	0.185	0.449	19.193	2.6456	-1.0924	-1.084E-005
9	UDCONU1	Combination	0.300	0.708	16.877	1.5690	-0.6664	-2.854E-005
10	UDCONU1	Combination	-2.239	1.003E-004	22.946	-1.944E-004	-3.9126	-2.567E-005
11	UDCONU1	Combination	-2.602E-004	2.307	66.016	23.8690	-0.0012	0.0010
12	UDCONU1	Combination	-4.313	-8.079E-005	39.546	3.980E-004	-9.2854	-3.704E-004
13	UDCONU1	Combination	4.321E-004	-7.149	37.089	26.9970	0.0017	5.832E-004
14	UDCONU1	Combination	1.132	3.329E-004	14.900	-0.0013	2.1304	5.960E-004
15	UDCONU1	Combination	3.326E-005	-1.927	12.659	8.5824	8.929E-005	3.932E-005
16	UDCONU1	Combination	1.046	-2.609E-005	12.535	1.307E-004	-0.3462	1.132E-004
17	UDCONU1	Combination	9.124E-003	1.073	22.454	1.3047	-0.0106	-1.044E-004

Table 31: Integrated Wall Forces

Area	OutputCase	CaseType	Fx	Fy	Fz	Mx	My	Mz
			kip	kip	kip	kip-ft	kip-ft	kip-ft
18	UDCONU1	Combination	0.010	1.817	20.675	1.9491	-0.0111	1.666E-004
19	UDCONU1	Combination	-2.677	3.923E-004	41.942	-9.084E-004	13.5463	0.0015
20	UDCONU1	Combination	0.010	8.985	28.810	-3.0149	0.0037	-2.801E-004
21	UDCONU1	Combination	-0.838	3.200E-004	49.223	-0.0013	-7.7428	9.883E-004
22	UDCONU1	Combination	0.014	2.539	15.951	0.8576	-0.0043	-3.574E-004
23	UDCONU1	Combination	3.790	-3.445E-004	49.640	0.0021	-22.4815	-0.0028
24	UDCONU1	Combination	-6.092E-005	-0.673	15.500	8.7705	-2.207E-004	1.788E-004
25	UDCONU1	Combination	4.265	-1.216E-003	36.179	0.0040	-1.2358	-0.0020
27	UDCONU1	Combination	-6.854E-005	-1.526	16.795	8.4406	-3.938E-004	-1.640E-004
28	UDCONU1	Combination	2.293	-7.039E-004	62.560	0.0029	-42.6013	0.0026
29	UDCONU1	Combination	-1.410E-004	-0.103	14.774	1.7791	-3.444E-004	1.231E-004
30	UDCONU1	Combination	-4.091	1.666E-003	33.048	-0.0019	5.2773	-0.0025
31	UDCONU1	Combination	9.954E-005	1.696	6.792	-5.5505	3.988E-004	-1.288E-005
32	UDCONU1	Combination	-0.449	-7.457E-003	86.937	0.0269	2.9283	-0.0136
33	UDCONU1	Combination	-2.923	1.162	25.699	-1.1648	-2.9306	-4.181E-004
34	UDCONU1	Combination	-1.674E-003	-5.498	95.208	99.9844	-0.0068	0.0036
35	UDCONU1	Combination	-1.597	-9.533E-003	15.573	0.0285	-4.7774	-1.656E-004
36	UDCONU1	Combination	2.435	-3.063E-003	7.179	0.0111	8.6356	-4.931E-004
37	UDCONU1	Combination	-1.025	-9.661E-003	151.999	0.0363	-15.8950	-0.0584
38	UDCONU1	Combination	3.327E-004	-2.296	55.532	18.3844	-5.939E-004	0.0037
39	UDCONU1	Combination	5.521	1.405E-003	30.668	-0.0064	19.1973	-0.0077
40	UDCONU1	Combination	-11.019	-5.494E-003	69.888	0.0359	91.8165	-0.0119
41	UDCONU1	Combination	-2.053E-004	3.838	1.549	-10.7644	-4.466E-004	9.248E-004
42	UDCONU1	Combination	0.168	-2.617E-003	149.229	-0.0676	-95.9588	-0.1051
1	UDCONU2	Combination	3.643	-1.467E-004	19.114	6.260E-004	13.4824	3.883E-005
2	UDCONU2	Combination	4.665	-1.641E-004	20.277	6.957E-004	17.5099	-2.720E-005
3	UDCONU2	Combination	1.695	-2.041E-003	50.034	0.0083	10.4392	5.323E-005
5	UDCONU2	Combination	7.989	-2.528E-003	55.476	0.0104	-69.7961	-0.0119
6	UDCONU2	Combination	2.786E-003	-4.823	17.677	31.7483	0.0112	-0.0038
7	UDCONU2	Combination	-1.965	3.332E-003	89.671	-0.0132	0.3536	-0.0039
8	UDCONU2	Combination	0.218	0.528	20.319	3.1355	-1.2947	-1.106E-005
9	UDCONU2	Combination	0.319	0.752	17.371	1.9465	-0.8267	-3.649E-005
10	UDCONU2	Combination	-2.537	1.114E-004	24.291	-2.135E-004	-4.3823	-2.163E-005
11	UDCONU2	Combination	-3.045E-004	2.571	71.811	27.5925	-0.0014	0.0012
12	UDCONU2	Combination	-4.939	-9.208E-005	41.560	4.556E-004	-10.1198	-4.243E-004
13	UDCONU2	Combination	4.924E-004	-8.130	38.029	31.5242	0.0020	6.617E-004
14	UDCONU2	Combination	1.417	3.770E-004	14.747	-0.0014	2.9891	6.815E-004
15	UDCONU2	Combination	3.620E-005	-2.235	13.015	10.0256	1.002E-004	4.405E-005
16	UDCONU2	Combination	1.281	-3.108E-005	12.330	1.529E-004	-0.0213	1.393E-004
17	UDCONU2	Combination	0.010	1.201	24.519	1.7580	-0.0143	-1.184E-004
18	UDCONU2	Combination	0.011	1.896	22.358	2.8913	-0.0164	2.047E-004
19	UDCONU2	Combination	-2.337	7.150E-004	45.039	-0.0020	4.6140	0.0028
20	UDCONU2	Combination	0.017	14.706	38.111	-8.0690	0.0097	-4.353E-004
21	UDCONU2	Combination	-0.346	5.330E-004	68.543	-0.0021	-10.3754	0.0015
22	UDCONU2	Combination	0.028	4.944	17.835	-3.3995	0.0199	-6.070E-004
23	UDCONU2	Combination	5.782	-5.082E-004	40.384	0.0031	-30.5426	-0.0045
24	UDCONU2	Combination	-7.847E-005	-0.511	17.067	10.7395	-3.043E-004	3.094E-004
25	UDCONU2	Combination	4.560	-1.605E-003	39.930	0.0054	0.0193	-0.0029
27	UDCONU2	Combination	-1.310E-004	-2.271	18.795	11.1996	-5.875E-004	-1.576E-004
28	UDCONU2	Combination	3.047	-5.730E-004	68.390	0.0023	-25.7700	0.0024
29	UDCONU2	Combination	-1.588E-004	-0.295	15.473	2.7782	-3.920E-004	1.321E-004
30	UDCONU2	Combination	-4.220	1.836E-003	35.290	-0.0023	5.8092	-0.0028
31	UDCONU2	Combination	1.090E-004	1.534	8.339	-4.3513	4.385E-004	-1.381E-005
32	UDCONU2	Combination	-0.605	-9.256E-003	96.041	0.0357	6.0517	-0.0160

Table 31: Integrated Wall Forces

Area	OutputCase	CaseType	Fx	Fy	Fz	Mx	My	Mz
			kip	kip	kip	kip-ft	kip-ft	kip-ft
33	UDCONU2	Combination	-4.249	1.689	28.457	-2.5308	-6.3663	-4.821E-004
34	UDCONU2	Combination	-1.931E-003	-5.849	107.425	115.0686	-0.0079	0.0041
35	UDCONU2	Combination	-1.865	-0.011	17.801	0.0335	-5.6124	-1.876E-004
36	UDCONU2	Combination	2.380	-3.025E-003	7.617	0.0102	7.8201	-5.446E-004
37	UDCONU2	Combination	-4.005	-8.877E-003	166.200	0.0227	-48.3775	-0.0674
38	UDCONU2	Combination	5.611E-004	-5.103	63.243	20.6179	-2.178E-004	0.0044
39	UDCONU2	Combination	7.952	1.812E-003	37.357	-0.0081	26.5279	-0.0099
40	UDCONU2	Combination	-17.162	-8.622E-003	93.322	0.0575	154.5727	-0.0186
41	UDCONU2	Combination	-2.252E-004	4.261	1.023	-11.9431	-4.885E-004	9.958E-004
42	UDCONU2	Combination	4.093	-0.013	185.477	-0.2112	-332.2592	-0.1684
1	LONG TERM (ACI L/240 LIMIT)	Combination	1.048	-1.297E-004	3.158	5.286E-004	3.5463	-5.771E-005
2	LONG TERM (ACI L/240 LIMIT)	Combination	1.309	-1.389E-004	3.519	5.624E-004	4.5969	-8.513E-005
3	LONG TERM (ACI L/240 LIMIT)	Combination	0.204	-1.948E-003	7.461	0.0078	-5.2198	-2.887E-004
5	LONG TERM (ACI L/240 LIMIT)	Combination	1.694	-2.619E-003	6.308	0.0106	-35.9302	-0.0164
6	LONG TERM (ACI L/240 LIMIT)	Combination	2.915E-003	1.278	-3.605	9.6022	0.0117	-0.0036
7	LONG TERM (ACI L/240 LIMIT)	Combination	-0.678	4.171E-003	12.423	-0.0169	-8.5254	-0.0040
8	LONG TERM (ACI L/240 LIMIT)	Combination	0.050	0.121	3.657	0.7646	-0.3156	5.627E-005
9	LONG TERM (ACI L/240 LIMIT)	Combination	0.158	0.372	2.505	0.2997	-0.1274	-6.608E-005
10	LONG TERM (ACI L/240 LIMIT)	Combination	-1.190	3.119E-005	4.532	-7.876E-005	-2.2440	9.032E-005
11	LONG TERM (ACI L/240 LIMIT)	Combination	-3.346E-004	0.132	13.788	5.6420	-0.0015	0.0012
12	LONG TERM (ACI L/240 LIMIT)	Combination	-0.270	-7.152E-005	3.674	3.072E-004	-1.1711	-3.381E-004
13	LONG TERM (ACI L/240 LIMIT)	Combination	5.946E-004	-2.690	8.115	7.4505	0.0024	0.0011
14	LONG TERM (ACI L/240 LIMIT)	Combination	0.796	3.369E-004	2.638	-0.0013	0.9061	6.055E-004
15	LONG TERM (ACI L/240 LIMIT)	Combination	6.858E-006	-1.286	-0.156	4.1990	5.306E-006	2.960E-006
16	LONG TERM (ACI L/240 LIMIT)	Combination	0.131	-3.513E-005	0.778	1.266E-004	-2.3716	5.357E-005
17	LONG TERM (ACI L/240 LIMIT)	Combination	-1.613E-003	-0.201	3.826	2.4768	-0.0205	-1.099E-004

Table 31: Integrated Wall Forces

Area	OutputCase	CaseType	Fx	Fy	Fz	Mx	My	Mz
			kip	kip	kip	kip-ft	kip-ft	kip-ft
18	LONG TERM (ACI L/240 LIMIT)	Combination	-9.436E-003	-1.677	1.339	6.2621	-0.0354	2.416E-004
19	LONG TERM (ACI L/240 LIMIT)	Combination	1.254	-2.710E-004	1.375	0.0015	-14.7585	0.0024
20	LONG TERM (ACI L/240 LIMIT)	Combination	6.422E-003	5.646	10.938	-3.3614	0.0043	-5.208E-005
21	LONG TERM (ACI L/240 LIMIT)	Combination	-0.255	6.797E-004	21.080	-0.0030	-2.9458	0.0028
22	LONG TERM (ACI L/240 LIMIT)	Combination	1.915E-003	0.313	0.143	-1.3880	0.0085	-6.121E-004
23	LONG TERM (ACI L/240 LIMIT)	Combination	3.908	-1.728E-003	-0.387	0.0072	-16.8855	-0.0019
24	LONG TERM (ACI L/240 LIMIT)	Combination	-6.969E-005	0.538	0.191	-3.3279	-2.421E-004	2.566E-004
25	LONG TERM (ACI L/240 LIMIT)	Combination	0.552	5.398E-005	4.297	-5.533E-004	-3.3200	7.974E-004
27	LONG TERM (ACI L/240 LIMIT)	Combination	5.477E-005	0.755	3.307	-4.4382	2.549E-004	-3.155E-005
28	LONG TERM (ACI L/240 LIMIT)	Combination	-0.383	-2.523E-004	17.873	6.555E-004	19.9695	0.0039
29	LONG TERM (ACI L/240 LIMIT)	Combination	-2.728E-006	0.421	1.993	-3.1590	-9.731E-006	-1.786E-005
30	LONG TERM (ACI L/240 LIMIT)	Combination	0.119	2.151E-004	3.827	4.193E-004	6.4592	-8.508E-004
31	LONG TERM (ACI L/240 LIMIT)	Combination	1.544E-004	0.696	1.284	-7.3656	6.189E-004	-8.699E-005
32	LONG TERM (ACI L/240 LIMIT)	Combination	0.158	-2.450E-004	13.686	0.0016	3.3470	-0.0036
33	LONG TERM (ACI L/240 LIMIT)	Combination	-1.898	0.755	3.854	-0.7284	-1.8330	-2.960E-004
34	LONG TERM (ACI L/240 LIMIT)	Combination	-1.818E-003	-1.421	20.775	22.5297	-0.0071	0.0025
35	LONG TERM (ACI L/240 LIMIT)	Combination	-0.898	-5.360E-003	4.487	0.0158	-2.6478	-2.463E-004
36	LONG TERM (ACI L/240 LIMIT)	Combination	0.339	-6.362E-004	-6.147	0.0020	0.8551	-6.714E-004
37	LONG TERM (ACI L/240 LIMIT)	Combination	-3.216	-0.027	29.129	0.1044	-24.0981	-0.0118
38	LONG TERM (ACI L/240 LIMIT)	Combination	-5.906E-004	-2.217	7.527	-3.4967	-0.0035	0.0049

Table 31: Integrated Wall Forces

Area	OutputCase	CaseType	Fx	Fy	Fz	Mx	My	Mz
			kip	kip	kip	kip-ft	kip-ft	kip-ft
39	LONG TERM (ACI L/240 LIMIT)	Combination	3.321	2.636E-004	9.310	-0.0022	10.6957	-0.0037
40	LONG TERM (ACI L/240 LIMIT)	Combination	-7.817	-8.748E-003	14.799	0.0453	67.3454	-0.0146
41	LONG TERM (ACI L/240 LIMIT)	Combination	-1.947E-004	3.245	-2.253	-9.1104	-4.583E-004	8.900E-004
42	LONG TERM (ACI L/240 LIMIT)	Combination	2.489	6.570E-003	44.907	-0.0617	-33.8790	-0.0586
1	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	0.796	-8.995E-005	3.029	3.693E-004	2.8414	-2.844E-005
2	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	1.044	-6.553E-005	2.891	2.698E-004	3.7958	-3.270E-005
3	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	0.185	-8.016E-004	5.956	0.0032	-0.0343	-8.676E-004
5	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	1.242	-7.152E-004	3.436	0.0030	-10.6837	-0.0038
6	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	1.891E-003	1.009	-1.247	6.4294	0.0076	-0.0027
7	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-0.114	1.425E-003	10.703	-0.0058	1.9448	-0.0028
8	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-0.114	-0.276	2.870	1.6957	-0.6999	-7.865E-005
9	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	0.139	0.327	2.076	-0.1634	0.0693	6.314E-007
10	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-0.400	2.246E-005	3.194	-4.577E-005	-0.6735	3.212E-005
11	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-1.326E-004	0.084	9.743	4.5125	-5.972E-004	5.598E-004
12	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-0.441	-1.438E-005	4.088	7.568E-005	-1.4364	-2.113E-004
13	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	1.162E-004	-1.798	4.329	6.6058	4.748E-004	4.967E-004
14	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	0.151	1.993E-004	1.398	-7.889E-004	-0.0095	2.872E-004
15	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	1.467E-005	-0.740	0.517	2.4940	4.151E-005	7.096E-007
16	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	0.349	-1.537E-005	0.727	5.057E-005	-0.3152	5.130E-005
17	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	1.948E-003	0.231	3.725	0.1800	-0.0015	4.543E-006

Table 31: Integrated Wall Forces

Area	OutputCase	CaseType	Fx	Fy	Fz	Mx	My	Mz
			kip	kip	kip	kip-ft	kip-ft	kip-ft
18	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-3.459E-003	-0.613	1.312	2.5643	-0.0145	1.102E-004
19	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	0.707	9.021E-005	2.287	-9.459E-005	-11.8153	0.0019
20	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	5.574E-003	4.956	9.195	-4.4155	0.0053	-1.022E-005
21	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-0.237	4.118E-004	17.112	-0.0017	-3.0782	0.0014
22	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	4.991E-003	0.874	1.238	-2.2927	0.0132	-3.152E-004
23	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	2.651	-6.024E-004	0.287	0.0026	-4.8721	-8.952E-004
24	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-2.820E-005	0.278	0.678	-1.3832	-1.082E-004	1.314E-004
25	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	0.745	-2.178E-004	4.120	7.078E-004	1.1042	-3.634E-004
27	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-9.114E-006	7.776E-004	2.826	-0.2536	-8.538E-006	-3.153E-005
28	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	1.325E-003	2.242E-004	14.243	-9.490E-004	19.3638	-1.451E-004
29	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-2.653E-005	0.115	1.670	-0.8643	-6.486E-005	5.343E-006
30	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-0.058	1.779E-004	3.268	-2.120E-004	2.0159	-3.734E-004
31	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	3.397E-005	0.218	3.023	-1.2862	1.359E-004	-1.250E-005
32	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-0.117	-1.095E-003	9.473	0.0062	3.2672	-0.0027
33	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-1.311	0.521	3.730	-1.0911	-2.7447	-6.232E-005
34	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-4.892E-004	-0.704	15.962	18.3822	-0.0019	9.220E-004
35	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-0.428	-2.555E-003	2.999	0.0078	-1.3050	-5.207E-005
36	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	0.299	-4.993E-004	-2.769	0.0011	0.4255	-2.222E-004
37	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-3.328	-8.810E-003	22.808	0.0288	-26.6199	-0.0169
38	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-3.983E-005	-1.646	7.729	-1.4634	-7.739E-004	0.0018

Table 31: Integrated Wall Forces

Area	OutputCase	CaseType	Fx	Fy	Fz	Mx	My	Mz
			kip	kip	kip	kip-ft	kip-ft	kip-ft
39	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	2.494	3.267E-004	7.029	-0.0017	7.5826	-0.0023
40	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-6.630	-5.941E-003	14.250	0.0313	46.8425	-0.0093
41	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	-5.884E-005	1.462	-1.184	-4.1179	-1.304E-004	2.499E-004
42	LONG TERM (ACI LIVE L/360 LIMIT)	Combination	2.623	-5.474E-003	38.416	-0.0863	-136.1394	-0.0648

5.2. Structure results

Table 32: Sum Of Reactions, Part 1 of 2

Table 32: Sum Of Reactions, Part 1 of 2

OutputCase	GlobalFX	GlobalFY	GlobalFZ	GlobalMX	GlobalMY	GlobalMZ
	kip	kip	kip	kip-ft	kip-ft	kip-ft
CASE-1	-3.232E-005	2.401E-004	2927.381	126515.7112	-257634	0.0226
CASE-1	-3.232E-005	2.401E-004	2927.381	126515.7112	-257634	0.0226
CASE-2	1.399E-004	3.064E-004	2443.395	106004.7048	-213429	0.0292
CASE-2	1.399E-004	3.064E-004	2443.395	106004.7048	-213429	0.0292
CASE-3	7.250E-005	-4.232E-005	2443.395	106004.7060	-213429	-0.0160
CASE-3	7.250E-005	-4.232E-005	2443.395	106004.7060	-213429	-0.0160
CASE-4	-2.510E-004	2.066E-004	2114.596	91322.1444	-184141	0.0223
CASE-4	-2.510E-004	2.066E-004	2114.596	91322.1444	-184141	0.0223
CASE-5	-3.059E-005	4.627E-005	2282.066	99167.7080	-198694	0.0033
CASE-5	-3.059E-005	4.627E-005	2282.066	99167.7080	-198694	0.0033
UDCONN1	-1.420E-004	8.827E-005	2282.066	99167.7030	-198694	0.0107
UDCONN2	-1.982E-004	1.309E-004	2927.381	126515.7073	-257634	0.0163
UDCONU1	-1.988E-004	1.236E-004	3194.892	138834.7842	-278171	0.0150
UDCONU2	-2.603E-004	1.741E-004	3770.984	162758.0504	-332736	0.0218
LONG TERM (ACI L/240 LIMIT)	1.512E-004	-3.152E-004	812.786	35193.5680	-73492.5526	-0.0450
LONG TERM (ACI LIVE L/360 LIMIT)	-6.915E-005	-1.549E-004	645.315	27348.0044	-58939.8788	-0.0259

Table 32: Sum Of Reactions, Part 2 of 2

Table 32: Sum Of Reactions, Part 2 of 2

OutputCase	GlobalX	GlobalY	GlobalZ
	ft	ft	ft
CASE-1	0.0000	0.0000	0.0000
CASE-1	0.0000	0.0000	0.0000
CASE-2	0.0000	0.0000	0.0000
CASE-2	0.0000	0.0000	0.0000
CASE-3	0.0000	0.0000	0.0000
CASE-3	0.0000	0.0000	0.0000
CASE-4	0.0000	0.0000	0.0000
CASE-4	0.0000	0.0000	0.0000
CASE-5	0.0000	0.0000	0.0000

Table 32: Sum Of Reactions, Part 2 of 2

OutputCase	GlobalX	GlobalY	GlobalZ
	ft	ft	ft
CASE-5	0.0000	0.0000	0.0000
UDCONN1	0.0000	0.0000	0.0000
UDCONN2	0.0000	0.0000	0.0000
UDCONU1	0.0000	0.0000	0.0000
UDCONU2	0.0000	0.0000	0.0000
LONG TERM (ACI L/240 LIMIT)	0.0000	0.0000	0.0000
LONG TERM (ACI LIVE L/360 LIMIT)	0.0000	0.0000	0.0000

Table 33: Nodal Displacements - Summary, Part 1 of 2

Table 33: Nodal Displacements - Summary, Part 1 of 2

Panel	Node	OutputCase	Ux	Uy	Uz	Rx	Ry	Rz
			in	in	in	Radians	Radians	Radians
1	283	CASE-1	0.000000	0.000000	-0.016981	0.000000	0.000000	0.000000
2	~4657	CASE-1	0.000000	0.000000	-0.009526	0.000000	0.000000	0.000000
3	~4950	CASE-1	0.000000	0.000000	0.001530	0.000000	0.000000	0.000000
11	~2920	CASE-1	0.000000	0.000000	-0.067426	0.000000	0.000000	0.000000
12	~5065	CASE-1	0.000000	0.000000	-0.068633	0.000000	0.000000	0.000000
13	~549	CASE-1	0.000000	0.000000	-0.039352	0.000000	0.000000	0.000000
21	~2316	CASE-1	0.000000	0.000000	-0.066939	0.000000	0.000000	0.000000
22	~2317	CASE-1	0.000000	0.000000	-0.066804	0.000000	0.000000	0.000000
23	~3153	CASE-1	0.000000	0.000000	-0.040401	0.000000	0.000000	0.000000
31	~412	CASE-1	0.000000	0.000000	-0.048002	0.000000	0.000000	0.000000
32	~448	CASE-1	0.000000	0.000000	-0.064326	0.000000	0.000000	0.000000
33	~556	CASE-1	0.000000	0.000000	-0.028873	0.000000	0.000000	0.000000
38	289	CASE-1	0.000000	0.000000	-0.082269	0.000000	0.000000	0.000000
39	~2387	CASE-1	0.000000	0.000000	-0.067282	0.000000	0.000000	0.000000
40	~1824	CASE-1	0.000000	0.000000	-0.091154	0.000000	0.000000	0.000000
41	~416	CASE-1	0.000000	0.000000	-0.122353	0.000000	0.000000	0.000000
42	~488	CASE-1	0.000000	0.000000	-0.193273	0.000000	0.000000	0.000000
43	~560	CASE-1	0.000000	0.000000	-0.124214	0.000000	0.000000	0.000000
44	298	CASE-1	0.000000	0.000000	-0.014952	0.000000	0.000000	0.000000
45	~1807	CASE-1	0.000000	0.000000	-0.014748	0.000000	0.000000	0.000000
46	~2449	CASE-1	0.000000	0.000000	-0.001773	0.000000	0.000000	0.000000
47	~1639	CASE-1	0.000000	0.000000	-0.033113	0.000000	0.000000	0.000000
48	~1196	CASE-1	0.000000	0.000000	-0.019756	0.000000	0.000000	0.000000
49	~1341	CASE-1	0.000000	0.000000	0.010125	0.000000	0.000000	0.000000
50	~2171	CASE-1	0.000000	0.000000	-0.031257	0.000000	0.000000	0.000000
51	~417	CASE-1	0.000000	0.000000	-0.128021	0.000000	0.000000	0.000000
52	~490	CASE-1	0.000000	0.000000	-0.209757	0.000000	0.000000	0.000000
53	~561	CASE-1	0.000000	0.000000	-0.130910	0.000000	0.000000	0.000000
54	~778	CASE-1	0.000000	0.000000	-0.484066	0.000000	0.000000	0.000000
57	~1167	CASE-1	0.000000	0.000000	-0.069244	0.000000	0.000000	0.000000
58	~1310	CASE-1	0.000000	0.000000	-0.228780	0.000000	0.000000	0.000000
59	~2021	CASE-1	0.000000	0.000000	-0.221013	0.000000	0.000000	0.000000
60	~2200	CASE-1	0.000000	0.000000	0.047859	0.000000	0.000000	0.000000
61	~421	CASE-1	0.000000	0.000000	-0.095757	0.000000	0.000000	0.000000
62	~3055	CASE-1	0.000000	0.000000	-0.212787	0.000000	0.000000	0.000000
63	~5032	CASE-1	0.000000	0.000000	-0.129275	0.000000	0.000000	0.000000
64	~2466	CASE-1	0.000000	0.000000	-0.814109	0.000000	0.000000	0.000000
67	295	CASE-1	0.000000	0.000000	0.137206	0.000000	0.000000	0.000000

Table 33: Nodal Displacements - Summary, Part 1 of 2

Panel	Node	OutputCase	Ux	Uy	Uz	Rx	Ry	Rz
			in	in	in	Radians	Radians	Radians
68	~1315	CASE-1	0.000000	0.000000	-0.564712	0.000000	0.000000	0.000000
69	~1351	CASE-1	0.000000	0.000000	-0.581922	0.000000	0.000000	0.000000
70	~2181	CASE-1	0.000000	0.000000	0.181993	0.000000	0.000000	0.000000
71	~2942	CASE-1	0.000000	0.000000	-0.113113	0.000000	0.000000	0.000000
72	~3058	CASE-1	0.000000	0.000000	-0.316211	0.000000	0.000000	0.000000
73	~3174	CASE-1	0.000000	0.000000	-0.234965	0.000000	0.000000	0.000000
74	~1606	CASE-1	0.000000	0.000000	-0.644724	0.000000	0.000000	0.000000
75	~1609	CASE-1	0.000000	0.000000	-0.635916	0.000000	0.000000	0.000000
76	~2502	CASE-1	0.000000	0.000000	-0.487081	0.000000	0.000000	0.000000
77	~1173	CASE-1	0.000000	0.000000	-0.119923	0.000000	0.000000	0.000000
78	~1317	CASE-1	0.000000	0.000000	-0.607101	0.000000	0.000000	0.000000
79	~1354	CASE-1	0.000000	0.000000	-0.642882	0.000000	0.000000	0.000000
80	~2186	CASE-1	0.000000	0.000000	0.218961	0.000000	0.000000	0.000000
81	~2945	CASE-1	0.000000	0.000000	-0.141435	0.000000	0.000000	0.000000
82	~3063	CASE-1	0.000000	0.000000	-0.387930	0.000000	0.000000	0.000000
83	~3179	CASE-1	0.000000	0.000000	-0.266727	0.000000	0.000000	0.000000
84	~3573	CASE-1	0.000000	0.000000	-0.308479	0.000000	0.000000	0.000000
85	~5021	CASE-1	0.000000	0.000000	-0.383006	0.000000	0.000000	0.000000
86	~4786	CASE-1	0.000000	0.000000	-0.385304	0.000000	0.000000	0.000000
87	~2507	CASE-1	0.000000	0.000000	-0.155332	0.000000	0.000000	0.000000
88	~1320	CASE-1	0.000000	0.000000	-0.580162	0.000000	0.000000	0.000000
89	~1356	CASE-1	0.000000	0.000000	-0.621760	0.000000	0.000000	0.000000
90	~2187	CASE-1	0.000000	0.000000	0.214674	0.000000	0.000000	0.000000
91	~2949	CASE-1	0.000000	0.000000	-0.140364	0.000000	0.000000	0.000000
92	~3064	CASE-1	0.000000	0.000000	-0.388062	0.000000	0.000000	0.000000
93	~3180	CASE-1	0.000000	0.000000	-0.273070	0.000000	0.000000	0.000000
94	~4282	CASE-1	0.000000	0.000000	-0.303397	0.000000	0.000000	0.000000
95	~5010	CASE-1	0.000000	0.000000	-0.701479	0.000000	0.000000	0.000000
96	~4990	CASE-1	0.000000	0.000000	-0.742933	0.000000	0.000000	0.000000
97	~4904	CASE-1	0.000000	0.000000	-0.292793	0.000000	0.000000	0.000000
98	~1324	CASE-1	0.000000	0.000000	-0.398508	0.000000	0.000000	0.000000
99	~1360	CASE-1	0.000000	0.000000	-0.433144	0.000000	0.000000	0.000000
100	~2192	CASE-1	0.000000	0.000000	0.137794	0.000000	0.000000	0.000000
101	~4336	CASE-1	0.000000	0.000000	-0.111648	0.000000	0.000000	0.000000
102	~4667	CASE-1	0.000000	0.000000	-0.331838	0.000000	0.000000	0.000000
103	~4589	CASE-1	0.000000	0.000000	-0.235784	0.000000	0.000000	0.000000
106	~4989	CASE-1	0.000000	0.000000	-0.750604	0.000000	0.000000	0.000000
107	~5008	CASE-1	0.000000	0.000000	-0.384080	0.000000	0.000000	0.000000
108	~1328	CASE-1	0.000000	0.000000	-0.079387	0.000000	0.000000	0.000000
109	~1400	CASE-1	0.000000	0.000000	-0.148947	0.000000	0.000000	0.000000
110	~1828	CASE-1	0.000000	0.000000	0.077212	0.000000	0.000000	0.000000
111	~4247	CASE-1	0.000000	0.000000	-0.002029	0.000000	0.000000	0.000000
113	~4880	CASE-1	0.000000	0.000000	0.018307	0.000000	0.000000	0.000000
121	~2962	CASE-1	0.000000	0.000000	-0.045142	0.000000	0.000000	0.000000
122	~3136	CASE-1	0.000000	0.000000	-0.112806	0.000000	0.000000	0.000000
123	~3194	CASE-1	0.000000	0.000000	-0.087609	0.000000	0.000000	0.000000
124	~3477	CASE-1	0.000000	0.000000	-0.026692	0.000000	0.000000	0.000000
131	~2963	CASE-1	0.000000	0.000000	-0.044492	0.000000	0.000000	0.000000
132	~3137	CASE-1	0.000000	0.000000	-0.117070	0.000000	0.000000	0.000000
133	~3195	CASE-1	0.000000	0.000000	-0.092555	0.000000	0.000000	0.000000
134	~3478	CASE-1	0.000000	0.000000	-0.033944	0.000000	0.000000	0.000000
141	~2854	CASE-1	0.000000	0.000000	-0.003118	0.000000	0.000000	0.000000
1	283	CASE-2	0.000000	0.000000	-0.014952	0.000000	0.000000	0.000000
2	~4657	CASE-2	0.000000	0.000000	-0.007642	0.000000	0.000000	0.000000

Table 33: Nodal Displacements - Summary, Part 1 of 2

Panel	Node	OutputCase	Ux	Uy	Uz	Rx	Ry	Rz
			in	in	in	Radians	Radians	Radians
3	~4950	CASE-2	0.000000	0.000000	0.000999	0.000000	0.000000	0.000000
11	~2920	CASE-2	0.000000	0.000000	-0.056081	0.000000	0.000000	0.000000
12	~4731	CASE-2	0.000000	0.000000	-0.056456	0.000000	0.000000	0.000000
13	~549	CASE-2	0.000000	0.000000	-0.030749	0.000000	0.000000	0.000000
21	~5062	CASE-2	0.000000	0.000000	-0.055957	0.000000	0.000000	0.000000
22	~2317	CASE-2	0.000000	0.000000	-0.055511	0.000000	0.000000	0.000000
23	~3153	CASE-2	0.000000	0.000000	-0.031440	0.000000	0.000000	0.000000
31	~412	CASE-2	0.000000	0.000000	-0.038534	0.000000	0.000000	0.000000
32	~448	CASE-2	0.000000	0.000000	-0.051117	0.000000	0.000000	0.000000
33	~556	CASE-2	0.000000	0.000000	-0.022731	0.000000	0.000000	0.000000
38	289	CASE-2	0.000000	0.000000	-0.068856	0.000000	0.000000	0.000000
39	~2387	CASE-2	0.000000	0.000000	-0.055933	0.000000	0.000000	0.000000
40	~1824	CASE-2	0.000000	0.000000	-0.075955	0.000000	0.000000	0.000000
41	~416	CASE-2	0.000000	0.000000	-0.095635	0.000000	0.000000	0.000000
42	~488	CASE-2	0.000000	0.000000	-0.151237	0.000000	0.000000	0.000000
43	~560	CASE-2	0.000000	0.000000	-0.096034	0.000000	0.000000	0.000000
44	298	CASE-2	0.000000	0.000000	-0.009578	0.000000	0.000000	0.000000
45	~1807	CASE-2	0.000000	0.000000	-0.009796	0.000000	0.000000	0.000000
46	~2452	CASE-2	0.000000	0.000000	-0.001507	0.000000	0.000000	0.000000
47	~1639	CASE-2	0.000000	0.000000	-0.027651	0.000000	0.000000	0.000000
48	~1196	CASE-2	0.000000	0.000000	-0.016505	0.000000	0.000000	0.000000
49	~1341	CASE-2	0.000000	0.000000	0.008888	0.000000	0.000000	0.000000
50	~2171	CASE-2	0.000000	0.000000	-0.026413	0.000000	0.000000	0.000000
51	~417	CASE-2	0.000000	0.000000	-0.098142	0.000000	0.000000	0.000000
52	~489	CASE-2	0.000000	0.000000	-0.160159	0.000000	0.000000	0.000000
53	~561	CASE-2	0.000000	0.000000	-0.099316	0.000000	0.000000	0.000000
54	~778	CASE-2	0.000000	0.000000	-0.266513	0.000000	0.000000	0.000000
57	~1167	CASE-2	0.000000	0.000000	-0.054075	0.000000	0.000000	0.000000
58	~1310	CASE-2	0.000000	0.000000	-0.170644	0.000000	0.000000	0.000000
59	~1346	CASE-2	0.000000	0.000000	-0.161406	0.000000	0.000000	0.000000
60	~2200	CASE-2	0.000000	0.000000	0.032298	0.000000	0.000000	0.000000
61	~421	CASE-2	0.000000	0.000000	-0.063422	0.000000	0.000000	0.000000
62	~493	CASE-2	0.000000	0.000000	-0.118187	0.000000	0.000000	0.000000
63	~5032	CASE-2	0.000000	0.000000	-0.066591	0.000000	0.000000	0.000000
64	~2466	CASE-2	0.000000	0.000000	-0.416488	0.000000	0.000000	0.000000
67	~1065	CASE-2	0.000000	0.000000	0.121872	0.000000	0.000000	0.000000
68	~1315	CASE-2	0.000000	0.000000	-0.383772	0.000000	0.000000	0.000000
69	~1351	CASE-2	0.000000	0.000000	-0.391725	0.000000	0.000000	0.000000
70	~2181	CASE-2	0.000000	0.000000	0.128594	0.000000	0.000000	0.000000
71	~2942	CASE-2	0.000000	0.000000	-0.054776	0.000000	0.000000	0.000000
72	~3116	CASE-2	0.000000	0.000000	-0.143544	0.000000	0.000000	0.000000
73	~3174	CASE-2	0.000000	0.000000	-0.111983	0.000000	0.000000	0.000000
74	~1606	CASE-2	0.000000	0.000000	-0.343133	0.000000	0.000000	0.000000
75	~1609	CASE-2	0.000000	0.000000	-0.343314	0.000000	0.000000	0.000000
76	~2502	CASE-2	0.000000	0.000000	-0.264718	0.000000	0.000000	0.000000
77	~1173	CASE-2	0.000000	0.000000	-0.092551	0.000000	0.000000	0.000000
78	~1318	CASE-2	0.000000	0.000000	-0.411880	0.000000	0.000000	0.000000
79	~1354	CASE-2	0.000000	0.000000	-0.428436	0.000000	0.000000	0.000000
80	~2186	CASE-2	0.000000	0.000000	0.154998	0.000000	0.000000	0.000000
81	~2945	CASE-2	0.000000	0.000000	-0.067835	0.000000	0.000000	0.000000
82	~4658	CASE-2	0.000000	0.000000	-0.168164	0.000000	0.000000	0.000000
83	~3179	CASE-2	0.000000	0.000000	-0.123099	0.000000	0.000000	0.000000
84	~3573	CASE-2	0.000000	0.000000	-0.163083	0.000000	0.000000	0.000000
85	~3745	CASE-2	0.000000	0.000000	-0.207416	0.000000	0.000000	0.000000

Table 33: Nodal Displacements - Summary, Part 1 of 2

Panel	Node	OutputCase	Ux	Uy	Uz	Rx	Ry	Rz
			in	in	in	Radians	Radians	Radians
86	~4786	CASE-2	0.000000	0.000000	-0.208526	0.000000	0.000000	0.000000
87	~2507	CASE-2	0.000000	0.000000	-0.079070	0.000000	0.000000	0.000000
88	~1320	CASE-2	0.000000	0.000000	-0.398557	0.000000	0.000000	0.000000
89	~1356	CASE-2	0.000000	0.000000	-0.418529	0.000000	0.000000	0.000000
90	~2187	CASE-2	0.000000	0.000000	0.152074	0.000000	0.000000	0.000000
91	~2949	CASE-2	0.000000	0.000000	-0.069293	0.000000	0.000000	0.000000
92	~3122	CASE-2	0.000000	0.000000	-0.167770	0.000000	0.000000	0.000000
93	~3180	CASE-2	0.000000	0.000000	-0.128246	0.000000	0.000000	0.000000
94	~4282	CASE-2	0.000000	0.000000	-0.152773	0.000000	0.000000	0.000000
95	~5010	CASE-2	0.000000	0.000000	-0.407724	0.000000	0.000000	0.000000
96	~4990	CASE-2	0.000000	0.000000	-0.430274	0.000000	0.000000	0.000000
97	~4904	CASE-2	0.000000	0.000000	-0.182014	0.000000	0.000000	0.000000
98	~1324	CASE-2	0.000000	0.000000	-0.288996	0.000000	0.000000	0.000000
99	~1360	CASE-2	0.000000	0.000000	-0.310622	0.000000	0.000000	0.000000
100	~2192	CASE-2	0.000000	0.000000	0.095617	0.000000	0.000000	0.000000
101	~4336	CASE-2	0.000000	0.000000	-0.054691	0.000000	0.000000	0.000000
102	~4667	CASE-2	0.000000	0.000000	-0.152196	0.000000	0.000000	0.000000
103	~4589	CASE-2	0.000000	0.000000	-0.113841	0.000000	0.000000	0.000000
106	~4989	CASE-2	0.000000	0.000000	-0.433917	0.000000	0.000000	0.000000
107	~5008	CASE-2	0.000000	0.000000	-0.227581	0.000000	0.000000	0.000000
108	~1328	CASE-2	0.000000	0.000000	-0.057246	0.000000	0.000000	0.000000
109	~1400	CASE-2	0.000000	0.000000	-0.105526	0.000000	0.000000	0.000000
110	~1828	CASE-2	0.000000	0.000000	0.055818	0.000000	0.000000	0.000000
111	~4247	CASE-2	0.000000	0.000000	-0.001793	0.000000	0.000000	0.000000
113	~4880	CASE-2	0.000000	0.000000	0.008514	0.000000	0.000000	0.000000
121	~2962	CASE-2	0.000000	0.000000	-0.039301	0.000000	0.000000	0.000000
122	~3136	CASE-2	0.000000	0.000000	-0.101190	0.000000	0.000000	0.000000
123	~3194	CASE-2	0.000000	0.000000	-0.079765	0.000000	0.000000	0.000000
124	~3477	CASE-2	0.000000	0.000000	-0.015002	0.000000	0.000000	0.000000
131	~2963	CASE-2	0.000000	0.000000	-0.038809	0.000000	0.000000	0.000000
132	~3137	CASE-2	0.000000	0.000000	-0.103352	0.000000	0.000000	0.000000
133	~3195	CASE-2	0.000000	0.000000	-0.084114	0.000000	0.000000	0.000000
134	~3420	CASE-2	0.000000	0.000000	-0.021203	0.000000	0.000000	0.000000
141	~2854	CASE-2	0.000000	0.000000	-0.002272	0.000000	0.000000	0.000000
1	283	CASE-3	0.000000	0.000000	-0.057029	0.000000	0.000000	0.000000
2	~4657	CASE-3	0.000000	0.000000	-0.016942	0.000000	0.000000	0.000000
3	~4947	CASE-3	0.000000	0.000000	-0.001999	0.000000	0.000000	0.000000
11	~2920	CASE-3	0.000000	0.000000	-0.186762	0.000000	0.000000	0.000000
12	~4731	CASE-3	0.000000	0.000000	-0.188130	0.000000	0.000000	0.000000
13	~549	CASE-3	0.000000	0.000000	-0.101900	0.000000	0.000000	0.000000
21	~5062	CASE-3	0.000000	0.000000	-0.182077	0.000000	0.000000	0.000000
22	~2317	CASE-3	0.000000	0.000000	-0.178919	0.000000	0.000000	0.000000
23	~3153	CASE-3	0.000000	0.000000	-0.103474	0.000000	0.000000	0.000000
31	~412	CASE-3	0.000000	0.000000	-0.107937	0.000000	0.000000	0.000000
32	~448	CASE-3	0.000000	0.000000	-0.147302	0.000000	0.000000	0.000000
33	~556	CASE-3	0.000000	0.000000	-0.064469	0.000000	0.000000	0.000000
38	289	CASE-3	0.000000	0.000000	-0.228078	0.000000	0.000000	0.000000
39	~2385	CASE-3	0.000000	0.000000	-0.192345	0.000000	0.000000	0.000000
40	~1824	CASE-3	0.000000	0.000000	-0.220394	0.000000	0.000000	0.000000
41	~416	CASE-3	0.000000	0.000000	-0.285126	0.000000	0.000000	0.000000
42	~488	CASE-3	0.000000	0.000000	-0.472299	0.000000	0.000000	0.000000
43	~560	CASE-3	0.000000	0.000000	-0.306269	0.000000	0.000000	0.000000
44	298	CASE-3	0.000000	0.000000	-0.019135	0.000000	0.000000	0.000000
45	~1807	CASE-3	0.000000	0.000000	-0.023122	0.000000	0.000000	0.000000

Table 33: Nodal Displacements - Summary, Part 1 of 2

Panel	Node	OutputCase	Ux	Uy	Uz	Rx	Ry	Rz
			in	in	in	Radians	Radians	Radians
46	~2299	CASE-3	0.000000	0.000000	-0.008625	0.000000	0.000000	0.000000
47	~1639	CASE-3	0.000000	0.000000	-0.102046	0.000000	0.000000	0.000000
48	~1196	CASE-3	0.000000	0.000000	-0.051712	0.000000	0.000000	0.000000
49	~1341	CASE-3	0.000000	0.000000	0.039851	0.000000	0.000000	0.000000
50	~2171	CASE-3	0.000000	0.000000	-0.073075	0.000000	0.000000	0.000000
51	~417	CASE-3	0.000000	0.000000	-0.290404	0.000000	0.000000	0.000000
52	~489	CASE-3	0.000000	0.000000	-0.503458	0.000000	0.000000	0.000000
53	~561	CASE-3	0.000000	0.000000	-0.315758	0.000000	0.000000	0.000000
54	~778	CASE-3	0.000000	0.000000	-0.825590	0.000000	0.000000	0.000000
57	~1167	CASE-3	0.000000	0.000000	-0.163200	0.000000	0.000000	0.000000
58	~1310	CASE-3	0.000000	0.000000	-0.552306	0.000000	0.000000	0.000000
59	~1346	CASE-3	0.000000	0.000000	-0.519295	0.000000	0.000000	0.000000
60	~2200	CASE-3	0.000000	0.000000	0.092654	0.000000	0.000000	0.000000
61	~421	CASE-3	0.000000	0.000000	-0.169762	0.000000	0.000000	0.000000
62	~493	CASE-3	0.000000	0.000000	-0.354975	0.000000	0.000000	0.000000
63	~565	CASE-3	0.000000	0.000000	-0.199676	0.000000	0.000000	0.000000
64	~2467	CASE-3	0.000000	0.000000	-1.332794	0.000000	0.000000	0.000000
67	~1065	CASE-3	0.000000	0.000000	0.410535	0.000000	0.000000	0.000000
68	~1315	CASE-3	0.000000	0.000000	-1.320070	0.000000	0.000000	0.000000
69	~1351	CASE-3	0.000000	0.000000	-1.345624	0.000000	0.000000	0.000000
70	~2181	CASE-3	0.000000	0.000000	0.406806	0.000000	0.000000	0.000000
71	~2942	CASE-3	0.000000	0.000000	-0.158334	0.000000	0.000000	0.000000
72	~3116	CASE-3	0.000000	0.000000	-0.456898	0.000000	0.000000	0.000000
73	~3174	CASE-3	0.000000	0.000000	-0.356767	0.000000	0.000000	0.000000
74	~1606	CASE-3	0.000000	0.000000	-1.023978	0.000000	0.000000	0.000000
75	~1609	CASE-3	0.000000	0.000000	-1.024365	0.000000	0.000000	0.000000
76	~2502	CASE-3	0.000000	0.000000	-0.783561	0.000000	0.000000	0.000000
77	~1173	CASE-3	0.000000	0.000000	-0.290495	0.000000	0.000000	0.000000
78	~1317	CASE-3	0.000000	0.000000	-1.393295	0.000000	0.000000	0.000000
79	~1354	CASE-3	0.000000	0.000000	-1.464033	0.000000	0.000000	0.000000
80	~2186	CASE-3	0.000000	0.000000	0.519198	0.000000	0.000000	0.000000
81	~2944	CASE-3	0.000000	0.000000	-0.204971	0.000000	0.000000	0.000000
82	~3063	CASE-3	0.000000	0.000000	-0.521283	0.000000	0.000000	0.000000
83	~3179	CASE-3	0.000000	0.000000	-0.376055	0.000000	0.000000	0.000000
84	~3573	CASE-3	0.000000	0.000000	-0.487193	0.000000	0.000000	0.000000
85	~5021	CASE-3	0.000000	0.000000	-0.608140	0.000000	0.000000	0.000000
86	~4786	CASE-3	0.000000	0.000000	-0.604977	0.000000	0.000000	0.000000
87	~2507	CASE-3	0.000000	0.000000	-0.210102	0.000000	0.000000	0.000000
88	~1320	CASE-3	0.000000	0.000000	-1.297043	0.000000	0.000000	0.000000
89	~1356	CASE-3	0.000000	0.000000	-1.399977	0.000000	0.000000	0.000000
90	~2187	CASE-3	0.000000	0.000000	0.504817	0.000000	0.000000	0.000000
91	~2949	CASE-3	0.000000	0.000000	-0.208059	0.000000	0.000000	0.000000
92	~3064	CASE-3	0.000000	0.000000	-0.521288	0.000000	0.000000	0.000000
93	~3180	CASE-3	0.000000	0.000000	-0.391757	0.000000	0.000000	0.000000
94	~4282	CASE-3	0.000000	0.000000	-0.489475	0.000000	0.000000	0.000000
95	~5010	CASE-3	0.000000	0.000000	-1.283870	0.000000	0.000000	0.000000
96	~4990	CASE-3	0.000000	0.000000	-1.346363	0.000000	0.000000	0.000000
97	~4904	CASE-3	0.000000	0.000000	-0.515975	0.000000	0.000000	0.000000
98	~1324	CASE-3	0.000000	0.000000	-0.869759	0.000000	0.000000	0.000000
99	~1360	CASE-3	0.000000	0.000000	-0.934637	0.000000	0.000000	0.000000
100	~2192	CASE-3	0.000000	0.000000	0.267260	0.000000	0.000000	0.000000
101	~4336	CASE-3	0.000000	0.000000	-0.149899	0.000000	0.000000	0.000000
102	~4667	CASE-3	0.000000	0.000000	-0.436513	0.000000	0.000000	0.000000
103	~4589	CASE-3	0.000000	0.000000	-0.316367	0.000000	0.000000	0.000000

Table 33: Nodal Displacements - Summary, Part 1 of 2

Panel	Node	OutputCase	Ux	Uy	Uz	Rx	Ry	Rz
			in	in	in	Radians	Radians	Radians
106	~4989	CASE-3	0.000000	0.000000	-1.351907	0.000000	0.000000	0.000000
107	~5008	CASE-3	0.000000	0.000000	-0.653144	0.000000	0.000000	0.000000
108	~1258	CASE-3	0.000000	0.000000	0.114119	0.000000	0.000000	0.000000
109	~1400	CASE-3	0.000000	0.000000	-0.214349	0.000000	0.000000	0.000000
110	~2195	CASE-3	0.000000	0.000000	0.095206	0.000000	0.000000	0.000000
111	~4386	CASE-3	0.000000	0.000000	-0.003184	0.000000	0.000000	0.000000
113	~4880	CASE-3	0.000000	0.000000	0.012203	0.000000	0.000000	0.000000
121	~2962	CASE-3	0.000000	0.000000	-0.097877	0.000000	0.000000	0.000000
122	~3078	CASE-3	0.000000	0.000000	-0.248666	0.000000	0.000000	0.000000
123	~3194	CASE-3	0.000000	0.000000	-0.187081	0.000000	0.000000	0.000000
124	~3477	CASE-3	0.000000	0.000000	-0.114580	0.000000	0.000000	0.000000
131	~2963	CASE-3	0.000000	0.000000	-0.097474	0.000000	0.000000	0.000000
132	~3079	CASE-3	0.000000	0.000000	-0.259667	0.000000	0.000000	0.000000
133	~3195	CASE-3	0.000000	0.000000	-0.205123	0.000000	0.000000	0.000000
134	~3478	CASE-3	0.000000	0.000000	-0.133087	0.000000	0.000000	0.000000
141	~2854	CASE-3	0.000000	0.000000	-0.007357	0.000000	0.000000	0.000000
1	283	CASE-4	0.000000	0.000000	-0.041171	0.000000	0.000000	0.000000
2	~4657	CASE-4	0.000000	0.000000	-0.011168	0.000000	0.000000	0.000000
3	~4947	CASE-4	0.000000	0.000000	-0.001683	0.000000	0.000000	0.000000
11	~2920	CASE-4	0.000000	0.000000	-0.108335	0.000000	0.000000	0.000000
12	~4731	CASE-4	0.000000	0.000000	-0.110328	0.000000	0.000000	0.000000
13	~549	CASE-4	0.000000	0.000000	-0.063034	0.000000	0.000000	0.000000
21	~2316	CASE-4	0.000000	0.000000	-0.105892	0.000000	0.000000	0.000000
22	~2317	CASE-4	0.000000	0.000000	-0.105488	0.000000	0.000000	0.000000
23	~3153	CASE-4	0.000000	0.000000	-0.063946	0.000000	0.000000	0.000000
31	~412	CASE-4	0.000000	0.000000	-0.066276	0.000000	0.000000	0.000000
32	~448	CASE-4	0.000000	0.000000	-0.090327	0.000000	0.000000	0.000000
33	~556	CASE-4	0.000000	0.000000	-0.041322	0.000000	0.000000	0.000000
38	289	CASE-4	0.000000	0.000000	-0.144030	0.000000	0.000000	0.000000
39	~2384	CASE-4	0.000000	0.000000	-0.121166	0.000000	0.000000	0.000000
40	~1824	CASE-4	0.000000	0.000000	-0.142910	0.000000	0.000000	0.000000
41	~416	CASE-4	0.000000	0.000000	-0.167765	0.000000	0.000000	0.000000
42	~488	CASE-4	0.000000	0.000000	-0.260968	0.000000	0.000000	0.000000
43	~560	CASE-4	0.000000	0.000000	-0.178192	0.000000	0.000000	0.000000
44	298	CASE-4	0.000000	0.000000	-0.013215	0.000000	0.000000	0.000000
45	~1807	CASE-4	0.000000	0.000000	-0.015735	0.000000	0.000000	0.000000
46	~2299	CASE-4	0.000000	0.000000	-0.006153	0.000000	0.000000	0.000000
47	~1639	CASE-4	0.000000	0.000000	-0.061929	0.000000	0.000000	0.000000
48	~1196	CASE-4	0.000000	0.000000	-0.032521	0.000000	0.000000	0.000000
49	~1341	CASE-4	0.000000	0.000000	0.025798	0.000000	0.000000	0.000000
50	~2171	CASE-4	0.000000	0.000000	-0.049469	0.000000	0.000000	0.000000
51	~417	CASE-4	0.000000	0.000000	-0.170029	0.000000	0.000000	0.000000
52	~489	CASE-4	0.000000	0.000000	-0.270359	0.000000	0.000000	0.000000
53	~561	CASE-4	0.000000	0.000000	-0.180861	0.000000	0.000000	0.000000
54	~778	CASE-4	0.000000	0.000000	-0.469219	0.000000	0.000000	0.000000
57	~1167	CASE-4	0.000000	0.000000	-0.097957	0.000000	0.000000	0.000000
58	~1310	CASE-4	0.000000	0.000000	-0.314765	0.000000	0.000000	0.000000
59	~1346	CASE-4	0.000000	0.000000	-0.295075	0.000000	0.000000	0.000000
60	~2200	CASE-4	0.000000	0.000000	0.055236	0.000000	0.000000	0.000000
61	~421	CASE-4	0.000000	0.000000	-0.097788	0.000000	0.000000	0.000000
62	~493	CASE-4	0.000000	0.000000	-0.189846	0.000000	0.000000	0.000000
63	~565	CASE-4	0.000000	0.000000	-0.111554	0.000000	0.000000	0.000000
64	~2466	CASE-4	0.000000	0.000000	-0.723857	0.000000	0.000000	0.000000
67	~1066	CASE-4	0.000000	0.000000	0.252674	0.000000	0.000000	0.000000

Table 33: Nodal Displacements - Summary, Part 1 of 2

Panel	Node	OutputCase	Ux	Uy	Uz	Rx	Ry	Rz
			in	in	in	Radians	Radians	Radians
68	~1315	CASE-4	0.000000	0.000000	-0.737717	0.000000	0.000000	0.000000
69	~1351	CASE-4	0.000000	0.000000	-0.746554	0.000000	0.000000	0.000000
70	~2181	CASE-4	0.000000	0.000000	0.242252	0.000000	0.000000	0.000000
71	~2942	CASE-4	0.000000	0.000000	-0.075376	0.000000	0.000000	0.000000
72	~3116	CASE-4	0.000000	0.000000	-0.205629	0.000000	0.000000	0.000000
73	~3174	CASE-4	0.000000	0.000000	-0.168086	0.000000	0.000000	0.000000
74	~1606	CASE-4	0.000000	0.000000	-0.593616	0.000000	0.000000	0.000000
75	~1609	CASE-4	0.000000	0.000000	-0.609624	0.000000	0.000000	0.000000
76	~2502	CASE-4	0.000000	0.000000	-0.467831	0.000000	0.000000	0.000000
77	~1069	CASE-4	0.000000	0.000000	0.182691	0.000000	0.000000	0.000000
78	~1317	CASE-4	0.000000	0.000000	-0.785484	0.000000	0.000000	0.000000
79	~1353	CASE-4	0.000000	0.000000	-0.808782	0.000000	0.000000	0.000000
80	~2685	CASE-4	0.000000	0.000000	0.295495	0.000000	0.000000	0.000000
81	~2944	CASE-4	0.000000	0.000000	-0.098973	0.000000	0.000000	0.000000
82	~3063	CASE-4	0.000000	0.000000	-0.233421	0.000000	0.000000	0.000000
83	~3179	CASE-4	0.000000	0.000000	-0.170927	0.000000	0.000000	0.000000
84	~3573	CASE-4	0.000000	0.000000	-0.275255	0.000000	0.000000	0.000000
85	~4916	CASE-4	0.000000	0.000000	-0.359727	0.000000	0.000000	0.000000
86	~4786	CASE-4	0.000000	0.000000	-0.365354	0.000000	0.000000	0.000000
87	~2507	CASE-4	0.000000	0.000000	-0.129527	0.000000	0.000000	0.000000
88	~1320	CASE-4	0.000000	0.000000	-0.731059	0.000000	0.000000	0.000000
89	~1356	CASE-4	0.000000	0.000000	-0.761008	0.000000	0.000000	0.000000
90	~2187	CASE-4	0.000000	0.000000	0.286845	0.000000	0.000000	0.000000
91	~2949	CASE-4	0.000000	0.000000	-0.103615	0.000000	0.000000	0.000000
92	~3064	CASE-4	0.000000	0.000000	-0.236986	0.000000	0.000000	0.000000
93	~3181	CASE-4	0.000000	0.000000	-0.182733	0.000000	0.000000	0.000000
94	~4282	CASE-4	0.000000	0.000000	-0.275479	0.000000	0.000000	0.000000
95	~5010	CASE-4	0.000000	0.000000	-0.780869	0.000000	0.000000	0.000000
96	~4990	CASE-4	0.000000	0.000000	-0.820022	0.000000	0.000000	0.000000
97	~4904	CASE-4	0.000000	0.000000	-0.330487	0.000000	0.000000	0.000000
98	~1324	CASE-4	0.000000	0.000000	-0.503235	0.000000	0.000000	0.000000
99	~1360	CASE-4	0.000000	0.000000	-0.533577	0.000000	0.000000	0.000000
100	~2192	CASE-4	0.000000	0.000000	0.158879	0.000000	0.000000	0.000000
101	~4336	CASE-4	0.000000	0.000000	-0.076657	0.000000	0.000000	0.000000
102	~4667	CASE-4	0.000000	0.000000	-0.208575	0.000000	0.000000	0.000000
103	~4589	CASE-4	0.000000	0.000000	-0.155457	0.000000	0.000000	0.000000
106	~4989	CASE-4	0.000000	0.000000	-0.824165	0.000000	0.000000	0.000000
107	~5008	CASE-4	0.000000	0.000000	-0.407966	0.000000	0.000000	0.000000
108	~1258	CASE-4	0.000000	0.000000	0.070925	0.000000	0.000000	0.000000
109	~1400	CASE-4	0.000000	0.000000	-0.132273	0.000000	0.000000	0.000000
110	~2195	CASE-4	0.000000	0.000000	0.045651	0.000000	0.000000	0.000000
111	~4386	CASE-4	0.000000	0.000000	-0.001903	0.000000	0.000000	0.000000
113	~4880	CASE-4	0.000000	0.000000	0.005360	0.000000	0.000000	0.000000
121	~2962	CASE-4	0.000000	0.000000	-0.061948	0.000000	0.000000	0.000000
122	~3078	CASE-4	0.000000	0.000000	-0.158273	0.000000	0.000000	0.000000
123	~3194	CASE-4	0.000000	0.000000	-0.123962	0.000000	0.000000	0.000000
124	~3477	CASE-4	0.000000	0.000000	-0.050277	0.000000	0.000000	0.000000
131	~2963	CASE-4	0.000000	0.000000	-0.060833	0.000000	0.000000	0.000000
132	~3137	CASE-4	0.000000	0.000000	-0.165025	0.000000	0.000000	0.000000
133	~3195	CASE-4	0.000000	0.000000	-0.134424	0.000000	0.000000	0.000000
134	~3478	CASE-4	0.000000	0.000000	-0.063777	0.000000	0.000000	0.000000
141	~2854	CASE-4	0.000000	0.000000	-0.004775	0.000000	0.000000	0.000000
1	283	CASE-5	0.000000	0.000000	-0.057491	0.000000	0.000000	0.000000
2	~4657	CASE-5	0.000000	0.000000	-0.016118	0.000000	0.000000	0.000000

Table 33: Nodal Displacements - Summary, Part 1 of 2

Panel	Node	OutputCase	Ux	Uy	Uz	Rx	Ry	Rz
			in	in	in	Radians	Radians	Radians
3	~4947	CASE-5	0.000000	0.000000	-0.002950	0.000000	0.000000	0.000000
11	~2920	CASE-5	0.000000	0.000000	-0.178673	0.000000	0.000000	0.000000
12	~4731	CASE-5	0.000000	0.000000	-0.179053	0.000000	0.000000	0.000000
13	~549	CASE-5	0.000000	0.000000	-0.093458	0.000000	0.000000	0.000000
21	~5062	CASE-5	0.000000	0.000000	-0.175179	0.000000	0.000000	0.000000
22	~2317	CASE-5	0.000000	0.000000	-0.171286	0.000000	0.000000	0.000000
23	~3153	CASE-5	0.000000	0.000000	-0.094047	0.000000	0.000000	0.000000
31	~412	CASE-5	0.000000	0.000000	-0.098665	0.000000	0.000000	0.000000
32	~448	CASE-5	0.000000	0.000000	-0.135640	0.000000	0.000000	0.000000
33	~556	CASE-5	0.000000	0.000000	-0.062508	0.000000	0.000000	0.000000
38	289	CASE-5	0.000000	0.000000	-0.212741	0.000000	0.000000	0.000000
39	~2384	CASE-5	0.000000	0.000000	-0.183362	0.000000	0.000000	0.000000
40	~1824	CASE-5	0.000000	0.000000	-0.209383	0.000000	0.000000	0.000000
41	~416	CASE-5	0.000000	0.000000	-0.264260	0.000000	0.000000	0.000000
42	~488	CASE-5	0.000000	0.000000	-0.431576	0.000000	0.000000	0.000000
43	~560	CASE-5	0.000000	0.000000	-0.284768	0.000000	0.000000	0.000000
44	298	CASE-5	0.000000	0.000000	-0.015995	0.000000	0.000000	0.000000
45	~1807	CASE-5	0.000000	0.000000	-0.019746	0.000000	0.000000	0.000000
46	~2299	CASE-5	0.000000	0.000000	-0.008525	0.000000	0.000000	0.000000
47	~1639	CASE-5	0.000000	0.000000	-0.092099	0.000000	0.000000	0.000000
48	~1196	CASE-5	0.000000	0.000000	-0.047398	0.000000	0.000000	0.000000
49	~1341	CASE-5	0.000000	0.000000	0.038398	0.000000	0.000000	0.000000
50	~2171	CASE-5	0.000000	0.000000	-0.069603	0.000000	0.000000	0.000000
51	~417	CASE-5	0.000000	0.000000	-0.268588	0.000000	0.000000	0.000000
52	~489	CASE-5	0.000000	0.000000	-0.449977	0.000000	0.000000	0.000000
53	~561	CASE-5	0.000000	0.000000	-0.288922	0.000000	0.000000	0.000000
54	~778	CASE-5	0.000000	0.000000	-0.669478	0.000000	0.000000	0.000000
57	~1167	CASE-5	0.000000	0.000000	-0.151916	0.000000	0.000000	0.000000
58	~1310	CASE-5	0.000000	0.000000	-0.501022	0.000000	0.000000	0.000000
59	~1346	CASE-5	0.000000	0.000000	-0.467337	0.000000	0.000000	0.000000
60	~2200	CASE-5	0.000000	0.000000	0.075014	0.000000	0.000000	0.000000
61	~421	CASE-5	0.000000	0.000000	-0.152758	0.000000	0.000000	0.000000
62	~493	CASE-5	0.000000	0.000000	-0.305385	0.000000	0.000000	0.000000
63	~565	CASE-5	0.000000	0.000000	-0.166457	0.000000	0.000000	0.000000
64	~2467	CASE-5	0.000000	0.000000	-1.027356	0.000000	0.000000	0.000000
67	~1066	CASE-5	0.000000	0.000000	0.403104	0.000000	0.000000	0.000000
68	~1315	CASE-5	0.000000	0.000000	-1.200126	0.000000	0.000000	0.000000
69	~1351	CASE-5	0.000000	0.000000	-1.210566	0.000000	0.000000	0.000000
70	~2181	CASE-5	0.000000	0.000000	0.381474	0.000000	0.000000	0.000000
71	~2942	CASE-5	0.000000	0.000000	-0.124784	0.000000	0.000000	0.000000
72	~3116	CASE-5	0.000000	0.000000	-0.349396	0.000000	0.000000	0.000000
73	~3174	CASE-5	0.000000	0.000000	-0.279853	0.000000	0.000000	0.000000
74	~1606	CASE-5	0.000000	0.000000	-0.833298	0.000000	0.000000	0.000000
75	~1609	CASE-5	0.000000	0.000000	-0.863103	0.000000	0.000000	0.000000
76	~2502	CASE-5	0.000000	0.000000	-0.668866	0.000000	0.000000	0.000000
77	~1069	CASE-5	0.000000	0.000000	0.280726	0.000000	0.000000	0.000000
78	~1317	CASE-5	0.000000	0.000000	-1.250759	0.000000	0.000000	0.000000
79	~1353	CASE-5	0.000000	0.000000	-1.315173	0.000000	0.000000	0.000000
80	~2186	CASE-5	0.000000	0.000000	0.469049	0.000000	0.000000	0.000000
81	~2944	CASE-5	0.000000	0.000000	-0.164243	0.000000	0.000000	0.000000
82	~3063	CASE-5	0.000000	0.000000	-0.426818	0.000000	0.000000	0.000000
83	~3179	CASE-5	0.000000	0.000000	-0.299097	0.000000	0.000000	0.000000
84	~3573	CASE-5	0.000000	0.000000	-0.403799	0.000000	0.000000	0.000000
85	~5021	CASE-5	0.000000	0.000000	-0.516732	0.000000	0.000000	0.000000

Table 33: Nodal Displacements - Summary, Part 1 of 2

Panel	Node	OutputCase	Ux	Uy	Uz	Rx	Ry	Rz
			in	in	in	Radians	Radians	Radians
86	~4786	CASE-5	0.000000	0.000000	-0.515875	0.000000	0.000000	0.000000
87	~2507	CASE-5	0.000000	0.000000	-0.172302	0.000000	0.000000	0.000000
88	~1320	CASE-5	0.000000	0.000000	-1.156916	0.000000	0.000000	0.000000
89	~1356	CASE-5	0.000000	0.000000	-1.239338	0.000000	0.000000	0.000000
90	~2187	CASE-5	0.000000	0.000000	0.455902	0.000000	0.000000	0.000000
91	~2949	CASE-5	0.000000	0.000000	-0.170384	0.000000	0.000000	0.000000
92	~3064	CASE-5	0.000000	0.000000	-0.430748	0.000000	0.000000	0.000000
93	~3180	CASE-5	0.000000	0.000000	-0.317932	0.000000	0.000000	0.000000
94	~4282	CASE-5	0.000000	0.000000	-0.445476	0.000000	0.000000	0.000000
95	~5010	CASE-5	0.000000	0.000000	-1.151737	0.000000	0.000000	0.000000
96	~4990	CASE-5	0.000000	0.000000	-1.202045	0.000000	0.000000	0.000000
97	~4904	CASE-5	0.000000	0.000000	-0.467265	0.000000	0.000000	0.000000
98	~1324	CASE-5	0.000000	0.000000	-0.782538	0.000000	0.000000	0.000000
99	~1360	CASE-5	0.000000	0.000000	-0.832213	0.000000	0.000000	0.000000
100	~2192	CASE-5	0.000000	0.000000	0.240157	0.000000	0.000000	0.000000
101	~4336	CASE-5	0.000000	0.000000	-0.123653	0.000000	0.000000	0.000000
102	~4667	CASE-5	0.000000	0.000000	-0.359821	0.000000	0.000000	0.000000
103	~4589	CASE-5	0.000000	0.000000	-0.261307	0.000000	0.000000	0.000000
106	~4989	CASE-5	0.000000	0.000000	-1.205342	0.000000	0.000000	0.000000
107	~5008	CASE-5	0.000000	0.000000	-0.583649	0.000000	0.000000	0.000000
108	~1258	CASE-5	0.000000	0.000000	0.109433	0.000000	0.000000	0.000000
109	~1400	CASE-5	0.000000	0.000000	-0.189731	0.000000	0.000000	0.000000
110	~2195	CASE-5	0.000000	0.000000	0.069372	0.000000	0.000000	0.000000
111	~4386	CASE-5	0.000000	0.000000	-0.002730	0.000000	0.000000	0.000000
113	~4880	CASE-5	0.000000	0.000000	0.006558	0.000000	0.000000	0.000000
121	~2962	CASE-5	0.000000	0.000000	-0.091864	0.000000	0.000000	0.000000
122	~3078	CASE-5	0.000000	0.000000	-0.234135	0.000000	0.000000	0.000000
123	~3194	CASE-5	0.000000	0.000000	-0.176622	0.000000	0.000000	0.000000
124	~3477	CASE-5	0.000000	0.000000	-0.092511	0.000000	0.000000	0.000000
131	~2963	CASE-5	0.000000	0.000000	-0.092436	0.000000	0.000000	0.000000
132	~3079	CASE-5	0.000000	0.000000	-0.246430	0.000000	0.000000	0.000000
133	~3195	CASE-5	0.000000	0.000000	-0.194852	0.000000	0.000000	0.000000
134	~3478	CASE-5	0.000000	0.000000	-0.111989	0.000000	0.000000	0.000000
141	~2854	CASE-5	0.000000	0.000000	-0.008266	0.000000	0.000000	0.000000

Table 33: Nodal Displacements - Summary, Part 2 of 2

Table 33: Nodal Displacements - Summary, Part 2 of 2

Panel	Node	OutputCase	GlobalX	GlobalY
			ft	ft
1	283	CASE-1	0.0000	0.0000
2	~4657	CASE-1	10.3260	14.9743
3	~4950	CASE-1	10.3260	24.3266
11	~2920	CASE-1	22.4113	11.3354
12	~5065	CASE-1	22.4113	13.6836
13	~549	CASE-1	22.4113	26.0583
21	~2316	CASE-1	24.6769	11.8374
22	~2317	CASE-1	25.0094	12.6207
23	~3153	CASE-1	25.4606	25.8902
31	~412	CASE-1	46.8708	11.8374
32	~448	CASE-1	46.8708	15.3930
33	~556	CASE-1	46.8708	26.0583
38	289	CASE-1	43.3941	85.7361

Table 33: Nodal Displacements - Summary, Part 2 of 2

Panel	Node	OutputCase	GlobalX	GlobalY
			ft	ft
39	~2387	CASE-1	43.3941	106.7793
40	~1824	CASE-1	43.3941	114.8483
41	~416	CASE-1	58.0729	11.8374
42	~488	CASE-1	58.0729	18.9485
43	~560	CASE-1	58.0729	26.0583
44	298	CASE-1	58.4094	46.3358
45	~1807	CASE-1	58.4094	50.2089
46	~2449	CASE-1	58.4094	61.8346
47	~1639	CASE-1	49.8549	83.1246
48	~1196	CASE-1	49.9094	86.2627
49	~1341	CASE-1	54.9931	96.8376
50	~2171	CASE-1	49.9094	114.8483
51	~417	CASE-1	61.1995	11.8374
52	~490	CASE-1	64.3260	18.9485
53	~561	CASE-1	61.1995	26.0583
54	~778	CASE-1	68.7466	46.3333
57	~1167	CASE-1	68.7466	83.2594
58	~1310	CASE-1	68.7466	93.5237
59	~2021	CASE-1	70.3764	100.1515
60	~2200	CASE-1	69.2446	114.8483
61	~421	CASE-1	72.5177	11.8374
62	~3055	CASE-1	82.5910	18.6131
63	~5032	CASE-1	82.5910	24.9409
64	~2466	CASE-1	80.7388	46.4339
67	295	CASE-1	73.7330	75.0833
68	~1315	CASE-1	83.2741	93.5237
69	~1351	CASE-1	83.2741	96.8376
70	~2181	CASE-1	83.2741	114.8483
71	~2942	CASE-1	93.6594	11.3354
72	~3058	CASE-1	93.6594	18.6131
73	~3174	CASE-1	93.6594	25.8902
74	~1606	CASE-1	86.5920	46.3659
75	~1609	CASE-1	90.0818	50.1364
76	~2502	CASE-1	94.3420	60.2479
77	~1173	CASE-1	86.5920	83.2594
78	~1317	CASE-1	89.9100	93.5237
79	~1354	CASE-1	92.2844	96.8376
80	~2186	CASE-1	94.6589	114.8483
81	~2945	CASE-1	101.9293	11.3354
82	~3063	CASE-1	107.1667	18.6131
83	~3179	CASE-1	107.1667	25.8902
84	~3573	CASE-1	96.4160	47.3374
85	~5021	CASE-1	96.4160	56.4522
86	~4786	CASE-1	96.0341	63.0647
87	~2507	CASE-1	99.3135	72.0476
88	~1320	CASE-1	97.6539	93.5237
89	~1356	CASE-1	97.6539	96.8376
90	~2187	CASE-1	97.6539	114.8483
91	~2949	CASE-1	113.7533	11.3354
92	~3064	CASE-1	110.4600	18.6131
93	~3180	CASE-1	110.4600	25.8902
94	~4282	CASE-1	113.6406	47.3852
95	~5010	CASE-1	118.8764	59.7199
96	~4990	CASE-1	119.8447	62.0011

Table 33: Nodal Displacements - Summary, Part 2 of 2

Panel	Node	OutputCase	GlobalX	GlobalY
			ft	ft
97	~4904	CASE-1	119.7427	72.9010
98	~1324	CASE-1	109.3991	93.5237
99	~1360	CASE-1	109.3991	96.8376
100	~2192	CASE-1	109.3991	114.8483
101	~4336	CASE-1	120.4927	11.3354
102	~4667	CASE-1	120.0760	22.2940
103	~4589	CASE-1	120.0760	25.9182
106	~4989	CASE-1	120.2701	63.0033
107	~5008	CASE-1	124.3872	72.7025
108	~1328	CASE-1	121.7427	93.5237
109	~1400	CASE-1	121.7427	100.1515
110	~1828	CASE-1	126.5730	114.8483
111	~4247	CASE-1	140.9529	7.9992
113	~4880	CASE-1	133.8486	33.1539
121	~2962	CASE-1	152.1418	11.3354
122	~3136	CASE-1	152.1418	22.2520
123	~3194	CASE-1	152.1418	25.8902
124	~3477	CASE-1	152.1418	40.0000
131	~2963	CASE-1	156.1324	11.3354
132	~3137	CASE-1	156.1324	22.2520
133	~3195	CASE-1	156.1324	25.8902
134	~3478	CASE-1	156.1324	40.0000
141	~2854	CASE-1	170.5100	3.8342
1	283	CASE-2	0.0000	0.0000
2	~4657	CASE-2	10.3260	14.9743
3	~4950	CASE-2	10.3260	24.3266
11	~2920	CASE-2	22.4113	11.3354
12	~4731	CASE-2	22.4113	12.8505
13	~549	CASE-2	22.4113	26.0583
21	~5062	CASE-2	24.4638	11.3354
22	~2317	CASE-2	25.0094	12.6207
23	~3153	CASE-2	25.4606	25.8902
31	~412	CASE-2	46.8708	11.8374
32	~448	CASE-2	46.8708	15.3930
33	~556	CASE-2	46.8708	26.0583
38	289	CASE-2	43.3941	85.7361
39	~2387	CASE-2	43.3941	106.7793
40	~1824	CASE-2	43.3941	114.8483
41	~416	CASE-2	58.0729	11.8374
42	~488	CASE-2	58.0729	18.9485
43	~560	CASE-2	58.0729	26.0583
44	298	CASE-2	58.4094	46.3358
45	~1807	CASE-2	58.4094	50.2089
46	~2452	CASE-2	58.4094	71.9586
47	~1639	CASE-2	49.8549	83.1246
48	~1196	CASE-2	49.9094	86.2627
49	~1341	CASE-2	54.9931	96.8376
50	~2171	CASE-2	49.9094	114.8483
51	~417	CASE-2	61.1995	11.8374
52	~489	CASE-2	61.1995	18.9485
53	~561	CASE-2	61.1995	26.0583
54	~778	CASE-2	68.7466	46.3333
57	~1167	CASE-2	68.7466	83.2594
58	~1310	CASE-2	68.7466	93.5237

Table 33: Nodal Displacements - Summary, Part 2 of 2

Panel	Node	OutputCase	GlobalX	GlobalY
			ft	ft
59	~1346	CASE-2	68.7466	96.8376
60	~2200	CASE-2	69.2446	114.8483
61	~421	CASE-2	72.5177	11.8374
62	~493	CASE-2	72.5177	18.9485
63	~5032	CASE-2	82.5910	24.9409
64	~2466	CASE-2	80.7388	46.4339
67	~1065	CASE-2	75.1120	75.0833
68	~1315	CASE-2	83.2741	93.5237
69	~1351	CASE-2	83.2741	96.8376
70	~2181	CASE-2	83.2741	114.8483
71	~2942	CASE-2	93.6594	11.3354
72	~3116	CASE-2	93.6594	22.2520
73	~3174	CASE-2	93.6594	25.8902
74	~1606	CASE-2	86.5920	46.3659
75	~1609	CASE-2	90.0818	50.1364
76	~2502	CASE-2	94.3420	60.2479
77	~1173	CASE-2	86.5920	83.2594
78	~1318	CASE-2	92.2844	93.5237
79	~1354	CASE-2	92.2844	96.8376
80	~2186	CASE-2	94.6589	114.8483
81	~2945	CASE-2	101.9293	11.3354
82	~4658	CASE-2	106.7603	18.6131
83	~3179	CASE-2	107.1667	25.8902
84	~3573	CASE-2	96.4160	47.3374
85	~3745	CASE-2	96.4160	57.4445
86	~4786	CASE-2	96.0341	63.0647
87	~2507	CASE-2	99.3135	72.0476
88	~1320	CASE-2	97.6539	93.5237
89	~1356	CASE-2	97.6539	96.8376
90	~2187	CASE-2	97.6539	114.8483
91	~2949	CASE-2	113.7533	11.3354
92	~3122	CASE-2	110.4600	22.2520
93	~3180	CASE-2	110.4600	25.8902
94	~4282	CASE-2	113.6406	47.3852
95	~5010	CASE-2	118.8764	59.7199
96	~4990	CASE-2	119.8447	62.0011
97	~4904	CASE-2	119.7427	72.9010
98	~1324	CASE-2	109.3991	93.5237
99	~1360	CASE-2	109.3991	96.8376
100	~2192	CASE-2	109.3991	114.8483
101	~4336	CASE-2	120.4927	11.3354
102	~4667	CASE-2	120.0760	22.2940
103	~4589	CASE-2	120.0760	25.9182
106	~4989	CASE-2	120.2701	63.0033
107	~5008	CASE-2	124.3872	72.7025
108	~1328	CASE-2	121.7427	93.5237
109	~1400	CASE-2	121.7427	100.1515
110	~1828	CASE-2	126.5730	114.8483
111	~4247	CASE-2	140.9529	7.9992
113	~4880	CASE-2	133.8486	33.1539
121	~2962	CASE-2	152.1418	11.3354
122	~3136	CASE-2	152.1418	22.2520
123	~3194	CASE-2	152.1418	25.8902
124	~3477	CASE-2	152.1418	40.0000

Table 33: Nodal Displacements - Summary, Part 2 of 2

Panel	Node	OutputCase	GlobalX	GlobalY
			ft	ft
131	~2963	CASE-2	156.1324	11.3354
132	~3137	CASE-2	156.1324	22.2520
133	~3195	CASE-2	156.1324	25.8902
134	~3420	CASE-2	156.1324	37.2541
141	~2854	CASE-2	170.5100	3.8342
1	283	CASE-3	0.0000	0.0000
2	~4657	CASE-3	10.3260	14.9743
3	~4947	CASE-3	11.3844	26.8200
11	~2920	CASE-3	22.4113	11.3354
12	~4731	CASE-3	22.4113	12.8505
13	~549	CASE-3	22.4113	26.0583
21	~5062	CASE-3	24.4638	11.3354
22	~2317	CASE-3	25.0094	12.6207
23	~3153	CASE-3	25.4606	25.8902
31	~412	CASE-3	46.8708	11.8374
32	~448	CASE-3	46.8708	15.3930
33	~556	CASE-3	46.8708	26.0583
38	289	CASE-3	43.3941	85.7361
39	~2385	CASE-3	43.3941	100.1515
40	~1824	CASE-3	43.3941	114.8483
41	~416	CASE-3	58.0729	11.8374
42	~488	CASE-3	58.0729	18.9485
43	~560	CASE-3	58.0729	26.0583
44	298	CASE-3	58.4094	46.3358
45	~1807	CASE-3	58.4094	50.2089
46	~2299	CASE-3	57.7083	71.9586
47	~1639	CASE-3	49.8549	83.1246
48	~1196	CASE-3	49.9094	86.2627
49	~1341	CASE-3	54.9931	96.8376
50	~2171	CASE-3	49.9094	114.8483
51	~417	CASE-3	61.1995	11.8374
52	~489	CASE-3	61.1995	18.9485
53	~561	CASE-3	61.1995	26.0583
54	~778	CASE-3	68.7466	46.3333
57	~1167	CASE-3	68.7466	83.2594
58	~1310	CASE-3	68.7466	93.5237
59	~1346	CASE-3	68.7466	96.8376
60	~2200	CASE-3	69.2446	114.8483
61	~421	CASE-3	72.5177	11.8374
62	~493	CASE-3	72.5177	18.9485
63	~565	CASE-3	72.5177	26.0583
64	~2467	CASE-3	77.7490	46.4339
67	~1065	CASE-3	75.1120	75.0833
68	~1315	CASE-3	83.2741	93.5237
69	~1351	CASE-3	83.2741	96.8376
70	~2181	CASE-3	83.2741	114.8483
71	~2942	CASE-3	93.6594	11.3354
72	~3116	CASE-3	93.6594	22.2520
73	~3174	CASE-3	93.6594	25.8902
74	~1606	CASE-3	86.5920	46.3659
75	~1609	CASE-3	90.0818	50.1364
76	~2502	CASE-3	94.3420	60.2479
77	~1173	CASE-3	86.5920	83.2594
78	~1317	CASE-3	89.9100	93.5237

Table 33: Nodal Displacements - Summary, Part 2 of 2

Panel	Node	OutputCase	GlobalX	GlobalY
			ft	ft
79	~1354	CASE-3	92.2844	96.8376
80	~2186	CASE-3	94.6589	114.8483
81	~2944	CASE-3	99.1727	11.3354
82	~3063	CASE-3	107.1667	18.6131
83	~3179	CASE-3	107.1667	25.8902
84	~3573	CASE-3	96.4160	47.3374
85	~5021	CASE-3	96.4160	56.4522
86	~4786	CASE-3	96.0341	63.0647
87	~2507	CASE-3	99.3135	72.0476
88	~1320	CASE-3	97.6539	93.5237
89	~1356	CASE-3	97.6539	96.8376
90	~2187	CASE-3	97.6539	114.8483
91	~2949	CASE-3	113.7533	11.3354
92	~3064	CASE-3	110.4600	18.6131
93	~3180	CASE-3	110.4600	25.8902
94	~4282	CASE-3	113.6406	47.3852
95	~5010	CASE-3	118.8764	59.7199
96	~4990	CASE-3	119.8447	62.0011
97	~4904	CASE-3	119.7427	72.9010
98	~1324	CASE-3	109.3991	93.5237
99	~1360	CASE-3	109.3991	96.8376
100	~2192	CASE-3	109.3991	114.8483
101	~4336	CASE-3	120.4927	11.3354
102	~4667	CASE-3	120.0760	22.2940
103	~4589	CASE-3	120.0760	25.9182
106	~4989	CASE-3	120.2701	63.0033
107	~5008	CASE-3	124.3872	72.7025
108	~1258	CASE-3	126.5730	89.2538
109	~1400	CASE-3	121.7427	100.1515
110	~2195	CASE-3	121.7427	114.8483
111	~4386	CASE-3	134.0759	0.3333
113	~4880	CASE-3	133.8486	33.1539
121	~2962	CASE-3	152.1418	11.3354
122	~3078	CASE-3	152.1418	18.6131
123	~3194	CASE-3	152.1418	25.8902
124	~3477	CASE-3	152.1418	40.0000
131	~2963	CASE-3	156.1324	11.3354
132	~3079	CASE-3	156.1324	18.6131
133	~3195	CASE-3	156.1324	25.8902
134	~3478	CASE-3	156.1324	40.0000
141	~2854	CASE-3	170.5100	3.8342
1	283	CASE-4	0.0000	0.0000
2	~4657	CASE-4	10.3260	14.9743
3	~4947	CASE-4	11.3844	26.8200
11	~2920	CASE-4	22.4113	11.3354
12	~4731	CASE-4	22.4113	12.8505
13	~549	CASE-4	22.4113	26.0583
21	~2316	CASE-4	24.6769	11.8374
22	~2317	CASE-4	25.0094	12.6207
23	~3153	CASE-4	25.4606	25.8902
31	~412	CASE-4	46.8708	11.8374
32	~448	CASE-4	46.8708	15.3930
33	~556	CASE-4	46.8708	26.0583
38	289	CASE-4	43.3941	85.7361

Table 33: Nodal Displacements - Summary, Part 2 of 2

Panel	Node	OutputCase	GlobalX	GlobalY
			ft	ft
39	~2384	CASE-4	43.3941	96.8376
40	~1824	CASE-4	43.3941	114.8483
41	~416	CASE-4	58.0729	11.8374
42	~488	CASE-4	58.0729	18.9485
43	~560	CASE-4	58.0729	26.0583
44	298	CASE-4	58.4094	46.3358
45	~1807	CASE-4	58.4094	50.2089
46	~2299	CASE-4	57.7083	71.9586
47	~1639	CASE-4	49.8549	83.1246
48	~1196	CASE-4	49.9094	86.2627
49	~1341	CASE-4	54.9931	96.8376
50	~2171	CASE-4	49.9094	114.8483
51	~417	CASE-4	61.1995	11.8374
52	~489	CASE-4	61.1995	18.9485
53	~561	CASE-4	61.1995	26.0583
54	~778	CASE-4	68.7466	46.3333
57	~1167	CASE-4	68.7466	83.2594
58	~1310	CASE-4	68.7466	93.5237
59	~1346	CASE-4	68.7466	96.8376
60	~2200	CASE-4	69.2446	114.8483
61	~421	CASE-4	72.5177	11.8374
62	~493	CASE-4	72.5177	18.9485
63	~565	CASE-4	72.5177	26.0583
64	~2466	CASE-4	80.7388	46.4339
67	~1066	CASE-4	77.7063	75.0833
68	~1315	CASE-4	83.2741	93.5237
69	~1351	CASE-4	83.2741	96.8376
70	~2181	CASE-4	83.2741	114.8483
71	~2942	CASE-4	93.6594	11.3354
72	~3116	CASE-4	93.6594	22.2520
73	~3174	CASE-4	93.6594	25.8902
74	~1606	CASE-4	86.5920	46.3659
75	~1609	CASE-4	90.0818	50.1364
76	~2502	CASE-4	94.3420	60.2479
77	~1069	CASE-4	86.5920	75.0833
78	~1317	CASE-4	89.9100	93.5237
79	~1353	CASE-4	89.9100	96.8376
80	~2685	CASE-4	92.2640	114.8483
81	~2944	CASE-4	99.1727	11.3354
82	~3063	CASE-4	107.1667	18.6131
83	~3179	CASE-4	107.1667	25.8902
84	~3573	CASE-4	96.4160	47.3374
85	~4916	CASE-4	96.4160	59.0602
86	~4786	CASE-4	96.0341	63.0647
87	~2507	CASE-4	99.3135	72.0476
88	~1320	CASE-4	97.6539	93.5237
89	~1356	CASE-4	97.6539	96.8376
90	~2187	CASE-4	97.6539	114.8483
91	~2949	CASE-4	113.7533	11.3354
92	~3064	CASE-4	110.4600	18.6131
93	~3181	CASE-4	113.7533	25.8902
94	~4282	CASE-4	113.6406	47.3852
95	~5010	CASE-4	118.8764	59.7199
96	~4990	CASE-4	119.8447	62.0011

Table 33: Nodal Displacements - Summary, Part 2 of 2

Panel	Node	OutputCase	GlobalX	GlobalY
			ft	ft
97	~4904	CASE-4	119.7427	72.9010
98	~1324	CASE-4	109.3991	93.5237
99	~1360	CASE-4	109.3991	96.8376
100	~2192	CASE-4	109.3991	114.8483
101	~4336	CASE-4	120.4927	11.3354
102	~4667	CASE-4	120.0760	22.2940
103	~4589	CASE-4	120.0760	25.9182
106	~4989	CASE-4	120.2701	63.0033
107	~5008	CASE-4	124.3872	72.7025
108	~1258	CASE-4	126.5730	89.2538
109	~1400	CASE-4	121.7427	100.1515
110	~2195	CASE-4	121.7427	114.8483
111	~4386	CASE-4	134.0759	0.3333
113	~4880	CASE-4	133.8486	33.1539
121	~2962	CASE-4	152.1418	11.3354
122	~3078	CASE-4	152.1418	18.6131
123	~3194	CASE-4	152.1418	25.8902
124	~3477	CASE-4	152.1418	40.0000
131	~2963	CASE-4	156.1324	11.3354
132	~3137	CASE-4	156.1324	22.2520
133	~3195	CASE-4	156.1324	25.8902
134	~3478	CASE-4	156.1324	40.0000
141	~2854	CASE-4	170.5100	3.8342
1	283	CASE-5	0.0000	0.0000
2	~4657	CASE-5	10.3260	14.9743
3	~4947	CASE-5	11.3844	26.8200
11	~2920	CASE-5	22.4113	11.3354
12	~4731	CASE-5	22.4113	12.8505
13	~549	CASE-5	22.4113	26.0583
21	~5062	CASE-5	24.4638	11.3354
22	~2317	CASE-5	25.0094	12.6207
23	~3153	CASE-5	25.4606	25.8902
31	~412	CASE-5	46.8708	11.8374
32	~448	CASE-5	46.8708	15.3930
33	~556	CASE-5	46.8708	26.0583
38	289	CASE-5	43.3941	85.7361
39	~2384	CASE-5	43.3941	96.8376
40	~1824	CASE-5	43.3941	114.8483
41	~416	CASE-5	58.0729	11.8374
42	~488	CASE-5	58.0729	18.9485
43	~560	CASE-5	58.0729	26.0583
44	298	CASE-5	58.4094	46.3358
45	~1807	CASE-5	58.4094	50.2089
46	~2299	CASE-5	57.7083	71.9586
47	~1639	CASE-5	49.8549	83.1246
48	~1196	CASE-5	49.9094	86.2627
49	~1341	CASE-5	54.9931	96.8376
50	~2171	CASE-5	49.9094	114.8483
51	~417	CASE-5	61.1995	11.8374
52	~489	CASE-5	61.1995	18.9485
53	~561	CASE-5	61.1995	26.0583
54	~778	CASE-5	68.7466	46.3333
57	~1167	CASE-5	68.7466	83.2594
58	~1310	CASE-5	68.7466	93.5237

Table 33: Nodal Displacements - Summary, Part 2 of 2

Panel	Node	OutputCase	GlobalX	GlobalY
			ft	ft
59	~1346	CASE-5	68.7466	96.8376
60	~2200	CASE-5	69.2446	114.8483
61	~421	CASE-5	72.5177	11.8374
62	~493	CASE-5	72.5177	18.9485
63	~565	CASE-5	72.5177	26.0583
64	~2467	CASE-5	77.7490	46.4339
67	~1066	CASE-5	77.7063	75.0833
68	~1315	CASE-5	83.2741	93.5237
69	~1351	CASE-5	83.2741	96.8376
70	~2181	CASE-5	83.2741	114.8483
71	~2942	CASE-5	93.6594	11.3354
72	~3116	CASE-5	93.6594	22.2520
73	~3174	CASE-5	93.6594	25.8902
74	~1606	CASE-5	86.5920	46.3659
75	~1609	CASE-5	90.0818	50.1364
76	~2502	CASE-5	94.3420	60.2479
77	~1069	CASE-5	86.5920	75.0833
78	~1317	CASE-5	89.9100	93.5237
79	~1353	CASE-5	89.9100	96.8376
80	~2186	CASE-5	94.6589	114.8483
81	~2944	CASE-5	99.1727	11.3354
82	~3063	CASE-5	107.1667	18.6131
83	~3179	CASE-5	107.1667	25.8902
84	~3573	CASE-5	96.4160	47.3374
85	~5021	CASE-5	96.4160	56.4522
86	~4786	CASE-5	96.0341	63.0647
87	~2507	CASE-5	99.3135	72.0476
88	~1320	CASE-5	97.6539	93.5237
89	~1356	CASE-5	97.6539	96.8376
90	~2187	CASE-5	97.6539	114.8483
91	~2949	CASE-5	113.7533	11.3354
92	~3064	CASE-5	110.4600	18.6131
93	~3180	CASE-5	110.4600	25.8902
94	~4282	CASE-5	113.6406	47.3852
95	~5010	CASE-5	118.8764	59.7199
96	~4990	CASE-5	119.8447	62.0011
97	~4904	CASE-5	119.7427	72.9010
98	~1324	CASE-5	109.3991	93.5237
99	~1360	CASE-5	109.3991	96.8376
100	~2192	CASE-5	109.3991	114.8483
101	~4336	CASE-5	120.4927	11.3354
102	~4667	CASE-5	120.0760	22.2940
103	~4589	CASE-5	120.0760	25.9182
106	~4989	CASE-5	120.2701	63.0033
107	~5008	CASE-5	124.3872	72.7025
108	~1258	CASE-5	126.5730	89.2538
109	~1400	CASE-5	121.7427	100.1515
110	~2195	CASE-5	121.7427	114.8483
111	~4386	CASE-5	134.0759	0.3333
113	~4880	CASE-5	133.8486	33.1539
121	~2962	CASE-5	152.1418	11.3354
122	~3078	CASE-5	152.1418	18.6131
123	~3194	CASE-5	152.1418	25.8902
124	~3477	CASE-5	152.1418	40.0000

Table 33: Nodal Displacements - Summary, Part 2 of 2

Panel	Node	OutputCase	GlobalX	GlobalY
			ft	ft
131	~2963	CASE-5	156.1324	11.3354
132	~3079	CASE-5	156.1324	18.6131
133	~3195	CASE-5	156.1324	25.8902
134	~3478	CASE-5	156.1324	40.0000
141	~2854	CASE-5	170.5100	3.8342

Design

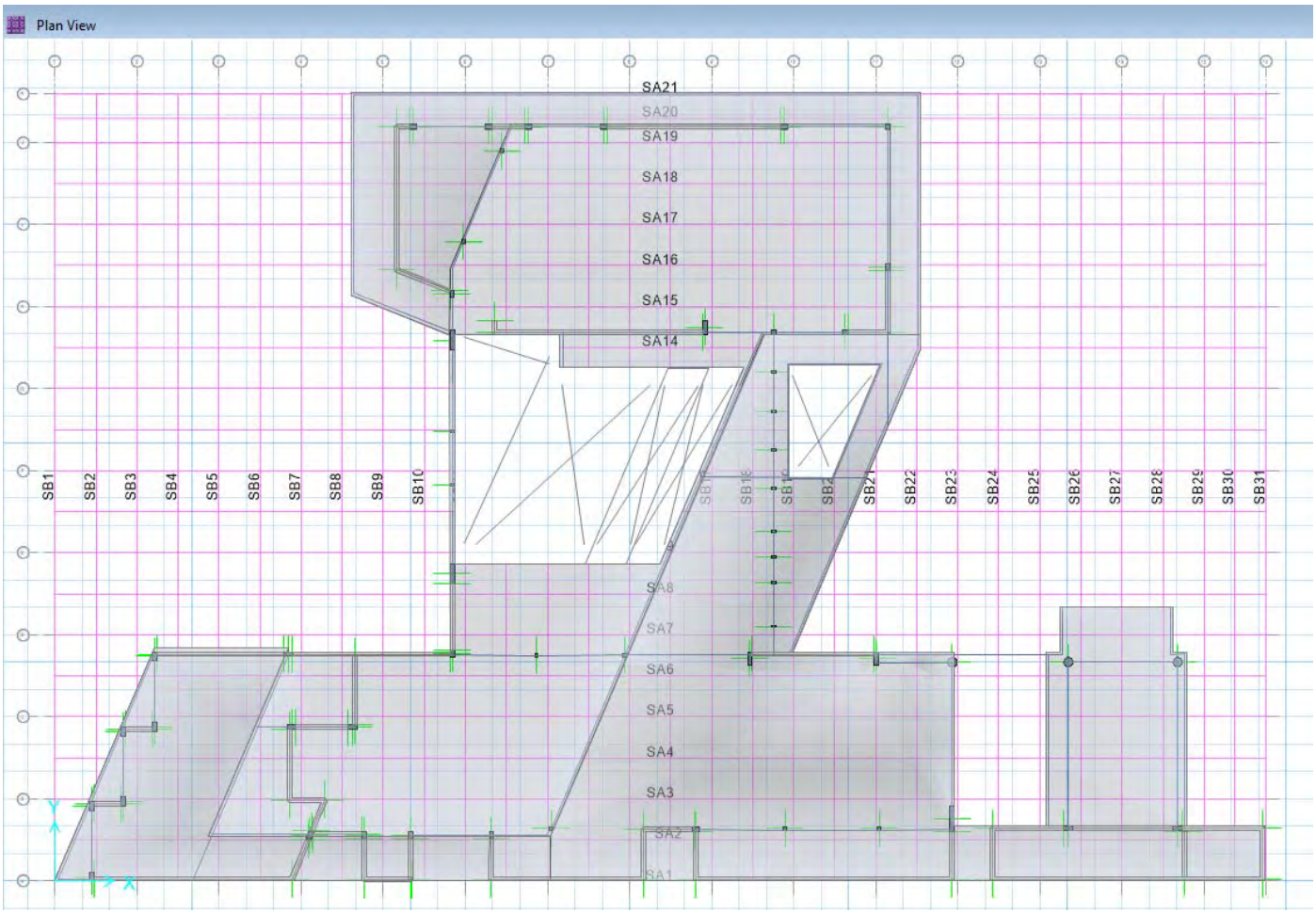


Figure 3: Finite element model

6. Design summary

This section provides design information for beams, strips, and punching checks.

6.1. Preferences

Table 34: Design Preferences 01 - Resistance Factors

Table 34: Design Preferences 01 - Resistance Factors

PhiTen	PhiComp	PhiShear
0.900000	0.650000	0.750000

Table 35: Design Preferences 02 - Rebar Cover - Slabs

Table 35: Design Preferences 02 - Rebar Cover - Slabs

CoverTop	CoverBot	BarSize	InnerLayer	PTCGSTop	PTCGSBotExt	PTCGSBotInt	SlabType
in	in			in	in	in	
0.7500	0.7500	#6	B	1.0000	1.7500	1.0000	Two Way

Table 36: Design Preferences 03 - Rebar Cover - Beams

Table 36: Design Preferences 03 - Rebar Cover - Beams

CoverTop	CoverBot	BarSizeF	BarSizeS	PTCGSTop	PTCGSBot
in	in			in	in
1.5000	1.5000	#9	#4	2.0000	2.0000

Table 37: Design Preferences 04 - Prestress Data

Table 37: Design Preferences 04 - Prestress Data

UserStress	InitConcRat	LLFraction
No	0.800000	0.500000

6.2. Overwrites

Table 38: Slab Design Overwrites 01 - Strip Based

Table 38: Slab Design Overwrites 01 - Strip Based

Strip	Layer	DesignType	RLLF	IgnorePT	RebarMat
SA1	A	Column	1.000000	No	A615Gr60
SA2	A	Middle	1.000000	No	A615Gr60
SA3	A	Column	1.000000	No	A615Gr60
SA4	A	Middle	1.000000	No	A615Gr60
SA5	A	Column	1.000000	No	A615Gr60

Table 38: Slab Design Overwrites 01 - Strip Based

Strip	Layer	DesignType	RLLF	IgnorePT	RebarMat
SA6	A	Middle	1.000000	No	A615Gr60
SA7	A	Column	1.000000	No	A615Gr60
SA8	A	Middle	1.000000	No	A615Gr60
SA9	A	Column	1.000000	No	A615Gr60
SB1	B	Column	1.000000	No	A615Gr60
SB2	B	Middle	1.000000	No	A615Gr60
SB3	B	Column	1.000000	No	A615Gr60
SB4	B	Middle	1.000000	No	A615Gr60
SB5	B	Column	1.000000	No	A615Gr60
SB6	B	Middle	1.000000	No	A615Gr60
SB7	B	Column	1.000000	No	A615Gr60
SB8	B	Middle	1.000000	No	A615Gr60
SB9	B	Column	1.000000	No	A615Gr60
SA10	A	Middle	1.000000	No	A615Gr60
SA11	A	Column	1.000000	No	A615Gr60
SA12	A	Middle	1.000000	No	A615Gr60
SA13	A	Column	1.000000	No	A615Gr60
SA14	A	Middle	1.000000	No	A615Gr60
SA15	A	Column	1.000000	No	A615Gr60
SA16	A	Middle	1.000000	No	A615Gr60
SA17	A	Column	1.000000	No	A615Gr60
SA18	A	Middle	1.000000	No	A615Gr60
SA19	A	Column	1.000000	No	A615Gr60
SA20	A	Middle	1.000000	No	A615Gr60
SA21	A	Column	1.000000	No	A615Gr60
SB10	B	Middle	1.000000	No	A615Gr60
SB11	B	Column	1.000000	No	A615Gr60
SB12	B	Middle	1.000000	No	A615Gr60
SB13	B	Column	1.000000	No	A615Gr60
SB14	B	Middle	1.000000	No	A615Gr60
SB15	B	Column	1.000000	No	A615Gr60
SB16	B	Middle	1.000000	No	A615Gr60
SB17	B	Column	1.000000	No	A615Gr60
SB18	B	Middle	1.000000	No	A615Gr60
SB19	B	Column	1.000000	No	A615Gr60
SB20	B	Middle	1.000000	No	A615Gr60
SB21	B	Column	1.000000	No	A615Gr60
SB22	B	Middle	1.000000	No	A615Gr60
SB23	B	Column	1.000000	No	A615Gr60
SB24	B	Middle	1.000000	No	A615Gr60
SB25	B	Column	1.000000	No	A615Gr60
SB26	B	Middle	1.000000	No	A615Gr60
SB27	B	Column	1.000000	No	A615Gr60
SB28	B	Middle	1.000000	No	A615Gr60
SB29	B	Column	1.000000	No	A615Gr60
SB30	B	Middle	1.000000	No	A615Gr60
SB31	B	Column	1.000000	No	A615Gr60

Table 39: Slab Design Overwrites 02 - Finite Element Based, Part 1 of 2

Table 39: Slab Design Overwrites 02 - Finite Element Based, Part 1 of 2

Area	RebarMat
4	A615Gr60
26	A615Gr60
44	A615Gr60
45	A615Gr60
46	A615Gr60
47	A615Gr60
48	A615Gr60
51	A615Gr60
52	A615Gr60

Table 39: Slab Design Overwrites 02 - Finite Element Based, Part 2 of 2

Table 39: Slab Design Overwrites 02 - Finite Element Based, Part 2 of 2

Area	RLLF	Design	IgnorePT
4	1.000000	Yes	No
26	1.000000	Yes	No
44	1.000000	Yes	No
45	1.000000	Yes	No
46	1.000000	Yes	No
47	1.000000	Yes	No
48	1.000000	Yes	No
51	1.000000	Yes	No
52	1.000000	Yes	No

Table 40: Beam Design Overwrites

Table 40: Beam Design Overwrites

Line	CoverType	RLLF	IgnorePT
4	Section	1.000000	No
5	Section	1.000000	No
6	Section	1.000000	No
7	Section	1.000000	No
11	Section	1.000000	No
14	Section	1.000000	No
19	Section	1.000000	No
20	Section	1.000000	No
32	Section	1.000000	No
33	Section	1.000000	No
45	Section	1.000000	No
46	Section	1.000000	No
63	Section	1.000000	No
64	Section	1.000000	No
65	Section	1.000000	No
66	Section	1.000000	No
67	Section	1.000000	No
68	Section	1.000000	No

Table 40: Beam Design Overwrites

Line	CoverType	RLLF	IgnorePT
69	Section	1.000000	No
70	Section	1.000000	No
71	Section	1.000000	No
72	Section	1.000000	No
73	Section	1.000000	No
74	Section	1.000000	No
75	Section	1.000000	No
76	Section	1.000000	No
77	Section	1.000000	No
78	Section	1.000000	No
79	Section	1.000000	No
80	Section	1.000000	No
81	Section	1.000000	No
82	Section	1.000000	No
83	Section	1.000000	No
84	Section	1.000000	No
85	Section	1.000000	No
86	Section	1.000000	No
87	Section	1.000000	No
88	Section	1.000000	No
89	Section	1.000000	No
90	Section	1.000000	No
91	Section	1.000000	No
92	Section	1.000000	No
93	Section	1.000000	No
94	Section	1.000000	No
95	Section	1.000000	No
96	Section	1.000000	No
97	Section	1.000000	No
98	Section	1.000000	No
99	Section	1.000000	No
100	Section	1.000000	No
101	Section	1.000000	No
102	Section	1.000000	No
103	Section	1.000000	No
104	Section	1.000000	No
105	Section	1.000000	No
106	Section	1.000000	No
107	Section	1.000000	No
108	Section	1.000000	No
112	Section	1.000000	No
113	Section	1.000000	No
114	Section	1.000000	No
115	Section	1.000000	No
116	Section	1.000000	No
117	Section	1.000000	No
118	Section	1.000000	No
119	Section	1.000000	No
120	Section	1.000000	No
121	Section	1.000000	No

Table 41: Punching Shear Design Overwrites 01 - General

Table 41: Punching Shear Design Overwrites 01 - General

Point	Check	LocType	EffDepth	ReinfType
2	Program Determined	Auto	Auto	None
4	Program Determined	Auto	Auto	None
6	Program Determined	Auto	Auto	None
16	Program Determined	Auto	Auto	None
18	Program Determined	Auto	Auto	None
20	Program Determined	Auto	Auto	None
24	Program Determined	Auto	Auto	None
26	Program Determined	Auto	Auto	None
30	Program Determined	Auto	Auto	None
32	Program Determined	Auto	Auto	None
34	Program Determined	Auto	Auto	None
36	Program Determined	Auto	Auto	None
40	Program Determined	Auto	Auto	None
42	Program Determined	Auto	Auto	None
44	Program Determined	Auto	Auto	None
46	Program Determined	Auto	Auto	None
48	Program Determined	Auto	Auto	None
50	Program Determined	Auto	Auto	None
52	Program Determined	Auto	Auto	None
54	Program Determined	Auto	Auto	None
56	Program Determined	Auto	Auto	None
58	Program Determined	Auto	Auto	None
60	Program Determined	Auto	Auto	None
66	Program Determined	Auto	Auto	None
68	Program Determined	Auto	Auto	None
70	Program Determined	Auto	Auto	None
72	Program Determined	Auto	Auto	None
74	Program Determined	Auto	Auto	None
76	Program Determined	Auto	Auto	None

Table 41: Punching Shear Design Overwrites 01 - General

Point	Check	LocType	EffDepth	ReinfType
78	Program Determined	Auto	Auto	None
80	Program Determined	Auto	Auto	None
82	Program Determined	Auto	Auto	None
84	Program Determined	Auto	Auto	None
86	Program Determined	Auto	Auto	None
92	Program Determined	Auto	Auto	None
94	Program Determined	Auto	Auto	None
96	Program Determined	Auto	Auto	None
98	Program Determined	Auto	Auto	None
100	Program Determined	Auto	Auto	None
102	Program Determined	Auto	Auto	None
104	Program Determined	Auto	Auto	None
106	Program Determined	Auto	Auto	None
108	Program Determined	Auto	Auto	None
110	Program Determined	Auto	Auto	None
112	Program Determined	Auto	Auto	None
114	Program Determined	Auto	Auto	None
116	Program Determined	Auto	Auto	None
118	Program Determined	Auto	Auto	None
120	Program Determined	Auto	Auto	None
122	Program Determined	Auto	Auto	None
309	Program Determined	Auto	Auto	None
315	Program Determined	Auto	Auto	None
317	Program Determined	Auto	Auto	None

6.3. Slab design

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA1	Span 1	Start		0.0000	0.0000
SA1	Span 1	Middle	UDCONU2	-3.8409	0.0971
SA1	Span 1	End	UDCONU2	-11.5980	0.2936
SA1	Span 2	Start	UDCONU2	-11.5980	0.2936
SA1	Span 2	Middle		0.0000	0.0000
SA1	Span 2	End	UDCONU2	-10.5780	0.2698
SA1	Span 3	Start	UDCONU2	-10.5780	0.2698
SA1	Span 3	Middle	UDCONU2	-3.0925	0.0953
SA1	Span 3	End	UDCONU2	-3.5633	0.1044
SA1	Span 4	Start	UDCONU2	-3.5633	0.1044
SA1	Span 4	Middle	UDCONU2	-2.5118	0.0630
SA1	Span 4	End	UDCONU1	-0.7563	0.0189
SA1	Span 5	Start	UDCONU1	-0.7563	0.0189
SA1	Span 5	Middle		0.0000	0.0000
SA1	Span 5	End	UDCONU2	-1.5123	0.0382
SA1	Span 6	Start	UDCONU2	-1.5123	0.0382
SA1	Span 6	Middle	UDCONU2	-3.9216	0.1070
SA1	Span 6	End	UDCONU2	-6.9290	0.1779
SA1	Span 7	Start	UDCONU2	-6.9290	0.1779
SA1	Span 7	Middle		0.0000	0.0000
SA1	Span 7	End	UDCONU2	-8.2945	0.2104
SA1	Span 8	Start	UDCONU2	-8.2945	0.2104
SA1	Span 8	Middle	UDCONU2	-1.5650	0.0412
SA1	Span 8	End	UDCONU2	-2.6429	0.0795
SA1	Span 9	Start	UDCONU2	-2.6429	0.0795
SA1	Span 9	Middle		0.0000	0.0000
SA1	Span 9	End	UDCONU2	-1.2665	0.0448
SA1	Span 10	Start	UDCONU2	-1.2665	0.0448
SA1	Span 10	Middle		0.0000	0.0000
SA1	Span 10	End	UDCONU2	-9.0029	0.2266
SA1	Span 11	Start	UDCONU2	-9.0029	0.2266
SA1	Span 11	Middle		0.0000	0.0000
SA1	Span 11	End	UDCONU2	-9.0950	0.2289
SA1	Span 12	Start	UDCONU2	-9.0950	0.2289
SA1	Span 12	Middle		0.0000	0.0000
SA1	Span 12	End	UDCONU2	-0.1848	0.0000
SA1	Span 13	Start	UDCONU2	-0.1848	0.0000
SA1	Span 13	Middle		0.0000	0.0000
SA1	Span 13	End	UDCONU2	-7.0775	0.1779
SA1	Span 14	Start	UDCONU2	-7.0775	0.1779
SA1	Span 14	Middle	UDCONU2	-1.3536	0.0339
SA1	Span 14	End	UDCONU2	-3.1567	0.0846
SA1	Span 15	Start	UDCONU2	-13.0385	0.3289
SA1	Span 15	Middle	UDCONU2	-3.8733	0.0972
SA1	Span 15	End	UDCONU2	-4.6278	0.1162
SA1	Span 16	Start	UDCONU2	-4.6278	0.1162
SA1	Span 16	Middle		0.0000	0.0000
SA1	Span 16	End	UDCONU2	-10.2297	0.2576

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA1	Span 17	Start	UDCONU2	-10.2297	0.2576
SA1	Span 17	Middle	UDCONU2	-2.5237	0.0633
SA1	Span 17	End	UDCONU2	-3.8057	0.1090
SA1	Span 18	Start	UDCONU2	-3.8057	0.1090
SA1	Span 18	Middle		0.0000	0.0000
SA1	Span 18	End	UDCONU2	-1.7840	0.0557
SA1	Span 19	Start	UDCONU2	-1.7840	0.0557
SA1	Span 19	Middle	UDCONU2	-3.5424	0.0967
SA1	Span 19	End	UDCONU2	-4.3757	0.1232
SA1	Span 20	Start	UDCONU2	-4.3757	0.1232
SA1	Span 20	Middle	UDCONU2	-1.9837	0.0608
SA1	Span 20	End	UDCONU2	-4.2171	0.1105
SA1	Span 21	Start	UDCONU2	-4.2171	0.1105
SA1	Span 21	Middle	UDCONU2	-2.2792	0.0578
SA1	Span 21	End	UDCONU2	-2.9326	0.0735
SA1	Span 22	Start	UDCONU2	-2.9326	0.0735
SA1	Span 22	Middle		0.0000	0.0000
SA1	Span 22	End	UDCONU2	-1.1292	0.0283
SA1	Span 23	Start	UDCONU2	-1.1292	0.0283
SA1	Span 23	Middle	UDCONU2	-0.2497	0.0000
SA1	Span 23	End	UDCONU2	-1.7405	0.0682
SA1	Span 24	Start	UDCONU2	-1.7405	0.0682
SA1	Span 24	Middle	UDCONU1	-0.4398	0.0282
SA1	Span 24	End	UDCONU2	-1.1546	0.0620
SA1	Span 25	Start	UDCONU2	-1.8951	0.0826
SA1	Span 25	Middle	UDCONU2	-2.3390	0.0909
SA1	Span 25	End	UDCONU2	-1.3652	0.0551
SA1	Span 26	Start	UDCONU2	-0.7077	0.0382
SA1	Span 26	Middle	UDCONU1	-2.6298	0.0808
SA1	Span 26	End	UDCONU2	-2.1665	0.0707
SA1	Span 27	Start	UDCONU2	-2.1665	0.0707
SA1	Span 27	Middle		0.0000	0.0000
SA1	Span 27	End	UDCONU2	-0.1896	0.0000
SA1	Span 28	Start	UDCONU2	-0.1896	0.0000
SA1	Span 28	Middle	UDCONU2	-0.0553	0.0000
SA1	Span 28	End	UDCONU2	-1.3783	0.0345
SA2	Cantilever Start	Start	UDCONU2	-3.8422	0.0987
SA2	Cantilever Start	Middle		0.0000	0.0000
SA2	Cantilever Start	End	UDCONU2	-11.0856	0.2821
SA2	Span 1	Start	UDCONU2	-11.0856	0.2821
SA2	Span 1	Middle	UDCONU2	-9.4007	0.2362
SA2	Span 1	End	UDCONU2	-6.4318	0.1624
SA2	Span 2	Start	UDCONU2	-6.4318	0.1624
SA2	Span 2	Middle	UDCONU1	-0.7065	0.0000
SA2	Span 2	End	UDCONU2	-2.9458	0.0738
SA2	Span 3	Start	UDCONU2	-2.9458	0.0738
SA2	Span 3	Middle		0.0000	0.0000
SA2	Span 3	End	UDCONU2	-4.7726	0.1197
SA2	Span 4	Start	UDCONU2	-4.7726	0.1197
SA2	Span 4	Middle		0.0000	0.0000
SA2	Span 4	End	UDCONU2	-2.8746	0.0720

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA2	Span 5	Start	UDCONU2	-2.8746	0.0720
SA2	Span 5	Middle	UDCONU2	-7.8587	0.1972
SA2	Span 5	End	UDCONU2	-23.2545	0.5898
SA2	Span 6	Start	UDCONU2	-23.2545	0.5898
SA2	Span 6	Middle		0.0000	0.0000
SA2	Span 6	End	UDCONU2	-22.4286	0.5721
SA2	Span 7	Start	UDCONU2	-21.9602	0.5605
SA2	Span 7	Middle		0.0000	0.0000
SA2	Span 7	End	UDCONU2	-30.0111	0.7629
SA2	Span 8	Start	UDCONU2	-30.0111	0.7629
SA2	Span 8	Middle		0.0000	0.0000
SA2	Span 8	End	UDCONU2	-27.9280	0.7106
SA2	Span 9	Start	UDCONU2	-27.9280	0.7106
SA2	Span 9	Middle		0.0000	0.0000
SA2	Span 9	End	UDCONU2	-21.4755	0.5531
SA2	Span 10	Start	UDCONU2	-21.4755	0.5531
SA2	Span 10	Middle	UDCONU2	-7.6591	0.2105
SA2	Span 10	End	UDCONU2	-11.6052	0.3102
SA2	Span 11	Start	UDCONU2	-11.6052	0.3102
SA2	Span 11	Middle		0.0000	0.0000
SA2	Span 11	End	UDCONU2	-11.4463	0.2875
SA2	Span 12	Start	UDCONU2	-11.4463	0.2875
SA2	Span 12	Middle		0.0000	0.0000
SA2	Span 12	End	UDCONU2	-7.1197	0.1788
SA2	Span 13	Start	UDCONU2	-7.1197	0.1788
SA2	Span 13	Middle		0.0000	0.0000
SA2	Span 13	End	UDCONU2	-16.9396	0.4312
SA2	Span 14	Start	UDCONU2	-16.9396	0.4312
SA2	Span 14	Middle	UDCONU2	-4.6691	0.1171
SA2	Span 14	End	UDCONU2	-0.1962	0.0000
SA2	Span 15	Start	UDCONU2	-0.1962	0.0000
SA2	Span 15	Middle		0.0000	0.0000
SA2	Span 15	End	UDCONU2	-12.6530	0.3179
SA2	Span 16	Start	UDCONU2	-12.6530	0.3179
SA2	Span 16	Middle	UDCONU2	-8.2065	0.2060
SA2	Span 16	End	UDCONU2	-33.3278	0.8423
SA2	Span 17	Start	UDCONU2	-33.3278	0.8423
SA2	Span 17	Middle		0.0000	0.0000
SA2	Span 17	End	UDCONU2	-39.4537	0.9989
SA2	Span 18	Start	UDCONU2	-39.4537	0.9989
SA2	Span 18	Middle		0.0000	0.0000
SA2	Span 18	End	UDCONU2	-51.4780	1.3079
SA2	Span 19	Start	UDCONU2	-51.4780	1.3079
SA2	Span 19	Middle	UDCONU2	-63.3816	1.6159
SA2	Span 19	End	UDCONU2	-36.4148	0.9212
SA2	Span 20	Start	UDCONU2	-36.4148	0.9212
SA2	Span 20	Middle	UDCONU1	-2.7822	0.0697
SA2	Span 20	End	UDCONU2	-14.7796	0.3716
SA2	Span 21	Start	UDCONU2	-14.7796	0.3716
SA2	Span 21	Middle		0.0000	0.0000
SA2	Span 21	End	UDCONU2	-28.9527	0.8199
SA2	Span 22	Start	UDCONU2	-28.9527	0.8199
SA2	Span 22	Middle	UDCONU2	-24.2595	0.7067

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA2	Span 22	End	UDCONU2	-43.5454	1.1194
SA2	Span 23	Start	UDCONU2	-43.5454	1.1194
SA2	Span 23	Middle		0.0000	0.0000
SA2	Span 23	End	UDCONU2	-41.8270	1.0651
SA2	Span 24	Start	UDCONU2	-41.8270	1.0651
SA2	Span 24	Middle		0.0000	0.0000
SA2	Span 24	End	UDCONU2	-11.1219	0.2813
SA2	Span 25	Start	UDCONU2	-11.1219	0.2813
SA2	Span 25	Middle	UDCONU2	-11.3710	0.2931
SA2	Span 25	End	UDCONU2	-48.8800	1.2567
SA2	Span 26	Start	UDCONU2	-48.8800	1.2567
SA2	Span 26	Middle		0.0000	0.0000
SA2	Span 26	End	UDCONU2	-32.1019	0.8263
SA2	Span 27	Start	UDCONU2	-32.1019	0.8263
SA2	Span 27	Middle	UDCONU2	-4.6999	0.1328
SA2	Span 27	End	UDCONU2	-34.2633	0.8789
SA2	Span 28	Start	UDCONU2	-34.2633	0.8789
SA2	Span 28	Middle		0.0000	0.0000
SA2	Span 28	End	UDCONU2	-39.1052	1.0070
SA2	Span 29	Start	UDCONU2	-39.1052	1.0070
SA2	Span 29	Middle	UDCONU2	-6.6310	0.2098
SA2	Span 29	End	UDCONU2	-24.8461	0.7424
SA2	Span 30	Start	UDCONU2	-24.8461	0.7424
SA2	Span 30	Middle		0.0000	0.0000
SA2	Span 30	End	UDCONU2	-14.1997	0.4408
SA2	Span 31	Start	UDCONU2	-14.1997	0.4408
SA2	Span 31	Middle	UDCONU2	-0.4983	0.0801
SA2	Span 31	End	UDCONU2	-1.4944	0.1028
SA2	Span 32	Start	UDCONU2	-1.4944	0.1028
SA2	Span 32	Middle	UDCONU1	-1.5032	0.0607
SA2	Span 32	End	UDCONU2	-8.1400	0.2123
SA2	Span 33	Start	UDCONU2	-8.1400	0.2123
SA2	Span 33	Middle		0.0000	0.0000
SA2	Span 33	End	UDCONU2	-17.2345	0.4336
SA2	Span 34	Start	UDCONU2	-17.2345	0.4336
SA2	Span 34	Middle		0.0000	0.0000
SA2	Span 34	End	UDCONU2	-16.7545	0.4215
SA2	Span 35	Start	UDCONU2	-16.7545	0.4215
SA2	Span 35	Middle	UDCONU2	-10.7058	0.2689
SA2	Span 35	End	UDCONU2	-5.8744	0.1516
SA2	Span 36	Start	UDCONU2	-5.8744	0.1516
SA2	Span 36	Middle	UDCONU2	-9.3445	0.2346
SA2	Span 36	End	UDCONU2	-19.3254	0.4865
SA2	Span 37	Start	UDCONU2	-19.3254	0.4865
SA2	Span 37	Middle		0.0000	0.0000
SA2	Span 37	End	UDCONU2	-18.1459	0.4689
SA2	Span 38	Start	UDCONU2	-18.1459	0.4689
SA2	Span 38	Middle		0.0000	0.0000
SA2	Span 38	End	UDCONU2	-15.7268	0.3955
SA2	Span 39	Start	UDCONU2	-15.7268	0.3955
SA2	Span 39	Middle		0.0000	0.0000
SA2	Span 39	End	UDCONU2	-6.1572	0.1627
SA2	Span 40	Start	UDCONU2	-6.1572	0.1627

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA2	Span 40	Middle	UDCONU1	-0.9548	0.0295
SA2	Span 40	End	UDCONU2	-1.5459	0.0423
SA3	Span 1	Start		0.0000	0.0000
SA3	Span 1	Middle		0.0000	0.0000
SA3	Span 1	End	UDCONU2	-6.2064	0.1567
SA3	Span 2	Start	UDCONU2	-6.2064	0.1567
SA3	Span 2	Middle		0.0000	0.0000
SA3	Span 2	End	UDCONU2	-7.8377	0.2034
SA3	Span 3	Start	UDCONU2	-7.8377	0.2034
SA3	Span 3	Middle		0.0000	0.0000
SA3	Span 3	End	UDCONU2	-43.8695	1.1121
SA3	Span 4	Start	UDCONU2	-43.8695	1.1121
SA3	Span 4	Middle		0.0000	0.0000
SA3	Span 4	End	UDCONU2	-45.2355	1.1472
SA3	Span 5	Start	UDCONU2	-45.2355	1.1472
SA3	Span 5	Middle		0.0000	0.0000
SA3	Span 5	End	UDCONU2	-14.4399	0.3630
SA3	Span 6	Start	UDCONU2	-14.4399	0.3630
SA3	Span 6	Middle		0.0000	0.0000
SA3	Span 6	End	UDCONU2	-1.6677	0.0418
SA3	Span 7	Start	UDCONU2	-1.6677	0.0418
SA3	Span 7	Middle	UDCONU2	-3.4897	0.0875
SA3	Span 7	End	UDCONU2	-4.4798	0.1123
SA3	Span 8	Start	UDCONU2	-4.4798	0.1123
SA3	Span 8	Middle	UDCONU2	-27.4404	0.6924
SA3	Span 8	End	UDCONU2	-48.5607	1.2327
SA3	Span 9	Start	UDCONU2	-48.5607	1.2327
SA3	Span 9	Middle		0.0000	0.0000
SA3	Span 9	End	UDCONU2	-12.2902	0.3272
SA3	Span 10	Start	UDCONU2	-12.2902	0.3272
SA3	Span 10	Middle		0.0000	0.0000
SA3	Span 10	End	UDCONU2	-39.7392	1.0107
SA3	Span 11	Start	UDCONU2	-39.7392	1.0107
SA3	Span 11	Middle	UDCONU2	-19.6931	0.4958
SA3	Span 11	End	UDCONU2	-6.8278	0.1728
SA3	Span 12	Start	UDCONU2	-6.8278	0.1728
SA3	Span 12	Middle	UDCONU1	-2.0420	0.0544
SA3	Span 12	End		0.0000	0.0000
SA3	Span 13	Start		0.0000	0.0000
SA3	Span 13	Middle	UDCONU2	-2.3382	0.0586
SA3	Span 13	End	UDCONU2	-4.2032	0.1054
SA3	Span 14	Start	UDCONU2	-4.2032	0.1054
SA3	Span 14	Middle		0.0000	0.0000
SA3	Span 14	End	UDCONU2	-7.3013	0.1832
SA3	Span 15	Start	UDCONU2	-7.3013	0.1832
SA3	Span 15	Middle	UDCONU2	-6.1513	0.1543
SA3	Span 15	End	UDCONU2	-20.6836	0.5398
SA3	Span 16	Start	UDCONU2	-20.6836	0.5398
SA3	Span 16	Middle	UDCONU2	-19.8102	0.5237
SA3	Span 16	End	UDCONU2	-12.5180	0.3435
SA3	Span 17	Start	UDCONU2	-3.7757	0.1384
SA3	Span 17	Middle	UDCONU2	-9.3004	0.2438
SA3	Span 17	End	UDCONU2	-17.9800	0.4661

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA3	Span 18	Start	UDCONU2	-17.9800	0.4661
SA3	Span 18	Middle	UDCONU2	-2.2601	0.0724
SA3	Span 18	End	UDCONU2	-13.5363	0.3579
SA3	Span 19	Start	UDCONU2	-13.5363	0.3579
SA3	Span 19	Middle	UDCONU2	-10.2062	0.2767
SA3	Span 19	End	UDCONU2	-25.8003	0.6713
SA3	Span 20	Start	UDCONU2	-25.8003	0.6713
SA3	Span 20	Middle	UDCONU2	-13.7655	0.4008
SA3	Span 20	End		0.0000	0.0000
SA3	Span 21	Start		0.0000	0.0000
SA3	Span 21	Middle		0.0000	0.0000
SA3	Span 21	End		0.0000	0.0000
SA3	Span 22	Start	UDCONU1	-4.6922	0.1305
SA3	Span 22	Middle		0.0000	0.0000
SA3	Span 22	End	UDCONU2	-2.9446	0.0738
SA3	Span 23	Start	UDCONU2	-2.9446	0.0738
SA3	Span 23	Middle	UDCONU2	-1.6147	0.0404
SA3	Span 23	End	UDCONU1	-2.0418	0.0512
SA3	Span 24	Start	UDCONU1	-2.0418	0.0512
SA3	Span 24	Middle	UDCONU2	-2.7383	0.0686
SA3	Span 24	End	UDCONU2	-3.3465	0.0839
SA3	Span 25	Start	UDCONU2	-3.3465	0.0839
SA3	Span 25	Middle	UDCONU1	-2.3126	0.0630
SA3	Span 25	End		0.0000	0.0000
SA3	Span 26	Start		0.0000	0.0000
SA3	Span 26	Middle		0.0000	0.0000
SA3	Span 26	End		0.0000	0.0000
SA4	Span 1	Start		0.0000	0.0000
SA4	Span 1	Middle		0.0000	0.0000
SA4	Span 1	End	UDCONU2	-12.4270	0.3123
SA4	Span 2	Start	UDCONU2	-12.4270	0.3123
SA4	Span 2	Middle		0.0000	0.0000
SA4	Span 2	End	UDCONU2	-13.9724	0.3514
SA4	Span 3	Start	UDCONU2	-13.9724	0.3514
SA4	Span 3	Middle	UDCONU2	-8.8503	0.2221
SA4	Span 3	End		0.0000	0.0000
SA4	Span 4	Start		0.0000	0.0000
SA4	Span 4	Middle		0.0000	0.0000
SA4	Span 4	End	UDCONU2	-1.9754	0.0495
SA4	Span 5	Start	UDCONU2	-1.9754	0.0495
SA4	Span 5	Middle	UDCONU2	-10.9272	0.2744
SA4	Span 5	End	UDCONU2	-42.4555	1.0758
SA4	Span 6	Start	UDCONU2	-19.9462	0.5065
SA4	Span 6	Middle	UDCONU2	-31.9661	0.8183
SA4	Span 6	End	UDCONU2	-31.9661	0.8183
SA4	Span 7	Start	UDCONU2	-10.9632	0.2852
SA4	Span 7	Middle		0.0000	0.0000
SA4	Span 7	End		0.0000	0.0000
SA4	Span 8	Start		0.0000	0.0000
SA4	Span 8	Middle		0.0000	0.0000
SA4	Span 8	End	UDCONU2	-0.1297	0.0000
SA4	Span 9	Start	UDCONU2	-0.1297	0.0000
SA4	Span 9	Middle	UDCONU2	-7.2638	0.1822

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA4	Span 9	End	UDCONU2	-15.3588	0.3862
SA4	Span 10	Start	UDCONU2	-15.3588	0.3862
SA4	Span 10	Middle	UDCONU2	-6.7677	0.1698
SA4	Span 10	End	UDCONU2	-0.7505	0.0000
SA4	Span 11	Start	UDCONU2	-2.1570	0.0578
SA4	Span 11	Middle	UDCONU2	-2.1570	0.0578
SA4	Span 11	End		0.0000	0.0000
SA4	Span 12	Start		0.0000	0.0000
SA4	Span 12	Middle		0.0000	0.0000
SA4	Span 12	End		0.0000	0.0000
SA4	Span 13	Start		0.0000	0.0000
SA4	Span 13	Middle		0.0000	0.0000
SA4	Span 13	End		0.0000	0.0000
SA4	Span 14	Start		0.0000	0.0000
SA4	Span 14	Middle		0.0000	0.0000
SA4	Span 14	End	UDCONU2	-2.7554	0.0715
SA4	Span 15	Start	UDCONU2	-2.7554	0.0715
SA4	Span 15	Middle	UDCONU2	-0.7613	0.0440
SA4	Span 15	End		0.0000	0.0000
SA4	Span 16	Start		0.0000	0.0000
SA4	Span 16	Middle		0.0000	0.0000
SA4	Span 16	End		0.0000	0.0000
SA4	Span 17	Start	UDCONU2	-0.9159	0.0344
SA4	Span 17	Middle		0.0000	0.0000
SA4	Span 17	End	UDCONU2	-5.8920	0.1489
SA4	Span 18	Start	UDCONU2	-5.8920	0.1489
SA4	Span 18	Middle	UDCONU2	-0.0602	0.0000
SA4	Span 18	End		0.0000	0.0000
SA4	Span 19	Start		0.0000	0.0000
SA4	Span 19	Middle		0.0000	0.0000
SA4	Span 19	End	UDCONU2	-6.0873	0.1549
SA4	Span 20	Start	UDCONU2	-6.0873	0.1549
SA4	Span 20	Middle	UDCONU2	-1.2393	0.0320
SA4	Span 20	End		0.0000	0.0000
SA4	Span 21	Start		0.0000	0.0000
SA4	Span 21	Middle		0.0000	0.0000
SA4	Span 21	End		0.0000	0.0000
SA5	Span 1	Start		0.0000	0.0000
SA5	Span 1	Middle		0.0000	0.0000
SA5	Span 1	End	UDCONU2	-1.3278	0.0364
SA5	Span 2	Start	UDCONU2	-1.3278	0.0364
SA5	Span 2	Middle	UDCONU2	-1.9762	0.0547
SA5	Span 2	End	UDCONU2	-2.9487	0.0802
SA5	Span 3	Start	UDCONU2	-2.9487	0.0802
SA5	Span 3	Middle		0.0000	0.0000
SA5	Span 3	End	UDCONU2	-6.4808	0.1743
SA5	Span 4	Start	UDCONU2	-6.4808	0.1743
SA5	Span 4	Middle		0.0000	0.0000
SA5	Span 4	End	UDCONU2	-37.4216	0.9469
SA5	Span 5	Start	UDCONU2	-37.4216	0.9469
SA5	Span 5	Middle		0.0000	0.0000
SA5	Span 5	End	UDCONU2	-38.4570	0.9734
SA5	Span 6	Start	UDCONU2	-38.4570	0.9734

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA5	Span 6	Middle	UDCONU2	-6.8356	0.1715
SA5	Span 6	End		0.0000	0.0000
SA5	Span 7	Start		0.0000	0.0000
SA5	Span 7	Middle	UDCONU2	-0.4877	0.0000
SA5	Span 7	End	UDCONU2	-12.6593	0.3181
SA5	Span 8	Start	UDCONU2	-12.6593	0.3181
SA5	Span 8	Middle	UDCONU2	-10.6334	0.2670
SA5	Span 8	End	UDCONU2	-13.1209	0.3297
SA5	Span 9	Start	UDCONU2	-13.1209	0.3297
SA5	Span 9	Middle	UDCONU2	-11.2142	0.2817
SA5	Span 9	End	UDCONU2	-12.7530	0.3205
SA5	Span 10	Start	UDCONU2	-12.7530	0.3205
SA5	Span 10	Middle		0.0000	0.0000
SA5	Span 10	End	UDCONU2	-11.6216	0.2978
SA5	Span 11	Start	UDCONU2	-11.6216	0.2978
SA5	Span 11	Middle		0.0000	0.0000
SA5	Span 11	End	UDCONU2	-11.3352	0.2920
SA5	Span 12	Start	UDCONU2	-11.3352	0.2920
SA5	Span 12	Middle		0.0000	0.0000
SA5	Span 12	End	UDCONU2	-9.0229	0.2341
SA5	Span 13	Start	UDCONU2	-9.0229	0.2341
SA5	Span 13	Middle	UDCONU2	-7.0330	0.1897
SA5	Span 13	End	UDCONU2	-34.8453	0.8881
SA5	Span 14	Start	UDCONU2	-34.8453	0.8881
SA5	Span 14	Middle		0.0000	0.0000
SA5	Span 14	End	UDCONU2	-101.8633	2.6448
SA5	Span 15	Start	UDCONU2	-101.8633	2.6448
SA5	Span 15	Middle		0.0000	0.0000
SA5	Span 15	End	UDCONU2	-94.6412	2.4466
SA5	Span 16	Start	UDCONU2	-94.6412	2.4466
SA5	Span 16	Middle	UDCONU2	-47.7428	1.2359
SA5	Span 16	End	UDCONU2	-18.4656	0.4821
SA5	Span 17	Start	UDCONU2	-18.4656	0.4821
SA5	Span 17	Middle	UDCONU1	-0.7284	0.0000
SA5	Span 17	End	UDCONU2	-0.0560	0.0000
SA5	Span 18	Start	UDCONU2	-0.0560	0.0000
SA5	Span 18	Middle		0.0000	0.0000
SA5	Span 18	End	UDCONU2	-6.4493	0.1618
SA5	Span 19	Start	UDCONU2	-6.4493	0.1618
SA5	Span 19	Middle	UDCONU2	-14.5276	0.3652
SA5	Span 19	End	UDCONU2	-23.6407	0.5959
SA5	Span 20	Start	UDCONU2	-25.0041	0.6305
SA5	Span 20	Middle	UDCONU2	-18.9153	0.4761
SA5	Span 20	End	UDCONU2	-4.1350	0.1037
SA5	Span 21	Start	UDCONU2	-4.1350	0.1037
SA5	Span 21	Middle	UDCONU2	-0.1504	0.0000
SA5	Span 21	End		0.0000	0.0000
SA5	Span 22	Start		0.0000	0.0000
SA5	Span 22	Middle	UDCONU2	-3.4470	0.1047
SA5	Span 22	End	UDCONU2	-0.0567	0.0000
SA5	Span 23	Start	UDCONU2	-0.0567	0.0000
SA5	Span 23	Middle	UDCONU2	-0.5056	0.0417
SA5	Span 23	End	UDCONU2	-1.9031	0.0684

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA5	Span 24	Start	UDCONU2	-1.9031	0.0684
SA5	Span 24	Middle	UDCONU2	-0.2633	0.0000
SA5	Span 24	End	UDCONU2	-2.3311	0.0584
SA5	Span 25	Start	UDCONU2	-2.3311	0.0584
SA5	Span 25	Middle	UDCONU2	-1.9732	0.0627
SA5	Span 25	End		0.0000	0.0000
SA5	Span 26	Start		0.0000	0.0000
SA5	Span 26	Middle		0.0000	0.0000
SA5	Span 26	End		0.0000	0.0000
SA5	Span 27	Start	UDCONU2	-1.8233	0.0535
SA5	Span 27	Middle		0.0000	0.0000
SA5	Span 27	End	UDCONU2	-6.6782	0.1683
SA5	Span 28	Start	UDCONU2	-6.6782	0.1683
SA5	Span 28	Middle	UDCONU2	-0.1207	0.0000
SA5	Span 28	End		0.0000	0.0000
SA5	Span 29	Start		0.0000	0.0000
SA5	Span 29	Middle		0.0000	0.0000
SA5	Span 29	End	UDCONU2	-2.7127	0.0694
SA5	Span 30	Start	UDCONU2	-2.7127	0.0694
SA5	Span 30	Middle	UDCONU1	-0.9660	0.0000
SA5	Span 30	End		0.0000	0.0000
SA5	Span 31	Start		0.0000	0.0000
SA5	Span 31	Middle		0.0000	0.0000
SA5	Span 31	End		0.0000	0.0000
SA6	Span 1	Start		0.0000	0.0000
SA6	Span 1	Middle		0.0000	0.0000
SA6	Span 1	End	UDCONU2	-0.6979	0.0175
SA6	Span 2	Start	UDCONU2	-0.6979	0.0175
SA6	Span 2	Middle	UDCONU2	-1.2322	0.0320
SA6	Span 2	End	UDCONU2	-7.8962	0.2006
SA6	Span 3	Start	UDCONU2	-7.8962	0.2006
SA6	Span 3	Middle	UDCONU2	-3.4290	0.0859
SA6	Span 3	End	UDCONU2	-1.0167	0.0000
SA6	Span 4	Start	UDCONU2	-1.0167	0.0000
SA6	Span 4	Middle	UDCONU2	-4.5801	0.1148
SA6	Span 4	End	UDCONU2	-5.7488	0.1442
SA6	Span 5	Start	UDCONU2	-5.7488	0.1442
SA6	Span 5	Middle		0.0000	0.0000
SA6	Span 5	End	UDCONU2	-1.8146	0.0544
SA6	Span 6	Start	UDCONU2	-1.8146	0.0544
SA6	Span 6	Middle	UDCONU2	-3.8967	0.1123
SA6	Span 6	End	UDCONU2	-26.8686	0.7078
SA6	Span 7	Start	UDCONU2	-26.8686	0.7078
SA6	Span 7	Middle	UDCONU2	-23.4432	0.6297
SA6	Span 7	End	UDCONU2	-19.0347	0.5157
SA6	Span 8	Start	UDCONU2	-19.0347	0.5157
SA6	Span 8	Middle	UDCONU2	-9.4871	0.2532
SA6	Span 8	End	UDCONU2	-8.4032	0.2274
SA6	Span 9	Start	UDCONU2	-8.4032	0.2274
SA6	Span 9	Middle	UDCONU2	-13.8608	0.3484
SA6	Span 9	End	UDCONU2	-39.9673	1.0121
SA6	Span 10	Start	UDCONU2	-39.9673	1.0121
SA6	Span 10	Middle		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA6	Span 10	End	UDCONU2	-30.4679	0.7694
SA6	Span 11	Start	UDCONU2	-30.4679	0.7694
SA6	Span 11	Middle	UDCONU2	-21.4078	0.5392
SA6	Span 11	End	UDCONU2	-36.1122	0.9134
SA6	Span 12	Start	UDCONU2	-36.1122	0.9134
SA6	Span 12	Middle		0.0000	0.0000
SA6	Span 12	End	UDCONU2	-10.6558	0.2676
SA6	Span 13	Start	UDCONU2	-12.9301	0.3249
SA6	Span 13	Middle	UDCONU2	-12.9301	0.3249
SA6	Span 13	End	UDCONU2	-1.2765	0.0351
SA6	Span 14	Start	UDCONU2	-1.2765	0.0351
SA6	Span 14	Middle	UDCONU2	-13.0134	0.3328
SA6	Span 14	End	UDCONU2	-91.0161	2.3663
SA6	Span 15	Start	UDCONU2	-91.0161	2.3663
SA6	Span 15	Middle	UDCONU2	-26.5646	0.6907
SA6	Span 15	End	UDCONU2	-17.5932	0.4783
SA6	Span 16	Start	UDCONU2	-17.5932	0.4783
SA6	Span 16	Middle	UDCONU2	-17.8590	0.5172
SA6	Span 16	End	UDCONU2	-60.8850	1.6337
SA6	Span 17	Start	UDCONU2	-60.8850	1.6337
SA6	Span 17	Middle		0.0000	0.0000
SA6	Span 17	End	UDCONU2	-60.3668	1.6157
SA6	Span 18	Start	UDCONU2	-60.3668	1.6157
SA6	Span 18	Middle	UDCONU2	-7.0272	0.2012
SA6	Span 18	End	UDCONU2	-19.7455	0.5105
SA6	Span 19	Start	UDCONU2	-19.7455	0.5105
SA6	Span 19	Middle	UDCONU2	-13.7248	0.3551
SA6	Span 19	End		0.0000	0.0000
SA6	Span 20	Start		0.0000	0.0000
SA6	Span 20	Middle		0.0000	0.0000
SA6	Span 20	End		0.0000	0.0000
SA6	Span 21	Start	UDCONU2	-6.6085	0.1730
SA6	Span 21	Middle		0.0000	0.0000
SA6	Span 21	End	UDCONU2	-28.8811	0.7399
SA6	Span 22	Start	UDCONU2	-28.8811	0.7399
SA6	Span 22	Middle	UDCONU2	-0.4436	0.0000
SA6	Span 22	End		0.0000	0.0000
SA6	Span 23	Start		0.0000	0.0000
SA6	Span 23	Middle	UDCONU2	-4.0764	0.1038
SA6	Span 23	End	UDCONU2	-21.8608	0.5516
SA6	Span 24	Start	UDCONU2	-21.8608	0.5516
SA6	Span 24	Middle	UDCONU2	-5.5545	0.1424
SA6	Span 24	End		0.0000	0.0000
SA6	Span 25	Start		0.0000	0.0000
SA6	Span 25	Middle		0.0000	0.0000
SA6	Span 25	End		0.0000	0.0000
SA7	Span 1	Start		0.0000	0.0000
SA7	Span 1	Middle		0.0000	0.0000
SA7	Span 1	End		0.0000	0.0000
SA7	Span 2	Start		0.0000	0.0000
SA7	Span 2	Middle		0.0000	0.0000
SA7	Span 2	End	UDCONU2	-1.0728	0.0269
SA7	Span 3	Start	UDCONU2	-1.0728	0.0269

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA7	Span 3	Middle	UDCONU2	-1.0858	0.0272
SA7	Span 3	End	UDCONU2	-0.8353	0.0209
SA7	Span 4	Start	UDCONU2	-0.8353	0.0209
SA7	Span 4	Middle	UDCONU2	-1.1111	0.0279
SA7	Span 4	End	UDCONU2	-1.0602	0.0266
SA7	Span 5	Start	UDCONU2	-1.0602	0.0266
SA7	Span 5	Middle		0.0000	0.0000
SA7	Span 5	End	UDCONU2	-1.0435	0.0262
SA7	Span 6	Start	UDCONU2	-1.0435	0.0262
SA7	Span 6	Middle		0.0000	0.0000
SA7	Span 6	End	UDCONU2	-0.9972	0.0251
SA7	Span 7	Start	UDCONU2	-0.9972	0.0251
SA7	Span 7	Middle		0.0000	0.0000
SA7	Span 7	End	UDCONU2	-0.4021	0.0111
SA7	Span 8	Start	UDCONU2	-0.3710	0.0114
SA7	Span 8	Middle	UDCONU2	-1.0804	0.0299
SA7	Span 8	End	UDCONU2	-1.4421	0.0405
SA7	Span 9	Start	UDCONU2	-1.4421	0.0405
SA7	Span 9	Middle	UDCONU2	-0.9573	0.0269
SA7	Span 9	End	UDCONU2	-23.9553	0.6241
SA7	Span 10	Start	UDCONU2	-23.9553	0.6241
SA7	Span 10	Middle		0.0000	0.0000
SA7	Span 10	End	UDCONU2	-26.0384	0.6580
SA7	Span 11	Start	UDCONU2	-26.0384	0.6580
SA7	Span 11	Middle		0.0000	0.0000
SA7	Span 11	End	UDCONU2	-23.5454	0.5987
SA7	Span 12	Start	UDCONU2	-23.5454	0.5987
SA7	Span 12	Middle	UDCONU2	-38.3711	0.9712
SA7	Span 12	End	UDCONU2	-19.5259	0.4918
SA7	Span 13	Start	UDCONU2	-19.5259	0.4918
SA7	Span 13	Middle	UDCONU2	-8.7279	0.2191
SA7	Span 13	End	UDCONU2	-40.0688	1.0147
SA7	Span 14	Start	UDCONU2	-40.0688	1.0147
SA7	Span 14	Middle		0.0000	0.0000
SA7	Span 14	End	UDCONU2	-32.2552	0.8150
SA7	Span 15	Start	UDCONU2	-32.2552	0.8150
SA7	Span 15	Middle		0.0000	0.0000
SA7	Span 15	End	UDCONU2	-31.4373	0.7941
SA7	Span 16	Start	UDCONU2	-35.8028	0.9055
SA7	Span 16	Middle	UDCONU2	-21.3611	0.5381
SA7	Span 16	End	UDCONU2	-1.4507	0.0363
SA7	Span 17	Start	UDCONU2	-1.4507	0.0363
SA7	Span 17	Middle	UDCONU2	-11.5002	0.2889
SA7	Span 17	End	UDCONU2	-62.8503	1.6021
SA7	Span 18	Start	UDCONU2	-62.8503	1.6021
SA7	Span 18	Middle		0.0000	0.0000
SA7	Span 18	End	UDCONU2	-73.5915	1.8818
SA7	Span 19	Start	UDCONU2	-73.5915	1.8818
SA7	Span 19	Middle		0.0000	0.0000
SA7	Span 19	End	UDCONU2	-31.4412	0.8057
SA7	Span 20	Start	UDCONU2	-31.4412	0.8057
SA7	Span 20	Middle		0.0000	0.0000
SA7	Span 20	End	UDCONU2	-28.2479	0.7197

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA7	Span 21	Start	UDCONU2	-28.2479	0.7197
SA7	Span 21	Middle	UDCONU2	-2.4469	0.0677
SA7	Span 21	End	UDCONU2	-2.8745	0.0747
SA7	Span 22	Start	UDCONU2	-2.8745	0.0747
SA7	Span 22	Middle		0.0000	0.0000
SA7	Span 22	End	UDCONU2	-0.2722	0.0068
SA7	Span 23	Start	UDCONU2	-0.2722	0.0068
SA7	Span 23	Middle	UDCONU2	-1.0290	0.0310
SA7	Span 23	End	UDCONU2	-2.9956	0.0833
SA7	Span 24	Start	UDCONU2	-2.9956	0.0833
SA7	Span 24	Middle		0.0000	0.0000
SA7	Span 24	End		0.0000	0.0000
SA7	Span 25	Start		0.0000	0.0000
SA7	Span 25	Middle		0.0000	0.0000
SA7	Span 25	End		0.0000	0.0000
SA7	Span 26	Start		0.0000	0.0000
SA7	Span 26	Middle		0.0000	0.0000
SA7	Span 26	End	UDCONU2	-2.6552	0.0698
SA7	Span 27	Start	UDCONU2	-10.7362	0.2743
SA7	Span 27	Middle	UDCONU2	-5.2792	0.1344
SA7	Span 27	End		0.0000	0.0000
SA7	Span 28	Start		0.0000	0.0000
SA7	Span 28	Middle	UDCONU2	-7.8884	0.1980
SA7	Span 28	End		0.0000	0.0000
SA7	Span 29	Start		0.0000	0.0000
SA7	Span 29	Middle		0.0000	0.0000
SA7	Span 29	End		0.0000	0.0000
SA8	Span 1	Start		0.0000	0.0000
SA8	Span 1	Middle		0.0000	0.0000
SA8	Span 1	End		0.0000	0.0000
SA8	Span 2	Start		0.0000	0.0000
SA8	Span 2	Middle		0.0000	0.0000
SA8	Span 2	End		0.0000	0.0000
SA8	Span 3	Start		0.0000	0.0000
SA8	Span 3	Middle		0.0000	0.0000
SA8	Span 3	End		0.0000	0.0000
SA8	Span 4	Start		0.0000	0.0000
SA8	Span 4	Middle		0.0000	0.0000
SA8	Span 4	End		0.0000	0.0000
SA8	Span 5	Start		0.0000	0.0000
SA8	Span 5	Middle		0.0000	0.0000
SA8	Span 5	End	UDCONU2	-61.7403	1.5733
SA8	Span 6	Start	UDCONU2	-61.7403	1.5733
SA8	Span 6	Middle		0.0000	0.0000
SA8	Span 6	End	UDCONU2	-57.7245	1.4700
SA8	Span 7	Start	UDCONU2	-57.7245	1.4700
SA8	Span 7	Middle		0.0000	0.0000
SA8	Span 7	End	UDCONU2	-42.1495	1.0680
SA8	Span 8	Start	UDCONU2	-42.1495	1.0680
SA8	Span 8	Middle	UDCONU2	-33.2351	0.8444
SA8	Span 8	End	UDCONU2	-7.5706	0.1900
SA8	Span 9	Start	UDCONU2	-7.5706	0.1900
SA8	Span 9	Middle		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA8	Span 9	End	UDCONU2	-12.5966	0.3165
SA8	Span 10	Start	UDCONU2	-12.5966	0.3165
SA8	Span 10	Middle		0.0000	0.0000
SA8	Span 10	End	UDCONU2	-41.6332	1.0579
SA8	Span 11	Start	UDCONU2	-49.3633	1.2560
SA8	Span 11	Middle	UDCONU2	-14.5766	0.3665
SA8	Span 11	End	UDCONU2	-6.6261	0.1662
SA8	Span 12	Start	UDCONU2	-6.6261	0.1662
SA8	Span 12	Middle	UDCONU2	-48.0418	1.2193
SA8	Span 12	End	UDCONU2	-61.6464	1.5708
SA8	Span 13	Start	UDCONU2	-61.6464	1.5708
SA8	Span 13	Middle		0.0000	0.0000
SA8	Span 13	End	UDCONU2	-46.5936	1.1825
SA8	Span 14	Start	UDCONU2	-46.5936	1.1825
SA8	Span 14	Middle	UDCONU2	-20.6778	0.5246
SA8	Span 14	End		0.0000	0.0000
SA8	Span 15	Start		0.0000	0.0000
SA8	Span 15	Middle		0.0000	0.0000
SA8	Span 15	End		0.0000	0.0000
SA8	Span 16	Start		0.0000	0.0000
SA8	Span 16	Middle		0.0000	0.0000
SA8	Span 16	End		0.0000	0.0000
SA8	Span 17	Start	UDCONU2	-0.3464	0.0087
SA8	Span 17	Middle	UDCONU2	-0.3464	0.0087
SA8	Span 17	End		0.0000	0.0000
SA8	Span 18	Start		0.0000	0.0000
SA8	Span 18	Middle		0.0000	0.0000
SA8	Span 18	End		0.0000	0.0000
SA8	Span 19	Start		0.0000	0.0000
SA8	Span 19	Middle		0.0000	0.0000
SA8	Span 19	End		0.0000	0.0000
SA9	Span 1	Start		0.0000	0.0000
SA9	Span 1	Middle		0.0000	0.0000
SA9	Span 1	End		0.0000	0.0000
SA9	Span 2	Start		0.0000	0.0000
SA9	Span 2	Middle		0.0000	0.0000
SA9	Span 2	End		0.0000	0.0000
SA9	Span 3	Start		0.0000	0.0000
SA9	Span 3	Middle		0.0000	0.0000
SA9	Span 3	End		0.0000	0.0000
SA9	Span 4	Start		0.0000	0.0000
SA9	Span 4	Middle		0.0000	0.0000
SA9	Span 4	End		0.0000	0.0000
SA9	Span 5	Start		0.0000	0.0000
SA9	Span 5	Middle		0.0000	0.0000
SA9	Span 5	End	UDCONU2	-40.3832	1.0842
SA9	Span 6	Start	UDCONU2	-40.3832	1.0842
SA9	Span 6	Middle		0.0000	0.0000
SA9	Span 6	End	UDCONU2	-16.4408	0.4204
SA9	Span 7	Start	UDCONU2	-16.4408	0.4204
SA9	Span 7	Middle	UDCONU2	-6.5598	0.1656
SA9	Span 7	End	UDCONU2	-0.7063	0.0177
SA9	Span 8	Start	UDCONU2	-0.7063	0.0177

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA9	Span 8	Middle	UDCONU2	-0.0070	0.0000
SA9	Span 8	End	UDCONU2	-2.7537	0.0726
SA9	Span 9	Start	UDCONU2	-3.2869	0.0860
SA9	Span 9	Middle	UDCONU2	-5.8305	0.1498
SA9	Span 9	End	UDCONU2	-14.5446	0.3684
SA9	Span 10	Start	UDCONU2	-18.6698	0.4705
SA9	Span 10	Middle	UDCONU2	-12.1225	0.3046
SA9	Span 10	End	UDCONU2	-5.3916	0.1352
SA9	Span 11	Start	UDCONU2	-5.3916	0.1352
SA9	Span 11	Middle	UDCONU2	-61.3998	1.5644
SA9	Span 11	End	UDCONU2	-106.4446	2.7507
SA9	Span 12	Start	UDCONU2	-106.4446	2.7507
SA9	Span 12	Middle		0.0000	0.0000
SA9	Span 12	End	UDCONU2	-67.8926	1.7346
SA9	Span 13	Start	UDCONU2	-67.8926	1.7346
SA9	Span 13	Middle	UDCONU2	-17.6329	0.4472
SA9	Span 13	End		0.0000	0.0000
SA9	Span 14	Start		0.0000	0.0000
SA9	Span 14	Middle		0.0000	0.0000
SA9	Span 14	End		0.0000	0.0000
SA9	Span 15	Start		0.0000	0.0000
SA9	Span 15	Middle		0.0000	0.0000
SA9	Span 15	End		0.0000	0.0000
SA9	Span 16	Start		0.0000	0.0000
SA9	Span 16	Middle		0.0000	0.0000
SA9	Span 16	End		0.0000	0.0000
SA9	Span 17	Start		0.0000	0.0000
SA9	Span 17	Middle		0.0000	0.0000
SA9	Span 17	End		0.0000	0.0000
SA9	Span 18	Start		0.0000	0.0000
SA9	Span 18	Middle		0.0000	0.0000
SA9	Span 18	End		0.0000	0.0000
SB1	Span 1	Start	UDCONU2	-0.9758	0.0274
SB1	Span 1	Middle		0.0000	0.0000
SB1	Span 1	End		0.0000	0.0000
SB1	Span 2	Start		0.0000	0.0000
SB1	Span 2	Middle		0.0000	0.0000
SB1	Span 2	End		0.0000	0.0000
SB1	Span 3	Start		0.0000	0.0000
SB1	Span 3	Middle		0.0000	0.0000
SB1	Span 3	End		0.0000	0.0000
SB1	Span 4	Start		0.0000	0.0000
SB1	Span 4	Middle		0.0000	0.0000
SB1	Span 4	End		0.0000	0.0000
SB1	Span 5	Start		0.0000	0.0000
SB1	Span 5	Middle		0.0000	0.0000
SB1	Span 5	End		0.0000	0.0000
SB1	Span 6	Start		0.0000	0.0000
SB1	Span 6	Middle		0.0000	0.0000
SB1	Span 6	End		0.0000	0.0000
SB1	Span 7	Start		0.0000	0.0000
SB1	Span 7	Middle		0.0000	0.0000
SB1	Span 7	End		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB1	Span 8	Start		0.0000	0.0000
SB1	Span 8	Middle		0.0000	0.0000
SB1	Span 8	End		0.0000	0.0000
SB1	Span 9	Start		0.0000	0.0000
SB1	Span 9	Middle		0.0000	0.0000
SB1	Span 9	End		0.0000	0.0000
SB1	Span 10	Start		0.0000	0.0000
SB1	Span 10	Middle		0.0000	0.0000
SB1	Span 10	End		0.0000	0.0000
SB2	Span 1	Start	UDCONU2	-4.6723	0.1442
SB2	Span 1	Middle		0.0000	0.0000
SB2	Span 1	End	UDCONU2	-4.8553	0.1406
SB2	Span 2	Start	UDCONU2	-4.8553	0.1406
SB2	Span 2	Middle		0.0000	0.0000
SB2	Span 2	End	UDCONU2	-6.2951	0.1741
SB2	Span 3	Start	UDCONU2	-6.2951	0.1741
SB2	Span 3	Middle		0.0000	0.0000
SB2	Span 3	End	UDCONU2	-11.1057	0.3072
SB2	Span 4	Start	UDCONU2	-11.1057	0.3072
SB2	Span 4	Middle		0.0000	0.0000
SB2	Span 4	End	UDCONU2	-10.5928	0.2922
SB2	Span 5	Start	UDCONU2	-10.5928	0.2922
SB2	Span 5	Middle		0.0000	0.0000
SB2	Span 5	End	UDCONU2	-9.9456	0.2737
SB2	Span 6	Start	UDCONU2	-9.9456	0.2737
SB2	Span 6	Middle	UDCONU2	-0.0860	0.0024
SB2	Span 6	End		0.0000	0.0000
SB2	Span 7	Start		0.0000	0.0000
SB2	Span 7	Middle		0.0000	0.0000
SB2	Span 7	End		0.0000	0.0000
SB2	Span 8	Start		0.0000	0.0000
SB2	Span 8	Middle		0.0000	0.0000
SB2	Span 8	End		0.0000	0.0000
SB2	Span 9	Start		0.0000	0.0000
SB2	Span 9	Middle		0.0000	0.0000
SB2	Span 9	End		0.0000	0.0000
SB2	Span 10	Start		0.0000	0.0000
SB2	Span 10	Middle		0.0000	0.0000
SB2	Span 10	End		0.0000	0.0000
SB2	Span 11	Start		0.0000	0.0000
SB2	Span 11	Middle		0.0000	0.0000
SB2	Span 11	End		0.0000	0.0000
SB2	Span 12	Start		0.0000	0.0000
SB2	Span 12	Middle		0.0000	0.0000
SB2	Span 12	End		0.0000	0.0000
SB2	Span 13	Start		0.0000	0.0000
SB2	Span 13	Middle		0.0000	0.0000
SB2	Span 13	End		0.0000	0.0000
SB2	Span 14	Start		0.0000	0.0000
SB2	Span 14	Middle		0.0000	0.0000
SB2	Span 14	End		0.0000	0.0000
SB3	Span 1	Start	UDCONU2	-8.0853	0.2319
SB3	Span 1	Middle	UDCONU2	-0.9250	0.0381

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB3	Span 1	End	UDCONU2	-26.0549	0.7345
SB3	Span 2	Start	UDCONU2	-26.0549	0.7345
SB3	Span 2	Middle		0.0000	0.0000
SB3	Span 2	End	UDCONU2	-28.0155	0.7901
SB3	Span 3	Start	UDCONU2	-28.0155	0.7901
SB3	Span 3	Middle		0.0000	0.0000
SB3	Span 3	End	UDCONU2	-10.5822	0.2943
SB3	Span 4	Start	UDCONU2	-10.5822	0.2943
SB3	Span 4	Middle	UDCONU2	-2.7450	0.0751
SB3	Span 4	End	UDCONU2	-23.0210	0.6497
SB3	Span 5	Start	UDCONU2	-23.0210	0.6497
SB3	Span 5	Middle		0.0000	0.0000
SB3	Span 5	End	UDCONU2	-26.4636	0.7402
SB3	Span 6	Start	UDCONU2	-26.4636	0.7402
SB3	Span 6	Middle		0.0000	0.0000
SB3	Span 6	End	UDCONU2	-25.7790	0.7223
SB3	Span 7	Start	UDCONU2	-25.7790	0.7223
SB3	Span 7	Middle		0.0000	0.0000
SB3	Span 7	End	UDCONU2	-10.4000	0.2904
SB3	Span 8	Start	UDCONU2	-10.4000	0.2904
SB3	Span 8	Middle	UDCONU2	-0.6659	0.0182
SB3	Span 8	End	UDCONU2	-1.0097	0.0283
SB3	Span 9	Start	UDCONU2	-1.0097	0.0283
SB3	Span 9	Middle		0.0000	0.0000
SB3	Span 9	End	UDCONU2	-0.5267	0.0149
SB3	Span 10	Start	UDCONU2	-0.5267	0.0149
SB3	Span 10	Middle		0.0000	0.0000
SB3	Span 10	End		0.0000	0.0000
SB3	Span 11	Start		0.0000	0.0000
SB3	Span 11	Middle		0.0000	0.0000
SB3	Span 11	End		0.0000	0.0000
SB3	Span 12	Start		0.0000	0.0000
SB3	Span 12	Middle		0.0000	0.0000
SB3	Span 12	End		0.0000	0.0000
SB3	Span 13	Start		0.0000	0.0000
SB3	Span 13	Middle		0.0000	0.0000
SB3	Span 13	End		0.0000	0.0000
SB3	Span 14	Start		0.0000	0.0000
SB3	Span 14	Middle		0.0000	0.0000
SB3	Span 14	End		0.0000	0.0000
SB3	Span 15	Start		0.0000	0.0000
SB3	Span 15	Middle		0.0000	0.0000
SB3	Span 15	End		0.0000	0.0000
SB3	Span 16	Start		0.0000	0.0000
SB3	Span 16	Middle		0.0000	0.0000
SB3	Span 16	End		0.0000	0.0000
SB3	Span 17	Start		0.0000	0.0000
SB3	Span 17	Middle		0.0000	0.0000
SB3	Span 17	End		0.0000	0.0000
SB4	Span 1	Start	UDCONU1	-3.1379	0.0885
SB4	Span 1	Middle		0.0000	0.0000
SB4	Span 1	End	UDCONU2	-7.5073	0.2058
SB4	Span 2	Start	UDCONU2	-7.5073	0.2058

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB4	Span 2	Middle		0.0000	0.0000
SB4	Span 2	End		0.0000	0.0000
SB4	Span 3	Start		0.0000	0.0000
SB4	Span 3	Middle	UDCONU2	-7.0683	0.2028
SB4	Span 3	End	UDCONU2	-10.2669	0.2866
SB4	Span 4	Start	UDCONU2	-10.2669	0.2866
SB4	Span 4	Middle	UDCONU2	-1.6695	0.0470
SB4	Span 4	End	UDCONU2	-7.8305	0.2244
SB4	Span 5	Start		0.0000	0.0000
SB4	Span 5	Middle		0.0000	0.0000
SB4	Span 5	End		0.0000	0.0000
SB4	Span 6	Start		0.0000	0.0000
SB4	Span 6	Middle		0.0000	0.0000
SB4	Span 6	End		0.0000	0.0000
SB4	Span 7	Start		0.0000	0.0000
SB4	Span 7	Middle		0.0000	0.0000
SB4	Span 7	End		0.0000	0.0000
SB4	Span 8	Start		0.0000	0.0000
SB4	Span 8	Middle		0.0000	0.0000
SB4	Span 8	End		0.0000	0.0000
SB4	Span 9	Start		0.0000	0.0000
SB4	Span 9	Middle		0.0000	0.0000
SB4	Span 9	End		0.0000	0.0000
SB4	Span 10	Start		0.0000	0.0000
SB4	Span 10	Middle		0.0000	0.0000
SB4	Span 10	End		0.0000	0.0000
SB4	Span 11	Start		0.0000	0.0000
SB4	Span 11	Middle		0.0000	0.0000
SB4	Span 11	End		0.0000	0.0000
SB5	Span 1	Start	UDCONU2	-6.4237	0.1877
SB5	Span 1	Middle	UDCONU2	-0.8826	0.0351
SB5	Span 1	End	UDCONU2	-10.8230	0.3049
SB5	Span 2	Start	UDCONU2	-10.8230	0.3049
SB5	Span 2	Middle		0.0000	0.0000
SB5	Span 2	End	UDCONU2	-1.7228	0.0553
SB5	Span 3	Start	UDCONU2	-1.7228	0.0553
SB5	Span 3	Middle	UDCONU2	-1.7309	0.0552
SB5	Span 3	End	UDCONU2	-0.3930	0.0000
SB5	Span 4	Start	UDCONU2	-0.3930	0.0000
SB5	Span 4	Middle	UDCONU2	-0.5823	0.0000
SB5	Span 4	End	UDCONU2	-0.4753	0.0000
SB5	Span 5	Start	UDCONU2	-0.4753	0.0000
SB5	Span 5	Middle	UDCONU2	-1.3895	0.0386
SB5	Span 5	End	UDCONU2	-7.8074	0.2171
SB5	Span 6	Start		0.0000	0.0000
SB5	Span 6	Middle		0.0000	0.0000
SB5	Span 6	End		0.0000	0.0000
SB5	Span 7	Start		0.0000	0.0000
SB5	Span 7	Middle		0.0000	0.0000
SB5	Span 7	End		0.0000	0.0000
SB5	Span 8	Start		0.0000	0.0000
SB5	Span 8	Middle		0.0000	0.0000
SB5	Span 8	End		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB5	Span 9	Start		0.0000	0.0000
SB5	Span 9	Middle		0.0000	0.0000
SB5	Span 9	End		0.0000	0.0000
SB5	Span 10	Start		0.0000	0.0000
SB5	Span 10	Middle		0.0000	0.0000
SB5	Span 10	End		0.0000	0.0000
SB5	Span 11	Start		0.0000	0.0000
SB5	Span 11	Middle		0.0000	0.0000
SB5	Span 11	End		0.0000	0.0000
SB5	Span 12	Start		0.0000	0.0000
SB5	Span 12	Middle		0.0000	0.0000
SB5	Span 12	End		0.0000	0.0000
SB6	Span 1	Start	UDCONU2	-10.7590	0.3077
SB6	Span 1	Middle	UDCONU2	-3.4760	0.1024
SB6	Span 1	End	UDCONU2	-7.9884	0.2191
SB6	Span 2	Start	UDCONU2	-6.7338	0.1846
SB6	Span 2	Middle	UDCONU2	-17.9580	0.5062
SB6	Span 2	End	UDCONU2	-6.7279	0.2080
SB6	Span 3	Start	UDCONU2	-6.7279	0.2080
SB6	Span 3	Middle	UDCONU2	-2.8465	0.0940
SB6	Span 3	End	UDCONU2	-10.7602	0.3039
SB6	Span 4	Start	UDCONU2	-10.7602	0.3039
SB6	Span 4	Middle		0.0000	0.0000
SB6	Span 4	End	UDCONU2	-4.3185	0.1282
SB6	Span 5	Start	UDCONU2	-4.3185	0.1282
SB6	Span 5	Middle		0.0000	0.0000
SB6	Span 5	End	UDCONU2	-3.3288	0.1020
SB6	Span 6	Start	UDCONU2	-3.3288	0.1020
SB6	Span 6	Middle	UDCONU2	-1.1682	0.0341
SB6	Span 6	End	UDCONU2	-5.9448	0.1656
SB6	Span 7	Start		0.0000	0.0000
SB6	Span 7	Middle		0.0000	0.0000
SB6	Span 7	End		0.0000	0.0000
SB6	Span 8	Start		0.0000	0.0000
SB6	Span 8	Middle		0.0000	0.0000
SB6	Span 8	End		0.0000	0.0000
SB6	Span 9	Start		0.0000	0.0000
SB6	Span 9	Middle		0.0000	0.0000
SB6	Span 9	End		0.0000	0.0000
SB6	Span 10	Start		0.0000	0.0000
SB6	Span 10	Middle		0.0000	0.0000
SB6	Span 10	End		0.0000	0.0000
SB6	Span 11	Start		0.0000	0.0000
SB6	Span 11	Middle		0.0000	0.0000
SB6	Span 11	End		0.0000	0.0000
SB6	Span 12	Start		0.0000	0.0000
SB6	Span 12	Middle		0.0000	0.0000
SB6	Span 12	End		0.0000	0.0000
SB6	Span 13	Start		0.0000	0.0000
SB6	Span 13	Middle		0.0000	0.0000
SB6	Span 13	End		0.0000	0.0000
SB7	Span 1	Start	UDCONU2	-5.7832	0.1612
SB7	Span 1	Middle		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB7	Span 1	End	UDCONU2	-6.2239	0.1706
SB7	Span 2	Start	UDCONU2	-6.2239	0.1706
SB7	Span 2	Middle	UDCONU2	-10.6325	0.2918
SB7	Span 2	End	UDCONU2	-8.8507	0.2428
SB7	Span 3	Start	UDCONU2	-8.8507	0.2428
SB7	Span 3	Middle	UDCONU2	-8.3951	0.2302
SB7	Span 3	End	UDCONU2	-8.8220	0.2494
SB7	Span 4	Start	UDCONU2	-8.8220	0.2494
SB7	Span 4	Middle		0.0000	0.0000
SB7	Span 4	End	UDCONU2	-18.0573	0.4969
SB7	Span 5	Start	UDCONU2	-18.0573	0.4969
SB7	Span 5	Middle		0.0000	0.0000
SB7	Span 5	End	UDCONU2	-21.8770	0.6027
SB7	Span 6	Start	UDCONU2	-21.8770	0.6027
SB7	Span 6	Middle		0.0000	0.0000
SB7	Span 6	End	UDCONU2	-9.2012	0.2524
SB7	Span 7	Start	UDCONU2	-9.2012	0.2524
SB7	Span 7	Middle	UDCONU2	-9.3945	0.2577
SB7	Span 7	End	UDCONU2	-14.3115	0.3933
SB7	Span 8	Start	UDCONU2	-14.3115	0.3933
SB7	Span 8	Middle		0.0000	0.0000
SB7	Span 8	End	UDCONU2	-10.4899	0.3267
SB7	Span 9	Start	UDCONU2	-10.4899	0.3267
SB7	Span 9	Middle	UDCONU2	-11.2761	0.3518
SB7	Span 9	End	UDCONU2	-15.3317	0.4669
SB7	Span 10	Start	UDCONU2	-15.3317	0.4669
SB7	Span 10	Middle		0.0000	0.0000
SB7	Span 10	End	UDCONU2	-10.4642	0.2872
SB7	Span 11	Start	UDCONU2	-10.4642	0.2872
SB7	Span 11	Middle		0.0000	0.0000
SB7	Span 11	End	UDCONU2	-7.0943	0.1945
SB7	Span 12	Start	UDCONU2	-7.0943	0.1945
SB7	Span 12	Middle	UDCONU2	-1.2066	0.0386
SB7	Span 12	End	UDCONU2	-4.5887	0.1305
SB7	Span 13	Start	UDCONU2	-4.5887	0.1305
SB7	Span 13	Middle		0.0000	0.0000
SB7	Span 13	End		0.0000	0.0000
SB7	Span 14	Start		0.0000	0.0000
SB7	Span 14	Middle		0.0000	0.0000
SB7	Span 14	End		0.0000	0.0000
SB7	Span 15	Start		0.0000	0.0000
SB7	Span 15	Middle		0.0000	0.0000
SB7	Span 15	End		0.0000	0.0000
SB7	Span 16	Start		0.0000	0.0000
SB7	Span 16	Middle		0.0000	0.0000
SB7	Span 16	End		0.0000	0.0000
SB7	Span 17	Start		0.0000	0.0000
SB7	Span 17	Middle		0.0000	0.0000
SB7	Span 17	End		0.0000	0.0000
SB7	Span 18	Start		0.0000	0.0000
SB7	Span 18	Middle		0.0000	0.0000
SB7	Span 18	End		0.0000	0.0000
SB7	Span 19	Start		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB7	Span 19	Middle		0.0000	0.0000
SB7	Span 19	End		0.0000	0.0000
SB7	Span 20	Start		0.0000	0.0000
SB7	Span 20	Middle		0.0000	0.0000
SB7	Span 20	End		0.0000	0.0000
SB8	Span 1	Start		0.0000	0.0000
SB8	Span 1	Middle	UDCONU2	-1.1070	0.0325
SB8	Span 1	End	UDCONU2	-13.4193	0.3899
SB8	Span 2	Start	UDCONU2	-13.4193	0.3899
SB8	Span 2	Middle	UDCONU2	-7.1849	0.2155
SB8	Span 2	End	UDCONU2	-11.4000	0.3298
SB8	Span 3	Start	UDCONU2	-11.4000	0.3298
SB8	Span 3	Middle		0.0000	0.0000
SB8	Span 3	End	UDCONU2	-7.2979	0.2038
SB8	Span 4	Start	UDCONU2	-7.2979	0.2038
SB8	Span 4	Middle	UDCONU2	-9.1867	0.2520
SB8	Span 4	End	UDCONU2	-34.5948	0.9572
SB8	Span 5	Start	UDCONU2	-34.5948	0.9572
SB8	Span 5	Middle		0.0000	0.0000
SB8	Span 5	End	UDCONU2	-34.2696	0.9481
SB8	Span 6	Start	UDCONU2	-34.2696	0.9481
SB8	Span 6	Middle		0.0000	0.0000
SB8	Span 6	End	UDCONU2	-18.1060	0.5083
SB8	Span 7	Start	UDCONU2	-18.1060	0.5083
SB8	Span 7	Middle	UDCONU2	-8.5938	0.2404
SB8	Span 7	End	UDCONU2	-1.8740	0.0587
SB8	Span 8	Start	UDCONU2	-11.7670	0.3512
SB8	Span 8	Middle		0.0000	0.0000
SB8	Span 8	End		0.0000	0.0000
SB8	Span 9	Start		0.0000	0.0000
SB8	Span 9	Middle		0.0000	0.0000
SB8	Span 9	End		0.0000	0.0000
SB8	Span 10	Start		0.0000	0.0000
SB8	Span 10	Middle		0.0000	0.0000
SB8	Span 10	End		0.0000	0.0000
SB8	Span 11	Start		0.0000	0.0000
SB8	Span 11	Middle		0.0000	0.0000
SB8	Span 11	End		0.0000	0.0000
SB8	Span 12	Start		0.0000	0.0000
SB8	Span 12	Middle		0.0000	0.0000
SB8	Span 12	End		0.0000	0.0000
SB8	Span 13	Start		0.0000	0.0000
SB8	Span 13	Middle	UDCONU2	-2.3501	0.0762
SB8	Span 13	End	UDCONU2	-0.8727	0.0329
SB8	Span 14	Start	UDCONU2	-0.8727	0.0329
SB8	Span 14	Middle	UDCONU2	-1.5807	0.0517
SB8	Span 14	End	UDCONU2	-2.9326	0.0870
SB8	Span 15	Start	UDCONU2	-2.9326	0.0870
SB8	Span 15	Middle	UDCONU2	-2.6920	0.0785
SB8	Span 15	End	UDCONU2	-0.8017	0.0224
SB9	Span 1	Start		0.0000	0.0000
SB9	Span 1	Middle		0.0000	0.0000
SB9	Span 1	End	UDCONU2	-3.5854	0.1124

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB9	Span 2	Start	UDCONU2	-3.5854	0.1124
SB9	Span 2	Middle	UDCONU2	-0.8992	0.0337
SB9	Span 2	End	UDCONU2	-1.3288	0.0714
SB9	Span 3	Start	UDCONU2	-1.3288	0.0714
SB9	Span 3	Middle	UDCONU2	-14.1697	0.4139
SB9	Span 3	End	UDCONU2	-25.1943	0.7104
SB9	Span 4	Start	UDCONU2	-25.1943	0.7104
SB9	Span 4	Middle	UDCONU2	-5.7370	0.1592
SB9	Span 4	End	UDCONU2	-0.2892	0.0000
SB9	Span 5	Start	UDCONU2	-0.2892	0.0000
SB9	Span 5	Middle	UDCONU2	-7.9729	0.2229
SB9	Span 5	End	UDCONU2	-17.6338	0.4985
SB9	Span 6	Start	UDCONU2	-17.6338	0.4985
SB9	Span 6	Middle	UDCONU2	-11.6485	0.3417
SB9	Span 6	End	UDCONU2	-18.8047	0.5486
SB9	Span 7	Start		0.0000	0.0000
SB9	Span 7	Middle		0.0000	0.0000
SB9	Span 7	End		0.0000	0.0000
SB9	Span 8	Start		0.0000	0.0000
SB9	Span 8	Middle		0.0000	0.0000
SB9	Span 8	End		0.0000	0.0000
SB9	Span 9	Start		0.0000	0.0000
SB9	Span 9	Middle		0.0000	0.0000
SB9	Span 9	End		0.0000	0.0000
SB9	Span 10	Start		0.0000	0.0000
SB9	Span 10	Middle		0.0000	0.0000
SB9	Span 10	End	UDCONU2	-0.5803	0.0192
SB9	Span 11	Start	UDCONU2	-0.5803	0.0192
SB9	Span 11	Middle	UDCONU2	-8.8482	0.2729
SB9	Span 11	End	UDCONU2	-27.9467	0.8144
SB9	Span 12	Start	UDCONU2	-27.9467	0.8144
SB9	Span 12	Middle	UDCONU2	-11.7021	0.3639
SB9	Span 12	End	UDCONU2	-5.3070	0.1913
SB9	Span 13	Start	UDCONU2	-5.3070	0.1913
SB9	Span 13	Middle	UDCONU2	-6.5465	0.2519
SB9	Span 13	End	UDCONU2	-19.6049	0.6181
SB9	Span 14	Start	UDCONU2	-19.6049	0.6181
SB9	Span 14	Middle		0.0000	0.0000
SB9	Span 14	End	UDCONU2	-34.1455	0.9593
SB9	Span 15	Start	UDCONU2	-34.1455	0.9593
SB9	Span 15	Middle	UDCONU2	-13.3126	0.3734
SB9	Span 15	End	UDCONU2	-1.4445	0.0406
SA10	Span 1	Start		0.0000	0.0000
SA10	Span 1	Middle		0.0000	0.0000
SA10	Span 1	End		0.0000	0.0000
SA10	Span 2	Start		0.0000	0.0000
SA10	Span 2	Middle		0.0000	0.0000
SA10	Span 2	End		0.0000	0.0000
SA10	Span 3	Start		0.0000	0.0000
SA10	Span 3	Middle		0.0000	0.0000
SA10	Span 3	End		0.0000	0.0000
SA10	Span 4	Start		0.0000	0.0000
SA10	Span 4	Middle		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA10	Span 4	End		0.0000	0.0000
SA10	Span 5	Start		0.0000	0.0000
SA10	Span 5	Middle		0.0000	0.0000
SA10	Span 5	End	UDCONU2	-24.7189	0.7443
SA10	Span 6	Start	UDCONU2	-24.7189	0.7443
SA10	Span 6	Middle		0.0000	0.0000
SA10	Span 6	End		0.0000	0.0000
SA10	Span 7	Start		0.0000	0.0000
SA10	Span 7	Middle		0.0000	0.0000
SA10	Span 7	End		0.0000	0.0000
SA10	Span 8	Start		0.0000	0.0000
SA10	Span 8	Middle		0.0000	0.0000
SA10	Span 8	End		0.0000	0.0000
SA10	Span 9	Start		0.0000	0.0000
SA10	Span 9	Middle		0.0000	0.0000
SA10	Span 9	End	UDCONU2	-4.2075	0.1055
SA10	Span 10	Start	UDCONU2	-4.6266	0.1160
SA10	Span 10	Middle		0.0000	0.0000
SA10	Span 10	End	UDCONU2	-8.9712	0.2263
SA10	Span 11	Start	UDCONU2	-8.9712	0.2263
SA10	Span 11	Middle	UDCONU2	-70.4768	1.8052
SA10	Span 11	End	UDCONU2	-125.7463	3.2702
SA10	Span 12	Start	UDCONU2	-125.7463	3.2702
SA10	Span 12	Middle		0.0000	0.0000
SA10	Span 12	End	UDCONU2	-80.3862	2.0615
SA10	Span 13	Start	UDCONU2	-80.3862	2.0615
SA10	Span 13	Middle	UDCONU2	-28.0035	0.7074
SA10	Span 13	End		0.0000	0.0000
SA10	Span 14	Start		0.0000	0.0000
SA10	Span 14	Middle		0.0000	0.0000
SA10	Span 14	End		0.0000	0.0000
SA10	Span 15	Start		0.0000	0.0000
SA10	Span 15	Middle		0.0000	0.0000
SA10	Span 15	End		0.0000	0.0000
SA10	Span 16	Start		0.0000	0.0000
SA10	Span 16	Middle		0.0000	0.0000
SA10	Span 16	End		0.0000	0.0000
SA10	Span 17	Start		0.0000	0.0000
SA10	Span 17	Middle		0.0000	0.0000
SA10	Span 17	End		0.0000	0.0000
SA10	Span 18	Start		0.0000	0.0000
SA10	Span 18	Middle		0.0000	0.0000
SA10	Span 18	End		0.0000	0.0000
SA11	Span 1	Start		0.0000	0.0000
SA11	Span 1	Middle		0.0000	0.0000
SA11	Span 1	End		0.0000	0.0000
SA11	Span 2	Start		0.0000	0.0000
SA11	Span 2	Middle		0.0000	0.0000
SA11	Span 2	End		0.0000	0.0000
SA11	Span 3	Start		0.0000	0.0000
SA11	Span 3	Middle		0.0000	0.0000
SA11	Span 3	End		0.0000	0.0000
SA11	Span 4	Start		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA11	Span 4	Middle		0.0000	0.0000
SA11	Span 4	End		0.0000	0.0000
SA11	Span 5	Start		0.0000	0.0000
SA11	Span 5	Middle		0.0000	0.0000
SA11	Span 5	End	UDCONU2	-11.3288	0.3414
SA11	Span 6	Start	UDCONU2	-10.7566	0.3855
SA11	Span 6	Middle		0.0000	0.0000
SA11	Span 6	End		0.0000	0.0000
SA11	Span 7	Start		0.0000	0.0000
SA11	Span 7	Middle		0.0000	0.0000
SA11	Span 7	End		0.0000	0.0000
SA11	Span 8	Start		0.0000	0.0000
SA11	Span 8	Middle		0.0000	0.0000
SA11	Span 8	End		0.0000	0.0000
SA11	Span 9	Start		0.0000	0.0000
SA11	Span 9	Middle		0.0000	0.0000
SA11	Span 9	End	UDCONU2	-9.3599	0.2398
SA11	Span 10	Start	UDCONU2	-9.3599	0.2398
SA11	Span 10	Middle		0.0000	0.0000
SA11	Span 10	End	UDCONU2	-3.9251	0.1060
SA11	Span 11	Start	UDCONU2	-3.9251	0.1060
SA11	Span 11	Middle	UDCONU2	-4.1449	0.1138
SA11	Span 11	End	UDCONU2	-7.7669	0.2047
SA11	Span 12	Start	UDCONU2	-7.7669	0.2047
SA11	Span 12	Middle	UDCONU2	-56.5762	1.4416
SA11	Span 12	End	UDCONU2	-118.7852	3.0799
SA11	Span 13	Start	UDCONU2	-118.7852	3.0799
SA11	Span 13	Middle		0.0000	0.0000
SA11	Span 13	End	UDCONU2	-114.9940	2.9847
SA11	Span 14	Start	UDCONU2	-33.8594	0.8781
SA11	Span 14	Middle	UDCONU2	-21.7676	0.5602
SA11	Span 14	End	UDCONU2	-7.0390	0.1868
SA11	Span 15	Start	UDCONU2	-0.5184	0.0000
SA11	Span 15	Middle	UDCONU2	-0.4356	0.0000
SA11	Span 15	End	UDCONU2	-0.1555	0.0045
SA11	Span 16	Start	UDCONU2	-0.1555	0.0045
SA11	Span 16	Middle		0.0000	0.0000
SA11	Span 16	End		0.0000	0.0000
SA11	Span 17	Start		0.0000	0.0000
SA11	Span 17	Middle		0.0000	0.0000
SA11	Span 17	End		0.0000	0.0000
SA11	Span 18	Start		0.0000	0.0000
SA11	Span 18	Middle		0.0000	0.0000
SA11	Span 18	End		0.0000	0.0000
SA11	Span 19	Start		0.0000	0.0000
SA11	Span 19	Middle		0.0000	0.0000
SA11	Span 19	End		0.0000	0.0000
SA11	Span 20	Start		0.0000	0.0000
SA11	Span 20	Middle		0.0000	0.0000
SA11	Span 20	End		0.0000	0.0000
SA12	Span 1	Start		0.0000	0.0000
SA12	Span 1	Middle		0.0000	0.0000
SA12	Span 1	End		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA12	Span 2	Start		0.0000	0.0000
SA12	Span 2	Middle		0.0000	0.0000
SA12	Span 2	End		0.0000	0.0000
SA12	Span 3	Start		0.0000	0.0000
SA12	Span 3	Middle		0.0000	0.0000
SA12	Span 3	End		0.0000	0.0000
SA12	Span 4	Start		0.0000	0.0000
SA12	Span 4	Middle		0.0000	0.0000
SA12	Span 4	End		0.0000	0.0000
SA12	Span 5	Start		0.0000	0.0000
SA12	Span 5	Middle		0.0000	0.0000
SA12	Span 5	End	UDCONU2	-4.4302	0.1492
SA12	Span 6	Start	UDCONU2	-5.0795	0.1627
SA12	Span 6	Middle		0.0000	0.0000
SA12	Span 6	End		0.0000	0.0000
SA12	Span 7	Start		0.0000	0.0000
SA12	Span 7	Middle		0.0000	0.0000
SA12	Span 7	End		0.0000	0.0000
SA12	Span 8	Start		0.0000	0.0000
SA12	Span 8	Middle		0.0000	0.0000
SA12	Span 8	End		0.0000	0.0000
SA12	Span 9	Start		0.0000	0.0000
SA12	Span 9	Middle		0.0000	0.0000
SA12	Span 9	End	UDCONU2	-2.7950	0.0716
SA12	Span 10	Start	UDCONU2	-2.7950	0.0716
SA12	Span 10	Middle	UDCONU2	-7.5637	0.1917
SA12	Span 10	End	UDCONU2	-11.8453	0.2983
SA12	Span 11	Start	UDCONU2	-19.1191	0.4817
SA12	Span 11	Middle	UDCONU2	-34.9607	0.8840
SA12	Span 11	End	UDCONU2	-71.2957	1.8218
SA12	Span 12	Start	UDCONU2	-71.2957	1.8218
SA12	Span 12	Middle		0.0000	0.0000
SA12	Span 12	End	UDCONU2	-15.6947	0.3963
SA12	Span 13	Start		0.0000	0.0000
SA12	Span 13	Middle		0.0000	0.0000
SA12	Span 13	End	UDCONU2	-5.8246	0.1523
SA12	Span 14	Start	UDCONU2	-5.8246	0.1523
SA12	Span 14	Middle		0.0000	0.0000
SA12	Span 14	End	UDCONU2	-11.2123	0.2879
SA12	Span 15	Start	UDCONU2	-8.4778	0.2184
SA12	Span 15	Middle		0.0000	0.0000
SA12	Span 15	End		0.0000	0.0000
SA12	Span 16	Start		0.0000	0.0000
SA12	Span 16	Middle		0.0000	0.0000
SA12	Span 16	End		0.0000	0.0000
SA12	Span 17	Start		0.0000	0.0000
SA12	Span 17	Middle		0.0000	0.0000
SA12	Span 17	End		0.0000	0.0000
SA12	Span 18	Start		0.0000	0.0000
SA12	Span 18	Middle		0.0000	0.0000
SA12	Span 18	End		0.0000	0.0000
SA12	Span 19	Start		0.0000	0.0000
SA12	Span 19	Middle		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA12	Span 19	End		0.0000	0.0000
SA13	Span 1	Start		0.0000	0.0000
SA13	Span 1	Middle		0.0000	0.0000
SA13	Span 1	End		0.0000	0.0000
SA13	Span 2	Start		0.0000	0.0000
SA13	Span 2	Middle		0.0000	0.0000
SA13	Span 2	End		0.0000	0.0000
SA13	Span 3	Start		0.0000	0.0000
SA13	Span 3	Middle		0.0000	0.0000
SA13	Span 3	End		0.0000	0.0000
SA13	Span 4	Start		0.0000	0.0000
SA13	Span 4	Middle		0.0000	0.0000
SA13	Span 4	End		0.0000	0.0000
SA13	Span 5	Start		0.0000	0.0000
SA13	Span 5	Middle		0.0000	0.0000
SA13	Span 5	End	UDCONU2	-3.5429	0.1322
SA13	Span 6	Start	UDCONU2	-3.5429	0.1322
SA13	Span 6	Middle		0.0000	0.0000
SA13	Span 6	End		0.0000	0.0000
SA13	Span 7	Start		0.0000	0.0000
SA13	Span 7	Middle		0.0000	0.0000
SA13	Span 7	End		0.0000	0.0000
SA13	Span 8	Start		0.0000	0.0000
SA13	Span 8	Middle		0.0000	0.0000
SA13	Span 8	End		0.0000	0.0000
SA13	Span 9	Start		0.0000	0.0000
SA13	Span 9	Middle	UDCONU1	-0.1553	0.0039
SA13	Span 9	End	UDCONU1	-0.0620	0.0016
SA13	Span 10	Start		0.0000	0.0000
SA13	Span 10	Middle		0.0000	0.0000
SA13	Span 10	End	UDCONU2	-23.7482	0.6091
SA13	Span 11	Start	UDCONU2	-30.6616	0.7848
SA13	Span 11	Middle	UDCONU2	-35.0685	0.8923
SA13	Span 11	End	UDCONU2	-62.5043	1.5955
SA13	Span 12	Start	UDCONU2	-62.5043	1.5955
SA13	Span 12	Middle		0.0000	0.0000
SA13	Span 12	End	UDCONU2	-27.2428	0.6940
SA13	Span 13	Start		0.0000	0.0000
SA13	Span 13	Middle		0.0000	0.0000
SA13	Span 13	End	UDCONU2	-2.3296	0.0633
SA13	Span 14	Start	UDCONU2	-1.9129	0.0532
SA13	Span 14	Middle		0.0000	0.0000
SA13	Span 14	End	UDCONU2	-11.7888	0.3067
SA13	Span 15	Start	UDCONU2	-11.7888	0.3067
SA13	Span 15	Middle		0.0000	0.0000
SA13	Span 15	End		0.0000	0.0000
SA13	Span 16	Start		0.0000	0.0000
SA13	Span 16	Middle		0.0000	0.0000
SA13	Span 16	End		0.0000	0.0000
SA13	Span 17	Start		0.0000	0.0000
SA13	Span 17	Middle		0.0000	0.0000
SA13	Span 17	End		0.0000	0.0000
SA13	Span 18	Start		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA13	Span 18	Middle		0.0000	0.0000
SA13	Span 18	End		0.0000	0.0000
SA13	Span 19	Start		0.0000	0.0000
SA13	Span 19	Middle		0.0000	0.0000
SA13	Span 19	End		0.0000	0.0000
SA14	Span 1	Start		0.0000	0.0000
SA14	Span 1	Middle		0.0000	0.0000
SA14	Span 1	End		0.0000	0.0000
SA14	Span 2	Start		0.0000	0.0000
SA14	Span 2	Middle		0.0000	0.0000
SA14	Span 2	End		0.0000	0.0000
SA14	Span 3	Start		0.0000	0.0000
SA14	Span 3	Middle		0.0000	0.0000
SA14	Span 3	End		0.0000	0.0000
SA14	Span 4	Start		0.0000	0.0000
SA14	Span 4	Middle		0.0000	0.0000
SA14	Span 4	End		0.0000	0.0000
SA14	Span 5	Start		0.0000	0.0000
SA14	Span 5	Middle		0.0000	0.0000
SA14	Span 5	End	UDCONU2	-6.5869	0.2068
SA14	Span 6	Start	UDCONU2	-8.4354	0.2358
SA14	Span 6	Middle		0.0000	0.0000
SA14	Span 6	End	UDCONU2	-4.2633	0.1086
SA14	Span 7	Start	UDCONU2	-4.2633	0.1086
SA14	Span 7	Middle	UDCONU2	-4.7968	0.1210
SA14	Span 7	End	UDCONU2	-5.2899	0.1336
SA14	Span 8	Start	UDCONU2	-5.2899	0.1336
SA14	Span 8	Middle	UDCONU2	-4.5526	0.1148
SA14	Span 8	End	UDCONU2	-3.3559	0.0908
SA14	Span 9	Start	UDCONU2	-3.3559	0.0908
SA14	Span 9	Middle	UDCONU2	-22.1155	0.5708
SA14	Span 9	End	UDCONU2	-34.5261	0.8841
SA14	Span 10	Start	UDCONU2	-34.5261	0.8841
SA14	Span 10	Middle	UDCONU2	-38.8206	0.9893
SA14	Span 10	End	UDCONU1	-37.7457	0.9648
SA14	Span 11	Start	UDCONU1	-37.7457	0.9648
SA14	Span 11	Middle		0.0000	0.0000
SA14	Span 11	End	UDCONU1	-27.9456	0.7052
SA14	Span 12	Start	UDCONU1	-27.9456	0.7052
SA14	Span 12	Middle		0.0000	0.0000
SA14	Span 12	End	UDCONU1	-19.8672	0.5003
SA14	Span 13	Start	UDCONU1	-19.8672	0.5003
SA14	Span 13	Middle	UDCONU1	-8.8245	0.2215
SA14	Span 13	End	UDCONU2	-19.1642	0.4902
SA14	Span 14	Start	UDCONU2	-19.1642	0.4902
SA14	Span 14	Middle	UDCONU2	-23.8212	0.6205
SA14	Span 14	End	UDCONU2	-32.8160	0.8662
SA14	Span 15	Start	UDCONU2	-32.8160	0.8662
SA14	Span 15	Middle		0.0000	0.0000
SA14	Span 15	End	UDCONU2	-69.2389	1.8023
SA14	Span 16	Start	UDCONU2	-69.2389	1.8023
SA14	Span 16	Middle		0.0000	0.0000
SA14	Span 16	End	UDCONU2	-44.9308	1.1709

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA14	Span 17	Start	UDCONU2	-44.9308	1.1709
SA14	Span 17	Middle	UDCONU2	-17.4667	0.4699
SA14	Span 17	End	UDCONU2	-8.5381	0.2452
SA14	Span 18	Start	UDCONU2	-8.5381	0.2452
SA14	Span 18	Middle		0.0000	0.0000
SA14	Span 18	End	UDCONU2	-10.7296	0.3071
SA14	Span 19	Start	UDCONU2	-10.7296	0.3071
SA14	Span 19	Middle		0.0000	0.0000
SA14	Span 19	End	UDCONU2	-42.5600	1.1630
SA14	Span 20	Start	UDCONU2	-42.5600	1.1630
SA14	Span 20	Middle		0.0000	0.0000
SA14	Span 20	End	UDCONU2	-49.9667	1.3472
SA14	Span 21	Start	UDCONU2	-49.9667	1.3472
SA14	Span 21	Middle		0.0000	0.0000
SA14	Span 21	End	UDCONU2	-60.8211	1.6183
SA14	Span 22	Start	UDCONU2	-60.8211	1.6183
SA14	Span 22	Middle	UDCONU2	-7.6106	0.1961
SA14	Span 22	End		0.0000	0.0000
SA14	Span 23	Start		0.0000	0.0000
SA14	Span 23	Middle		0.0000	0.0000
SA14	Span 23	End		0.0000	0.0000
SA14	Span 24	Start		0.0000	0.0000
SA14	Span 24	Middle		0.0000	0.0000
SA14	Span 24	End		0.0000	0.0000
SA14	Span 25	Start		0.0000	0.0000
SA14	Span 25	Middle		0.0000	0.0000
SA14	Span 25	End		0.0000	0.0000
SA14	Span 26	Start		0.0000	0.0000
SA14	Span 26	Middle		0.0000	0.0000
SA14	Span 26	End		0.0000	0.0000
SA15	Span 1	Start		0.0000	0.0000
SA15	Span 1	Middle		0.0000	0.0000
SA15	Span 1	End		0.0000	0.0000
SA15	Span 2	Start		0.0000	0.0000
SA15	Span 2	Middle		0.0000	0.0000
SA15	Span 2	End		0.0000	0.0000
SA15	Span 3	Start		0.0000	0.0000
SA15	Span 3	Middle		0.0000	0.0000
SA15	Span 3	End		0.0000	0.0000
SA15	Span 4	Start		0.0000	0.0000
SA15	Span 4	Middle	UDCONU2	-1.4963	0.0428
SA15	Span 4	End	UDCONU2	-10.4928	0.2915
SA15	Span 5	Start	UDCONU2	-10.4928	0.2915
SA15	Span 5	Middle	UDCONU2	-19.4457	0.5187
SA15	Span 5	End	UDCONU2	-36.5083	0.9527
SA15	Span 6	Start	UDCONU2	-36.5083	0.9527
SA15	Span 6	Middle		0.0000	0.0000
SA15	Span 6	End	UDCONU2	-45.0689	1.1634
SA15	Span 7	Start	UDCONU2	-45.0689	1.1634
SA15	Span 7	Middle		0.0000	0.0000
SA15	Span 7	End	UDCONU2	-36.1959	0.9194
SA15	Span 8	Start	UDCONU2	-36.1959	0.9194
SA15	Span 8	Middle	UDCONU2	-35.3642	0.9008

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA15	Span 8	End	UDCONU2	-34.4860	0.8719
SA15	Span 9	Start	UDCONU2	-34.4860	0.8719
SA15	Span 9	Middle	UDCONU2	-13.6646	0.3468
SA15	Span 9	End	UDCONU1	-7.2807	0.1827
SA15	Span 10	Start	UDCONU1	-7.2807	0.1827
SA15	Span 10	Middle	UDCONU2	-2.9338	0.0892
SA15	Span 10	End	UDCONU2	-4.6686	0.1291
SA15	Span 11	Start	UDCONU2	-7.8508	0.2055
SA15	Span 11	Middle	UDCONU2	-28.7912	0.7405
SA15	Span 11	End	UDCONU2	-35.6280	0.9259
SA15	Span 12	Start	UDCONU2	-35.6280	0.9259
SA15	Span 12	Middle	UDCONU2	-34.0892	0.8758
SA15	Span 12	End	UDCONU2	-12.2540	0.3213
SA15	Span 13	Start	UDCONU2	-12.2540	0.3213
SA15	Span 13	Middle	UDCONU2	-24.3261	0.6789
SA15	Span 13	End	UDCONU2	-43.7381	1.1723
SA15	Span 14	Start	UDCONU2	-43.7381	1.1723
SA15	Span 14	Middle		0.0000	0.0000
SA15	Span 14	End	UDCONU2	-50.9855	1.3057
SA15	Span 15	Start	UDCONU2	-50.9855	1.3057
SA15	Span 15	Middle	UDCONU2	-23.7988	0.6124
SA15	Span 15	End		0.0000	0.0000
SA15	Span 16	Start		0.0000	0.0000
SA15	Span 16	Middle		0.0000	0.0000
SA15	Span 16	End		0.0000	0.0000
SA15	Span 17	Start		0.0000	0.0000
SA15	Span 17	Middle		0.0000	0.0000
SA15	Span 17	End		0.0000	0.0000
SA15	Span 18	Start		0.0000	0.0000
SA15	Span 18	Middle		0.0000	0.0000
SA15	Span 18	End		0.0000	0.0000
SA15	Span 19	Start		0.0000	0.0000
SA15	Span 19	Middle		0.0000	0.0000
SA15	Span 19	End		0.0000	0.0000
SA16	Span 1	Start		0.0000	0.0000
SA16	Span 1	Middle		0.0000	0.0000
SA16	Span 1	End		0.0000	0.0000
SA16	Span 2	Start		0.0000	0.0000
SA16	Span 2	Middle		0.0000	0.0000
SA16	Span 2	End		0.0000	0.0000
SA16	Span 3	Start		0.0000	0.0000
SA16	Span 3	Middle		0.0000	0.0000
SA16	Span 3	End		0.0000	0.0000
SA16	Span 4	Start		0.0000	0.0000
SA16	Span 4	Middle	UDCONU2	-1.4129	0.0383
SA16	Span 4	End	UDCONU2	-30.1520	0.7632
SA16	Span 5	Start	UDCONU2	-30.1520	0.7632
SA16	Span 5	Middle		0.0000	0.0000
SA16	Span 5	End	UDCONU2	-48.8439	1.3217
SA16	Span 6	Start	UDCONU2	-48.8439	1.3217
SA16	Span 6	Middle	UDCONU2	-19.9160	0.5760
SA16	Span 6	End	UDCONU2	-49.4549	1.2825
SA16	Span 7	Start	UDCONU2	-49.7877	1.2683

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA16	Span 7	Middle		0.0000	0.0000
SA16	Span 7	End	UDCONU2	-45.0065	1.1677
SA16	Span 8	Start	UDCONU2	-45.0065	1.1677
SA16	Span 8	Middle		0.0000	0.0000
SA16	Span 8	End	UDCONU2	-49.0971	1.2645
SA16	Span 9	Start	UDCONU2	-49.0971	1.2645
SA16	Span 9	Middle	UDCONU2	-18.5183	0.4793
SA16	Span 9	End	UDCONU2	-0.1405	0.0000
SA16	Span 10	Start	UDCONU2	-0.1405	0.0000
SA16	Span 10	Middle		0.0000	0.0000
SA16	Span 10	End		0.0000	0.0000
SA16	Span 11	Start		0.0000	0.0000
SA16	Span 11	Middle	UDCONU2	-1.8849	0.0650
SA16	Span 11	End	UDCONU2	-1.8849	0.0650
SA16	Span 12	Start	UDCONU2	-1.3096	0.0575
SA16	Span 12	Middle	UDCONU2	-1.7863	0.0747
SA16	Span 12	End	UDCONU2	-1.4692	0.0679
SA16	Span 13	Start	UDCONU2	-1.4692	0.0679
SA16	Span 13	Middle	UDCONU2	-7.9556	0.2154
SA16	Span 13	End	UDCONU2	-34.2330	0.8908
SA16	Span 14	Start	UDCONU2	-34.2330	0.8908
SA16	Span 14	Middle		0.0000	0.0000
SA16	Span 14	End	UDCONU2	-60.6363	1.5612
SA16	Span 15	Start	UDCONU2	-60.6363	1.5612
SA16	Span 15	Middle	UDCONU2	-19.0074	0.4950
SA16	Span 15	End		0.0000	0.0000
SA16	Span 16	Start		0.0000	0.0000
SA16	Span 16	Middle		0.0000	0.0000
SA16	Span 16	End		0.0000	0.0000
SA16	Span 17	Start		0.0000	0.0000
SA16	Span 17	Middle		0.0000	0.0000
SA16	Span 17	End		0.0000	0.0000
SA16	Span 18	Start		0.0000	0.0000
SA16	Span 18	Middle		0.0000	0.0000
SA16	Span 18	End		0.0000	0.0000
SA16	Span 19	Start		0.0000	0.0000
SA16	Span 19	Middle		0.0000	0.0000
SA16	Span 19	End		0.0000	0.0000
SA17	Span 1	Start		0.0000	0.0000
SA17	Span 1	Middle		0.0000	0.0000
SA17	Span 1	End		0.0000	0.0000
SA17	Span 2	Start		0.0000	0.0000
SA17	Span 2	Middle		0.0000	0.0000
SA17	Span 2	End		0.0000	0.0000
SA17	Span 3	Start		0.0000	0.0000
SA17	Span 3	Middle		0.0000	0.0000
SA17	Span 3	End		0.0000	0.0000
SA17	Span 4	Start		0.0000	0.0000
SA17	Span 4	Middle		0.0000	0.0000
SA17	Span 4	End	UDCONU2	-22.0580	0.5641
SA17	Span 5	Start	UDCONU2	-38.6245	0.9838
SA17	Span 5	Middle	UDCONU2	-23.9150	0.6357
SA17	Span 5	End	UDCONU2	-51.9178	1.3418

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA17	Span 6	Start	UDCONU2	-51.9178	1.3418
SA17	Span 6	Middle		0.0000	0.0000
SA17	Span 6	End	UDCONU2	-57.4041	1.4835
SA17	Span 7	Start	UDCONU2	-57.4041	1.4835
SA17	Span 7	Middle	UDCONU2	-59.6237	1.5403
SA17	Span 7	End	UDCONU2	-57.8162	1.4929
SA17	Span 8	Start	UDCONU2	-57.8162	1.4929
SA17	Span 8	Middle	UDCONU2	-17.3481	0.4543
SA17	Span 8	End		0.0000	0.0000
SA17	Span 9	Start		0.0000	0.0000
SA17	Span 9	Middle		0.0000	0.0000
SA17	Span 9	End		0.0000	0.0000
SA17	Span 10	Start		0.0000	0.0000
SA17	Span 10	Middle		0.0000	0.0000
SA17	Span 10	End		0.0000	0.0000
SA17	Span 11	Start		0.0000	0.0000
SA17	Span 11	Middle		0.0000	0.0000
SA17	Span 11	End		0.0000	0.0000
SA17	Span 12	Start		0.0000	0.0000
SA17	Span 12	Middle	UDCONU2	-3.0374	0.0845
SA17	Span 12	End	UDCONU2	-16.2911	0.4212
SA17	Span 13	Start	UDCONU2	-16.2911	0.4212
SA17	Span 13	Middle		0.0000	0.0000
SA17	Span 13	End	UDCONU2	-33.1160	0.8461
SA17	Span 14	Start	UDCONU2	-33.1160	0.8461
SA17	Span 14	Middle	UDCONU2	-9.7948	0.2563
SA17	Span 14	End		0.0000	0.0000
SA17	Span 15	Start		0.0000	0.0000
SA17	Span 15	Middle		0.0000	0.0000
SA17	Span 15	End		0.0000	0.0000
SA17	Span 16	Start		0.0000	0.0000
SA17	Span 16	Middle		0.0000	0.0000
SA17	Span 16	End		0.0000	0.0000
SA17	Span 17	Start		0.0000	0.0000
SA17	Span 17	Middle		0.0000	0.0000
SA17	Span 17	End		0.0000	0.0000
SA17	Span 18	Start		0.0000	0.0000
SA17	Span 18	Middle		0.0000	0.0000
SA17	Span 18	End		0.0000	0.0000
SA18	Span 1	Start		0.0000	0.0000
SA18	Span 1	Middle		0.0000	0.0000
SA18	Span 1	End		0.0000	0.0000
SA18	Span 2	Start		0.0000	0.0000
SA18	Span 2	Middle		0.0000	0.0000
SA18	Span 2	End		0.0000	0.0000
SA18	Span 3	Start		0.0000	0.0000
SA18	Span 3	Middle		0.0000	0.0000
SA18	Span 3	End		0.0000	0.0000
SA18	Span 4	Start		0.0000	0.0000
SA18	Span 4	Middle		0.0000	0.0000
SA18	Span 4	End	UDCONU2	-23.3208	0.5878
SA18	Span 5	Start	UDCONU2	-40.7254	1.0326
SA18	Span 5	Middle	UDCONU2	-18.6361	0.4815

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA18	Span 5	End	UDCONU2	-31.1586	0.8074
SA18	Span 6	Start	UDCONU2	-31.1586	0.8074
SA18	Span 6	Middle		0.0000	0.0000
SA18	Span 6	End	UDCONU2	-58.3189	1.5031
SA18	Span 7	Start	UDCONU2	-58.3189	1.5031
SA18	Span 7	Middle	UDCONU2	-33.7070	0.8701
SA18	Span 7	End	UDCONU2	-1.6254	0.0564
SA18	Span 8	Start	UDCONU2	-1.6254	0.0564
SA18	Span 8	Middle	UDCONU2	-0.6610	0.0355
SA18	Span 8	End		0.0000	0.0000
SA18	Span 9	Start		0.0000	0.0000
SA18	Span 9	Middle		0.0000	0.0000
SA18	Span 9	End		0.0000	0.0000
SA18	Span 10	Start		0.0000	0.0000
SA18	Span 10	Middle	UDCONU2	-0.6025	0.0454
SA18	Span 10	End	UDCONU2	-0.9034	0.0475
SA18	Span 11	Start	UDCONU2	-0.9034	0.0475
SA18	Span 11	Middle	UDCONU2	-2.4702	0.0708
SA18	Span 11	End	UDCONU2	-14.1103	0.3626
SA18	Span 12	Start	UDCONU2	-14.1103	0.3626
SA18	Span 12	Middle		0.0000	0.0000
SA18	Span 12	End	UDCONU2	-25.7318	0.6555
SA18	Span 13	Start	UDCONU2	-25.7318	0.6555
SA18	Span 13	Middle	UDCONU2	-8.3955	0.2128
SA18	Span 13	End		0.0000	0.0000
SA18	Span 14	Start		0.0000	0.0000
SA18	Span 14	Middle		0.0000	0.0000
SA18	Span 14	End		0.0000	0.0000
SA18	Span 15	Start		0.0000	0.0000
SA18	Span 15	Middle		0.0000	0.0000
SA18	Span 15	End		0.0000	0.0000
SA18	Span 16	Start		0.0000	0.0000
SA18	Span 16	Middle		0.0000	0.0000
SA18	Span 16	End		0.0000	0.0000
SA18	Span 17	Start		0.0000	0.0000
SA18	Span 17	Middle		0.0000	0.0000
SA18	Span 17	End		0.0000	0.0000
SA19	Span 1	Start		0.0000	0.0000
SA19	Span 1	Middle		0.0000	0.0000
SA19	Span 1	End		0.0000	0.0000
SA19	Span 2	Start		0.0000	0.0000
SA19	Span 2	Middle		0.0000	0.0000
SA19	Span 2	End		0.0000	0.0000
SA19	Span 3	Start		0.0000	0.0000
SA19	Span 3	Middle		0.0000	0.0000
SA19	Span 3	End		0.0000	0.0000
SA19	Span 4	Start		0.0000	0.0000
SA19	Span 4	Middle		0.0000	0.0000
SA19	Span 4	End	UDCONU2	-30.1603	0.7720
SA19	Span 5	Start	UDCONU2	-30.1603	0.7720
SA19	Span 5	Middle		0.0000	0.0000
SA19	Span 5	End	UDCONU2	-51.7148	1.3453
SA19	Span 6	Start	UDCONU2	-51.7148	1.3453

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA19	Span 6	Middle		0.0000	0.0000
SA19	Span 6	End	UDCONU2	-31.9564	0.8510
SA19	Span 7	Start	UDCONU2	-31.9564	0.8510
SA19	Span 7	Middle		0.0000	0.0000
SA19	Span 7	End	UDCONU2	-28.0653	0.7296
SA19	Span 8	Start	UDCONU2	-28.0653	0.7296
SA19	Span 8	Middle	UDCONU2	-4.1181	0.1199
SA19	Span 8	End	UDCONU2	-10.6634	0.2822
SA19	Span 9	Start	UDCONU2	-10.6634	0.2822
SA19	Span 9	Middle		0.0000	0.0000
SA19	Span 9	End	UDCONU2	-32.1313	0.8291
SA19	Span 10	Start	UDCONU2	-32.1313	0.8291
SA19	Span 10	Middle		0.0000	0.0000
SA19	Span 10	End	UDCONU2	-37.0647	0.9573
SA19	Span 11	Start	UDCONU2	-37.0647	0.9573
SA19	Span 11	Middle		0.0000	0.0000
SA19	Span 11	End	UDCONU2	-54.5592	1.4129
SA19	Span 12	Start	UDCONU2	-54.5592	1.4129
SA19	Span 12	Middle	UDCONU2	-50.7010	1.3141
SA19	Span 12	End	UDCONU2	-58.8061	1.5218
SA19	Span 13	Start	UDCONU2	-58.8061	1.5218
SA19	Span 13	Middle		0.0000	0.0000
SA19	Span 13	End	UDCONU2	-59.0282	1.5238
SA19	Span 14	Start	UDCONU2	-59.0282	1.5238
SA19	Span 14	Middle		0.0000	0.0000
SA19	Span 14	End	UDCONU2	-40.8504	1.0649
SA19	Span 15	Start	UDCONU2	-40.8504	1.0649
SA19	Span 15	Middle		0.0000	0.0000
SA19	Span 15	End	UDCONU2	-33.4728	0.8548
SA19	Span 16	Start	UDCONU2	-33.4728	0.8548
SA19	Span 16	Middle		0.0000	0.0000
SA19	Span 16	End	UDCONU2	-14.9289	0.3771
SA19	Span 17	Start	UDCONU2	-14.9289	0.3771
SA19	Span 17	Middle	UDCONU2	-10.1705	0.2599
SA19	Span 17	End	UDCONU2	-25.8415	0.6517
SA19	Span 18	Start	UDCONU2	-25.8415	0.6517
SA19	Span 18	Middle		0.0000	0.0000
SA19	Span 18	End	UDCONU2	-26.0008	0.6565
SA19	Span 19	Start	UDCONU2	-26.0008	0.6565
SA19	Span 19	Middle		0.0000	0.0000
SA19	Span 19	End	UDCONU2	-8.4253	0.2673
SA19	Span 20	Start	UDCONU2	-8.4253	0.2673
SA19	Span 20	Middle	UDCONU2	-5.6127	0.1984
SA19	Span 20	End	UDCONU2	-4.2630	0.1684
SA19	Span 21	Start	UDCONU2	-4.2630	0.1684
SA19	Span 21	Middle	UDCONU2	-9.4224	0.3023
SA19	Span 21	End	UDCONU2	-33.5170	0.8517
SA19	Span 22	Start	UDCONU2	-33.5170	0.8517
SA19	Span 22	Middle		0.0000	0.0000
SA19	Span 22	End	UDCONU2	-33.3777	0.8475
SA19	Span 23	Start	UDCONU2	-33.3777	0.8475
SA19	Span 23	Middle		0.0000	0.0000
SA19	Span 23	End	UDCONU2	-16.8233	0.4346

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA19	Span 24	Start	UDCONU2	-16.8233	0.4346
SA19	Span 24	Middle	UDCONU2	-6.1018	0.1537
SA19	Span 24	End	UDCONU2	-14.2355	0.3637
SA19	Span 25	Start	UDCONU2	-14.2355	0.3637
SA19	Span 25	Middle		0.0000	0.0000
SA19	Span 25	End	UDCONU2	-37.1607	0.9445
SA19	Span 26	Start	UDCONU2	-37.1607	0.9445
SA19	Span 26	Middle	UDCONU2	-15.0860	0.3815
SA19	Span 26	End		0.0000	0.0000
SA19	Span 27	Start		0.0000	0.0000
SA19	Span 27	Middle		0.0000	0.0000
SA19	Span 27	End		0.0000	0.0000
SA19	Span 28	Start		0.0000	0.0000
SA19	Span 28	Middle		0.0000	0.0000
SA19	Span 28	End		0.0000	0.0000
SA19	Span 29	Start		0.0000	0.0000
SA19	Span 29	Middle		0.0000	0.0000
SA19	Span 29	End		0.0000	0.0000
SA19	Span 30	Start		0.0000	0.0000
SA19	Span 30	Middle		0.0000	0.0000
SA19	Span 30	End		0.0000	0.0000
SA20	Span 1	Start		0.0000	0.0000
SA20	Span 1	Middle		0.0000	0.0000
SA20	Span 1	End		0.0000	0.0000
SA20	Span 2	Start		0.0000	0.0000
SA20	Span 2	Middle		0.0000	0.0000
SA20	Span 2	End		0.0000	0.0000
SA20	Span 3	Start		0.0000	0.0000
SA20	Span 3	Middle		0.0000	0.0000
SA20	Span 3	End		0.0000	0.0000
SA20	Span 4	Start		0.0000	0.0000
SA20	Span 4	Middle	UDCONU2	-1.6613	0.0474
SA20	Span 4	End	UDCONU2	-30.6039	0.8028
SA20	Span 5	Start	UDCONU2	-30.6039	0.8028
SA20	Span 5	Middle		0.0000	0.0000
SA20	Span 5	End	UDCONU2	-48.3809	1.2673
SA20	Span 6	Start	UDCONU2	-48.3809	1.2673
SA20	Span 6	Middle		0.0000	0.0000
SA20	Span 6	End	UDCONU2	-33.9314	0.9240
SA20	Span 7	Start	UDCONU2	-33.9314	0.9240
SA20	Span 7	Middle		0.0000	0.0000
SA20	Span 7	End	UDCONU2	-29.7083	0.7769
SA20	Span 8	Start	UDCONU2	-29.7083	0.7769
SA20	Span 8	Middle	UDCONU2	-2.5483	0.0766
SA20	Span 8	End	UDCONU2	-3.3179	0.0919
SA20	Span 9	Start	UDCONU2	-3.3179	0.0919
SA20	Span 9	Middle		0.0000	0.0000
SA20	Span 9	End	UDCONU2	-13.0985	0.3417
SA20	Span 10	Start	UDCONU2	-13.0985	0.3417
SA20	Span 10	Middle		0.0000	0.0000
SA20	Span 10	End	UDCONU2	-18.9214	0.4986
SA20	Span 11	Start	UDCONU2	-18.9214	0.4986
SA20	Span 11	Middle		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA20	Span 11	End	UDCONU2	-36.5236	0.9461
SA20	Span 12	Start	UDCONU2	-36.5236	0.9461
SA20	Span 12	Middle		0.0000	0.0000
SA20	Span 12	End	UDCONU2	-40.1016	1.0459
SA20	Span 13	Start	UDCONU2	-40.1016	1.0459
SA20	Span 13	Middle		0.0000	0.0000
SA20	Span 13	End	UDCONU2	-35.2786	0.8921
SA20	Span 14	Start	UDCONU2	-35.2786	0.8921
SA20	Span 14	Middle		0.0000	0.0000
SA20	Span 14	End	UDCONU2	-21.0246	0.5295
SA20	Span 15	Start	UDCONU2	-21.0246	0.5295
SA20	Span 15	Middle	UDCONU2	-18.1453	0.4566
SA20	Span 15	End	UDCONU2	-29.5648	0.7464
SA20	Span 16	Start	UDCONU2	-29.5648	0.7464
SA20	Span 16	Middle		0.0000	0.0000
SA20	Span 16	End	UDCONU2	-32.4677	0.8204
SA20	Span 17	Start	UDCONU2	-32.4677	0.8204
SA20	Span 17	Middle		0.0000	0.0000
SA20	Span 17	End	UDCONU2	-19.9060	0.5645
SA20	Span 18	Start	UDCONU2	-19.9060	0.5645
SA20	Span 18	Middle	UDCONU2	-13.0056	0.3929
SA20	Span 18	End	UDCONU2	-9.0444	0.2965
SA20	Span 19	Start	UDCONU2	-9.0444	0.2965
SA20	Span 19	Middle	UDCONU2	-21.4708	0.6240
SA20	Span 19	End	UDCONU2	-40.5220	1.0352
SA20	Span 20	Start	UDCONU2	-40.5220	1.0352
SA20	Span 20	Middle		0.0000	0.0000
SA20	Span 20	End	UDCONU2	-37.4153	0.9518
SA20	Span 21	Start	UDCONU2	-37.4153	0.9518
SA20	Span 21	Middle		0.0000	0.0000
SA20	Span 21	End	UDCONU2	-17.1079	0.4414
SA20	Span 22	Start	UDCONU2	-17.1079	0.4414
SA20	Span 22	Middle	UDCONU2	-6.2718	0.1591
SA20	Span 22	End	UDCONU2	-11.2877	0.2835
SA20	Span 23	Start	UDCONU2	-11.2877	0.2835
SA20	Span 23	Middle		0.0000	0.0000
SA20	Span 23	End	UDCONU2	-34.3755	0.8723
SA20	Span 24	Start	UDCONU2	-34.3755	0.8723
SA20	Span 24	Middle	UDCONU2	-3.8421	0.0973
SA20	Span 24	End		0.0000	0.0000
SA20	Span 25	Start		0.0000	0.0000
SA20	Span 25	Middle		0.0000	0.0000
SA20	Span 25	End		0.0000	0.0000
SA20	Span 26	Start		0.0000	0.0000
SA20	Span 26	Middle		0.0000	0.0000
SA20	Span 26	End		0.0000	0.0000
SA20	Span 27	Start		0.0000	0.0000
SA20	Span 27	Middle		0.0000	0.0000
SA20	Span 27	End		0.0000	0.0000
SA20	Span 28	Start		0.0000	0.0000
SA20	Span 28	Middle		0.0000	0.0000
SA20	Span 28	End		0.0000	0.0000
SA21	Span 1	Start		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SA21	Span 1	Middle		0.0000	0.0000
SA21	Span 1	End		0.0000	0.0000
SA21	Span 2	Start		0.0000	0.0000
SA21	Span 2	Middle		0.0000	0.0000
SA21	Span 2	End		0.0000	0.0000
SA21	Span 3	Start		0.0000	0.0000
SA21	Span 3	Middle		0.0000	0.0000
SA21	Span 3	End		0.0000	0.0000
SA21	Span 4	Start		0.0000	0.0000
SA21	Span 4	Middle	UDCONU2	-0.9754	0.0295
SA21	Span 4	End	UDCONU2	-13.4389	0.3584
SA21	Span 5	Start	UDCONU2	-13.4389	0.3584
SA21	Span 5	Middle	UDCONU2	-14.3722	0.3801
SA21	Span 5	End	UDCONU2	-0.4853	0.0000
SA21	Span 6	Start	UDCONU2	-0.4853	0.0000
SA21	Span 6	Middle	UDCONU2	-10.7686	0.2743
SA21	Span 6	End	UDCONU2	-10.3070	0.2614
SA21	Span 7	Start	UDCONU2	-10.3070	0.2614
SA21	Span 7	Middle	UDCONU2	-12.8160	0.3297
SA21	Span 7	End	UDCONU2	-11.0470	0.3039
SA21	Span 8	Start	UDCONU2	-11.0470	0.3039
SA21	Span 8	Middle	UDCONU2	-7.3511	0.2160
SA21	Span 8	End	UDCONU2	-4.6871	0.1530
SA21	Span 9	Start	UDCONU2	-4.6871	0.1530
SA21	Span 9	Middle	UDCONU2	-13.7880	0.3703
SA21	Span 9	End	UDCONU2	-7.6982	0.2060
SA21	Span 10	Start	UDCONU2	-7.6982	0.2060
SA21	Span 10	Middle	UDCONU2	-2.5714	0.0691
SA21	Span 10	End	UDCONU2	-4.3934	0.1103
SA21	Span 11	Start	UDCONU2	-4.3934	0.1103
SA21	Span 11	Middle	UDCONU2	-4.3783	0.1099
SA21	Span 11	End		0.0000	0.0000
SA21	Span 12	Start		0.0000	0.0000
SA21	Span 12	Middle		0.0000	0.0000
SA21	Span 12	End		0.0000	0.0000
SA21	Span 13	Start		0.0000	0.0000
SA21	Span 13	Middle		0.0000	0.0000
SA21	Span 13	End		0.0000	0.0000
SA21	Span 14	Start		0.0000	0.0000
SA21	Span 14	Middle		0.0000	0.0000
SA21	Span 14	End		0.0000	0.0000
SA21	Span 15	Start		0.0000	0.0000
SA21	Span 15	Middle		0.0000	0.0000
SA21	Span 15	End		0.0000	0.0000
SB10	Span 1	Start	UDCONU2	-2.4958	0.0738
SB10	Span 1	Middle		0.0000	0.0000
SB10	Span 1	End	UDCONU2	-4.6565	0.1462
SB10	Span 2	Start	UDCONU2	-4.6565	0.1462
SB10	Span 2	Middle		0.0000	0.0000
SB10	Span 2	End	UDCONU2	-2.7388	0.0794
SB10	Span 3	Start	UDCONU2	-2.7388	0.0794
SB10	Span 3	Middle	UDCONU2	-2.4792	0.0736
SB10	Span 3	End	UDCONU2	-2.6412	0.0808

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB10	Span 4	Start	UDCONU2	-2.6412	0.0808
SB10	Span 4	Middle	UDCONU2	-10.1811	0.2806
SB10	Span 4	End	UDCONU2	-22.1367	0.6099
SB10	Span 5	Start	UDCONU2	-22.1367	0.6099
SB10	Span 5	Middle	UDCONU1	-0.3778	0.0000
SB10	Span 5	End		0.0000	0.0000
SB10	Span 6	Start		0.0000	0.0000
SB10	Span 6	Middle		0.0000	0.0000
SB10	Span 6	End	UDCONU2	-0.7176	0.0000
SB10	Span 7	Start	UDCONU2	-0.7176	0.0000
SB10	Span 7	Middle	UDCONU2	-7.0434	0.2064
SB10	Span 7	End	UDCONU2	-30.6605	0.8621
SB10	Span 8	Start		0.0000	0.0000
SB10	Span 8	Middle		0.0000	0.0000
SB10	Span 8	End		0.0000	0.0000
SB10	Span 9	Start		0.0000	0.0000
SB10	Span 9	Middle		0.0000	0.0000
SB10	Span 9	End		0.0000	0.0000
SB10	Span 10	Start		0.0000	0.0000
SB10	Span 10	Middle		0.0000	0.0000
SB10	Span 10	End		0.0000	0.0000
SB10	Span 11	Start		0.0000	0.0000
SB10	Span 11	Middle		0.0000	0.0000
SB10	Span 11	End	UDCONU2	-5.3461	0.1465
SB10	Span 12	Start	UDCONU2	-5.3461	0.1465
SB10	Span 12	Middle	UDCONU2	-16.7959	0.4878
SB10	Span 12	End	UDCONU2	-14.4637	0.4422
SB10	Span 13	Start	UDCONU2	-6.2427	0.2167
SB10	Span 13	Middle	UDCONU2	-6.4898	0.2234
SB10	Span 13	End	UDCONU2	-14.5455	0.4432
SB10	Span 14	Start	UDCONU2	-14.5455	0.4432
SB10	Span 14	Middle		0.0000	0.0000
SB10	Span 14	End	UDCONU2	-22.7631	0.6546
SB10	Span 15	Start	UDCONU2	-22.7631	0.6546
SB10	Span 15	Middle	UDCONU2	-12.2305	0.3466
SB10	Span 15	End	UDCONU2	-3.2278	0.0902
SB11	Span 1	Start	UDCONU2	-1.2888	0.0411
SB11	Span 1	Middle	UDCONU2	-0.9473	0.0326
SB11	Span 1	End	UDCONU2	-12.4249	0.3412
SB11	Span 2	Start	UDCONU2	-12.4249	0.3412
SB11	Span 2	Middle		0.0000	0.0000
SB11	Span 2	End		0.0000	0.0000
SB11	Span 3	Start		0.0000	0.0000
SB11	Span 3	Middle		0.0000	0.0000
SB11	Span 3	End		0.0000	0.0000
SB11	Span 4	Start		0.0000	0.0000
SB11	Span 4	Middle	UDCONU2	-26.2091	0.7319
SB11	Span 4	End	UDCONU2	-88.8128	2.5061
SB11	Span 5	Start	UDCONU2	-88.8128	2.5061
SB11	Span 5	Middle		0.0000	0.0000
SB11	Span 5	End	UDCONU2	-93.9730	2.6553
SB11	Span 6	Start	UDCONU2	-93.9730	2.6553
SB11	Span 6	Middle		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB11	Span 6	End	UDCONU2	-93.9826	2.6607
SB11	Span 7	Start	UDCONU2	-93.9826	2.6607
SB11	Span 7	Middle		0.0000	0.0000
SB11	Span 7	End	UDCONU2	-77.6342	2.1893
SB11	Span 8	Start	UDCONU2	-77.6342	2.1893
SB11	Span 8	Middle		0.0000	0.0000
SB11	Span 8	End	UDCONU2	-37.0618	1.0739
SB11	Span 9	Start	UDCONU2	-37.0618	1.0739
SB11	Span 9	Middle	UDCONU2	-36.8645	1.0757
SB11	Span 9	End	UDCONU2	-37.5428	1.0790
SB11	Span 10	Start	UDCONU2	-37.5428	1.0790
SB11	Span 10	Middle		0.0000	0.0000
SB11	Span 10	End	UDCONU2	-39.4729	1.1104
SB11	Span 11	Start	UDCONU2	-39.4729	1.1104
SB11	Span 11	Middle	UDCONU2	-29.6888	0.8273
SB11	Span 11	End	UDCONU2	-1.5427	0.0439
SB11	Span 12	Start	UDCONU2	-1.5427	0.0439
SB11	Span 12	Middle	UDCONU2	-3.0155	0.0990
SB11	Span 12	End	UDCONU2	-2.3485	0.0710
SB11	Span 13	Start	UDCONU2	-2.3485	0.0710
SB11	Span 13	Middle		0.0000	0.0000
SB11	Span 13	End	UDCONU2	-1.3262	0.0438
SB11	Span 14	Start	UDCONU2	-1.3262	0.0438
SB11	Span 14	Middle	UDCONU2	-0.4892	0.0134
SB11	Span 14	End	UDCONU2	-0.8688	0.0243
SB11	Span 15	Start	UDCONU2	-0.8688	0.0243
SB11	Span 15	Middle	UDCONU2	-0.3089	0.0085
SB11	Span 15	End	UDCONU2	-0.2980	0.0082
SB11	Span 16	Start	UDCONU2	-0.2980	0.0082
SB11	Span 16	Middle	UDCONU2	-0.3018	0.0083
SB11	Span 16	End	UDCONU2	-1.5452	0.0424
SB11	Span 17	Start	UDCONU2	-21.3509	0.6070
SB11	Span 17	Middle	UDCONU2	-21.9482	0.6047
SB11	Span 17	End	UDCONU2	-17.1049	0.4705
SB11	Span 18	Start	UDCONU2	-17.1049	0.4705
SB11	Span 18	Middle		0.0000	0.0000
SB11	Span 18	End	UDCONU2	-18.0296	0.4961
SB11	Span 19	Start	UDCONU2	-18.0296	0.4961
SB11	Span 19	Middle		0.0000	0.0000
SB11	Span 19	End	UDCONU2	-14.8435	0.4080
SB11	Span 20	Start	UDCONU2	-14.8435	0.4080
SB11	Span 20	Middle		0.0000	0.0000
SB11	Span 20	End	UDCONU2	-1.9661	0.0538
SB11	Span 21	Start	UDCONU2	-1.9661	0.0538
SB11	Span 21	Middle		0.0000	0.0000
SB11	Span 21	End	UDCONU2	-19.7154	0.5428
SB11	Span 22	Start	UDCONU2	-19.7154	0.5428
SB11	Span 22	Middle		0.0000	0.0000
SB11	Span 22	End	UDCONU2	-17.9437	0.4995
SB11	Span 23	Start	UDCONU2	-17.9437	0.4995
SB11	Span 23	Middle	UDCONU2	-16.2503	0.4521
SB11	Span 23	End	UDCONU2	-12.7020	0.3534
SB11	Span 24	Start	UDCONU2	-12.7020	0.3534

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB11	Span 24	Middle	UDCONU2	-19.7121	0.5499
SB11	Span 24	End	UDCONU2	-20.0404	0.5573
SB11	Span 25	Start	UDCONU2	-20.0404	0.5573
SB11	Span 25	Middle		0.0000	0.0000
SB11	Span 25	End	UDCONU2	-14.3473	0.3986
SB11	Span 26	Start	UDCONU2	-14.3473	0.3986
SB11	Span 26	Middle	UDCONU2	-7.3313	0.2037
SB11	Span 26	End	UDCONU2	-4.6163	0.1269
SB12	Span 1	Start	UDCONU2	-3.0171	0.0907
SB12	Span 1	Middle		0.0000	0.0000
SB12	Span 1	End	UDCONU2	-3.5543	0.1018
SB12	Span 2	Start	UDCONU2	-3.5543	0.1018
SB12	Span 2	Middle	UDCONU1	-3.5686	0.1093
SB12	Span 2	End	UDCONU2	-4.8588	0.1470
SB12	Span 3	Start	UDCONU2	-4.8588	0.1470
SB12	Span 3	Middle	UDCONU2	-6.3858	0.1750
SB12	Span 3	End	UDCONU2	-17.3993	0.4786
SB12	Span 4	Start	UDCONU2	-17.3993	0.4786
SB12	Span 4	Middle	UDCONU1	-2.6674	0.0730
SB12	Span 4	End		0.0000	0.0000
SB12	Span 5	Start		0.0000	0.0000
SB12	Span 5	Middle		0.0000	0.0000
SB12	Span 5	End		0.0000	0.0000
SB12	Span 6	Start		0.0000	0.0000
SB12	Span 6	Middle	UDCONU2	-47.4226	1.3337
SB12	Span 6	End	UDCONU2	-110.1727	3.1387
SB12	Span 7	Start	UDCONU2	-110.1727	3.1387
SB12	Span 7	Middle		0.0000	0.0000
SB12	Span 7	End	UDCONU2	-77.4454	2.1797
SB12	Span 8	Start	UDCONU2	-77.4454	2.1797
SB12	Span 8	Middle	UDCONU2	-52.5910	1.4689
SB12	Span 8	End	UDCONU2	-49.5113	1.3824
SB12	Span 9	Start		0.0000	0.0000
SB12	Span 9	Middle		0.0000	0.0000
SB12	Span 9	End		0.0000	0.0000
SB12	Span 10	Start		0.0000	0.0000
SB12	Span 10	Middle		0.0000	0.0000
SB12	Span 10	End		0.0000	0.0000
SB12	Span 11	Start		0.0000	0.0000
SB12	Span 11	Middle		0.0000	0.0000
SB12	Span 11	End	UDCONU2	-27.4225	0.7726
SB12	Span 12	Start	UDCONU2	-27.4225	0.7726
SB12	Span 12	Middle		0.0000	0.0000
SB12	Span 12	End	UDCONU2	-35.2879	0.9766
SB12	Span 13	Start	UDCONU2	-35.2879	0.9766
SB12	Span 13	Middle		0.0000	0.0000
SB12	Span 13	End	UDCONU2	-14.9765	0.4117
SB12	Span 14	Start	UDCONU2	-14.9765	0.4117
SB12	Span 14	Middle	UDCONU2	-1.4838	0.0406
SB12	Span 14	End	UDCONU2	-0.8815	0.0000
SB12	Span 15	Start	UDCONU2	-0.8815	0.0000
SB12	Span 15	Middle	UDCONU2	-25.7606	0.7107
SB12	Span 15	End	UDCONU2	-58.4453	1.6306

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB12	Span 16	Start	UDCONU2	-58.4453	1.6306
SB12	Span 16	Middle		0.0000	0.0000
SB12	Span 16	End	UDCONU2	-54.7187	1.5264
SB12	Span 17	Start	UDCONU2	-54.7187	1.5264
SB12	Span 17	Middle		0.0000	0.0000
SB12	Span 17	End	UDCONU2	-53.5788	1.4943
SB12	Span 18	Start	UDCONU2	-53.5788	1.4943
SB12	Span 18	Middle	UDCONU2	-50.1655	1.3982
SB12	Span 18	End	UDCONU2	-35.3976	0.9834
SB12	Span 19	Start	UDCONU2	-35.3976	0.9834
SB12	Span 19	Middle		0.0000	0.0000
SB12	Span 19	End	UDCONU2	-33.3445	0.9247
SB12	Span 20	Start	UDCONU2	-33.3445	0.9247
SB12	Span 20	Middle	UDCONU2	-18.6169	0.5148
SB12	Span 20	End	UDCONU2	-15.8111	0.4357
SB13	Span 1	Start	UDCONU2	-5.0947	0.1413
SB13	Span 1	Middle	UDCONU2	-6.6552	0.1870
SB13	Span 1	End	UDCONU2	-56.1568	1.5778
SB13	Span 2	Start	UDCONU2	-56.1568	1.5778
SB13	Span 2	Middle		0.0000	0.0000
SB13	Span 2	End	UDCONU2	-49.7762	1.3968
SB13	Span 3	Start	UDCONU2	-49.7762	1.3968
SB13	Span 3	Middle		0.0000	0.0000
SB13	Span 3	End	UDCONU2	-35.3196	0.9871
SB13	Span 4	Start	UDCONU2	-35.3196	0.9871
SB13	Span 4	Middle	UDCONU2	-12.4739	0.3426
SB13	Span 4	End		0.0000	0.0000
SB13	Span 5	Start		0.0000	0.0000
SB13	Span 5	Middle		0.0000	0.0000
SB13	Span 5	End	UDCONU2	-2.1576	0.0591
SB13	Span 6	Start	UDCONU2	-2.1576	0.0591
SB13	Span 6	Middle	UDCONU2	-71.1961	2.0015
SB13	Span 6	End	UDCONU2	-133.1186	3.8166
SB13	Span 7	Start	UDCONU2	-133.1186	3.8166
SB13	Span 7	Middle		0.0000	0.0000
SB13	Span 7	End	UDCONU2	-84.7580	2.3892
SB13	Span 8	Start	UDCONU2	-84.7580	2.3892
SB13	Span 8	Middle	UDCONU2	-46.2038	1.2881
SB13	Span 8	End	UDCONU2	-28.8158	0.7989
SB13	Span 9	Start		0.0000	0.0000
SB13	Span 9	Middle		0.0000	0.0000
SB13	Span 9	End		0.0000	0.0000
SB13	Span 10	Start		0.0000	0.0000
SB13	Span 10	Middle		0.0000	0.0000
SB13	Span 10	End		0.0000	0.0000
SB13	Span 11	Start		0.0000	0.0000
SB13	Span 11	Middle	UDCONU2	-29.6471	0.8374
SB13	Span 11	End	UDCONU2	-11.5967	0.3227
SB13	Span 12	Start	UDCONU2	-11.5967	0.3227
SB13	Span 12	Middle		0.0000	0.0000
SB13	Span 12	End		0.0000	0.0000
SB13	Span 13	Start		0.0000	0.0000
SB13	Span 13	Middle	UDCONU2	-6.7974	0.1863

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB13	Span 13	End	UDCONU2	-39.5672	1.0998
SB13	Span 14	Start	UDCONU2	-39.5672	1.0998
SB13	Span 14	Middle		0.0000	0.0000
SB13	Span 14	End	UDCONU2	-57.7942	1.6121
SB13	Span 15	Start	UDCONU2	-57.7942	1.6121
SB13	Span 15	Middle	UDCONU2	-30.1781	0.8348
SB13	Span 15	End	UDCONU2	-21.6811	0.5983
SB14	Span 1	Start	UDCONU2	-9.4564	0.2625
SB14	Span 1	Middle	UDCONU2	-16.1353	0.4437
SB14	Span 1	End	UDCONU1	-2.6023	0.0712
SB14	Span 2	Start	UDCONU1	-2.6023	0.0712
SB14	Span 2	Middle		0.0000	0.0000
SB14	Span 2	End	UDCONU2	-2.0291	0.0555
SB14	Span 3	Start	UDCONU2	-1.2328	0.0337
SB14	Span 3	Middle		0.0000	0.0000
SB14	Span 3	End	UDCONU2	-5.5597	0.1523
SB14	Span 4	Start	UDCONU2	-5.5597	0.1523
SB14	Span 4	Middle	UDCONU2	-63.9712	1.7882
SB14	Span 4	End	UDCONU2	-115.0713	3.2782
SB14	Span 5	Start	UDCONU2	-115.0713	3.2782
SB14	Span 5	Middle		0.0000	0.0000
SB14	Span 5	End	UDCONU2	-75.0796	2.1099
SB14	Span 6	Start	UDCONU2	-75.0796	2.1099
SB14	Span 6	Middle	UDCONU2	-36.0967	1.0039
SB14	Span 6	End	UDCONU2	-0.6314	0.0179
SB14	Span 7	Start		0.0000	0.0000
SB14	Span 7	Middle		0.0000	0.0000
SB14	Span 7	End		0.0000	0.0000
SB14	Span 8	Start		0.0000	0.0000
SB14	Span 8	Middle		0.0000	0.0000
SB14	Span 8	End		0.0000	0.0000
SB14	Span 9	Start		0.0000	0.0000
SB14	Span 9	Middle	UDCONU2	-46.7203	1.3000
SB14	Span 9	End	UDCONU2	-9.6012	0.2667
SB14	Span 10	Start	UDCONU2	-9.6012	0.2667
SB14	Span 10	Middle		0.0000	0.0000
SB14	Span 10	End		0.0000	0.0000
SB14	Span 11	Start		0.0000	0.0000
SB14	Span 11	Middle	UDCONU2	-2.6762	0.0733
SB14	Span 11	End	UDCONU2	-30.1143	0.8424
SB14	Span 12	Start	UDCONU2	-30.1143	0.8424
SB14	Span 12	Middle		0.0000	0.0000
SB14	Span 12	End	UDCONU2	-59.9291	1.6746
SB14	Span 13	Start	UDCONU2	-59.9291	1.6746
SB14	Span 13	Middle	UDCONU2	-25.9509	0.7211
SB14	Span 13	End	UDCONU2	-17.5796	0.4858
SB15	Span 1	Start	UDCONU2	-12.5284	0.3570
SB15	Span 1	Middle		0.0000	0.0000
SB15	Span 1	End	UDCONU2	-12.6350	0.3795
SB15	Span 2	Start	UDCONU2	-12.6350	0.3795
SB15	Span 2	Middle	UDCONU2	-9.7746	0.2873
SB15	Span 2	End	UDCONU2	-36.4566	1.0094
SB15	Span 3	Start	UDCONU2	-36.4566	1.0094

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB15	Span 3	Middle	UDCONU2	-48.1773	1.3393
SB15	Span 3	End	UDCONU2	-14.8350	0.4078
SB15	Span 4	Start	UDCONU2	-14.8350	0.4078
SB15	Span 4	Middle		0.0000	0.0000
SB15	Span 4	End	UDCONU2	-2.2418	0.0614
SB15	Span 5	Start	UDCONU2	-2.2418	0.0614
SB15	Span 5	Middle	UDCONU2	-58.4960	1.6408
SB15	Span 5	End	UDCONU2	-114.4285	3.2579
SB15	Span 6	Start	UDCONU2	-114.4285	3.2579
SB15	Span 6	Middle	UDCONU2	-109.4691	3.1114
SB15	Span 6	End	UDCONU2	-93.5749	2.6436
SB15	Span 7	Start	UDCONU2	-93.5749	2.6436
SB15	Span 7	Middle		0.0000	0.0000
SB15	Span 7	End	UDCONU2	-84.2493	2.3722
SB15	Span 8	Start	UDCONU2	-84.2493	2.3722
SB15	Span 8	Middle	UDCONU2	-55.7098	1.5528
SB15	Span 8	End	UDCONU2	-19.0805	0.5267
SB15	Span 9	Start		0.0000	0.0000
SB15	Span 9	Middle		0.0000	0.0000
SB15	Span 9	End		0.0000	0.0000
SB15	Span 10	Start		0.0000	0.0000
SB15	Span 10	Middle		0.0000	0.0000
SB15	Span 10	End		0.0000	0.0000
SB15	Span 11	Start		0.0000	0.0000
SB15	Span 11	Middle	UDCONU2	-82.1792	2.3121
SB15	Span 11	End	UDCONU2	-19.4021	0.5341
SB15	Span 12	Start	UDCONU2	-19.4021	0.5341
SB15	Span 12	Middle		0.0000	0.0000
SB15	Span 12	End		0.0000	0.0000
SB15	Span 13	Start		0.0000	0.0000
SB15	Span 13	Middle		0.0000	0.0000
SB15	Span 13	End	UDCONU2	-20.3326	0.5735
SB15	Span 14	Start	UDCONU2	-20.3326	0.5735
SB15	Span 14	Middle	UDCONU2	-56.8200	1.5998
SB15	Span 14	End	UDCONU2	-11.5933	0.3202
SB16	Span 1	Start	UDCONU2	-13.2685	0.3678
SB16	Span 1	Middle	UDCONU2	-105.3639	2.9896
SB16	Span 1	End	UDCONU2	-54.5528	1.5199
SB16	Span 2	Start	UDCONU2	-11.1144	0.3051
SB16	Span 2	Middle		0.0000	0.0000
SB16	Span 2	End		0.0000	0.0000
SB16	Span 3	Start		0.0000	0.0000
SB16	Span 3	Middle	UDCONU2	-34.1171	0.9566
SB16	Span 3	End	UDCONU2	-106.0345	3.0183
SB16	Span 4	Start	UDCONU2	-106.0345	3.0183
SB16	Span 4	Middle		0.0000	0.0000
SB16	Span 4	End	UDCONU2	-59.8486	1.6782
SB16	Span 5	Start	UDCONU2	-59.8486	1.6782
SB16	Span 5	Middle	UDCONU2	-31.4773	0.8732
SB16	Span 5	End	UDCONU2	-4.7096	0.1309
SB16	Span 6	Start	UDCONU2	-4.3701	0.1217
SB16	Span 6	Middle		0.0000	0.0000
SB16	Span 6	End	UDCONU2	-2.0879	0.0591

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB16	Span 7	Start	UDCONU2	-2.0879	0.0591
SB16	Span 7	Middle	UDCONU2	-0.2224	0.0078
SB16	Span 7	End		0.0000	0.0000
SB16	Span 8	Start		0.0000	0.0000
SB16	Span 8	Middle		0.0000	0.0000
SB16	Span 8	End		0.0000	0.0000
SB16	Span 9	Start	UDCONU2	-11.9364	0.3326
SB16	Span 9	Middle	UDCONU2	-122.3629	3.4940
SB16	Span 9	End	UDCONU2	-44.3334	1.2308
SB16	Span 10	Start	UDCONU2	-44.3334	1.2308
SB16	Span 10	Middle	UDCONU2	-0.3957	0.0000
SB16	Span 10	End		0.0000	0.0000
SB16	Span 11	Start		0.0000	0.0000
SB16	Span 11	Middle		0.0000	0.0000
SB16	Span 11	End	UDCONU2	-14.1326	0.3967
SB16	Span 12	Start	UDCONU2	-14.1326	0.3967
SB16	Span 12	Middle	UDCONU2	-40.5463	1.1332
SB16	Span 12	End	UDCONU2	-4.0159	0.1110
SB17	Span 1	Start	UDCONU2	-2.8567	0.0819
SB17	Span 1	Middle		0.0000	0.0000
SB17	Span 1	End	UDCONU2	-2.9786	0.1308
SB17	Span 2	Start	UDCONU2	-2.9786	0.1308
SB17	Span 2	Middle	UDCONU2	-15.2850	0.4597
SB17	Span 2	End	UDCONU2	-79.6037	2.2397
SB17	Span 3	Start	UDCONU2	-79.6037	2.2397
SB17	Span 3	Middle		0.0000	0.0000
SB17	Span 3	End	UDCONU2	-85.8209	2.4216
SB17	Span 4	Start	UDCONU2	-85.8209	2.4216
SB17	Span 4	Middle	UDCONU2	-37.8988	1.0628
SB17	Span 4	End	UDCONU2	-7.4798	0.2063
SB17	Span 5	Start	UDCONU2	-7.4798	0.2063
SB17	Span 5	Middle		0.0000	0.0000
SB17	Span 5	End		0.0000	0.0000
SB17	Span 6	Start		0.0000	0.0000
SB17	Span 6	Middle	UDCONU2	-37.2229	1.0564
SB17	Span 6	End	UDCONU2	-83.7048	2.3669
SB17	Span 7	Start	UDCONU2	-83.7048	2.3669
SB17	Span 7	Middle		0.0000	0.0000
SB17	Span 7	End	UDCONU2	-41.3859	1.1564
SB17	Span 8	Start	UDCONU2	-41.3859	1.1564
SB17	Span 8	Middle	UDCONU2	-24.5899	0.6822
SB17	Span 8	End	UDCONU2	-10.5440	0.2945
SB17	Span 9	Start	UDCONU2	-10.5440	0.2945
SB17	Span 9	Middle	UDCONU2	-3.1524	0.0948
SB17	Span 9	End	UDCONU2	-2.4503	0.0786
SB17	Span 10	Start	UDCONU2	-2.4503	0.0786
SB17	Span 10	Middle		0.0000	0.0000
SB17	Span 10	End	UDCONU2	-2.7993	0.0835
SB17	Span 11	Start	UDCONU2	-2.7993	0.0835
SB17	Span 11	Middle	UDCONU2	-0.1317	0.0000
SB17	Span 11	End	UDCONU2	-0.7741	0.0281
SB17	Span 12	Start	UDCONU2	-0.7741	0.0281
SB17	Span 12	Middle	UDCONU2	-2.0938	0.0625

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB17	Span 12	End		0.0000	0.0000
SB17	Span 13	Start		0.0000	0.0000
SB17	Span 13	Middle	UDCONU2	-53.2391	1.4955
SB17	Span 13	End	UDCONU2	-107.5368	3.0830
SB17	Span 14	Start	UDCONU2	-107.5368	3.0830
SB17	Span 14	Middle		0.0000	0.0000
SB17	Span 14	End	UDCONU2	-157.3566	4.5540
SB17	Span 15	Start	UDCONU2	-157.3566	4.5540
SB17	Span 15	Middle		0.0000	0.0000
SB17	Span 15	End	UDCONU2	-65.1219	1.8211
SB17	Span 16	Start	UDCONU2	-65.1219	1.8211
SB17	Span 16	Middle	UDCONU2	-4.0242	0.1102
SB17	Span 16	End		0.0000	0.0000
SB17	Span 17	Start		0.0000	0.0000
SB17	Span 17	Middle		0.0000	0.0000
SB17	Span 17	End	UDCONU2	-13.3916	0.3718
SB17	Span 18	Start	UDCONU2	-13.3916	0.3718
SB17	Span 18	Middle	UDCONU2	-38.3855	1.0704
SB17	Span 18	End	UDCONU2	-3.9537	0.1090
SB18	Span 1	Start	UDCONU2	-5.6273	0.1583
SB18	Span 1	Middle	UDCONU2	-27.9210	0.7809
SB18	Span 1	End	UDCONU2	-71.9194	2.0279
SB18	Span 2	Start	UDCONU2	-71.9194	2.0279
SB18	Span 2	Middle	UDCONU2	-37.7898	1.0641
SB18	Span 2	End	UDCONU2	-11.3865	0.3264
SB18	Span 3	Start	UDCONU2	-11.3865	0.3264
SB18	Span 3	Middle		0.0000	0.0000
SB18	Span 3	End		0.0000	0.0000
SB18	Span 4	Start		0.0000	0.0000
SB18	Span 4	Middle	UDCONU2	-63.1010	1.7634
SB18	Span 4	End	UDCONU2	-141.0059	4.0641
SB18	Span 5	Start	UDCONU2	-141.0059	4.0641
SB18	Span 5	Middle		0.0000	0.0000
SB18	Span 5	End	UDCONU2	-102.9981	2.9832
SB18	Span 6	Start	UDCONU2	-102.9981	2.9832
SB18	Span 6	Middle		0.0000	0.0000
SB18	Span 6	End	UDCONU2	-32.5862	0.9195
SB18	Span 7	Start	UDCONU2	-32.5862	0.9195
SB18	Span 7	Middle	UDCONU2	-16.5796	0.4596
SB18	Span 7	End	UDCONU2	-17.6480	0.4887
SB18	Span 8	Start	UDCONU2	-17.6480	0.4887
SB18	Span 8	Middle	UDCONU2	-16.6904	0.4601
SB18	Span 8	End	UDCONU2	-16.9138	0.4652
SB18	Span 9	Start	UDCONU2	-16.9138	0.4652
SB18	Span 9	Middle		0.0000	0.0000
SB18	Span 9	End	UDCONU2	-15.5586	0.4277
SB18	Span 10	Start	UDCONU2	-15.5586	0.4277
SB18	Span 10	Middle	UDCONU2	-10.3020	0.2827
SB18	Span 10	End	UDCONU2	-16.7724	0.4750
SB18	Span 11	Start	UDCONU2	-16.7724	0.4750
SB18	Span 11	Middle	UDCONU2	-16.6531	0.4683
SB18	Span 11	End	UDCONU2	-21.4202	0.5984
SB18	Span 12	Start	UDCONU2	-21.4202	0.5984

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB18	Span 12	Middle		0.0000	0.0000
SB18	Span 12	End	UDCONU2	-159.6651	4.6313
SB18	Span 13	Start	UDCONU2	-159.6651	4.6313
SB18	Span 13	Middle		0.0000	0.0000
SB18	Span 13	End	UDCONU2	-54.9676	1.5440
SB18	Span 14	Start	UDCONU2	-54.9676	1.5440
SB18	Span 14	Middle	UDCONU2	-5.0210	0.1461
SB18	Span 14	End		0.0000	0.0000
SB18	Span 15	Start		0.0000	0.0000
SB18	Span 15	Middle		0.0000	0.0000
SB18	Span 15	End	UDCONU2	-14.6241	0.4028
SB18	Span 16	Start	UDCONU2	-14.6241	0.4028
SB18	Span 16	Middle	UDCONU2	-49.6193	1.3840
SB18	Span 16	End	UDCONU2	-10.6391	0.2944
SB19	Span 1	Start	UDCONU2	-3.3969	0.0964
SB19	Span 1	Middle	UDCONU2	-28.7999	0.8094
SB19	Span 1	End	UDCONU2	-90.6038	2.5733
SB19	Span 2	Start	UDCONU2	-90.6038	2.5733
SB19	Span 2	Middle		0.0000	0.0000
SB19	Span 2	End	UDCONU2	-108.1373	3.0921
SB19	Span 3	Start	UDCONU2	-108.1373	3.0921
SB19	Span 3	Middle	UDCONU2	-51.6978	1.4471
SB19	Span 3	End	UDCONU2	-11.6543	0.3303
SB19	Span 4	Start	UDCONU2	-11.6543	0.3303
SB19	Span 4	Middle		0.0000	0.0000
SB19	Span 4	End		0.0000	0.0000
SB19	Span 5	Start		0.0000	0.0000
SB19	Span 5	Middle	UDCONU2	-37.9982	1.0714
SB19	Span 5	End	UDCONU2	-150.7787	4.3629
SB19	Span 6	Start	UDCONU2	-150.7787	4.3629
SB19	Span 6	Middle	UDCONU2	-44.2144	1.2469
SB19	Span 6	End	UDCONU2	-19.8759	0.5493
SB19	Span 7	Start	UDCONU2	-19.8759	0.5493
SB19	Span 7	Middle		0.0000	0.0000
SB19	Span 7	End	UDCONU2	-20.0650	0.5539
SB19	Span 8	Start	UDCONU2	-20.0650	0.5539
SB19	Span 8	Middle	UDCONU2	-12.5267	0.3441
SB19	Span 8	End	UDCONU2	-20.1599	0.5560
SB19	Span 9	Start	UDCONU2	-20.1599	0.5560
SB19	Span 9	Middle		0.0000	0.0000
SB19	Span 9	End	UDCONU2	-23.4666	0.6481
SB19	Span 10	Start	UDCONU2	-23.4666	0.6481
SB19	Span 10	Middle		0.0000	0.0000
SB19	Span 10	End	UDCONU2	-19.8050	0.5464
SB19	Span 11	Start	UDCONU2	-19.8050	0.5464
SB19	Span 11	Middle		0.0000	0.0000
SB19	Span 11	End	UDCONU2	-20.6673	0.5697
SB19	Span 12	Start	UDCONU2	-20.6673	0.5697
SB19	Span 12	Middle	UDCONU2	-10.2133	0.2803
SB19	Span 12	End	UDCONU2	-18.8933	0.5203
SB19	Span 13	Start	UDCONU2	-18.8933	0.5203
SB19	Span 13	Middle		0.0000	0.0000
SB19	Span 13	End	UDCONU2	-20.7673	0.5719

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB19	Span 14	Start	UDCONU2	-6.7798	0.1865
SB19	Span 14	Middle		0.0000	0.0000
SB19	Span 14	End	UDCONU2	-14.5179	0.4021
SB19	Span 15	Start	UDCONU2	-14.5179	0.4021
SB19	Span 15	Middle		0.0000	0.0000
SB19	Span 15	End	UDCONU2	-20.0093	0.5570
SB19	Span 16	Start	UDCONU2	-20.0093	0.5570
SB19	Span 16	Middle	UDCONU2	-6.1209	0.1683
SB19	Span 16	End	UDCONU2	-6.5845	0.1811
SB19	Span 17	Start	UDCONU2	-6.5845	0.1811
SB19	Span 17	Middle		0.0000	0.0000
SB19	Span 17	End	UDCONU2	-11.2555	0.3109
SB19	Span 18	Start	UDCONU2	-11.2555	0.3109
SB19	Span 18	Middle		0.0000	0.0000
SB19	Span 18	End	UDCONU2	-15.5594	0.4314
SB19	Span 19	Start	UDCONU2	-16.9008	0.4729
SB19	Span 19	Middle	UDCONU2	-10.8993	0.3082
SB19	Span 19	End	UDCONU2	-102.5274	2.9090
SB19	Span 20	Start	UDCONU2	-102.5274	2.9090
SB19	Span 20	Middle		0.0000	0.0000
SB19	Span 20	End	UDCONU2	-39.2567	1.0967
SB19	Span 21	Start	UDCONU2	-39.2567	1.0967
SB19	Span 21	Middle	UDCONU2	-6.4387	0.1930
SB19	Span 21	End		0.0000	0.0000
SB19	Span 22	Start		0.0000	0.0000
SB19	Span 22	Middle		0.0000	0.0000
SB19	Span 22	End	UDCONU2	-21.6723	0.6006
SB19	Span 23	Start	UDCONU2	-21.6723	0.6006
SB19	Span 23	Middle		0.0000	0.0000
SB19	Span 23	End	UDCONU2	-61.1179	1.7201
SB19	Span 24	Start	UDCONU2	-61.1179	1.7201
SB19	Span 24	Middle	UDCONU2	-19.4436	0.5448
SB19	Span 24	End	UDCONU2	-13.0230	0.3600
SB20	Span 1	Start	UDCONU2	-7.5973	0.2128
SB20	Span 1	Middle	UDCONU2	-30.6515	0.8480
SB20	Span 1	End	UDCONU2	-66.8105	1.8695
SB20	Span 2	Start	UDCONU2	-66.8105	1.8695
SB20	Span 2	Middle	UDCONU2	-33.3009	0.9237
SB20	Span 2	End	UDCONU2	-6.4875	0.1803
SB20	Span 3	Start	UDCONU2	-6.4875	0.1803
SB20	Span 3	Middle		0.0000	0.0000
SB20	Span 3	End		0.0000	0.0000
SB20	Span 4	Start		0.0000	0.0000
SB20	Span 4	Middle	UDCONU2	-8.0161	0.2549
SB20	Span 4	End	UDCONU2	-33.5515	0.9649
SB20	Span 5	Start		0.0000	0.0000
SB20	Span 5	Middle	UDCONU2	-4.7987	0.1323
SB20	Span 5	End	UDCONU2	-8.5197	0.2344
SB20	Span 6	Start	UDCONU2	-8.5197	0.2344
SB20	Span 6	Middle	UDCONU2	-8.4993	0.2333
SB20	Span 6	End	UDCONU2	-21.6390	0.6050
SB20	Span 7	Start	UDCONU2	-21.6390	0.6050
SB20	Span 7	Middle		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB20	Span 7	End	UDCONU2	-7.9553	0.2252
SB20	Span 8	Start	UDCONU2	-6.3156	0.1794
SB20	Span 8	Middle		0.0000	0.0000
SB20	Span 8	End		0.0000	0.0000
SB20	Span 9	Start		0.0000	0.0000
SB20	Span 9	Middle	UDCONU2	-24.3098	0.6728
SB20	Span 9	End	UDCONU2	-40.1331	1.1126
SB20	Span 10	Start	UDCONU2	-40.1331	1.1126
SB20	Span 10	Middle		0.0000	0.0000
SB20	Span 10	End	UDCONU2	-28.3409	0.8081
SB20	Span 11	Start	UDCONU2	-28.3409	0.8081
SB20	Span 11	Middle	UDCONU2	-9.6683	0.3078
SB20	Span 11	End		0.0000	0.0000
SB20	Span 12	Start		0.0000	0.0000
SB20	Span 12	Middle		0.0000	0.0000
SB20	Span 12	End	UDCONU2	-13.2846	0.3677
SB20	Span 13	Start	UDCONU2	-13.2846	0.3677
SB20	Span 13	Middle		0.0000	0.0000
SB20	Span 13	End	UDCONU2	-29.1843	0.8106
SB20	Span 14	Start	UDCONU2	-29.1843	0.8106
SB20	Span 14	Middle	UDCONU2	-16.3768	0.4533
SB20	Span 14	End	UDCONU2	-11.4661	0.3158
SB21	Span 1	Start	UDCONU2	-3.1227	0.0866
SB21	Span 1	Middle	UDCONU2	-25.6326	0.7071
SB21	Span 1	End	UDCONU2	-80.3976	2.2606
SB21	Span 2	Start	UDCONU2	-80.3976	2.2606
SB21	Span 2	Middle		0.0000	0.0000
SB21	Span 2	End	UDCONU2	-96.9184	2.7414
SB21	Span 3	Start	UDCONU2	-96.9184	2.7414
SB21	Span 3	Middle	UDCONU2	-47.9603	1.3332
SB21	Span 3	End	UDCONU2	-13.4608	0.3698
SB21	Span 4	Start	UDCONU2	-13.4608	0.3698
SB21	Span 4	Middle		0.0000	0.0000
SB21	Span 4	End		0.0000	0.0000
SB21	Span 5	Start		0.0000	0.0000
SB21	Span 5	Middle	UDCONU2	-6.7230	0.1843
SB21	Span 5	End	UDCONU2	-80.7358	2.2704
SB21	Span 6	Start	UDCONU2	-80.7358	2.2704
SB21	Span 6	Middle		0.0000	0.0000
SB21	Span 6	End	UDCONU2	-107.1804	3.0432
SB21	Span 7	Start	UDCONU2	-107.1804	3.0432
SB21	Span 7	Middle		0.0000	0.0000
SB21	Span 7	End	UDCONU2	-20.3613	0.6054
SB21	Span 8	Start	UDCONU2	-20.3613	0.6054
SB21	Span 8	Middle		0.0000	0.0000
SB21	Span 8	End		0.0000	0.0000
SB21	Span 9	Start		0.0000	0.0000
SB21	Span 9	Middle		0.0000	0.0000
SB21	Span 9	End		0.0000	0.0000
SB21	Span 10	Start		0.0000	0.0000
SB21	Span 10	Middle		0.0000	0.0000
SB21	Span 10	End		0.0000	0.0000
SB21	Span 11	Start		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB21	Span 11	Middle		0.0000	0.0000
SB21	Span 11	End		0.0000	0.0000
SB21	Span 12	Start		0.0000	0.0000
SB21	Span 12	Middle	UDCONU2	-0.2089	0.0000
SB21	Span 12	End	UDCONU2	-5.2119	0.1429
SB21	Span 13	Start	UDCONU2	-5.2119	0.1429
SB21	Span 13	Middle	UDCONU2	-1.6739	0.0458
SB21	Span 13	End	UDCONU2	-4.4056	0.1208
SB21	Span 14	Start	UDCONU2	-10.5361	0.2902
SB21	Span 14	Middle	UDCONU2	-57.2805	1.5974
SB21	Span 14	End	UDCONU2	-71.4097	2.0014
SB21	Span 15	Start	UDCONU2	-117.8464	3.3593
SB21	Span 15	Middle		0.0000	0.0000
SB21	Span 15	End	UDCONU2	-92.2781	2.8036
SB21	Span 16	Start	UDCONU2	-92.2781	2.8036
SB21	Span 16	Middle		0.0000	0.0000
SB21	Span 16	End	UDCONU2	-35.9459	1.2080
SB21	Span 17	Start	UDCONU2	-35.9459	1.2080
SB21	Span 17	Middle	UDCONU2	-34.2783	1.0741
SB21	Span 17	End	UDCONU2	-46.2321	1.3167
SB21	Span 18	Start	UDCONU2	-46.2321	1.3167
SB21	Span 18	Middle		0.0000	0.0000
SB21	Span 18	End	UDCONU2	-42.0025	1.1932
SB21	Span 19	Start	UDCONU2	-42.0025	1.1932
SB21	Span 19	Middle	UDCONU2	-3.8215	0.1079
SB21	Span 19	End	UDCONU2	-0.2649	0.0000
SB21	Span 20	Start	UDCONU2	-0.2649	0.0000
SB21	Span 20	Middle		0.0000	0.0000
SB21	Span 20	End	UDCONU2	-8.6964	0.2385
SB21	Span 21	Start	UDCONU2	-8.6964	0.2385
SB21	Span 21	Middle		0.0000	0.0000
SB21	Span 21	End	UDCONU2	-34.9787	0.9786
SB21	Span 22	Start	UDCONU2	-34.9787	0.9786
SB21	Span 22	Middle	UDCONU2	-15.8618	0.4400
SB21	Span 22	End	UDCONU2	-9.2309	0.2540
SB22	Span 1	Start	UDCONU2	-3.2831	0.0928
SB22	Span 1	Middle	UDCONU2	-15.9322	0.4472
SB22	Span 1	End	UDCONU2	-58.8726	1.6428
SB22	Span 2	Start	UDCONU2	-58.8726	1.6428
SB22	Span 2	Middle	UDCONU2	-47.8632	1.3318
SB22	Span 2	End	UDCONU2	-27.6923	0.7644
SB22	Span 3	Start	UDCONU2	-27.6923	0.7644
SB22	Span 3	Middle		0.0000	0.0000
SB22	Span 3	End		0.0000	0.0000
SB22	Span 4	Start		0.0000	0.0000
SB22	Span 4	Middle	UDCONU2	-4.4731	0.1225
SB22	Span 4	End	UDCONU2	-22.3192	0.6211
SB22	Span 5	Start	UDCONU2	-22.3192	0.6211
SB22	Span 5	Middle	UDCONU2	-13.4249	0.3756
SB22	Span 5	End		0.0000	0.0000
SB22	Span 6	Start		0.0000	0.0000
SB22	Span 6	Middle		0.0000	0.0000
SB22	Span 6	End		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB22	Span 7	Start		0.0000	0.0000
SB22	Span 7	Middle		0.0000	0.0000
SB22	Span 7	End		0.0000	0.0000
SB22	Span 8	Start		0.0000	0.0000
SB22	Span 8	Middle		0.0000	0.0000
SB22	Span 8	End	UDCONU2	-2.2687	0.0709
SB22	Span 9	Start	UDCONU2	-8.8627	0.2608
SB22	Span 9	Middle	UDCONU2	-64.0089	1.8620
SB22	Span 9	End	UDCONU2	-33.2993	1.0023
SB22	Span 10	Start	UDCONU2	-26.1042	0.7958
SB22	Span 10	Middle	UDCONU2	-22.2033	0.6858
SB22	Span 10	End	UDCONU2	-0.6397	0.0272
SB22	Span 11	Start	UDCONU2	-0.6397	0.0272
SB22	Span 11	Middle		0.0000	0.0000
SB22	Span 11	End	UDCONU2	-3.3798	0.0926
SB22	Span 12	Start	UDCONU2	-3.3798	0.0926
SB22	Span 12	Middle	UDCONU2	-5.5300	0.1517
SB22	Span 12	End	UDCONU2	-1.5840	0.0439
SB23	Span 1	Start	UDCONU2	-0.9832	0.0353
SB23	Span 1	Middle		0.0000	0.0000
SB23	Span 1	End	UDCONU2	-1.2660	0.0454
SB23	Span 2	Start	UDCONU2	-1.2660	0.0454
SB23	Span 2	Middle	UDCONU2	-3.5322	0.1473
SB23	Span 2	End	UDCONU2	-20.1139	0.6619
SB23	Span 3	Start	UDCONU2	-20.1139	0.6619
SB23	Span 3	Middle		0.0000	0.0000
SB23	Span 3	End	UDCONU2	-19.3722	0.6569
SB23	Span 4	Start	UDCONU2	-19.3722	0.6569
SB23	Span 4	Middle	UDCONU2	-17.0523	0.9032
SB23	Span 4	End	UDCONU2	-36.7149	1.0371
SB23	Span 5	Start	UDCONU2	-36.7149	1.0371
SB23	Span 5	Middle		0.0000	0.0000
SB23	Span 5	End	UDCONU2	-14.6017	0.4041
SB23	Span 6	Start	UDCONU2	-14.6017	0.4041
SB23	Span 6	Middle		0.0000	0.0000
SB23	Span 6	End		0.0000	0.0000
SB23	Span 7	Start		0.0000	0.0000
SB23	Span 7	Middle	UDCONU2	-1.7798	0.0487
SB23	Span 7	End	UDCONU2	-15.1300	0.4189
SB23	Span 8	Start	UDCONU2	-15.1300	0.4189
SB23	Span 8	Middle		0.0000	0.0000
SB23	Span 8	End	UDCONU2	-10.9575	0.3023
SB23	Span 9	Start	UDCONU2	-10.9575	0.3023
SB23	Span 9	Middle		0.0000	0.0000
SB23	Span 9	End	UDCONU2	-4.7116	0.1358
SB23	Span 10	Start	UDCONU2	-4.7116	0.1358
SB23	Span 10	Middle		0.0000	0.0000
SB23	Span 10	End		0.0000	0.0000
SB23	Span 11	Start		0.0000	0.0000
SB23	Span 11	Middle		0.0000	0.0000
SB23	Span 11	End		0.0000	0.0000
SB23	Span 12	Start		0.0000	0.0000
SB23	Span 12	Middle		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB23	Span 12	End		0.0000	0.0000
SB23	Span 13	Start		0.0000	0.0000
SB23	Span 13	Middle		0.0000	0.0000
SB23	Span 13	End		0.0000	0.0000
SB23	Span 14	Start		0.0000	0.0000
SB23	Span 14	Middle		0.0000	0.0000
SB23	Span 14	End		0.0000	0.0000
SB23	Span 15	Start		0.0000	0.0000
SB23	Span 15	Middle		0.0000	0.0000
SB23	Span 15	End		0.0000	0.0000
SB23	Span 16	Start		0.0000	0.0000
SB23	Span 16	Middle		0.0000	0.0000
SB23	Span 16	End		0.0000	0.0000
SB23	Span 17	Start		0.0000	0.0000
SB23	Span 17	Middle		0.0000	0.0000
SB23	Span 17	End		0.0000	0.0000
SB24	Span 1	Start	UDCONU2	-2.1232	0.0719
SB24	Span 1	Middle		0.0000	0.0000
SB24	Span 1	End	UDCONU2	-2.3651	0.0861
SB24	Span 2	Start	UDCONU2	-2.3651	0.0861
SB24	Span 2	Middle	UDCONU2	-0.2667	0.0385
SB24	Span 2	End	UDCONU2	-5.2434	0.1546
SB24	Span 3	Start	UDCONU2	-5.4032	0.1607
SB24	Span 3	Middle		0.0000	0.0000
SB24	Span 3	End		0.0000	0.0000
SB24	Span 4	Start		0.0000	0.0000
SB24	Span 4	Middle		0.0000	0.0000
SB24	Span 4	End		0.0000	0.0000
SB24	Span 5	Start		0.0000	0.0000
SB24	Span 5	Middle		0.0000	0.0000
SB24	Span 5	End		0.0000	0.0000
SB24	Span 6	Start		0.0000	0.0000
SB24	Span 6	Middle		0.0000	0.0000
SB24	Span 6	End		0.0000	0.0000
SB24	Span 7	Start		0.0000	0.0000
SB24	Span 7	Middle		0.0000	0.0000
SB24	Span 7	End		0.0000	0.0000
SB24	Span 8	Start		0.0000	0.0000
SB24	Span 8	Middle		0.0000	0.0000
SB24	Span 8	End		0.0000	0.0000
SB24	Span 9	Start		0.0000	0.0000
SB24	Span 9	Middle		0.0000	0.0000
SB24	Span 9	End		0.0000	0.0000
SB24	Span 10	Start		0.0000	0.0000
SB24	Span 10	Middle		0.0000	0.0000
SB24	Span 10	End		0.0000	0.0000
SB24	Span 11	Start		0.0000	0.0000
SB24	Span 11	Middle		0.0000	0.0000
SB24	Span 11	End		0.0000	0.0000
SB24	Span 12	Start		0.0000	0.0000
SB24	Span 12	Middle		0.0000	0.0000
SB24	Span 12	End		0.0000	0.0000
SB24	Span 13	Start		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB24	Span 13	Middle		0.0000	0.0000
SB24	Span 13	End		0.0000	0.0000
SB25	Span 1	Start	UDCONU2	-3.7249	0.1148
SB25	Span 1	Middle	UDCONU2	-55.1345	1.5364
SB25	Span 1	End	UDCONU2	-8.0430	0.2217
SB25	Span 2	Start	UDCONU2	-1.0650	0.0292
SB25	Span 2	Middle		0.0000	0.0000
SB25	Span 2	End		0.0000	0.0000
SB25	Span 3	Start		0.0000	0.0000
SB25	Span 3	Middle	UDCONU2	-6.1514	0.1693
SB25	Span 3	End	UDCONU2	-14.5669	0.4050
SB25	Span 4	Start	UDCONU2	-14.5669	0.4050
SB25	Span 4	Middle		0.0000	0.0000
SB25	Span 4	End	UDCONU2	-0.3708	0.0103
SB25	Span 5	Start	UDCONU2	-0.3708	0.0103
SB25	Span 5	Middle	UDCONU2	-0.0022	0.0000
SB25	Span 5	End		0.0000	0.0000
SB25	Span 6	Start		0.0000	0.0000
SB25	Span 6	Middle		0.0000	0.0000
SB25	Span 6	End		0.0000	0.0000
SB25	Span 7	Start		0.0000	0.0000
SB25	Span 7	Middle		0.0000	0.0000
SB25	Span 7	End		0.0000	0.0000
SB25	Span 8	Start		0.0000	0.0000
SB25	Span 8	Middle		0.0000	0.0000
SB25	Span 8	End		0.0000	0.0000
SB25	Span 9	Start		0.0000	0.0000
SB25	Span 9	Middle		0.0000	0.0000
SB25	Span 9	End		0.0000	0.0000
SB25	Span 10	Start		0.0000	0.0000
SB25	Span 10	Middle		0.0000	0.0000
SB25	Span 10	End		0.0000	0.0000
SB25	Span 11	Start		0.0000	0.0000
SB25	Span 11	Middle		0.0000	0.0000
SB25	Span 11	End		0.0000	0.0000
SB26	Span 1	Start	UDCONU2	-3.6668	0.1061
SB26	Span 1	Middle	UDCONU1	-25.2608	0.7031
SB26	Span 1	End	UDCONU2	-60.7183	1.6954
SB26	Span 2	Start	UDCONU2	-60.7183	1.6954
SB26	Span 2	Middle	UDCONU2	-14.1523	0.3904
SB26	Span 2	End	UDCONU2	-2.7985	0.0766
SB26	Span 3	Start	UDCONU2	-2.7985	0.0766
SB26	Span 3	Middle		0.0000	0.0000
SB26	Span 3	End		0.0000	0.0000
SB26	Span 4	Start		0.0000	0.0000
SB26	Span 4	Middle	UDCONU2	-7.9604	0.2183
SB26	Span 4	End	UDCONU2	-64.4839	1.8114
SB26	Span 5	Start	UDCONU2	-64.4839	1.8114
SB26	Span 5	Middle		0.0000	0.0000
SB26	Span 5	End	UDCONU2	-16.6620	0.4611
SB26	Span 6	Start	UDCONU2	-16.6620	0.4611
SB26	Span 6	Middle	UDCONU2	-0.6760	0.0000
SB26	Span 6	End		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB26	Span 7	Start		0.0000	0.0000
SB26	Span 7	Middle		0.0000	0.0000
SB26	Span 7	End		0.0000	0.0000
SB26	Span 8	Start		0.0000	0.0000
SB26	Span 8	Middle		0.0000	0.0000
SB26	Span 8	End		0.0000	0.0000
SB26	Span 9	Start		0.0000	0.0000
SB26	Span 9	Middle		0.0000	0.0000
SB26	Span 9	End		0.0000	0.0000
SB26	Span 10	Start		0.0000	0.0000
SB26	Span 10	Middle		0.0000	0.0000
SB26	Span 10	End		0.0000	0.0000
SB26	Span 11	Start		0.0000	0.0000
SB26	Span 11	Middle		0.0000	0.0000
SB26	Span 11	End		0.0000	0.0000
SB26	Span 12	Start		0.0000	0.0000
SB26	Span 12	Middle		0.0000	0.0000
SB26	Span 12	End		0.0000	0.0000
SB27	Span 1	Start	UDCONU2	-2.8434	0.0802
SB27	Span 1	Middle	UDCONU2	-48.1847	1.3526
SB27	Span 1	End	UDCONU2	-19.0398	0.5331
SB27	Span 2	Start	UDCONU2	-3.4021	0.0965
SB27	Span 2	Middle		0.0000	0.0000
SB27	Span 2	End		0.0000	0.0000
SB27	Span 3	Start		0.0000	0.0000
SB27	Span 3	Middle	UDCONU2	-3.5652	0.0976
SB27	Span 3	End	UDCONU2	-28.2556	0.7801
SB27	Span 4	Start	UDCONU2	-28.2556	0.7801
SB27	Span 4	Middle		0.0000	0.0000
SB27	Span 4	End	UDCONU2	-12.2861	0.3374
SB27	Span 5	Start	UDCONU2	-12.2861	0.3374
SB27	Span 5	Middle	UDCONU2	-1.2469	0.0346
SB27	Span 5	End		0.0000	0.0000
SB27	Span 6	Start		0.0000	0.0000
SB27	Span 6	Middle		0.0000	0.0000
SB27	Span 6	End		0.0000	0.0000
SB27	Span 7	Start		0.0000	0.0000
SB27	Span 7	Middle		0.0000	0.0000
SB27	Span 7	End		0.0000	0.0000
SB27	Span 8	Start		0.0000	0.0000
SB27	Span 8	Middle		0.0000	0.0000
SB27	Span 8	End		0.0000	0.0000
SB27	Span 9	Start		0.0000	0.0000
SB27	Span 9	Middle		0.0000	0.0000
SB27	Span 9	End		0.0000	0.0000
SB27	Span 10	Start		0.0000	0.0000
SB27	Span 10	Middle		0.0000	0.0000
SB27	Span 10	End		0.0000	0.0000
SB27	Span 11	Start		0.0000	0.0000
SB27	Span 11	Middle		0.0000	0.0000
SB27	Span 11	End		0.0000	0.0000
SB28	Span 1	Start	UDCONU2	-4.6668	0.1395
SB28	Span 1	Middle	UDCONU1	-17.8057	0.5139

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB28	Span 1	End	UDCONU2	-94.1895	2.6864
SB28	Span 2	Start	UDCONU2	-94.1895	2.6864
SB28	Span 2	Middle	UDCONU2	-14.1866	0.3983
SB28	Span 2	End	UDCONU2	-4.5157	0.1254
SB28	Span 3	Start	UDCONU2	-4.5157	0.1254
SB28	Span 3	Middle		0.0000	0.0000
SB28	Span 3	End		0.0000	0.0000
SB28	Span 4	Start		0.0000	0.0000
SB28	Span 4	Middle	UDCONU2	-4.0083	0.1098
SB28	Span 4	End	UDCONU2	-51.5632	1.4429
SB28	Span 5	Start	UDCONU2	-51.5632	1.4429
SB28	Span 5	Middle		0.0000	0.0000
SB28	Span 5	End	UDCONU2	-13.2568	0.3674
SB28	Span 6	Start	UDCONU2	-13.2568	0.3674
SB28	Span 6	Middle	UDCONU2	-0.4098	0.0000
SB28	Span 6	End		0.0000	0.0000
SB28	Span 7	Start		0.0000	0.0000
SB28	Span 7	Middle		0.0000	0.0000
SB28	Span 7	End		0.0000	0.0000
SB28	Span 8	Start		0.0000	0.0000
SB28	Span 8	Middle		0.0000	0.0000
SB28	Span 8	End		0.0000	0.0000
SB28	Span 9	Start		0.0000	0.0000
SB28	Span 9	Middle		0.0000	0.0000
SB28	Span 9	End		0.0000	0.0000
SB28	Span 10	Start		0.0000	0.0000
SB28	Span 10	Middle		0.0000	0.0000
SB28	Span 10	End		0.0000	0.0000
SB28	Span 11	Start		0.0000	0.0000
SB28	Span 11	Middle		0.0000	0.0000
SB28	Span 11	End		0.0000	0.0000
SB28	Span 12	Start		0.0000	0.0000
SB28	Span 12	Middle		0.0000	0.0000
SB28	Span 12	End		0.0000	0.0000
SB29	Span 1	Start	UDCONU2	-2.0316	0.0720
SB29	Span 1	Middle	UDCONU2	-2.1196	0.0706
SB29	Span 1	End	UDCONU1	-1.6607	0.0515
SB29	Span 2	Start	UDCONU1	-1.6607	0.0515
SB29	Span 2	Middle	UDCONU2	-3.3778	0.1126
SB29	Span 2	End	UDCONU2	-28.7351	0.7935
SB29	Span 3	Start	UDCONU2	-28.7351	0.7935
SB29	Span 3	Middle	UDCONU2	-0.7783	0.0214
SB29	Span 3	End	UDCONU1	-0.0454	0.0000
SB29	Span 4	Start	UDCONU1	-0.0454	0.0000
SB29	Span 4	Middle		0.0000	0.0000
SB29	Span 4	End		0.0000	0.0000
SB29	Span 5	Start		0.0000	0.0000
SB29	Span 5	Middle	UDCONU2	-1.8753	0.0518
SB29	Span 5	End		0.0000	0.0000
SB29	Span 6	Start		0.0000	0.0000
SB29	Span 6	Middle		0.0000	0.0000
SB29	Span 6	End		0.0000	0.0000
SB29	Span 7	Start		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB29	Span 7	Middle		0.0000	0.0000
SB29	Span 7	End		0.0000	0.0000
SB29	Span 8	Start		0.0000	0.0000
SB29	Span 8	Middle		0.0000	0.0000
SB29	Span 8	End		0.0000	0.0000
SB29	Span 9	Start		0.0000	0.0000
SB29	Span 9	Middle		0.0000	0.0000
SB29	Span 9	End		0.0000	0.0000
SB29	Span 10	Start		0.0000	0.0000
SB29	Span 10	Middle		0.0000	0.0000
SB29	Span 10	End		0.0000	0.0000
SB29	Span 11	Start		0.0000	0.0000
SB29	Span 11	Middle		0.0000	0.0000
SB29	Span 11	End		0.0000	0.0000
SB29	Span 12	Start		0.0000	0.0000
SB29	Span 12	Middle		0.0000	0.0000
SB29	Span 12	End		0.0000	0.0000
SB30	Span 1	Start	UDCONU2	-1.9895	0.0593
SB30	Span 1	Middle	UDCONU2	-4.8944	0.1374
SB30	Span 1	End		0.0000	0.0000
SB30	Span 2	Start		0.0000	0.0000
SB30	Span 2	Middle		0.0000	0.0000
SB30	Span 2	End		0.0000	0.0000
SB30	Span 3	Start		0.0000	0.0000
SB30	Span 3	Middle		0.0000	0.0000
SB30	Span 3	End		0.0000	0.0000
SB30	Span 4	Start		0.0000	0.0000
SB30	Span 4	Middle		0.0000	0.0000
SB30	Span 4	End		0.0000	0.0000
SB30	Span 5	Start		0.0000	0.0000
SB30	Span 5	Middle		0.0000	0.0000
SB30	Span 5	End		0.0000	0.0000
SB30	Span 6	Start		0.0000	0.0000
SB30	Span 6	Middle		0.0000	0.0000
SB30	Span 6	End		0.0000	0.0000
SB30	Span 7	Start		0.0000	0.0000
SB30	Span 7	Middle		0.0000	0.0000
SB30	Span 7	End		0.0000	0.0000
SB30	Span 8	Start		0.0000	0.0000
SB30	Span 8	Middle		0.0000	0.0000
SB30	Span 8	End		0.0000	0.0000
SB30	Span 9	Start		0.0000	0.0000
SB30	Span 9	Middle		0.0000	0.0000
SB30	Span 9	End		0.0000	0.0000
SB30	Span 10	Start		0.0000	0.0000
SB30	Span 10	Middle		0.0000	0.0000
SB30	Span 10	End		0.0000	0.0000
SB31	Span 1	Start	UDCONU2	-1.2211	0.0338
SB31	Span 1	Middle	UDCONU2	-0.0913	0.0000
SB31	Span 1	End	UDCONU2	-2.3418	0.0642
SB31	Span 2	Start	UDCONU2	-2.3418	0.0642
SB31	Span 2	Middle		0.0000	0.0000
SB31	Span 2	End		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 1 of 3

Strip	SpanID	Location	FTopCombo	FTopMoment	FTopArea
				kip-ft	in2
SB31	Span 3	Start		0.0000	0.0000
SB31	Span 3	Middle		0.0000	0.0000
SB31	Span 3	End		0.0000	0.0000
SB31	Span 4	Start		0.0000	0.0000
SB31	Span 4	Middle		0.0000	0.0000
SB31	Span 4	End		0.0000	0.0000
SB31	Span 5	Start		0.0000	0.0000
SB31	Span 5	Middle		0.0000	0.0000
SB31	Span 5	End		0.0000	0.0000
SB31	Span 6	Start		0.0000	0.0000
SB31	Span 6	Middle		0.0000	0.0000
SB31	Span 6	End		0.0000	0.0000
SB31	Span 7	Start		0.0000	0.0000
SB31	Span 7	Middle		0.0000	0.0000
SB31	Span 7	End		0.0000	0.0000
SB31	Span 8	Start		0.0000	0.0000
SB31	Span 8	Middle		0.0000	0.0000
SB31	Span 8	End		0.0000	0.0000
SB31	Span 9	Start		0.0000	0.0000
SB31	Span 9	Middle		0.0000	0.0000
SB31	Span 9	End		0.0000	0.0000
SB31	Span 10	Start		0.0000	0.0000
SB31	Span 10	Middle		0.0000	0.0000
SB31	Span 10	End		0.0000	0.0000
SB31	Span 11	Start		0.0000	0.0000
SB31	Span 11	Middle		0.0000	0.0000
SB31	Span 11	End		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA1	Span 1	Start	UDCONU2	0.0112	2.818E-004
SA1	Span 1	Middle	UDCONU2	0.6840	0.0184
SA1	Span 1	End	UDCONU2	0.0000	0.0000
SA1	Span 2	Start	UDCONU2	0.0000	0.0000
SA1	Span 2	Middle		0.0000	0.0000
SA1	Span 2	End	UDCONU2	0.0000	0.0000
SA1	Span 3	Start	UDCONU2	0.0000	0.0000
SA1	Span 3	Middle	UDCONU2	2.4456	0.0790
SA1	Span 3	End	UDCONU2	4.2875	0.1226
SA1	Span 4	Start	UDCONU2	4.2875	0.1226
SA1	Span 4	Middle	UDCONU2	4.3546	0.1093
SA1	Span 4	End	UDCONU1	2.7432	0.0688
SA1	Span 5	Start	UDCONU1	2.7432	0.0688
SA1	Span 5	Middle		0.0000	0.0000
SA1	Span 5	End	UDCONU2	2.9617	0.0745
SA1	Span 6	Start	UDCONU2	2.9617	0.0745
SA1	Span 6	Middle	UDCONU2	5.5402	0.1477
SA1	Span 6	End	UDCONU2	3.4904	0.0915
SA1	Span 7	Start	UDCONU2	3.4904	0.0915

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA1	Span 7	Middle		0.0000	0.0000
SA1	Span 7	End	UDCONU2	1.4940	0.0393
SA1	Span 8	Start	UDCONU2	1.4940	0.0393
SA1	Span 8	Middle	UDCONU2	2.5254	0.0666
SA1	Span 8	End	UDCONU2	0.1685	0.0166
SA1	Span 9	Start	UDCONU2	0.1685	0.0166
SA1	Span 9	Middle		0.0000	0.0000
SA1	Span 9	End	UDCONU2	0.3745	0.0224
SA1	Span 10	Start	UDCONU2	0.3745	0.0224
SA1	Span 10	Middle		0.0000	0.0000
SA1	Span 10	End	UDCONU2	0.3892	0.0000
SA1	Span 11	Start	UDCONU2	0.3892	0.0000
SA1	Span 11	Middle		0.0000	0.0000
SA1	Span 11	End	UDCONU2	0.0000	0.0000
SA1	Span 12	Start	UDCONU2	0.0000	0.0000
SA1	Span 12	Middle	UDCONU2	6.3029	0.1584
SA1	Span 12	End	UDCONU2	5.4029	0.1357
SA1	Span 13	Start	UDCONU2	5.4029	0.1357
SA1	Span 13	Middle		0.0000	0.0000
SA1	Span 13	End	UDCONU2	0.0337	0.0000
SA1	Span 14	Start	UDCONU2	0.0337	0.0000
SA1	Span 14	Middle	UDCONU1	1.2157	0.0313
SA1	Span 14	End	UDCONU2	1.3428	0.0391
SA1	Span 15	Start	UDCONU2	1.3428	0.0391
SA1	Span 15	Middle	UDCONU2	5.2093	0.1308
SA1	Span 15	End	UDCONU2	6.4638	0.1624
SA1	Span 16	Start	UDCONU2	6.4638	0.1624
SA1	Span 16	Middle		0.0000	0.0000
SA1	Span 16	End	UDCONU2	1.3620	0.0341
SA1	Span 17	Start	UDCONU2	1.3620	0.0341
SA1	Span 17	Middle	UDCONU2	5.6144	0.1410
SA1	Span 17	End	UDCONU2	4.3750	0.1372
SA1	Span 18	Start	UDCONU2	4.3750	0.1372
SA1	Span 18	Middle		0.0000	0.0000
SA1	Span 18	End	UDCONU2	1.7494	0.0549
SA1	Span 19	Start	UDCONU2	1.7494	0.0549
SA1	Span 19	Middle	UDCONU2	1.5517	0.0424
SA1	Span 19	End	UDCONU2	0.8105	0.0337
SA1	Span 20	Start	UDCONU2	0.8105	0.0337
SA1	Span 20	Middle	UDCONU2	1.1393	0.0361
SA1	Span 20	End	UDCONU2	1.4635	0.0414
SA1	Span 21	Start	UDCONU2	1.4635	0.0414
SA1	Span 21	Middle	UDCONU2	1.8811	0.0472
SA1	Span 21	End	UDCONU2	0.0158	0.0000
SA1	Span 22	Start	UDCONU2	0.0158	0.0000
SA1	Span 22	Middle		0.0000	0.0000
SA1	Span 22	End	UDCONU2	0.7716	0.0193
SA1	Span 23	Start	UDCONU2	0.7716	0.0193
SA1	Span 23	Middle	UDCONU2	2.0544	0.0522
SA1	Span 23	End	UDCONU2	0.2787	0.0298
SA1	Span 24	Start	UDCONU2	0.2787	0.0298
SA1	Span 24	Middle	UDCONU2	1.9984	0.0725
SA1	Span 24	End	UDCONU2	2.9899	0.1080

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA1	Span 25	Start	UDCONU2	2.9899	0.1080
SA1	Span 25	Middle	UDCONU2	0.8217	0.0547
SA1	Span 25	End	UDCONU2	0.6179	0.0363
SA1	Span 26	Start	UDCONU2	0.2818	0.0262
SA1	Span 26	Middle	UDCONU1	1.1422	0.0435
SA1	Span 26	End	UDCONU1	3.6039	0.1054
SA1	Span 27	Start	UDCONU1	3.6039	0.1054
SA1	Span 27	Middle		0.0000	0.0000
SA1	Span 27	End	UDCONU2	2.8131	0.0719
SA1	Span 28	Start	UDCONU2	2.8131	0.0719
SA1	Span 28	Middle	UDCONU2	2.2790	0.0571
SA1	Span 28	End	UDCONU2	1.3109	0.0330
SA2	Cantilever Start	Start		0.0000	0.0000
SA2	Cantilever Start	Middle		0.0000	0.0000
SA2	Cantilever Start	End	UDCONU2	0.0000	0.0000
SA2	Span 1	Start	UDCONU2	0.0000	0.0000
SA2	Span 1	Middle	UDCONU2	1.1589	0.0000
SA2	Span 1	End	UDCONU2	8.8195	0.2225
SA2	Span 2	Start	UDCONU2	8.8195	0.2225
SA2	Span 2	Middle	UDCONU2	18.3726	0.4624
SA2	Span 2	End	UDCONU2	7.8530	0.1971
SA2	Span 3	Start	UDCONU2	7.8530	0.1971
SA2	Span 3	Middle		0.0000	0.0000
SA2	Span 3	End	UDCONU2	11.5252	0.2895
SA2	Span 4	Start	UDCONU2	11.5252	0.2895
SA2	Span 4	Middle		0.0000	0.0000
SA2	Span 4	End	UDCONU2	12.2965	0.3089
SA2	Span 5	Start	UDCONU2	12.2965	0.3089
SA2	Span 5	Middle	UDCONU2	12.2448	0.3076
SA2	Span 5	End	UDCONU2	2.8060	0.0741
SA2	Span 6	Start	UDCONU2	2.8060	0.0741
SA2	Span 6	Middle		0.0000	0.0000
SA2	Span 6	End	UDCONU2	0.9126	0.0000
SA2	Span 7	Start	UDCONU2	0.8658	0.0000
SA2	Span 7	Middle		0.0000	0.0000
SA2	Span 7	End	UDCONU2	0.5791	0.0000
SA2	Span 8	Start	UDCONU2	0.5791	0.0000
SA2	Span 8	Middle		0.0000	0.0000
SA2	Span 8	End	UDCONU2	1.1950	0.0411
SA2	Span 9	Start	UDCONU2	1.1950	0.0411
SA2	Span 9	Middle		0.0000	0.0000
SA2	Span 9	End	UDCONU2	0.7992	0.0322
SA2	Span 10	Start	UDCONU2	0.7992	0.0322
SA2	Span 10	Middle	UDCONU2	3.4114	0.1039
SA2	Span 10	End	UDCONU2	3.2926	0.1013
SA2	Span 11	Start	UDCONU2	3.2926	0.1013
SA2	Span 11	Middle		0.0000	0.0000
SA2	Span 11	End	UDCONU2	5.3759	0.1348
SA2	Span 12	Start	UDCONU2	5.3759	0.1348
SA2	Span 12	Middle		0.0000	0.0000
SA2	Span 12	End	UDCONU2	9.8292	0.2470

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA2	Span 13	Start	UDCONU2	9.8292	0.2470
SA2	Span 13	Middle		0.0000	0.0000
SA2	Span 13	End	UDCONU2	1.0083	0.0000
SA2	Span 14	Start	UDCONU2	1.0083	0.0000
SA2	Span 14	Middle	UDCONU2	15.7298	0.3956
SA2	Span 14	End	UDCONU2	12.8338	0.3225
SA2	Span 15	Start	UDCONU2	12.8338	0.3225
SA2	Span 15	Middle		0.0000	0.0000
SA2	Span 15	End	UDCONU2	0.1523	0.0000
SA2	Span 16	Start	UDCONU2	0.1523	0.0000
SA2	Span 16	Middle	UDCONU2	7.7495	0.1945
SA2	Span 16	End	UDCONU1	3.9745	0.0996
SA2	Span 17	Start	UDCONU1	3.9745	0.0996
SA2	Span 17	Middle		0.0000	0.0000
SA2	Span 17	End	UDCONU2	6.4144	0.1609
SA2	Span 18	Start	UDCONU2	6.4144	0.1609
SA2	Span 18	Middle		0.0000	0.0000
SA2	Span 18	End	UDCONU2	8.9991	0.2259
SA2	Span 19	Start	UDCONU2	8.9991	0.2259
SA2	Span 19	Middle	UDCONU2	8.5126	0.2137
SA2	Span 19	End	UDCONU2	6.1585	0.1545
SA2	Span 20	Start	UDCONU2	6.1585	0.1545
SA2	Span 20	Middle	UDCONU2	18.7020	0.4707
SA2	Span 20	End	UDCONU2	12.1746	0.3059
SA2	Span 21	Start	UDCONU2	12.1746	0.3059
SA2	Span 21	Middle		0.0000	0.0000
SA2	Span 21	End	UDCONU2	0.3180	0.0891
SA2	Span 22	Start	UDCONU2	0.3180	0.0891
SA2	Span 22	Middle	UDCONU2	1.4528	0.1269
SA2	Span 22	End	UDCONU2	0.1844	0.0687
SA2	Span 23	Start	UDCONU2	0.1844	0.0687
SA2	Span 23	Middle		0.0000	0.0000
SA2	Span 23	End	UDCONU2	1.7187	0.0485
SA2	Span 24	Start	UDCONU2	1.7187	0.0485
SA2	Span 24	Middle		0.0000	0.0000
SA2	Span 24	End	UDCONU2	6.4469	0.1637
SA2	Span 25	Start	UDCONU2	6.4469	0.1637
SA2	Span 25	Middle	UDCONU2	8.8026	0.2257
SA2	Span 25	End	UDCONU2	0.0528	0.0000
SA2	Span 26	Start	UDCONU2	0.0528	0.0000
SA2	Span 26	Middle		0.0000	0.0000
SA2	Span 26	End	UDCONU2	0.7232	0.0335
SA2	Span 27	Start	UDCONU2	0.7232	0.0335
SA2	Span 27	Middle	UDCONU2	4.3855	0.1249
SA2	Span 27	End	UDCONU2	0.3050	0.0000
SA2	Span 28	Start	UDCONU2	0.3050	0.0000
SA2	Span 28	Middle		0.0000	0.0000
SA2	Span 28	End	UDCONU2	0.0671	0.0000
SA2	Span 29	Start	UDCONU2	0.0671	0.0000
SA2	Span 29	Middle	UDCONU2	8.7003	0.2618
SA2	Span 29	End	UDCONU2	3.1930	0.1955
SA2	Span 30	Start	UDCONU2	3.1930	0.1955
SA2	Span 30	Middle		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA2	Span 30	End	UDCONU2	1.7106	0.1239
SA2	Span 31	Start	UDCONU2	1.7106	0.1239
SA2	Span 31	Middle	UDCONU2	2.5124	0.1371
SA2	Span 31	End	UDCONU2	0.4855	0.0739
SA2	Span 32	Start	UDCONU2	0.4855	0.0739
SA2	Span 32	Middle	UDCONU2	2.7712	0.0973
SA2	Span 32	End	UDCONU2	2.8625	0.0798
SA2	Span 33	Start	UDCONU2	2.8625	0.0798
SA2	Span 33	Middle		0.0000	0.0000
SA2	Span 33	End	UDCONU2	0.8192	0.0000
SA2	Span 34	Start	UDCONU2	0.8192	0.0000
SA2	Span 34	Middle		0.0000	0.0000
SA2	Span 34	End	UDCONU2	1.9807	0.0496
SA2	Span 35	Start	UDCONU2	1.9807	0.0496
SA2	Span 35	Middle	UDCONU2	0.2029	0.0000
SA2	Span 35	End	UDCONU2	0.2689	0.0000
SA2	Span 36	Start	UDCONU2	0.2689	0.0000
SA2	Span 36	Middle	UDCONU2	0.0631	0.0000
SA2	Span 36	End	UDCONU1	3.1661	0.0793
SA2	Span 37	Start	UDCONU1	3.1661	0.0793
SA2	Span 37	Middle		0.0000	0.0000
SA2	Span 37	End	UDCONU1	4.5325	0.1252
SA2	Span 38	Start	UDCONU1	4.5325	0.1252
SA2	Span 38	Middle		0.0000	0.0000
SA2	Span 38	End	UDCONU1	6.7929	0.1704
SA2	Span 39	Start	UDCONU1	6.7929	0.1704
SA2	Span 39	Middle		0.0000	0.0000
SA2	Span 39	End	UDCONU2	2.8401	0.0794
SA2	Span 40	Start	UDCONU2	2.8401	0.0794
SA2	Span 40	Middle	UDCONU2	2.9087	0.0792
SA2	Span 40	End	UDCONU2	1.2165	0.0321
SA3	Span 1	Start		0.0000	0.0000
SA3	Span 1	Middle		0.0000	0.0000
SA3	Span 1	End	UDCONU2	1.7238	0.0457
SA3	Span 2	Start	UDCONU2	0.6895	0.0193
SA3	Span 2	Middle		0.0000	0.0000
SA3	Span 2	End	UDCONU2	1.2138	0.0370
SA3	Span 3	Start	UDCONU2	1.2138	0.0370
SA3	Span 3	Middle		0.0000	0.0000
SA3	Span 3	End	UDCONU2	0.0000	0.0000
SA3	Span 4	Start	UDCONU2	0.0000	0.0000
SA3	Span 4	Middle		0.0000	0.0000
SA3	Span 4	End	UDCONU2	0.0000	0.0000
SA3	Span 5	Start	UDCONU2	0.0000	0.0000
SA3	Span 5	Middle		0.0000	0.0000
SA3	Span 5	End	UDCONU2	3.8052	0.0954
SA3	Span 6	Start	UDCONU2	3.8052	0.0954
SA3	Span 6	Middle	UDCONU2	21.9706	0.5535
SA3	Span 6	End	UDCONU2	13.8770	0.3488
SA3	Span 7	Start	UDCONU2	13.8770	0.3488
SA3	Span 7	Middle	UDCONU2	14.7841	0.3717
SA3	Span 7	End	UDCONU2	13.1535	0.3306
SA3	Span 8	Start	UDCONU2	13.1535	0.3306

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA3	Span 8	Middle	UDCONU2	12.7136	0.3195
SA3	Span 8	End	UDCONU2	4.0325	0.1124
SA3	Span 9	Start	UDCONU2	4.0325	0.1124
SA3	Span 9	Middle		0.0000	0.0000
SA3	Span 9	End	UDCONU2	2.1180	0.0716
SA3	Span 10	Start	UDCONU2	2.1180	0.0716
SA3	Span 10	Middle		0.0000	0.0000
SA3	Span 10	End	UDCONU2	0.0000	0.0000
SA3	Span 11	Start	UDCONU2	0.0000	0.0000
SA3	Span 11	Middle	UDCONU2	10.1348	0.2551
SA3	Span 11	End	UDCONU2	14.9872	0.3783
SA3	Span 12	Start	UDCONU2	14.9872	0.3783
SA3	Span 12	Middle	UDCONU2	26.0766	0.6577
SA3	Span 12	End	UDCONU2	24.7884	0.6250
SA3	Span 13	Start	UDCONU2	24.7884	0.6250
SA3	Span 13	Middle	UDCONU2	18.3014	0.4606
SA3	Span 13	End	UDCONU1	9.3576	0.2349
SA3	Span 14	Start	UDCONU1	9.3576	0.2349
SA3	Span 14	Middle		0.0000	0.0000
SA3	Span 14	End	UDCONU1	5.0584	0.1268
SA3	Span 15	Start	UDCONU1	7.3847	0.1853
SA3	Span 15	Middle	UDCONU2	27.5631	0.6955
SA3	Span 15	End	UDCONU2	17.0521	0.4481
SA3	Span 16	Start	UDCONU2	17.0521	0.4481
SA3	Span 16	Middle	UDCONU2	6.6204	0.1896
SA3	Span 16	End	UDCONU2	8.1138	0.2474
SA3	Span 17	Start	UDCONU2	8.1138	0.2474
SA3	Span 17	Middle	UDCONU2	19.0162	0.4919
SA3	Span 17	End	UDCONU2	6.3140	0.1721
SA3	Span 18	Start	UDCONU2	15.2123	0.3965
SA3	Span 18	Middle	UDCONU2	18.1930	0.4727
SA3	Span 18	End	UDCONU2	14.3550	0.3766
SA3	Span 19	Start	UDCONU2	8.2662	0.2252
SA3	Span 19	Middle	UDCONU2	25.1950	0.6500
SA3	Span 19	End	UDCONU2	7.0351	0.2011
SA3	Span 20	Start	UDCONU2	7.0351	0.2011
SA3	Span 20	Middle	UDCONU2	7.0104	0.2309
SA3	Span 20	End		0.0000	0.0000
SA3	Span 21	Start		0.0000	0.0000
SA3	Span 21	Middle		0.0000	0.0000
SA3	Span 21	End		0.0000	0.0000
SA3	Span 22	Start	UDCONU2	3.9592	0.1142
SA3	Span 22	Middle		0.0000	0.0000
SA3	Span 22	End	UDCONU2	14.3469	0.3607
SA3	Span 23	Start	UDCONU2	14.3469	0.3607
SA3	Span 23	Middle	UDCONU2	9.5846	0.2406
SA3	Span 23	End	UDCONU2	4.3554	0.1092
SA3	Span 24	Start	UDCONU2	4.3554	0.1092
SA3	Span 24	Middle	UDCONU2	7.9959	0.2007
SA3	Span 24	End	UDCONU2	15.7748	0.3967
SA3	Span 25	Start	UDCONU2	15.7748	0.3967
SA3	Span 25	Middle	UDCONU2	2.4464	0.0660
SA3	Span 25	End		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA3	Span 26	Start		0.0000	0.0000
SA3	Span 26	Middle		0.0000	0.0000
SA3	Span 26	End		0.0000	0.0000
SA4	Span 1	Start		0.0000	0.0000
SA4	Span 1	Middle		0.0000	0.0000
SA4	Span 1	End	UDCONU2	3.0210	0.0757
SA4	Span 2	Start	UDCONU2	3.0210	0.0757
SA4	Span 2	Middle		0.0000	0.0000
SA4	Span 2	End	UDCONU2	4.9418	0.1241
SA4	Span 3	Start	UDCONU2	4.9418	0.1241
SA4	Span 3	Middle	UDCONU2	26.3336	0.6642
SA4	Span 3	End	UDCONU2	24.3952	0.6150
SA4	Span 4	Start	UDCONU2	24.3952	0.6150
SA4	Span 4	Middle		0.0000	0.0000
SA4	Span 4	End	UDCONU2	7.3225	0.1837
SA4	Span 5	Start	UDCONU2	6.0786	0.1525
SA4	Span 5	Middle	UDCONU2	4.9736	0.1247
SA4	Span 5	End	UDCONU2	0.0000	0.0000
SA4	Span 6	Start	UDCONU2	5.0703	0.1311
SA4	Span 6	Middle	UDCONU2	5.0703	0.1311
SA4	Span 6	End	UDCONU2	8.1926	0.2155
SA4	Span 7	Start	UDCONU2	8.1926	0.2155
SA4	Span 7	Middle	UDCONU2	30.3688	0.7709
SA4	Span 7	End	UDCONU2	30.0384	0.7605
SA4	Span 8	Start	UDCONU2	30.0384	0.7605
SA4	Span 8	Middle	UDCONU2	26.2122	0.6612
SA4	Span 8	End	UDCONU1	7.8816	0.1978
SA4	Span 9	Start	UDCONU1	7.8816	0.1978
SA4	Span 9	Middle	UDCONU1	1.7593	0.0441
SA4	Span 9	End	UDCONU2	2.0782	0.0521
SA4	Span 10	Start	UDCONU2	4.4665	0.1120
SA4	Span 10	Middle	UDCONU2	11.8542	0.2978
SA4	Span 10	End	UDCONU2	19.4468	0.4896
SA4	Span 11	Start	UDCONU2	20.3728	0.5168
SA4	Span 11	Middle	UDCONU2	20.1750	0.5241
SA4	Span 11	End	UDCONU2	21.2824	0.5560
SA4	Span 12	Start	UDCONU2	21.2824	0.5560
SA4	Span 12	Middle	UDCONU2	18.1250	0.4753
SA4	Span 12	End	UDCONU2	16.9105	0.4397
SA4	Span 13	Start	UDCONU2	23.2182	0.6010
SA4	Span 13	Middle	UDCONU2	27.2908	0.7056
SA4	Span 13	End	UDCONU2	27.2908	0.7056
SA4	Span 14	Start	UDCONU2	26.1838	0.6733
SA4	Span 14	Middle	UDCONU2	23.5902	0.5999
SA4	Span 14	End	UDCONU2	1.9667	0.0534
SA4	Span 15	Start	UDCONU2	1.9667	0.0534
SA4	Span 15	Middle	UDCONU2	1.7026	0.0676
SA4	Span 15	End		0.0000	0.0000
SA4	Span 16	Start		0.0000	0.0000
SA4	Span 16	Middle		0.0000	0.0000
SA4	Span 16	End		0.0000	0.0000
SA4	Span 17	Start	UDCONU2	4.7423	0.1303
SA4	Span 17	Middle		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA4	Span 17	End	UDCONU2	0.0000	0.0000
SA4	Span 18	Start	UDCONU2	0.0000	0.0000
SA4	Span 18	Middle	UDCONU2	12.4696	0.3182
SA4	Span 18	End	UDCONU2	16.7236	0.4207
SA4	Span 19	Start	UDCONU2	16.7236	0.4207
SA4	Span 19	Middle	UDCONU2	15.7189	0.3953
SA4	Span 19	End	UDCONU2	1.3716	0.0366
SA4	Span 20	Start	UDCONU2	1.3716	0.0366
SA4	Span 20	Middle	UDCONU2	3.2398	0.0822
SA4	Span 20	End		0.0000	0.0000
SA4	Span 21	Start		0.0000	0.0000
SA4	Span 21	Middle		0.0000	0.0000
SA4	Span 21	End		0.0000	0.0000
SA5	Span 1	Start		0.0000	0.0000
SA5	Span 1	Middle		0.0000	0.0000
SA5	Span 1	End	UDCONU2	2.6314	0.0691
SA5	Span 2	Start	UDCONU2	2.6314	0.0691
SA5	Span 2	Middle	UDCONU2	1.1317	0.0335
SA5	Span 2	End	UDCONU2	1.1896	0.0360
SA5	Span 3	Start	UDCONU2	1.1896	0.0360
SA5	Span 3	Middle		0.0000	0.0000
SA5	Span 3	End	UDCONU2	1.4608	0.0483
SA5	Span 4	Start	UDCONU2	1.4608	0.0483
SA5	Span 4	Middle		0.0000	0.0000
SA5	Span 4	End	UDCONU2	0.1895	0.0000
SA5	Span 5	Start	UDCONU2	0.1895	0.0000
SA5	Span 5	Middle		0.0000	0.0000
SA5	Span 5	End	UDCONU1	0.5113	0.0000
SA5	Span 6	Start	UDCONU1	0.5113	0.0000
SA5	Span 6	Middle	UDCONU2	25.5415	0.6441
SA5	Span 6	End	UDCONU2	24.8293	0.6260
SA5	Span 7	Start	UDCONU2	24.8293	0.6260
SA5	Span 7	Middle	UDCONU2	18.1475	0.4567
SA5	Span 7	End	UDCONU2	7.2779	0.1826
SA5	Span 8	Start	UDCONU2	7.2779	0.1826
SA5	Span 8	Middle	UDCONU2	8.0186	0.2012
SA5	Span 8	End	UDCONU2	5.7183	0.1434
SA5	Span 9	Start	UDCONU2	5.7183	0.1434
SA5	Span 9	Middle	UDCONU2	7.0803	0.1776
SA5	Span 9	End	UDCONU2	4.0851	0.1024
SA5	Span 10	Start	UDCONU2	4.0851	0.1024
SA5	Span 10	Middle		0.0000	0.0000
SA5	Span 10	End	UDCONU2	4.2998	0.1157
SA5	Span 11	Start	UDCONU2	4.2998	0.1157
SA5	Span 11	Middle		0.0000	0.0000
SA5	Span 11	End	UDCONU2	3.2478	0.0814
SA5	Span 12	Start	UDCONU2	3.2478	0.0814
SA5	Span 12	Middle		0.0000	0.0000
SA5	Span 12	End	UDCONU2	1.6985	0.0502
SA5	Span 13	Start	UDCONU2	1.6985	0.0502
SA5	Span 13	Middle	UDCONU2	3.2126	0.0935
SA5	Span 13	End	UDCONU2	1.1218	0.0416
SA5	Span 14	Start	UDCONU2	1.1218	0.0416

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA5	Span 14	Middle		0.0000	0.0000
SA5	Span 14	End	UDCONU2	0.2017	0.0000
SA5	Span 15	Start	UDCONU2	0.2017	0.0000
SA5	Span 15	Middle		0.0000	0.0000
SA5	Span 15	End	UDCONU2	0.0000	0.0000
SA5	Span 16	Start	UDCONU2	0.0000	0.0000
SA5	Span 16	Middle	UDCONU2	0.9675	0.0490
SA5	Span 16	End	UDCONU2	11.0119	0.2939
SA5	Span 17	Start	UDCONU2	11.0119	0.2939
SA5	Span 17	Middle	UDCONU2	30.5072	0.7797
SA5	Span 17	End	UDCONU2	26.9011	0.6876
SA5	Span 18	Start	UDCONU2	26.9011	0.6876
SA5	Span 18	Middle	UDCONU2	20.7529	0.5300
SA5	Span 18	End	UDCONU1	7.0508	0.1769
SA5	Span 19	Start	UDCONU1	7.0508	0.1769
SA5	Span 19	Middle	UDCONU1	1.7553	0.0440
SA5	Span 19	End	UDCONU2	1.6043	0.0402
SA5	Span 20	Start	UDCONU2	2.0679	0.0518
SA5	Span 20	Middle	UDCONU2	6.6377	0.1665
SA5	Span 20	End	UDCONU2	9.5221	0.2391
SA5	Span 21	Start	UDCONU2	18.1464	0.4567
SA5	Span 21	Middle	UDCONU2	31.5503	0.8064
SA5	Span 21	End	UDCONU2	32.3788	0.8344
SA5	Span 22	Start	UDCONU2	32.3788	0.8344
SA5	Span 22	Middle	UDCONU2	14.7766	0.3921
SA5	Span 22	End	UDCONU2	19.7807	0.5102
SA5	Span 23	Start	UDCONU2	26.4921	0.6876
SA5	Span 23	Middle	UDCONU2	27.0661	0.7148
SA5	Span 23	End	UDCONU2	30.5275	0.7914
SA5	Span 24	Start	UDCONU2	30.5275	0.7914
SA5	Span 24	Middle	UDCONU2	27.6919	0.7065
SA5	Span 24	End	UDCONU2	7.1619	0.1818
SA5	Span 25	Start	UDCONU2	7.1619	0.1818
SA5	Span 25	Middle	UDCONU2	7.0718	0.1906
SA5	Span 25	End		0.0000	0.0000
SA5	Span 26	Start		0.0000	0.0000
SA5	Span 26	Middle		0.0000	0.0000
SA5	Span 26	End		0.0000	0.0000
SA5	Span 27	Start	UDCONU2	5.6328	0.1491
SA5	Span 27	Middle		0.0000	0.0000
SA5	Span 27	End	UDCONU2	0.0000	0.0000
SA5	Span 28	Start	UDCONU2	0.0000	0.0000
SA5	Span 28	Middle	UDCONU2	13.9273	0.3520
SA5	Span 28	End	UDCONU2	19.2515	0.4854
SA5	Span 29	Start	UDCONU2	19.2515	0.4854
SA5	Span 29	Middle	UDCONU2	16.9901	0.4295
SA5	Span 29	End	UDCONU2	1.8064	0.0467
SA5	Span 30	Start	UDCONU2	1.8064	0.0467
SA5	Span 30	Middle	UDCONU1	3.0509	0.0772
SA5	Span 30	End		0.0000	0.0000
SA5	Span 31	Start		0.0000	0.0000
SA5	Span 31	Middle		0.0000	0.0000
SA5	Span 31	End		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA6	Span 1	Start		0.0000	0.0000
SA6	Span 1	Middle		0.0000	0.0000
SA6	Span 1	End	UDCONU2	0.5398	0.0135
SA6	Span 2	Start	UDCONU2	0.5398	0.0135
SA6	Span 2	Middle	UDCONU2	2.7235	0.0694
SA6	Span 2	End	UDCONU2	5.6164	0.1433
SA6	Span 3	Start	UDCONU2	5.6164	0.1433
SA6	Span 3	Middle	UDCONU2	13.7830	0.3464
SA6	Span 3	End	UDCONU2	11.9034	0.2990
SA6	Span 4	Start	UDCONU2	11.9034	0.2990
SA6	Span 4	Middle	UDCONU2	11.2106	0.2816
SA6	Span 4	End	UDCONU2	2.5207	0.0632
SA6	Span 5	Start	UDCONU2	2.3407	0.0586
SA6	Span 5	Middle		0.0000	0.0000
SA6	Span 5	End	UDCONU2	4.9833	0.1339
SA6	Span 6	Start	UDCONU2	4.9833	0.1339
SA6	Span 6	Middle	UDCONU2	2.5412	0.0783
SA6	Span 6	End	UDCONU2	2.0840	0.0641
SA6	Span 7	Start	UDCONU2	2.0840	0.0641
SA6	Span 7	Middle	UDCONU2	10.2477	0.2963
SA6	Span 7	End	UDCONU2	13.7261	0.3816
SA6	Span 8	Start	UDCONU2	13.7261	0.3816
SA6	Span 8	Middle	UDCONU2	22.7536	0.5989
SA6	Span 8	End	UDCONU2	7.3638	0.2013
SA6	Span 9	Start	UDCONU2	7.3638	0.2013
SA6	Span 9	Middle	UDCONU2	8.9197	0.2239
SA6	Span 9	End	UDCONU1	1.4991	0.0376
SA6	Span 10	Start	UDCONU1	1.4991	0.0376
SA6	Span 10	Middle		0.0000	0.0000
SA6	Span 10	End	UDCONU1	1.1608	0.0000
SA6	Span 11	Start	UDCONU1	1.1608	0.0000
SA6	Span 11	Middle	UDCONU1	0.3029	0.0000
SA6	Span 11	End	UDCONU2	2.6230	0.0657
SA6	Span 12	Start	UDCONU2	2.6230	0.0657
SA6	Span 12	Middle		0.0000	0.0000
SA6	Span 12	End	UDCONU2	6.5000	0.1630
SA6	Span 13	Start	UDCONU2	6.6472	0.1667
SA6	Span 13	Middle	UDCONU2	24.6412	0.6213
SA6	Span 13	End	UDCONU2	24.6412	0.6213
SA6	Span 14	Start	UDCONU2	21.1669	0.5361
SA6	Span 14	Middle	UDCONU2	18.3531	0.4676
SA6	Span 14	End	UDCONU2	3.5306	0.1165
SA6	Span 15	Start	UDCONU2	3.5306	0.1165
SA6	Span 15	Middle	UDCONU2	0.7820	0.0405
SA6	Span 15	End	UDCONU2	8.2078	0.2418
SA6	Span 16	Start	UDCONU2	18.4231	0.5071
SA6	Span 16	Middle	UDCONU2	23.9940	0.6456
SA6	Span 16	End	UDCONU2	15.8516	0.4667
SA6	Span 17	Start	UDCONU2	7.4267	0.2710
SA6	Span 17	Middle		0.0000	0.0000
SA6	Span 17	End	UDCONU2	5.9250	0.2286
SA6	Span 18	Start	UDCONU2	5.9250	0.2286
SA6	Span 18	Middle	UDCONU2	22.8275	0.5789

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA6	Span 18	End	UDCONU2	13.6884	0.3571
SA6	Span 19	Start	UDCONU2	13.6884	0.3571
SA6	Span 19	Middle	UDCONU2	13.5966	0.3519
SA6	Span 19	End		0.0000	0.0000
SA6	Span 20	Start		0.0000	0.0000
SA6	Span 20	Middle		0.0000	0.0000
SA6	Span 20	End		0.0000	0.0000
SA6	Span 21	Start	UDCONU2	7.4536	0.1942
SA6	Span 21	Middle		0.0000	0.0000
SA6	Span 21	End	UDCONU2	0.0000	0.0000
SA6	Span 22	Start	UDCONU2	0.0000	0.0000
SA6	Span 22	Middle	UDCONU2	13.1202	0.3301
SA6	Span 22	End	UDCONU2	18.1891	0.4578
SA6	Span 23	Start	UDCONU2	18.1891	0.4578
SA6	Span 23	Middle	UDCONU2	14.9830	0.3767
SA6	Span 23	End	UDCONU2	8.1197	0.2046
SA6	Span 24	Start	UDCONU2	8.1197	0.2046
SA6	Span 24	Middle	UDCONU2	4.6088	0.1187
SA6	Span 24	End		0.0000	0.0000
SA6	Span 25	Start		0.0000	0.0000
SA6	Span 25	Middle		0.0000	0.0000
SA6	Span 25	End		0.0000	0.0000
SA7	Span 1	Start		0.0000	0.0000
SA7	Span 1	Middle		0.0000	0.0000
SA7	Span 1	End		0.0000	0.0000
SA7	Span 2	Start		0.0000	0.0000
SA7	Span 2	Middle		0.0000	0.0000
SA7	Span 2	End	UDCONU2	2.0724	0.0536
SA7	Span 3	Start	UDCONU2	2.0724	0.0536
SA7	Span 3	Middle	UDCONU2	1.9802	0.0497
SA7	Span 3	End	UDCONU2	0.2312	0.0000
SA7	Span 4	Start	UDCONU2	0.2312	0.0000
SA7	Span 4	Middle	UDCONU2	0.9911	0.0249
SA7	Span 4	End	UDCONU2	1.0263	0.0257
SA7	Span 5	Start	UDCONU2	1.0263	0.0257
SA7	Span 5	Middle		0.0000	0.0000
SA7	Span 5	End	UDCONU2	0.1928	0.0048
SA7	Span 6	Start	UDCONU2	0.1928	0.0048
SA7	Span 6	Middle		0.0000	0.0000
SA7	Span 6	End	UDCONU2	0.4829	0.0130
SA7	Span 7	Start	UDCONU2	0.4829	0.0130
SA7	Span 7	Middle		0.0000	0.0000
SA7	Span 7	End	UDCONU2	0.4635	0.0126
SA7	Span 8	Start	UDCONU2	0.4635	0.0126
SA7	Span 8	Middle	UDCONU2	0.8041	0.0229
SA7	Span 8	End	UDCONU2	1.4243	0.0400
SA7	Span 9	Start	UDCONU2	1.4243	0.0400
SA7	Span 9	Middle	UDCONU2	0.8691	0.0247
SA7	Span 9	End	UDCONU2	16.8022	0.4430
SA7	Span 10	Start	UDCONU2	16.8022	0.4430
SA7	Span 10	Middle		0.0000	0.0000
SA7	Span 10	End	UDCONU2	19.8072	0.5127
SA7	Span 11	Start	UDCONU2	19.8072	0.5127

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA7	Span 11	Middle		0.0000	0.0000
SA7	Span 11	End	UDCONU2	20.8339	0.5298
SA7	Span 12	Start	UDCONU2	20.8339	0.5298
SA7	Span 12	Middle	UDCONU2	20.4234	0.5143
SA7	Span 12	End	UDCONU2	8.7791	0.2205
SA7	Span 13	Start	UDCONU2	8.7791	0.2205
SA7	Span 13	Middle	UDCONU2	13.8868	0.3491
SA7	Span 13	End	UDCONU2	9.3728	0.2353
SA7	Span 14	Start	UDCONU2	9.3728	0.2353
SA7	Span 14	Middle		0.0000	0.0000
SA7	Span 14	End	UDCONU2	12.2742	0.3084
SA7	Span 15	Start	UDCONU2	12.2742	0.3084
SA7	Span 15	Middle		0.0000	0.0000
SA7	Span 15	End	UDCONU2	17.3482	0.4365
SA7	Span 16	Start	UDCONU2	21.2532	0.5353
SA7	Span 16	Middle	UDCONU2	21.7066	0.5468
SA7	Span 16	End	UDCONU2	16.4686	0.4143
SA7	Span 17	Start	UDCONU2	16.4686	0.4143
SA7	Span 17	Middle	UDCONU2	3.3228	0.0833
SA7	Span 17	End	UDCONU2	0.2001	0.0000
SA7	Span 18	Start	UDCONU2	0.2001	0.0000
SA7	Span 18	Middle		0.0000	0.0000
SA7	Span 18	End	UDCONU2	0.0000	0.0000
SA7	Span 19	Start	UDCONU2	0.0000	0.0000
SA7	Span 19	Middle		0.0000	0.0000
SA7	Span 19	End	UDCONU2	4.0621	0.1018
SA7	Span 20	Start	UDCONU2	4.0621	0.1018
SA7	Span 20	Middle		0.0000	0.0000
SA7	Span 20	End	UDCONU2	5.4580	0.1432
SA7	Span 21	Start	UDCONU2	5.9562	0.1535
SA7	Span 21	Middle	UDCONU2	1.8156	0.0517
SA7	Span 21	End	UDCONU2	4.8140	0.1225
SA7	Span 22	Start	UDCONU2	4.8140	0.1225
SA7	Span 22	Middle		0.0000	0.0000
SA7	Span 22	End	UDCONU2	4.0927	0.1039
SA7	Span 23	Start	UDCONU2	4.0927	0.1039
SA7	Span 23	Middle	UDCONU2	1.4156	0.0408
SA7	Span 23	End	UDCONU2	2.0431	0.0548
SA7	Span 24	Start	UDCONU2	2.0431	0.0548
SA7	Span 24	Middle		0.0000	0.0000
SA7	Span 24	End		0.0000	0.0000
SA7	Span 25	Start		0.0000	0.0000
SA7	Span 25	Middle		0.0000	0.0000
SA7	Span 25	End		0.0000	0.0000
SA7	Span 26	Start		0.0000	0.0000
SA7	Span 26	Middle		0.0000	0.0000
SA7	Span 26	End	UDCONU2	0.3916	0.0125
SA7	Span 27	Start	UDCONU2	6.9265	0.1785
SA7	Span 27	Middle	UDCONU2	12.6740	0.3188
SA7	Span 27	End	UDCONU2	19.5722	0.4929
SA7	Span 28	Start	UDCONU2	19.5722	0.4929
SA7	Span 28	Middle	UDCONU2	15.2783	0.3848
SA7	Span 28	End		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA7	Span 29	Start		0.0000	0.0000
SA7	Span 29	Middle		0.0000	0.0000
SA7	Span 29	End		0.0000	0.0000
SA8	Span 1	Start		0.0000	0.0000
SA8	Span 1	Middle		0.0000	0.0000
SA8	Span 1	End		0.0000	0.0000
SA8	Span 2	Start		0.0000	0.0000
SA8	Span 2	Middle		0.0000	0.0000
SA8	Span 2	End		0.0000	0.0000
SA8	Span 3	Start		0.0000	0.0000
SA8	Span 3	Middle		0.0000	0.0000
SA8	Span 3	End		0.0000	0.0000
SA8	Span 4	Start		0.0000	0.0000
SA8	Span 4	Middle		0.0000	0.0000
SA8	Span 4	End		0.0000	0.0000
SA8	Span 5	Start		0.0000	0.0000
SA8	Span 5	Middle		0.0000	0.0000
SA8	Span 5	End	UDCONU2	23.4311	0.6207
SA8	Span 6	Start	UDCONU2	22.0929	0.5879
SA8	Span 6	Middle		0.0000	0.0000
SA8	Span 6	End	UDCONU2	8.7402	0.2216
SA8	Span 7	Start	UDCONU2	8.7402	0.2216
SA8	Span 7	Middle		0.0000	0.0000
SA8	Span 7	End	UDCONU2	27.7569	0.7004
SA8	Span 8	Start	UDCONU2	27.7569	0.7004
SA8	Span 8	Middle	UDCONU2	50.7990	1.2904
SA8	Span 8	End	UDCONU2	52.0074	1.3215
SA8	Span 9	Start	UDCONU2	52.0074	1.3215
SA8	Span 9	Middle	UDCONU2	55.3824	1.4091
SA8	Span 9	End	UDCONU2	67.0636	1.7116
SA8	Span 10	Start	UDCONU2	67.0636	1.7116
SA8	Span 10	Middle		0.0000	0.0000
SA8	Span 10	End	UDCONU2	30.3040	0.7684
SA8	Span 11	Start	UDCONU2	30.3040	0.7684
SA8	Span 11	Middle	UDCONU2	37.6940	0.9539
SA8	Span 11	End	UDCONU2	28.3726	0.7161
SA8	Span 12	Start	UDCONU2	28.3726	0.7161
SA8	Span 12	Middle	UDCONU2	14.9053	0.3748
SA8	Span 12	End	UDCONU2	0.0000	0.0000
SA8	Span 13	Start	UDCONU2	0.0000	0.0000
SA8	Span 13	Middle		0.0000	0.0000
SA8	Span 13	End	UDCONU2	0.8654	0.0000
SA8	Span 14	Start	UDCONU2	0.8654	0.0000
SA8	Span 14	Middle	UDCONU2	1.5453	0.0396
SA8	Span 14	End		0.0000	0.0000
SA8	Span 15	Start		0.0000	0.0000
SA8	Span 15	Middle		0.0000	0.0000
SA8	Span 15	End		0.0000	0.0000
SA8	Span 16	Start		0.0000	0.0000
SA8	Span 16	Middle		0.0000	0.0000
SA8	Span 16	End		0.0000	0.0000
SA8	Span 17	Start	UDCONU2	0.2808	0.0071
SA8	Span 17	Middle	UDCONU2	2.1944	0.0562

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA8	Span 17	End	UDCONU2	3.1296	0.0797
SA8	Span 18	Start	UDCONU2	3.1296	0.0797
SA8	Span 18	Middle	UDCONU2	2.7765	0.0708
SA8	Span 18	End		0.0000	0.0000
SA8	Span 19	Start		0.0000	0.0000
SA8	Span 19	Middle		0.0000	0.0000
SA8	Span 19	End		0.0000	0.0000
SA9	Span 1	Start		0.0000	0.0000
SA9	Span 1	Middle		0.0000	0.0000
SA9	Span 1	End		0.0000	0.0000
SA9	Span 2	Start		0.0000	0.0000
SA9	Span 2	Middle		0.0000	0.0000
SA9	Span 2	End		0.0000	0.0000
SA9	Span 3	Start		0.0000	0.0000
SA9	Span 3	Middle		0.0000	0.0000
SA9	Span 3	End		0.0000	0.0000
SA9	Span 4	Start		0.0000	0.0000
SA9	Span 4	Middle		0.0000	0.0000
SA9	Span 4	End		0.0000	0.0000
SA9	Span 5	Start		0.0000	0.0000
SA9	Span 5	Middle		0.0000	0.0000
SA9	Span 5	End	UDCONU2	19.8104	0.5651
SA9	Span 6	Start	UDCONU2	19.4520	0.4897
SA9	Span 6	Middle		0.0000	0.0000
SA9	Span 6	End	UDCONU2	3.3678	0.0847
SA9	Span 7	Start	UDCONU2	3.3678	0.0847
SA9	Span 7	Middle	UDCONU2	13.5211	0.3445
SA9	Span 7	End	UDCONU2	15.9048	0.4065
SA9	Span 8	Start	UDCONU2	15.9048	0.4065
SA9	Span 8	Middle	UDCONU2	24.3934	0.6312
SA9	Span 8	End	UDCONU2	26.4162	0.6860
SA9	Span 9	Start	UDCONU2	22.5554	0.5844
SA9	Span 9	Middle	UDCONU2	14.3320	0.3680
SA9	Span 9	End	UDCONU2	20.5116	0.5179
SA9	Span 10	Start	UDCONU2	20.5116	0.5179
SA9	Span 10	Middle	UDCONU2	26.7661	0.6752
SA9	Span 10	End	UDCONU2	29.4080	0.7424
SA9	Span 11	Start	UDCONU2	29.4080	0.7424
SA9	Span 11	Middle	UDCONU2	6.1730	0.1548
SA9	Span 11	End	UDCONU2	0.0000	0.0000
SA9	Span 12	Start	UDCONU2	0.0000	0.0000
SA9	Span 12	Middle		0.0000	0.0000
SA9	Span 12	End	UDCONU2	0.1101	0.0000
SA9	Span 13	Start	UDCONU2	0.1101	0.0000
SA9	Span 13	Middle	UDCONU2	0.7312	0.0197
SA9	Span 13	End		0.0000	0.0000
SA9	Span 14	Start		0.0000	0.0000
SA9	Span 14	Middle		0.0000	0.0000
SA9	Span 14	End		0.0000	0.0000
SA9	Span 15	Start		0.0000	0.0000
SA9	Span 15	Middle		0.0000	0.0000
SA9	Span 15	End		0.0000	0.0000
SA9	Span 16	Start		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA9	Span 16	Middle		0.0000	0.0000
SA9	Span 16	End		0.0000	0.0000
SA9	Span 17	Start		0.0000	0.0000
SA9	Span 17	Middle		0.0000	0.0000
SA9	Span 17	End		0.0000	0.0000
SA9	Span 18	Start		0.0000	0.0000
SA9	Span 18	Middle		0.0000	0.0000
SA9	Span 18	End		0.0000	0.0000
SB1	Span 1	Start	UDCONU2	1.3926	0.0388
SB1	Span 1	Middle	UDCONU2	3.3058	0.0909
SB1	Span 1	End		0.0000	0.0000
SB1	Span 2	Start		0.0000	0.0000
SB1	Span 2	Middle		0.0000	0.0000
SB1	Span 2	End		0.0000	0.0000
SB1	Span 3	Start		0.0000	0.0000
SB1	Span 3	Middle		0.0000	0.0000
SB1	Span 3	End		0.0000	0.0000
SB1	Span 4	Start		0.0000	0.0000
SB1	Span 4	Middle		0.0000	0.0000
SB1	Span 4	End		0.0000	0.0000
SB1	Span 5	Start		0.0000	0.0000
SB1	Span 5	Middle		0.0000	0.0000
SB1	Span 5	End		0.0000	0.0000
SB1	Span 6	Start		0.0000	0.0000
SB1	Span 6	Middle		0.0000	0.0000
SB1	Span 6	End		0.0000	0.0000
SB1	Span 7	Start		0.0000	0.0000
SB1	Span 7	Middle		0.0000	0.0000
SB1	Span 7	End		0.0000	0.0000
SB1	Span 8	Start		0.0000	0.0000
SB1	Span 8	Middle		0.0000	0.0000
SB1	Span 8	End		0.0000	0.0000
SB1	Span 9	Start		0.0000	0.0000
SB1	Span 9	Middle		0.0000	0.0000
SB1	Span 9	End		0.0000	0.0000
SB1	Span 10	Start		0.0000	0.0000
SB1	Span 10	Middle		0.0000	0.0000
SB1	Span 10	End		0.0000	0.0000
SB2	Span 1	Start	UDCONU2	3.4372	0.1104
SB2	Span 1	Middle		0.0000	0.0000
SB2	Span 1	End	UDCONU2	3.5342	0.1121
SB2	Span 2	Start	UDCONU2	3.5342	0.1121
SB2	Span 2	Middle		0.0000	0.0000
SB2	Span 2	End	UDCONU2	3.9106	0.1174
SB2	Span 3	Start	UDCONU2	3.9106	0.1174
SB2	Span 3	Middle	UDCONU2	14.1023	0.3875
SB2	Span 3	End	UDCONU2	0.1386	0.0000
SB2	Span 4	Start	UDCONU2	0.1386	0.0000
SB2	Span 4	Middle		0.0000	0.0000
SB2	Span 4	End	UDCONU2	0.0000	0.0000
SB2	Span 5	Start	UDCONU2	0.0000	0.0000
SB2	Span 5	Middle		0.0000	0.0000
SB2	Span 5	End	UDCONU2	0.3834	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB2	Span 6	Start	UDCONU2	1.0006	0.0290
SB2	Span 6	Middle	UDCONU2	1.3887	0.0385
SB2	Span 6	End		0.0000	0.0000
SB2	Span 7	Start		0.0000	0.0000
SB2	Span 7	Middle		0.0000	0.0000
SB2	Span 7	End		0.0000	0.0000
SB2	Span 8	Start		0.0000	0.0000
SB2	Span 8	Middle		0.0000	0.0000
SB2	Span 8	End		0.0000	0.0000
SB2	Span 9	Start		0.0000	0.0000
SB2	Span 9	Middle		0.0000	0.0000
SB2	Span 9	End		0.0000	0.0000
SB2	Span 10	Start		0.0000	0.0000
SB2	Span 10	Middle		0.0000	0.0000
SB2	Span 10	End		0.0000	0.0000
SB2	Span 11	Start		0.0000	0.0000
SB2	Span 11	Middle		0.0000	0.0000
SB2	Span 11	End		0.0000	0.0000
SB2	Span 12	Start		0.0000	0.0000
SB2	Span 12	Middle		0.0000	0.0000
SB2	Span 12	End		0.0000	0.0000
SB2	Span 13	Start		0.0000	0.0000
SB2	Span 13	Middle		0.0000	0.0000
SB2	Span 13	End		0.0000	0.0000
SB2	Span 14	Start		0.0000	0.0000
SB2	Span 14	Middle		0.0000	0.0000
SB2	Span 14	End		0.0000	0.0000
SB3	Span 1	Start	UDCONU2	7.5505	0.2146
SB3	Span 1	Middle	UDCONU2	25.1934	0.7031
SB3	Span 1	End	UDCONU2	4.6650	0.1436
SB3	Span 2	Start	UDCONU2	4.6650	0.1436
SB3	Span 2	Middle		0.0000	0.0000
SB3	Span 2	End	UDCONU2	4.5782	0.1422
SB3	Span 3	Start	UDCONU2	4.5782	0.1422
SB3	Span 3	Middle		0.0000	0.0000
SB3	Span 3	End	UDCONU2	3.5865	0.1021
SB3	Span 4	Start	UDCONU2	3.5865	0.1021
SB3	Span 4	Middle	UDCONU2	14.3017	0.3930
SB3	Span 4	End	UDCONU2	1.1464	0.0464
SB3	Span 5	Start	UDCONU2	1.1464	0.0464
SB3	Span 5	Middle		0.0000	0.0000
SB3	Span 5	End	UDCONU2	0.3547	0.0000
SB3	Span 6	Start	UDCONU2	0.3547	0.0000
SB3	Span 6	Middle		0.0000	0.0000
SB3	Span 6	End	UDCONU2	0.1378	0.0000
SB3	Span 7	Start	UDCONU2	0.1378	0.0000
SB3	Span 7	Middle		0.0000	0.0000
SB3	Span 7	End	UDCONU2	1.2062	0.0378
SB3	Span 8	Start	UDCONU2	1.6657	0.0467
SB3	Span 8	Middle	UDCONU2	5.0645	0.1395
SB3	Span 8	End	UDCONU2	2.9872	0.0847
SB3	Span 9	Start	UDCONU2	2.9872	0.0847
SB3	Span 9	Middle		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB3	Span 9	End	UDCONU2	2.6966	0.0760
SB3	Span 10	Start	UDCONU2	2.6966	0.0760
SB3	Span 10	Middle		0.0000	0.0000
SB3	Span 10	End		0.0000	0.0000
SB3	Span 11	Start		0.0000	0.0000
SB3	Span 11	Middle		0.0000	0.0000
SB3	Span 11	End		0.0000	0.0000
SB3	Span 12	Start		0.0000	0.0000
SB3	Span 12	Middle		0.0000	0.0000
SB3	Span 12	End		0.0000	0.0000
SB3	Span 13	Start		0.0000	0.0000
SB3	Span 13	Middle		0.0000	0.0000
SB3	Span 13	End		0.0000	0.0000
SB3	Span 14	Start		0.0000	0.0000
SB3	Span 14	Middle		0.0000	0.0000
SB3	Span 14	End		0.0000	0.0000
SB3	Span 15	Start		0.0000	0.0000
SB3	Span 15	Middle		0.0000	0.0000
SB3	Span 15	End		0.0000	0.0000
SB3	Span 16	Start		0.0000	0.0000
SB3	Span 16	Middle		0.0000	0.0000
SB3	Span 16	End		0.0000	0.0000
SB3	Span 17	Start		0.0000	0.0000
SB3	Span 17	Middle		0.0000	0.0000
SB3	Span 17	End		0.0000	0.0000
SB4	Span 1	Start	UDCONU1	2.8853	0.0815
SB4	Span 1	Middle		0.0000	0.0000
SB4	Span 1	End	UDCONU1	2.7887	0.0774
SB4	Span 2	Start	UDCONU1	2.7887	0.0774
SB4	Span 2	Middle	UDCONU2	21.8103	0.6068
SB4	Span 2	End	UDCONU2	16.5848	0.4595
SB4	Span 3	Start	UDCONU2	16.5848	0.4595
SB4	Span 3	Middle	UDCONU2	17.6745	0.4896
SB4	Span 3	End	UDCONU2	6.5350	0.1840
SB4	Span 4	Start	UDCONU2	6.5350	0.1840
SB4	Span 4	Middle	UDCONU2	17.3074	0.4774
SB4	Span 4	End	UDCONU2	7.1491	0.2057
SB4	Span 5	Start		0.0000	0.0000
SB4	Span 5	Middle		0.0000	0.0000
SB4	Span 5	End		0.0000	0.0000
SB4	Span 6	Start		0.0000	0.0000
SB4	Span 6	Middle		0.0000	0.0000
SB4	Span 6	End		0.0000	0.0000
SB4	Span 7	Start		0.0000	0.0000
SB4	Span 7	Middle		0.0000	0.0000
SB4	Span 7	End		0.0000	0.0000
SB4	Span 8	Start		0.0000	0.0000
SB4	Span 8	Middle		0.0000	0.0000
SB4	Span 8	End		0.0000	0.0000
SB4	Span 9	Start		0.0000	0.0000
SB4	Span 9	Middle		0.0000	0.0000
SB4	Span 9	End		0.0000	0.0000
SB4	Span 10	Start		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB4	Span 10	Middle		0.0000	0.0000
SB4	Span 10	End		0.0000	0.0000
SB4	Span 11	Start		0.0000	0.0000
SB4	Span 11	Middle		0.0000	0.0000
SB4	Span 11	End		0.0000	0.0000
SB5	Span 1	Start	UDCONU2	3.5920	0.1059
SB5	Span 1	Middle	UDCONU2	14.5795	0.4116
SB5	Span 1	End	UDCONU2	15.1525	0.4217
SB5	Span 2	Start	UDCONU2	15.1525	0.4217
SB5	Span 2	Middle		0.0000	0.0000
SB5	Span 2	End	UDCONU2	15.5090	0.4345
SB5	Span 3	Start	UDCONU2	15.5090	0.4345
SB5	Span 3	Middle	UDCONU2	15.9192	0.4455
SB5	Span 3	End	UDCONU2	15.3366	0.4309
SB5	Span 4	Start	UDCONU2	15.3366	0.4309
SB5	Span 4	Middle	UDCONU2	11.5805	0.3272
SB5	Span 4	End	UDCONU2	11.5625	0.3233
SB5	Span 5	Start	UDCONU2	11.5625	0.3233
SB5	Span 5	Middle	UDCONU2	16.3084	0.4512
SB5	Span 5	End	UDCONU2	2.2768	0.0649
SB5	Span 6	Start		0.0000	0.0000
SB5	Span 6	Middle		0.0000	0.0000
SB5	Span 6	End		0.0000	0.0000
SB5	Span 7	Start		0.0000	0.0000
SB5	Span 7	Middle		0.0000	0.0000
SB5	Span 7	End		0.0000	0.0000
SB5	Span 8	Start		0.0000	0.0000
SB5	Span 8	Middle		0.0000	0.0000
SB5	Span 8	End		0.0000	0.0000
SB5	Span 9	Start		0.0000	0.0000
SB5	Span 9	Middle		0.0000	0.0000
SB5	Span 9	End		0.0000	0.0000
SB5	Span 10	Start		0.0000	0.0000
SB5	Span 10	Middle		0.0000	0.0000
SB5	Span 10	End		0.0000	0.0000
SB5	Span 11	Start		0.0000	0.0000
SB5	Span 11	Middle		0.0000	0.0000
SB5	Span 11	End		0.0000	0.0000
SB5	Span 12	Start		0.0000	0.0000
SB5	Span 12	Middle		0.0000	0.0000
SB5	Span 12	End		0.0000	0.0000
SB6	Span 1	Start	UDCONU2	8.9457	0.2493
SB6	Span 1	Middle	UDCONU2	12.7153	0.3564
SB6	Span 1	End	UDCONU2	10.7552	0.2952
SB6	Span 2	Start	UDCONU2	10.7552	0.2952
SB6	Span 2	Middle	UDCONU2	7.0911	0.2065
SB6	Span 2	End	UDCONU2	4.3379	0.1424
SB6	Span 3	Start	UDCONU2	4.3379	0.1424
SB6	Span 3	Middle	UDCONU2	10.3460	0.3023
SB6	Span 3	End	UDCONU2	8.8590	0.2515
SB6	Span 4	Start	UDCONU2	8.8590	0.2515
SB6	Span 4	Middle		0.0000	0.0000
SB6	Span 4	End	UDCONU2	8.9699	0.2560

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB6	Span 5	Start	UDCONU2	8.9699	0.2560
SB6	Span 5	Middle		0.0000	0.0000
SB6	Span 5	End	UDCONU2	9.7452	0.2782
SB6	Span 6	Start	UDCONU2	10.5122	0.3001
SB6	Span 6	Middle	UDCONU2	13.6672	0.3776
SB6	Span 6	End	UDCONU2	4.9735	0.1371
SB6	Span 7	Start		0.0000	0.0000
SB6	Span 7	Middle		0.0000	0.0000
SB6	Span 7	End		0.0000	0.0000
SB6	Span 8	Start		0.0000	0.0000
SB6	Span 8	Middle		0.0000	0.0000
SB6	Span 8	End		0.0000	0.0000
SB6	Span 9	Start		0.0000	0.0000
SB6	Span 9	Middle		0.0000	0.0000
SB6	Span 9	End		0.0000	0.0000
SB6	Span 10	Start		0.0000	0.0000
SB6	Span 10	Middle		0.0000	0.0000
SB6	Span 10	End		0.0000	0.0000
SB6	Span 11	Start		0.0000	0.0000
SB6	Span 11	Middle		0.0000	0.0000
SB6	Span 11	End		0.0000	0.0000
SB6	Span 12	Start		0.0000	0.0000
SB6	Span 12	Middle		0.0000	0.0000
SB6	Span 12	End		0.0000	0.0000
SB6	Span 13	Start		0.0000	0.0000
SB6	Span 13	Middle		0.0000	0.0000
SB6	Span 13	End		0.0000	0.0000
SB7	Span 1	Start	UDCONU2	6.8653	0.1910
SB7	Span 1	Middle		0.0000	0.0000
SB7	Span 1	End	UDCONU2	7.2631	0.2053
SB7	Span 2	Start	UDCONU2	7.2631	0.2053
SB7	Span 2	Middle	UDCONU2	2.5843	0.0707
SB7	Span 2	End	UDCONU2	6.2540	0.1807
SB7	Span 3	Start	UDCONU2	6.2540	0.1807
SB7	Span 3	Middle	UDCONU2	5.2680	0.1504
SB7	Span 3	End	UDCONU2	5.8094	0.1679
SB7	Span 4	Start	UDCONU2	5.8094	0.1679
SB7	Span 4	Middle		0.0000	0.0000
SB7	Span 4	End	UDCONU2	19.0045	0.5231
SB7	Span 5	Start	UDCONU2	19.0045	0.5231
SB7	Span 5	Middle		0.0000	0.0000
SB7	Span 5	End	UDCONU2	22.5945	0.6226
SB7	Span 6	Start	UDCONU2	22.5945	0.6226
SB7	Span 6	Middle		0.0000	0.0000
SB7	Span 6	End	UDCONU2	9.1646	0.2541
SB7	Span 7	Start	UDCONU2	9.1646	0.2541
SB7	Span 7	Middle	UDCONU2	5.5045	0.1508
SB7	Span 7	End	UDCONU2	2.0920	0.0963
SB7	Span 8	Start	UDCONU2	2.0920	0.0963
SB7	Span 8	Middle		0.0000	0.0000
SB7	Span 8	End	UDCONU2	2.0853	0.0960
SB7	Span 9	Start	UDCONU2	2.0853	0.0960
SB7	Span 9	Middle	UDCONU2	2.1411	0.0956

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB7	Span 9	End	UDCONU2	4.7959	0.1415
SB7	Span 10	Start	UDCONU2	4.7959	0.1415
SB7	Span 10	Middle		0.0000	0.0000
SB7	Span 10	End	UDCONU2	6.9752	0.1912
SB7	Span 11	Start	UDCONU2	6.9752	0.1912
SB7	Span 11	Middle		0.0000	0.0000
SB7	Span 11	End	UDCONU2	6.3309	0.1735
SB7	Span 12	Start	UDCONU2	6.3309	0.1735
SB7	Span 12	Middle	UDCONU2	6.7760	0.1913
SB7	Span 12	End	UDCONU2	5.7323	0.1641
SB7	Span 13	Start	UDCONU2	5.7323	0.1641
SB7	Span 13	Middle		0.0000	0.0000
SB7	Span 13	End		0.0000	0.0000
SB7	Span 14	Start		0.0000	0.0000
SB7	Span 14	Middle		0.0000	0.0000
SB7	Span 14	End		0.0000	0.0000
SB7	Span 15	Start		0.0000	0.0000
SB7	Span 15	Middle		0.0000	0.0000
SB7	Span 15	End		0.0000	0.0000
SB7	Span 16	Start		0.0000	0.0000
SB7	Span 16	Middle		0.0000	0.0000
SB7	Span 16	End		0.0000	0.0000
SB7	Span 17	Start		0.0000	0.0000
SB7	Span 17	Middle		0.0000	0.0000
SB7	Span 17	End		0.0000	0.0000
SB7	Span 18	Start		0.0000	0.0000
SB7	Span 18	Middle		0.0000	0.0000
SB7	Span 18	End		0.0000	0.0000
SB7	Span 19	Start		0.0000	0.0000
SB7	Span 19	Middle		0.0000	0.0000
SB7	Span 19	End		0.0000	0.0000
SB7	Span 20	Start		0.0000	0.0000
SB7	Span 20	Middle		0.0000	0.0000
SB7	Span 20	End		0.0000	0.0000
SB8	Span 1	Start	UDCONU2	1.1542	0.0341
SB8	Span 1	Middle	UDCONU2	1.9282	0.0550
SB8	Span 1	End	UDCONU2	11.2958	0.3306
SB8	Span 2	Start	UDCONU2	11.2958	0.3306
SB8	Span 2	Middle	UDCONU2	18.2509	0.5207
SB8	Span 2	End	UDCONU2	8.7250	0.2430
SB8	Span 3	Start	UDCONU2	8.7250	0.2430
SB8	Span 3	Middle		0.0000	0.0000
SB8	Span 3	End	UDCONU2	8.9647	0.2496
SB8	Span 4	Start	UDCONU2	8.9647	0.2496
SB8	Span 4	Middle	UDCONU2	15.9879	0.4396
SB8	Span 4	End	UDCONU2	4.1608	0.1140
SB8	Span 5	Start	UDCONU2	4.1608	0.1140
SB8	Span 5	Middle		0.0000	0.0000
SB8	Span 5	End	UDCONU2	5.9488	0.1630
SB8	Span 6	Start	UDCONU2	5.9488	0.1630
SB8	Span 6	Middle		0.0000	0.0000
SB8	Span 6	End	UDCONU2	1.1972	0.0429
SB8	Span 7	Start	UDCONU2	1.1972	0.0429

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB8	Span 7	Middle	UDCONU2	3.9854	0.1139
SB8	Span 7	End	UDCONU2	7.8724	0.2218
SB8	Span 8	Start	UDCONU2	7.8724	0.2218
SB8	Span 8	Middle		0.0000	0.0000
SB8	Span 8	End		0.0000	0.0000
SB8	Span 9	Start		0.0000	0.0000
SB8	Span 9	Middle		0.0000	0.0000
SB8	Span 9	End		0.0000	0.0000
SB8	Span 10	Start		0.0000	0.0000
SB8	Span 10	Middle		0.0000	0.0000
SB8	Span 10	End		0.0000	0.0000
SB8	Span 11	Start		0.0000	0.0000
SB8	Span 11	Middle		0.0000	0.0000
SB8	Span 11	End		0.0000	0.0000
SB8	Span 12	Start		0.0000	0.0000
SB8	Span 12	Middle		0.0000	0.0000
SB8	Span 12	End		0.0000	0.0000
SB8	Span 13	Start		0.0000	0.0000
SB8	Span 13	Middle	UDCONU2	0.2894	0.0167
SB8	Span 13	End	UDCONU2	0.0000	0.0000
SB8	Span 14	Start	UDCONU2	0.0000	0.0000
SB8	Span 14	Middle	UDCONU2	0.2289	0.0156
SB8	Span 14	End	UDCONU2	0.0000	0.0000
SB8	Span 15	Start	UDCONU2	0.0000	0.0000
SB8	Span 15	Middle	UDCONU2	0.0000	0.0000
SB8	Span 15	End	UDCONU2	0.3518	0.0101
SB9	Span 1	Start		0.0000	0.0000
SB9	Span 1	Middle		0.0000	0.0000
SB9	Span 1	End	UDCONU2	4.6100	0.1405
SB9	Span 2	Start	UDCONU2	4.6100	0.1405
SB9	Span 2	Middle	UDCONU2	1.8677	0.0602
SB9	Span 2	End	UDCONU2	2.2588	0.0969
SB9	Span 3	Start	UDCONU2	2.2588	0.0969
SB9	Span 3	Middle	UDCONU2	14.3803	0.4197
SB9	Span 3	End	UDCONU2	14.2636	0.4148
SB9	Span 4	Start	UDCONU2	14.2636	0.4148
SB9	Span 4	Middle	UDCONU2	26.4230	0.7311
SB9	Span 4	End	UDCONU2	34.1984	0.9466
SB9	Span 5	Start	UDCONU2	34.1984	0.9466
SB9	Span 5	Middle	UDCONU2	34.3460	0.9503
SB9	Span 5	End	UDCONU2	9.1280	0.2639
SB9	Span 6	Start	UDCONU2	9.1280	0.2639
SB9	Span 6	Middle	UDCONU2	22.5779	0.6483
SB9	Span 6	End	UDCONU2	15.2015	0.4297
SB9	Span 7	Start		0.0000	0.0000
SB9	Span 7	Middle		0.0000	0.0000
SB9	Span 7	End		0.0000	0.0000
SB9	Span 8	Start		0.0000	0.0000
SB9	Span 8	Middle		0.0000	0.0000
SB9	Span 8	End		0.0000	0.0000
SB9	Span 9	Start		0.0000	0.0000
SB9	Span 9	Middle		0.0000	0.0000
SB9	Span 9	End		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB9	Span 10	Start		0.0000	0.0000
SB9	Span 10	Middle		0.0000	0.0000
SB9	Span 10	End	UDCONU2	3.2835	0.0933
SB9	Span 11	Start	UDCONU2	3.2835	0.0933
SB9	Span 11	Middle	UDCONU2	3.2797	0.1203
SB9	Span 11	End	UDCONU2	0.0000	0.0352
SB9	Span 12	Start	UDCONU2	0.0000	0.0352
SB9	Span 12	Middle	UDCONU2	0.0000	0.0348
SB9	Span 12	End	UDCONU2	0.0000	0.0373
SB9	Span 13	Start	UDCONU2	0.0000	0.0373
SB9	Span 13	Middle	UDCONU2	0.0000	0.0590
SB9	Span 13	End	UDCONU2	0.0000	0.0642
SB9	Span 14	Start	UDCONU2	0.0000	0.0642
SB9	Span 14	Middle		0.0000	0.0000
SB9	Span 14	End	UDCONU2	0.0000	0.0734
SB9	Span 15	Start	UDCONU2	0.0000	0.0734
SB9	Span 15	Middle	UDCONU2	0.7329	0.0000
SB9	Span 15	End	UDCONU2	0.6760	0.0000
SA10	Span 1	Start		0.0000	0.0000
SA10	Span 1	Middle		0.0000	0.0000
SA10	Span 1	End		0.0000	0.0000
SA10	Span 2	Start		0.0000	0.0000
SA10	Span 2	Middle		0.0000	0.0000
SA10	Span 2	End		0.0000	0.0000
SA10	Span 3	Start		0.0000	0.0000
SA10	Span 3	Middle		0.0000	0.0000
SA10	Span 3	End		0.0000	0.0000
SA10	Span 4	Start		0.0000	0.0000
SA10	Span 4	Middle		0.0000	0.0000
SA10	Span 4	End		0.0000	0.0000
SA10	Span 5	Start		0.0000	0.0000
SA10	Span 5	Middle		0.0000	0.0000
SA10	Span 5	End	UDCONU2	26.3613	0.7859
SA10	Span 6	Start	UDCONU2	26.3613	0.7859
SA10	Span 6	Middle		0.0000	0.0000
SA10	Span 6	End		0.0000	0.0000
SA10	Span 7	Start		0.0000	0.0000
SA10	Span 7	Middle		0.0000	0.0000
SA10	Span 7	End		0.0000	0.0000
SA10	Span 8	Start		0.0000	0.0000
SA10	Span 8	Middle		0.0000	0.0000
SA10	Span 8	End		0.0000	0.0000
SA10	Span 9	Start		0.0000	0.0000
SA10	Span 9	Middle		0.0000	0.0000
SA10	Span 9	End	UDCONU2	8.6320	0.2169
SA10	Span 10	Start	UDCONU2	13.8500	0.3483
SA10	Span 10	Middle		0.0000	0.0000
SA10	Span 10	End	UDCONU2	11.2381	0.2833
SA10	Span 11	Start	UDCONU2	11.2381	0.2833
SA10	Span 11	Middle	UDCONU2	0.8371	0.0000
SA10	Span 11	End	UDCONU2	0.0000	0.0000
SA10	Span 12	Start	UDCONU2	0.0000	0.0000
SA10	Span 12	Middle		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA10	Span 12	End	UDCONU2	0.0000	0.0000
SA10	Span 13	Start	UDCONU2	0.0000	0.0000
SA10	Span 13	Middle	UDCONU2	2.1552	0.0562
SA10	Span 13	End		0.0000	0.0000
SA10	Span 14	Start		0.0000	0.0000
SA10	Span 14	Middle		0.0000	0.0000
SA10	Span 14	End		0.0000	0.0000
SA10	Span 15	Start		0.0000	0.0000
SA10	Span 15	Middle		0.0000	0.0000
SA10	Span 15	End		0.0000	0.0000
SA10	Span 16	Start		0.0000	0.0000
SA10	Span 16	Middle		0.0000	0.0000
SA10	Span 16	End		0.0000	0.0000
SA10	Span 17	Start		0.0000	0.0000
SA10	Span 17	Middle		0.0000	0.0000
SA10	Span 17	End		0.0000	0.0000
SA10	Span 18	Start		0.0000	0.0000
SA10	Span 18	Middle		0.0000	0.0000
SA10	Span 18	End		0.0000	0.0000
SA11	Span 1	Start		0.0000	0.0000
SA11	Span 1	Middle		0.0000	0.0000
SA11	Span 1	End		0.0000	0.0000
SA11	Span 2	Start		0.0000	0.0000
SA11	Span 2	Middle		0.0000	0.0000
SA11	Span 2	End		0.0000	0.0000
SA11	Span 3	Start		0.0000	0.0000
SA11	Span 3	Middle		0.0000	0.0000
SA11	Span 3	End		0.0000	0.0000
SA11	Span 4	Start		0.0000	0.0000
SA11	Span 4	Middle		0.0000	0.0000
SA11	Span 4	End		0.0000	0.0000
SA11	Span 5	Start		0.0000	0.0000
SA11	Span 5	Middle		0.0000	0.0000
SA11	Span 5	End	UDCONU2	10.6327	0.3761
SA11	Span 6	Start	UDCONU2	10.6327	0.3761
SA11	Span 6	Middle		0.0000	0.0000
SA11	Span 6	End		0.0000	0.0000
SA11	Span 7	Start		0.0000	0.0000
SA11	Span 7	Middle		0.0000	0.0000
SA11	Span 7	End		0.0000	0.0000
SA11	Span 8	Start		0.0000	0.0000
SA11	Span 8	Middle		0.0000	0.0000
SA11	Span 8	End		0.0000	0.0000
SA11	Span 9	Start		0.0000	0.0000
SA11	Span 9	Middle		0.0000	0.0000
SA11	Span 9	End	UDCONU2	3.9187	0.1006
SA11	Span 10	Start	UDCONU2	3.9187	0.1006
SA11	Span 10	Middle		0.0000	0.0000
SA11	Span 10	End	UDCONU2	2.7646	0.0764
SA11	Span 11	Start	UDCONU2	2.7646	0.0764
SA11	Span 11	Middle	UDCONU2	2.2358	0.0655
SA11	Span 11	End	UDCONU2	2.7136	0.0778
SA11	Span 12	Start	UDCONU2	2.7136	0.0778

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA11	Span 12	Middle	UDCONU2	0.2913	0.0000
SA11	Span 12	End	UDCONU2	0.0000	0.0000
SA11	Span 13	Start	UDCONU2	0.0000	0.0000
SA11	Span 13	Middle		0.0000	0.0000
SA11	Span 13	End	UDCONU2	13.5430	0.3483
SA11	Span 14	Start	UDCONU2	0.0000	0.0000
SA11	Span 14	Middle	UDCONU2	1.4783	0.0453
SA11	Span 14	End	UDCONU2	23.3190	0.5931
SA11	Span 15	Start	UDCONU2	23.3190	0.5931
SA11	Span 15	Middle	UDCONU2	19.1772	0.4876
SA11	Span 15	End	UDCONU2	4.7601	0.1212
SA11	Span 16	Start	UDCONU2	4.7601	0.1212
SA11	Span 16	Middle		0.0000	0.0000
SA11	Span 16	End		0.0000	0.0000
SA11	Span 17	Start		0.0000	0.0000
SA11	Span 17	Middle		0.0000	0.0000
SA11	Span 17	End		0.0000	0.0000
SA11	Span 18	Start		0.0000	0.0000
SA11	Span 18	Middle		0.0000	0.0000
SA11	Span 18	End		0.0000	0.0000
SA11	Span 19	Start		0.0000	0.0000
SA11	Span 19	Middle		0.0000	0.0000
SA11	Span 19	End		0.0000	0.0000
SA11	Span 20	Start		0.0000	0.0000
SA11	Span 20	Middle		0.0000	0.0000
SA11	Span 20	End		0.0000	0.0000
SA12	Span 1	Start		0.0000	0.0000
SA12	Span 1	Middle		0.0000	0.0000
SA12	Span 1	End		0.0000	0.0000
SA12	Span 2	Start		0.0000	0.0000
SA12	Span 2	Middle		0.0000	0.0000
SA12	Span 2	End		0.0000	0.0000
SA12	Span 3	Start		0.0000	0.0000
SA12	Span 3	Middle		0.0000	0.0000
SA12	Span 3	End		0.0000	0.0000
SA12	Span 4	Start		0.0000	0.0000
SA12	Span 4	Middle		0.0000	0.0000
SA12	Span 4	End		0.0000	0.0000
SA12	Span 5	Start		0.0000	0.0000
SA12	Span 5	Middle		0.0000	0.0000
SA12	Span 5	End	UDCONU2	4.3560	0.1474
SA12	Span 6	Start	UDCONU2	4.3560	0.1474
SA12	Span 6	Middle		0.0000	0.0000
SA12	Span 6	End		0.0000	0.0000
SA12	Span 7	Start		0.0000	0.0000
SA12	Span 7	Middle		0.0000	0.0000
SA12	Span 7	End		0.0000	0.0000
SA12	Span 8	Start		0.0000	0.0000
SA12	Span 8	Middle		0.0000	0.0000
SA12	Span 8	End		0.0000	0.0000
SA12	Span 9	Start		0.0000	0.0000
SA12	Span 9	Middle		0.0000	0.0000
SA12	Span 9	End	UDCONU2	0.2908	0.0086

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA12	Span 10	Start	UDCONU2	0.2908	0.0086
SA12	Span 10	Middle	UDCONU2	2.0720	0.0534
SA12	Span 10	End	UDCONU2	2.5510	0.0642
SA12	Span 11	Start	UDCONU2	4.1385	0.1038
SA12	Span 11	Middle	UDCONU2	1.6134	0.0404
SA12	Span 11	End	UDCONU2	0.0000	0.0000
SA12	Span 12	Start	UDCONU2	0.0000	0.0000
SA12	Span 12	Middle		0.0000	0.0000
SA12	Span 12	End	UDCONU2	0.7276	0.0000
SA12	Span 13	Start		0.0000	0.0000
SA12	Span 13	Middle	UDCONU2	26.4690	0.6745
SA12	Span 13	End	UDCONU2	50.6061	1.2915
SA12	Span 14	Start	UDCONU2	50.6061	1.2915
SA12	Span 14	Middle		0.0000	0.0000
SA12	Span 14	End	UDCONU2	26.7158	0.6843
SA12	Span 15	Start	UDCONU2	22.5757	0.5783
SA12	Span 15	Middle		0.0000	0.0000
SA12	Span 15	End		0.0000	0.0000
SA12	Span 16	Start		0.0000	0.0000
SA12	Span 16	Middle		0.0000	0.0000
SA12	Span 16	End		0.0000	0.0000
SA12	Span 17	Start		0.0000	0.0000
SA12	Span 17	Middle		0.0000	0.0000
SA12	Span 17	End		0.0000	0.0000
SA12	Span 18	Start		0.0000	0.0000
SA12	Span 18	Middle		0.0000	0.0000
SA12	Span 18	End		0.0000	0.0000
SA12	Span 19	Start		0.0000	0.0000
SA12	Span 19	Middle		0.0000	0.0000
SA12	Span 19	End		0.0000	0.0000
SA13	Span 1	Start		0.0000	0.0000
SA13	Span 1	Middle		0.0000	0.0000
SA13	Span 1	End		0.0000	0.0000
SA13	Span 2	Start		0.0000	0.0000
SA13	Span 2	Middle		0.0000	0.0000
SA13	Span 2	End		0.0000	0.0000
SA13	Span 3	Start		0.0000	0.0000
SA13	Span 3	Middle		0.0000	0.0000
SA13	Span 3	End		0.0000	0.0000
SA13	Span 4	Start		0.0000	0.0000
SA13	Span 4	Middle		0.0000	0.0000
SA13	Span 4	End		0.0000	0.0000
SA13	Span 5	Start		0.0000	0.0000
SA13	Span 5	Middle		0.0000	0.0000
SA13	Span 5	End	UDCONU2	3.3956	0.1285
SA13	Span 6	Start	UDCONU2	3.3956	0.1285
SA13	Span 6	Middle		0.0000	0.0000
SA13	Span 6	End		0.0000	0.0000
SA13	Span 7	Start		0.0000	0.0000
SA13	Span 7	Middle		0.0000	0.0000
SA13	Span 7	End		0.0000	0.0000
SA13	Span 8	Start		0.0000	0.0000
SA13	Span 8	Middle		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA13	Span 8	End		0.0000	0.0000
SA13	Span 9	Start		0.0000	0.0000
SA13	Span 9	Middle	UDCONU2	0.2160	0.0055
SA13	Span 9	End	UDCONU2	0.1321	0.0033
SA13	Span 10	Start		0.0000	0.0000
SA13	Span 10	Middle		0.0000	0.0000
SA13	Span 10	End	UDCONU2	4.3726	0.1186
SA13	Span 11	Start	UDCONU2	18.6310	0.4789
SA13	Span 11	Middle	UDCONU2	2.6049	0.0709
SA13	Span 11	End	UDCONU2	0.0000	0.0000
SA13	Span 12	Start	UDCONU2	0.0000	0.0000
SA13	Span 12	Middle		0.0000	0.0000
SA13	Span 12	End	UDCONU2	23.2604	0.5929
SA13	Span 13	Start		0.0000	0.0000
SA13	Span 13	Middle		0.0000	0.0000
SA13	Span 13	End	UDCONU2	22.9079	0.5833
SA13	Span 14	Start	UDCONU2	22.9079	0.5833
SA13	Span 14	Middle		0.0000	0.0000
SA13	Span 14	End	UDCONU2	16.8433	0.4369
SA13	Span 15	Start	UDCONU2	16.8433	0.4369
SA13	Span 15	Middle		0.0000	0.0000
SA13	Span 15	End		0.0000	0.0000
SA13	Span 16	Start		0.0000	0.0000
SA13	Span 16	Middle		0.0000	0.0000
SA13	Span 16	End		0.0000	0.0000
SA13	Span 17	Start		0.0000	0.0000
SA13	Span 17	Middle		0.0000	0.0000
SA13	Span 17	End		0.0000	0.0000
SA13	Span 18	Start		0.0000	0.0000
SA13	Span 18	Middle		0.0000	0.0000
SA13	Span 18	End		0.0000	0.0000
SA13	Span 19	Start		0.0000	0.0000
SA13	Span 19	Middle		0.0000	0.0000
SA13	Span 19	End		0.0000	0.0000
SA14	Span 1	Start		0.0000	0.0000
SA14	Span 1	Middle		0.0000	0.0000
SA14	Span 1	End		0.0000	0.0000
SA14	Span 2	Start		0.0000	0.0000
SA14	Span 2	Middle		0.0000	0.0000
SA14	Span 2	End		0.0000	0.0000
SA14	Span 3	Start		0.0000	0.0000
SA14	Span 3	Middle		0.0000	0.0000
SA14	Span 3	End		0.0000	0.0000
SA14	Span 4	Start		0.0000	0.0000
SA14	Span 4	Middle		0.0000	0.0000
SA14	Span 4	End		0.0000	0.0000
SA14	Span 5	Start		0.0000	0.0000
SA14	Span 5	Middle		0.0000	0.0000
SA14	Span 5	End	UDCONU2	6.8312	0.2214
SA14	Span 6	Start	UDCONU2	6.8312	0.2214
SA14	Span 6	Middle		0.0000	0.0000
SA14	Span 6	End	UDCONU2	1.4304	0.0371
SA14	Span 7	Start	UDCONU2	1.4304	0.0371

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA14	Span 7	Middle	UDCONU2	2.0001	0.0502
SA14	Span 7	End	UDCONU2	2.8757	0.0723
SA14	Span 8	Start	UDCONU2	2.8757	0.0723
SA14	Span 8	Middle	UDCONU2	5.4600	0.1379
SA14	Span 8	End	UDCONU1	2.2204	0.0589
SA14	Span 9	Start	UDCONU1	2.2204	0.0589
SA14	Span 9	Middle	UDCONU1	0.4699	0.0000
SA14	Span 9	End	UDCONU2	2.5466	0.0750
SA14	Span 10	Start	UDCONU2	2.5466	0.0750
SA14	Span 10	Middle	UDCONU2	13.7919	0.3539
SA14	Span 10	End	UDCONU2	15.7483	0.3961
SA14	Span 11	Start	UDCONU2	15.7483	0.3961
SA14	Span 11	Middle		0.0000	0.0000
SA14	Span 11	End	UDCONU2	16.7721	0.4219
SA14	Span 12	Start	UDCONU2	16.7721	0.4219
SA14	Span 12	Middle		0.0000	0.0000
SA14	Span 12	End	UDCONU1	15.4881	0.3895
SA14	Span 13	Start	UDCONU1	15.4881	0.3895
SA14	Span 13	Middle	UDCONU1	20.9635	0.5280
SA14	Span 13	End	UDCONU2	6.3942	0.1674
SA14	Span 14	Start	UDCONU2	6.3942	0.1674
SA14	Span 14	Middle	UDCONU2	4.0513	0.1208
SA14	Span 14	End	UDCONU2	8.2171	0.2436
SA14	Span 15	Start	UDCONU2	8.2171	0.2436
SA14	Span 15	Middle		0.0000	0.0000
SA14	Span 15	End	UDCONU2	2.7265	0.0894
SA14	Span 16	Start	UDCONU2	2.7265	0.0894
SA14	Span 16	Middle		0.0000	0.0000
SA14	Span 16	End	UDCONU2	4.8699	0.1531
SA14	Span 17	Start	UDCONU2	4.8699	0.1531
SA14	Span 17	Middle	UDCONU2	21.2559	0.5658
SA14	Span 17	End	UDCONU2	29.0833	0.7652
SA14	Span 18	Start	UDCONU2	29.0833	0.7652
SA14	Span 18	Middle		0.0000	0.0000
SA14	Span 18	End	UDCONU2	28.0468	0.7395
SA14	Span 19	Start	UDCONU2	28.0468	0.7395
SA14	Span 19	Middle		0.0000	0.0000
SA14	Span 19	End	UDCONU2	17.2388	0.5184
SA14	Span 20	Start	UDCONU2	17.2388	0.5184
SA14	Span 20	Middle		0.0000	0.0000
SA14	Span 20	End	UDCONU2	28.7492	0.7987
SA14	Span 21	Start	UDCONU2	28.7492	0.7987
SA14	Span 21	Middle		0.0000	0.0000
SA14	Span 21	End	UDCONU2	23.9362	0.6734
SA14	Span 22	Start	UDCONU2	23.9362	0.6734
SA14	Span 22	Middle	UDCONU2	7.0450	0.1818
SA14	Span 22	End		0.0000	0.0000
SA14	Span 23	Start		0.0000	0.0000
SA14	Span 23	Middle		0.0000	0.0000
SA14	Span 23	End		0.0000	0.0000
SA14	Span 24	Start		0.0000	0.0000
SA14	Span 24	Middle		0.0000	0.0000
SA14	Span 24	End		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA14	Span 25	Start		0.0000	0.0000
SA14	Span 25	Middle		0.0000	0.0000
SA14	Span 25	End		0.0000	0.0000
SA14	Span 26	Start		0.0000	0.0000
SA14	Span 26	Middle		0.0000	0.0000
SA14	Span 26	End		0.0000	0.0000
SA15	Span 1	Start		0.0000	0.0000
SA15	Span 1	Middle		0.0000	0.0000
SA15	Span 1	End		0.0000	0.0000
SA15	Span 2	Start		0.0000	0.0000
SA15	Span 2	Middle		0.0000	0.0000
SA15	Span 2	End		0.0000	0.0000
SA15	Span 3	Start		0.0000	0.0000
SA15	Span 3	Middle		0.0000	0.0000
SA15	Span 3	End		0.0000	0.0000
SA15	Span 4	Start		0.0000	0.0000
SA15	Span 4	Middle	UDCONU2	0.1600	0.0093
SA15	Span 4	End	UDCONU2	0.1219	0.0280
SA15	Span 5	Start	UDCONU2	0.1219	0.0280
SA15	Span 5	Middle	UDCONU2	0.0000	0.0000
SA15	Span 5	End	UDCONU2	0.0407	0.0000
SA15	Span 6	Start	UDCONU2	0.0302	0.0000
SA15	Span 6	Middle		0.0000	0.0000
SA15	Span 6	End	UDCONU2	0.0000	0.0000
SA15	Span 7	Start	UDCONU2	0.0000	0.0000
SA15	Span 7	Middle		0.0000	0.0000
SA15	Span 7	End	UDCONU2	4.6837	0.1213
SA15	Span 8	Start	UDCONU2	4.6837	0.1213
SA15	Span 8	Middle	UDCONU2	7.8339	0.2031
SA15	Span 8	End	UDCONU2	11.5029	0.2889
SA15	Span 9	Start	UDCONU2	11.5029	0.2889
SA15	Span 9	Middle	UDCONU2	33.0127	0.8376
SA15	Span 9	End	UDCONU2	30.2737	0.7715
SA15	Span 10	Start	UDCONU2	30.2737	0.7715
SA15	Span 10	Middle	UDCONU2	23.8422	0.6165
SA15	Span 10	End	UDCONU1	11.3654	0.2902
SA15	Span 11	Start	UDCONU1	11.3654	0.2902
SA15	Span 11	Middle	UDCONU1	10.5089	0.2715
SA15	Span 11	End	UDCONU1	0.4238	0.0380
SA15	Span 12	Start	UDCONU1	0.4238	0.0380
SA15	Span 12	Middle	UDCONU1	7.6308	0.2091
SA15	Span 12	End	UDCONU2	20.6773	0.5341
SA15	Span 13	Start	UDCONU2	28.7157	0.7460
SA15	Span 13	Middle	UDCONU2	31.3669	0.8270
SA15	Span 13	End	UDCONU2	26.7714	0.7407
SA15	Span 14	Start	UDCONU2	8.8456	0.2862
SA15	Span 14	Middle		0.0000	0.0000
SA15	Span 14	End	UDCONU2	2.4643	0.1458
SA15	Span 15	Start	UDCONU2	2.4643	0.1458
SA15	Span 15	Middle	UDCONU2	8.1894	0.2088
SA15	Span 15	End		0.0000	0.0000
SA15	Span 16	Start		0.0000	0.0000
SA15	Span 16	Middle		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA15	Span 16	End		0.0000	0.0000
SA15	Span 17	Start		0.0000	0.0000
SA15	Span 17	Middle		0.0000	0.0000
SA15	Span 17	End		0.0000	0.0000
SA15	Span 18	Start		0.0000	0.0000
SA15	Span 18	Middle		0.0000	0.0000
SA15	Span 18	End		0.0000	0.0000
SA15	Span 19	Start		0.0000	0.0000
SA15	Span 19	Middle		0.0000	0.0000
SA15	Span 19	End		0.0000	0.0000
SA16	Span 1	Start		0.0000	0.0000
SA16	Span 1	Middle		0.0000	0.0000
SA16	Span 1	End		0.0000	0.0000
SA16	Span 2	Start		0.0000	0.0000
SA16	Span 2	Middle		0.0000	0.0000
SA16	Span 2	End		0.0000	0.0000
SA16	Span 3	Start		0.0000	0.0000
SA16	Span 3	Middle		0.0000	0.0000
SA16	Span 3	End		0.0000	0.0000
SA16	Span 4	Start		0.0000	0.0000
SA16	Span 4	Middle		0.0000	0.0000
SA16	Span 4	End	UDCONU2	0.9838	0.0000
SA16	Span 5	Start	UDCONU2	0.1666	0.0000
SA16	Span 5	Middle		0.0000	0.0000
SA16	Span 5	End	UDCONU2	0.0000	0.0741
SA16	Span 6	Start	UDCONU2	0.0000	0.0741
SA16	Span 6	Middle	UDCONU2	0.0000	0.0667
SA16	Span 6	End	UDCONU2	0.0000	0.0000
SA16	Span 7	Start	UDCONU2	0.0000	0.0000
SA16	Span 7	Middle		0.0000	0.0000
SA16	Span 7	End	UDCONU2	0.0000	0.0000
SA16	Span 8	Start	UDCONU2	0.0000	0.0000
SA16	Span 8	Middle		0.0000	0.0000
SA16	Span 8	End	UDCONU2	0.0000	0.0000
SA16	Span 9	Start	UDCONU2	0.0000	0.0000
SA16	Span 9	Middle	UDCONU2	29.0616	0.7511
SA16	Span 9	End	UDCONU2	40.4325	1.0391
SA16	Span 10	Start	UDCONU2	40.4325	1.0391
SA16	Span 10	Middle	UDCONU2	40.7415	1.0412
SA16	Span 10	End	UDCONU2	30.5911	0.7871
SA16	Span 11	Start	UDCONU2	30.5911	0.7871
SA16	Span 11	Middle	UDCONU2	24.3702	0.6299
SA16	Span 11	End	UDCONU1	13.3166	0.3469
SA16	Span 12	Start	UDCONU2	8.8014	0.2455
SA16	Span 12	Middle	UDCONU2	22.8139	0.6037
SA16	Span 12	End	UDCONU2	33.7455	0.8836
SA16	Span 13	Start	UDCONU2	38.5491	1.0090
SA16	Span 13	Middle	UDCONU2	38.5491	1.0090
SA16	Span 13	End	UDCONU2	20.7474	0.5383
SA16	Span 14	Start	UDCONU2	2.6846	0.0932
SA16	Span 14	Middle		0.0000	0.0000
SA16	Span 14	End	UDCONU2	0.0000	0.0328
SA16	Span 15	Start	UDCONU2	0.0000	0.0356

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA16	Span 15	Middle	UDCONU2	5.0802	0.1327
SA16	Span 15	End		0.0000	0.0000
SA16	Span 16	Start		0.0000	0.0000
SA16	Span 16	Middle		0.0000	0.0000
SA16	Span 16	End		0.0000	0.0000
SA16	Span 17	Start		0.0000	0.0000
SA16	Span 17	Middle		0.0000	0.0000
SA16	Span 17	End		0.0000	0.0000
SA16	Span 18	Start		0.0000	0.0000
SA16	Span 18	Middle		0.0000	0.0000
SA16	Span 18	End		0.0000	0.0000
SA16	Span 19	Start		0.0000	0.0000
SA16	Span 19	Middle		0.0000	0.0000
SA16	Span 19	End		0.0000	0.0000
SA17	Span 1	Start		0.0000	0.0000
SA17	Span 1	Middle		0.0000	0.0000
SA17	Span 1	End		0.0000	0.0000
SA17	Span 2	Start		0.0000	0.0000
SA17	Span 2	Middle		0.0000	0.0000
SA17	Span 2	End		0.0000	0.0000
SA17	Span 3	Start		0.0000	0.0000
SA17	Span 3	Middle		0.0000	0.0000
SA17	Span 3	End		0.0000	0.0000
SA17	Span 4	Start		0.0000	0.0000
SA17	Span 4	Middle		0.0000	0.0000
SA17	Span 4	End	UDCONU2	0.2433	0.0000
SA17	Span 5	Start	UDCONU2	0.0000	0.0000
SA17	Span 5	Middle	UDCONU2	0.0000	0.0000
SA17	Span 5	End	UDCONU2	0.0000	0.0000
SA17	Span 6	Start	UDCONU2	0.0000	0.0000
SA17	Span 6	Middle		0.0000	0.0000
SA17	Span 6	End	UDCONU2	0.0000	0.0000
SA17	Span 7	Start	UDCONU2	0.0000	0.0000
SA17	Span 7	Middle	UDCONU2	0.0000	0.0000
SA17	Span 7	End	UDCONU2	0.0000	0.0000
SA17	Span 8	Start	UDCONU2	0.0000	0.0000
SA17	Span 8	Middle	UDCONU2	18.0178	0.4725
SA17	Span 8	End	UDCONU2	31.2253	0.8070
SA17	Span 9	Start	UDCONU2	31.2253	0.8070
SA17	Span 9	Middle	UDCONU2	38.7172	0.9945
SA17	Span 9	End	UDCONU2	32.7785	0.8460
SA17	Span 10	Start	UDCONU2	32.7785	0.8460
SA17	Span 10	Middle	UDCONU2	29.8691	0.7729
SA17	Span 10	End	UDCONU2	23.8367	0.6246
SA17	Span 11	Start	UDCONU2	21.5503	0.5708
SA17	Span 11	Middle	UDCONU2	27.8777	0.7356
SA17	Span 11	End	UDCONU2	30.2809	0.7946
SA17	Span 12	Start	UDCONU2	31.8018	0.8279
SA17	Span 12	Middle	UDCONU2	31.8018	0.8279
SA17	Span 12	End	UDCONU2	14.0084	0.3605
SA17	Span 13	Start	UDCONU2	1.6263	0.0523
SA17	Span 13	Middle		0.0000	0.0000
SA17	Span 13	End	UDCONU2	0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA17	Span 14	Start	UDCONU2	0.0000	0.0000
SA17	Span 14	Middle	UDCONU2	2.3870	0.0614
SA17	Span 14	End		0.0000	0.0000
SA17	Span 15	Start		0.0000	0.0000
SA17	Span 15	Middle		0.0000	0.0000
SA17	Span 15	End		0.0000	0.0000
SA17	Span 16	Start		0.0000	0.0000
SA17	Span 16	Middle		0.0000	0.0000
SA17	Span 16	End		0.0000	0.0000
SA17	Span 17	Start		0.0000	0.0000
SA17	Span 17	Middle		0.0000	0.0000
SA17	Span 17	End		0.0000	0.0000
SA17	Span 18	Start		0.0000	0.0000
SA17	Span 18	Middle		0.0000	0.0000
SA17	Span 18	End		0.0000	0.0000
SA18	Span 1	Start		0.0000	0.0000
SA18	Span 1	Middle		0.0000	0.0000
SA18	Span 1	End		0.0000	0.0000
SA18	Span 2	Start		0.0000	0.0000
SA18	Span 2	Middle		0.0000	0.0000
SA18	Span 2	End		0.0000	0.0000
SA18	Span 3	Start		0.0000	0.0000
SA18	Span 3	Middle		0.0000	0.0000
SA18	Span 3	End		0.0000	0.0000
SA18	Span 4	Start		0.0000	0.0000
SA18	Span 4	Middle		0.0000	0.0000
SA18	Span 4	End	UDCONU2	0.1359	0.0000
SA18	Span 5	Start	UDCONU2	0.0000	0.0000
SA18	Span 5	Middle	UDCONU2	0.0000	0.0000
SA18	Span 5	End	UDCONU2	0.2042	0.0000
SA18	Span 6	Start	UDCONU2	0.2042	0.0000
SA18	Span 6	Middle		0.0000	0.0000
SA18	Span 6	End	UDCONU2	0.1326	0.0000
SA18	Span 7	Start	UDCONU2	0.0000	0.0000
SA18	Span 7	Middle	UDCONU2	17.9085	0.4662
SA18	Span 7	End	UDCONU2	31.9306	0.8221
SA18	Span 8	Start	UDCONU2	31.9306	0.8221
SA18	Span 8	Middle	UDCONU2	36.0311	0.9288
SA18	Span 8	End	UDCONU2	23.7195	0.6217
SA18	Span 9	Start	UDCONU2	23.7195	0.6217
SA18	Span 9	Middle	UDCONU2	21.2652	0.5714
SA18	Span 9	End	UDCONU2	21.2652	0.5714
SA18	Span 10	Start	UDCONU2	21.1613	0.5699
SA18	Span 10	Middle	UDCONU2	24.6155	0.6520
SA18	Span 10	End	UDCONU2	31.6559	0.8242
SA18	Span 11	Start	UDCONU2	35.9479	0.9255
SA18	Span 11	Middle	UDCONU2	35.9479	0.9255
SA18	Span 11	End	UDCONU2	23.0444	0.5896
SA18	Span 12	Start	UDCONU2	7.1304	0.1869
SA18	Span 12	Middle		0.0000	0.0000
SA18	Span 12	End	UDCONU2	0.0675	0.0000
SA18	Span 13	Start	UDCONU2	0.0675	0.0000
SA18	Span 13	Middle	UDCONU2	6.5068	0.1641

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA18	Span 13	End		0.0000	0.0000
SA18	Span 14	Start		0.0000	0.0000
SA18	Span 14	Middle		0.0000	0.0000
SA18	Span 14	End		0.0000	0.0000
SA18	Span 15	Start		0.0000	0.0000
SA18	Span 15	Middle		0.0000	0.0000
SA18	Span 15	End		0.0000	0.0000
SA18	Span 16	Start		0.0000	0.0000
SA18	Span 16	Middle		0.0000	0.0000
SA18	Span 16	End		0.0000	0.0000
SA18	Span 17	Start		0.0000	0.0000
SA18	Span 17	Middle		0.0000	0.0000
SA18	Span 17	End		0.0000	0.0000
SA19	Span 1	Start		0.0000	0.0000
SA19	Span 1	Middle		0.0000	0.0000
SA19	Span 1	End		0.0000	0.0000
SA19	Span 2	Start		0.0000	0.0000
SA19	Span 2	Middle		0.0000	0.0000
SA19	Span 2	End		0.0000	0.0000
SA19	Span 3	Start		0.0000	0.0000
SA19	Span 3	Middle		0.0000	0.0000
SA19	Span 3	End		0.0000	0.0000
SA19	Span 4	Start		0.0000	0.0000
SA19	Span 4	Middle		0.0000	0.0000
SA19	Span 4	End	UDCONU2	0.4410	0.0000
SA19	Span 5	Start	UDCONU2	0.0000	0.0000
SA19	Span 5	Middle		0.0000	0.0000
SA19	Span 5	End	UDCONU2	0.0000	0.0000
SA19	Span 6	Start	UDCONU2	0.0000	0.0000
SA19	Span 6	Middle		0.0000	0.0000
SA19	Span 6	End	UDCONU2	0.0000	0.0393
SA19	Span 7	Start	UDCONU2	0.0000	0.0393
SA19	Span 7	Middle		0.0000	0.0000
SA19	Span 7	End	UDCONU2	0.0000	0.0000
SA19	Span 8	Start	UDCONU2	0.0000	0.0000
SA19	Span 8	Middle	UDCONU2	0.8239	0.0373
SA19	Span 8	End	UDCONU2	2.3663	0.0737
SA19	Span 9	Start	UDCONU2	2.3663	0.0737
SA19	Span 9	Middle		0.0000	0.0000
SA19	Span 9	End	UDCONU1	0.5487	0.0000
SA19	Span 10	Start	UDCONU1	0.5487	0.0000
SA19	Span 10	Middle		0.0000	0.0000
SA19	Span 10	End	UDCONU2	0.2560	0.0324
SA19	Span 11	Start	UDCONU2	0.2560	0.0324
SA19	Span 11	Middle		0.0000	0.0000
SA19	Span 11	End	UDCONU2	6.6341	0.1944
SA19	Span 12	Start	UDCONU2	6.6341	0.1944
SA19	Span 12	Middle	UDCONU2	6.1249	0.1806
SA19	Span 12	End	UDCONU2	5.1587	0.1547
SA19	Span 13	Start	UDCONU2	5.1587	0.1547
SA19	Span 13	Middle		0.0000	0.0000
SA19	Span 13	End	UDCONU1	7.9647	0.2272
SA19	Span 14	Start	UDCONU1	7.9647	0.2272

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA19	Span 14	Middle		0.0000	0.0000
SA19	Span 14	End	UDCONU2	16.2237	0.4123
SA19	Span 15	Start	UDCONU2	16.2237	0.4123
SA19	Span 15	Middle		0.0000	0.0000
SA19	Span 15	End	UDCONU2	20.6193	0.5281
SA19	Span 16	Start	UDCONU2	20.6193	0.5281
SA19	Span 16	Middle		0.0000	0.0000
SA19	Span 16	End	UDCONU2	32.6404	0.8266
SA19	Span 17	Start	UDCONU2	32.6404	0.8266
SA19	Span 17	Middle	UDCONU2	34.5640	0.8739
SA19	Span 17	End	UDCONU2	13.4070	0.3494
SA19	Span 18	Start	UDCONU2	13.4070	0.3494
SA19	Span 18	Middle		0.0000	0.0000
SA19	Span 18	End	UDCONU2	9.4106	0.2783
SA19	Span 19	Start	UDCONU2	9.4106	0.2783
SA19	Span 19	Middle		0.0000	0.0000
SA19	Span 19	End	UDCONU2	8.4914	0.2689
SA19	Span 20	Start	UDCONU2	8.4914	0.2689
SA19	Span 20	Middle	UDCONU2	7.5408	0.2468
SA19	Span 20	End	UDCONU2	7.4184	0.2476
SA19	Span 21	Start	UDCONU2	7.4184	0.2476
SA19	Span 21	Middle	UDCONU2	8.1502	0.2667
SA19	Span 21	End	UDCONU2	6.9613	0.2322
SA19	Span 22	Start	UDCONU2	6.9613	0.2322
SA19	Span 22	Middle		0.0000	0.0000
SA19	Span 22	End	UDCONU2	10.3400	0.2696
SA19	Span 23	Start	UDCONU2	10.3400	0.2696
SA19	Span 23	Middle		0.0000	0.0000
SA19	Span 23	End	UDCONU2	19.5475	0.5035
SA19	Span 24	Start	UDCONU2	30.0549	0.7596
SA19	Span 24	Middle	UDCONU2	32.9506	0.8327
SA19	Span 24	End	UDCONU2	26.4055	0.6669
SA19	Span 25	Start	UDCONU2	11.7710	0.3016
SA19	Span 25	Middle		0.0000	0.0000
SA19	Span 25	End	UDCONU2	0.1640	0.0000
SA19	Span 26	Start	UDCONU2	0.1640	0.0000
SA19	Span 26	Middle	UDCONU2	7.0942	0.1790
SA19	Span 26	End		0.0000	0.0000
SA19	Span 27	Start		0.0000	0.0000
SA19	Span 27	Middle		0.0000	0.0000
SA19	Span 27	End		0.0000	0.0000
SA19	Span 28	Start		0.0000	0.0000
SA19	Span 28	Middle		0.0000	0.0000
SA19	Span 28	End		0.0000	0.0000
SA19	Span 29	Start		0.0000	0.0000
SA19	Span 29	Middle		0.0000	0.0000
SA19	Span 29	End		0.0000	0.0000
SA19	Span 30	Start		0.0000	0.0000
SA19	Span 30	Middle		0.0000	0.0000
SA19	Span 30	End		0.0000	0.0000
SA20	Span 1	Start		0.0000	0.0000
SA20	Span 1	Middle		0.0000	0.0000
SA20	Span 1	End		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA20	Span 2	Start		0.0000	0.0000
SA20	Span 2	Middle		0.0000	0.0000
SA20	Span 2	End		0.0000	0.0000
SA20	Span 3	Start		0.0000	0.0000
SA20	Span 3	Middle		0.0000	0.0000
SA20	Span 3	End		0.0000	0.0000
SA20	Span 4	Start		0.0000	0.0000
SA20	Span 4	Middle		0.0000	0.0000
SA20	Span 4	End	UDCONU2	0.5662	0.0000
SA20	Span 5	Start	UDCONU2	0.0000	0.0000
SA20	Span 5	Middle		0.0000	0.0000
SA20	Span 5	End	UDCONU2	0.0000	0.0355
SA20	Span 6	Start	UDCONU2	0.0000	0.0355
SA20	Span 6	Middle		0.0000	0.0000
SA20	Span 6	End	UDCONU2	0.0000	0.0596
SA20	Span 7	Start	UDCONU2	0.0000	0.0596
SA20	Span 7	Middle		0.0000	0.0000
SA20	Span 7	End	UDCONU2	0.0000	0.0000
SA20	Span 8	Start	UDCONU2	0.0000	0.0000
SA20	Span 8	Middle	UDCONU2	3.3294	0.0962
SA20	Span 8	End	UDCONU2	7.3772	0.1938
SA20	Span 9	Start	UDCONU2	7.3772	0.1938
SA20	Span 9	Middle		0.0000	0.0000
SA20	Span 9	End	UDCONU2	8.7277	0.2316
SA20	Span 10	Start	UDCONU2	8.7277	0.2316
SA20	Span 10	Middle		0.0000	0.0000
SA20	Span 10	End	UDCONU2	9.8338	0.2693
SA20	Span 11	Start	UDCONU2	9.8338	0.2693
SA20	Span 11	Middle		0.0000	0.0000
SA20	Span 11	End	UDCONU2	13.5479	0.3672
SA20	Span 12	Start	UDCONU2	13.5479	0.3672
SA20	Span 12	Middle		0.0000	0.0000
SA20	Span 12	End	UDCONU2	16.9981	0.4276
SA20	Span 13	Start	UDCONU2	16.9981	0.4276
SA20	Span 13	Middle		0.0000	0.0000
SA20	Span 13	End	UDCONU2	22.1160	0.5572
SA20	Span 14	Start	UDCONU2	22.1160	0.5572
SA20	Span 14	Middle		0.0000	0.0000
SA20	Span 14	End	UDCONU2	29.1374	0.7356
SA20	Span 15	Start	UDCONU2	29.1374	0.7356
SA20	Span 15	Middle	UDCONU2	26.3957	0.6658
SA20	Span 15	End	UDCONU2	3.3307	0.0835
SA20	Span 16	Start	UDCONU2	3.3307	0.0835
SA20	Span 16	Middle		0.0000	0.0000
SA20	Span 16	End	UDCONU2	2.4404	0.1107
SA20	Span 17	Start	UDCONU2	2.4404	0.1107
SA20	Span 17	Middle		0.0000	0.0000
SA20	Span 17	End	UDCONU2	1.9001	0.1111
SA20	Span 18	Start	UDCONU2	1.9001	0.1111
SA20	Span 18	Middle	UDCONU2	0.8962	0.0846
SA20	Span 18	End	UDCONU2	0.8346	0.0858
SA20	Span 19	Start	UDCONU2	0.8346	0.0858
SA20	Span 19	Middle	UDCONU2	1.3925	0.1064

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA20	Span 19	End	UDCONU2	0.0000	0.0677
SA20	Span 20	Start	UDCONU2	0.0000	0.0677
SA20	Span 20	Middle		0.0000	0.0000
SA20	Span 20	End	UDCONU2	2.3009	0.0694
SA20	Span 21	Start	UDCONU2	2.3009	0.0694
SA20	Span 21	Middle		0.0000	0.0000
SA20	Span 21	End	UDCONU2	11.4101	0.2976
SA20	Span 22	Start	UDCONU2	23.3183	0.5894
SA20	Span 22	Middle	UDCONU2	29.4468	0.7434
SA20	Span 22	End	UDCONU2	26.1691	0.6601
SA20	Span 23	Start	UDCONU2	13.2925	0.3341
SA20	Span 23	Middle		0.0000	0.0000
SA20	Span 23	End	UDCONU2	2.9007	0.0727
SA20	Span 24	Start	UDCONU2	3.4765	0.0919
SA20	Span 24	Middle	UDCONU2	4.5683	0.1155
SA20	Span 24	End		0.0000	0.0000
SA20	Span 25	Start		0.0000	0.0000
SA20	Span 25	Middle		0.0000	0.0000
SA20	Span 25	End		0.0000	0.0000
SA20	Span 26	Start		0.0000	0.0000
SA20	Span 26	Middle		0.0000	0.0000
SA20	Span 26	End		0.0000	0.0000
SA20	Span 27	Start		0.0000	0.0000
SA20	Span 27	Middle		0.0000	0.0000
SA20	Span 27	End		0.0000	0.0000
SA20	Span 28	Start		0.0000	0.0000
SA20	Span 28	Middle		0.0000	0.0000
SA20	Span 28	End		0.0000	0.0000
SA21	Span 1	Start		0.0000	0.0000
SA21	Span 1	Middle		0.0000	0.0000
SA21	Span 1	End		0.0000	0.0000
SA21	Span 2	Start		0.0000	0.0000
SA21	Span 2	Middle		0.0000	0.0000
SA21	Span 2	End		0.0000	0.0000
SA21	Span 3	Start		0.0000	0.0000
SA21	Span 3	Middle		0.0000	0.0000
SA21	Span 3	End		0.0000	0.0000
SA21	Span 4	Start		0.0000	0.0000
SA21	Span 4	Middle	UDCONU2	0.8215	0.0256
SA21	Span 4	End	UDCONU2	0.0000	0.0174
SA21	Span 5	Start	UDCONU2	0.0000	0.0174
SA21	Span 5	Middle	UDCONU2	2.5552	0.0680
SA21	Span 5	End	UDCONU2	6.0706	0.1549
SA21	Span 6	Start	UDCONU2	6.0706	0.1549
SA21	Span 6	Middle	UDCONU2	9.9438	0.2541
SA21	Span 6	End	UDCONU2	13.5144	0.3428
SA21	Span 7	Start	UDCONU2	13.5144	0.3428
SA21	Span 7	Middle	UDCONU2	12.0057	0.3040
SA21	Span 7	End	UDCONU2	1.0185	0.0513
SA21	Span 8	Start	UDCONU2	1.0185	0.0513
SA21	Span 8	Middle	UDCONU2	0.0000	0.0313
SA21	Span 8	End	UDCONU2	0.0000	0.0315
SA21	Span 9	Start	UDCONU2	0.0000	0.0315

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SA21	Span 9	Middle	UDCONU2	0.8231	0.0555
SA21	Span 9	End	UDCONU2	4.8081	0.1332
SA21	Span 10	Start	UDCONU2	11.5330	0.2952
SA21	Span 10	Middle	UDCONU2	14.6171	0.3700
SA21	Span 10	End	UDCONU2	13.5052	0.3407
SA21	Span 11	Start	UDCONU2	7.5727	0.1904
SA21	Span 11	Middle	UDCONU2	1.7973	0.0450
SA21	Span 11	End		0.0000	0.0000
SA21	Span 12	Start		0.0000	0.0000
SA21	Span 12	Middle		0.0000	0.0000
SA21	Span 12	End		0.0000	0.0000
SA21	Span 13	Start		0.0000	0.0000
SA21	Span 13	Middle		0.0000	0.0000
SA21	Span 13	End		0.0000	0.0000
SA21	Span 14	Start		0.0000	0.0000
SA21	Span 14	Middle		0.0000	0.0000
SA21	Span 14	End		0.0000	0.0000
SA21	Span 15	Start		0.0000	0.0000
SA21	Span 15	Middle		0.0000	0.0000
SA21	Span 15	End		0.0000	0.0000
SB10	Span 1	Start	UDCONU2	1.7715	0.0540
SB10	Span 1	Middle		0.0000	0.0000
SB10	Span 1	End	UDCONU2	3.5193	0.1150
SB10	Span 2	Start	UDCONU2	3.5193	0.1150
SB10	Span 2	Middle		0.0000	0.0000
SB10	Span 2	End	UDCONU2	2.3246	0.0681
SB10	Span 3	Start	UDCONU2	2.3246	0.0681
SB10	Span 3	Middle	UDCONU2	1.0787	0.0352
SB10	Span 3	End	UDCONU2	1.2419	0.0441
SB10	Span 4	Start	UDCONU2	1.2419	0.0441
SB10	Span 4	Middle	UDCONU2	9.3924	0.2589
SB10	Span 4	End	UDCONU2	9.9400	0.2777
SB10	Span 5	Start	UDCONU2	9.9400	0.2777
SB10	Span 5	Middle	UDCONU2	21.2835	0.5863
SB10	Span 5	End	UDCONU2	35.4814	0.9821
SB10	Span 6	Start	UDCONU2	35.4814	0.9821
SB10	Span 6	Middle	UDCONU2	41.7676	1.1586
SB10	Span 6	End	UDCONU2	28.8422	0.7971
SB10	Span 7	Start	UDCONU2	28.8422	0.7971
SB10	Span 7	Middle	UDCONU2	25.9448	0.7185
SB10	Span 7	End	UDCONU2	4.4325	0.1550
SB10	Span 8	Start		0.0000	0.0000
SB10	Span 8	Middle		0.0000	0.0000
SB10	Span 8	End		0.0000	0.0000
SB10	Span 9	Start		0.0000	0.0000
SB10	Span 9	Middle		0.0000	0.0000
SB10	Span 9	End		0.0000	0.0000
SB10	Span 10	Start		0.0000	0.0000
SB10	Span 10	Middle		0.0000	0.0000
SB10	Span 10	End		0.0000	0.0000
SB10	Span 11	Start		0.0000	0.0000
SB10	Span 11	Middle		0.0000	0.0000
SB10	Span 11	End	UDCONU2	7.5597	0.2073

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB10	Span 12	Start	UDCONU2	7.5597	0.2073
SB10	Span 12	Middle	UDCONU2	6.0650	0.1822
SB10	Span 12	End	UDCONU2	0.1277	0.0416
SB10	Span 13	Start	UDCONU2	0.1277	0.0416
SB10	Span 13	Middle	UDCONU2	0.5935	0.0579
SB10	Span 13	End	UDCONU2	0.0000	0.0355
SB10	Span 14	Start	UDCONU2	0.0000	0.0355
SB10	Span 14	Middle		0.0000	0.0000
SB10	Span 14	End	UDCONU2	0.0000	0.0383
SB10	Span 15	Start	UDCONU2	0.0000	0.0383
SB10	Span 15	Middle	UDCONU2	1.6325	0.0696
SB10	Span 15	End	UDCONU2	2.6125	0.0733
SB11	Span 1	Start	UDCONU2	2.1586	0.0649
SB11	Span 1	Middle	UDCONU2	1.0047	0.0341
SB11	Span 1	End	UDCONU2	4.1726	0.1143
SB11	Span 2	Start	UDCONU2	4.1726	0.1143
SB11	Span 2	Middle	UDCONU2	17.3104	0.4762
SB11	Span 2	End	UDCONU2	33.0320	0.9135
SB11	Span 3	Start	UDCONU2	33.0320	0.9135
SB11	Span 3	Middle	UDCONU2	44.5015	1.2356
SB11	Span 3	End	UDCONU2	30.8030	0.8512
SB11	Span 4	Start	UDCONU2	30.8030	0.8512
SB11	Span 4	Middle	UDCONU2	16.5361	0.4548
SB11	Span 4	End	UDCONU2	0.0000	0.0000
SB11	Span 5	Start	UDCONU2	0.0000	0.0000
SB11	Span 5	Middle		0.0000	0.0000
SB11	Span 5	End	UDCONU2	0.0000	0.0000
SB11	Span 6	Start	UDCONU2	0.0000	0.0000
SB11	Span 6	Middle		0.0000	0.0000
SB11	Span 6	End	UDCONU2	0.6295	0.0000
SB11	Span 7	Start	UDCONU2	0.6295	0.0000
SB11	Span 7	Middle		0.0000	0.0000
SB11	Span 7	End	UDCONU2	0.0996	0.0394
SB11	Span 8	Start	UDCONU2	0.0996	0.0394
SB11	Span 8	Middle		0.0000	0.0000
SB11	Span 8	End	UDCONU2	13.0818	0.4059
SB11	Span 9	Start	UDCONU2	13.0818	0.4059
SB11	Span 9	Middle	UDCONU2	18.7159	0.5691
SB11	Span 9	End	UDCONU2	35.4703	1.0099
SB11	Span 10	Start	UDCONU2	35.4703	1.0099
SB11	Span 10	Middle		0.0000	0.0000
SB11	Span 10	End	UDCONU2	25.8423	0.7264
SB11	Span 11	Start	UDCONU2	25.8423	0.7264
SB11	Span 11	Middle	UDCONU2	25.2699	0.7039
SB11	Span 11	End	UDCONU2	1.8255	0.0517
SB11	Span 12	Start	UDCONU2	1.8255	0.0517
SB11	Span 12	Middle	UDCONU2	3.9210	0.1243
SB11	Span 12	End	UDCONU2	1.3241	0.0428
SB11	Span 13	Start	UDCONU2	1.3241	0.0428
SB11	Span 13	Middle		0.0000	0.0000
SB11	Span 13	End	UDCONU2	0.8580	0.0309
SB11	Span 14	Start	UDCONU2	0.8580	0.0309
SB11	Span 14	Middle	UDCONU2	0.4264	0.0117

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB11	Span 14	End	UDCONU2	0.4586	0.0126
SB11	Span 15	Start	UDCONU2	0.4586	0.0126
SB11	Span 15	Middle	UDCONU2	0.4108	0.0112
SB11	Span 15	End	UDCONU2	0.5512	0.0151
SB11	Span 16	Start	UDCONU2	0.5512	0.0151
SB11	Span 16	Middle	UDCONU2	0.5463	0.0150
SB11	Span 16	End	UDCONU2	0.4156	0.0151
SB11	Span 17	Start	UDCONU2	11.5161	0.3348
SB11	Span 17	Middle	UDCONU2	9.1784	0.2518
SB11	Span 17	End	UDCONU2	14.7929	0.4066
SB11	Span 18	Start	UDCONU2	14.7929	0.4066
SB11	Span 18	Middle		0.0000	0.0000
SB11	Span 18	End	UDCONU2	15.2454	0.4224
SB11	Span 19	Start	UDCONU2	15.2454	0.4224
SB11	Span 19	Middle		0.0000	0.0000
SB11	Span 19	End	UDCONU2	14.6852	0.4036
SB11	Span 20	Start	UDCONU2	14.6852	0.4036
SB11	Span 20	Middle		0.0000	0.0000
SB11	Span 20	End	UDCONU2	13.0296	0.3581
SB11	Span 21	Start	UDCONU2	13.0296	0.3581
SB11	Span 21	Middle		0.0000	0.0000
SB11	Span 21	End	UDCONU2	1.6386	0.0512
SB11	Span 22	Start	UDCONU2	1.6386	0.0512
SB11	Span 22	Middle		0.0000	0.0000
SB11	Span 22	End	UDCONU2	2.4104	0.0718
SB11	Span 23	Start	UDCONU2	2.4104	0.0718
SB11	Span 23	Middle	UDCONU2	3.1026	0.0902
SB11	Span 23	End	UDCONU2	3.8314	0.1094
SB11	Span 24	Start	UDCONU2	3.8314	0.1094
SB11	Span 24	Middle	UDCONU2	9.0581	0.2590
SB11	Span 24	End	UDCONU1	0.5987	0.0000
SB11	Span 25	Start	UDCONU1	0.5987	0.0000
SB11	Span 25	Middle		0.0000	0.0000
SB11	Span 25	End	UDCONU1	1.7712	0.0518
SB11	Span 26	Start	UDCONU1	1.7712	0.0518
SB11	Span 26	Middle	UDCONU2	8.0722	0.2263
SB11	Span 26	End	UDCONU2	6.8335	0.1878
SB12	Span 1	Start	UDCONU2	1.8681	0.0593
SB12	Span 1	Middle		0.0000	0.0000
SB12	Span 1	End	UDCONU2	2.0097	0.0714
SB12	Span 2	Start	UDCONU2	2.0097	0.0714
SB12	Span 2	Middle	UDCONU2	0.9706	0.0389
SB12	Span 2	End	UDCONU2	4.8086	0.1325
SB12	Span 3	Start	UDCONU2	4.8086	0.1325
SB12	Span 3	Middle	UDCONU2	6.7650	0.1854
SB12	Span 3	End	UDCONU2	7.2146	0.1978
SB12	Span 4	Start	UDCONU2	7.2146	0.1978
SB12	Span 4	Middle	UDCONU2	16.5365	0.4548
SB12	Span 4	End	UDCONU2	31.9870	0.8843
SB12	Span 5	Start	UDCONU2	31.9870	0.8843
SB12	Span 5	Middle	UDCONU2	43.9695	1.2206
SB12	Span 5	End	UDCONU1	25.8075	0.7124
SB12	Span 6	Start	UDCONU1	25.8075	0.7124

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB12	Span 6	Middle	UDCONU1	9.6098	0.2723
SB12	Span 6	End	UDCONU2	0.0000	0.0000
SB12	Span 7	Start	UDCONU2	0.0000	0.0000
SB12	Span 7	Middle		0.0000	0.0000
SB12	Span 7	End	UDCONU2	0.0615	0.0000
SB12	Span 8	Start	UDCONU2	0.0615	0.0000
SB12	Span 8	Middle	UDCONU2	40.2286	1.1271
SB12	Span 8	End	UDCONU2	36.2167	1.0081
SB12	Span 9	Start		0.0000	0.0000
SB12	Span 9	Middle		0.0000	0.0000
SB12	Span 9	End		0.0000	0.0000
SB12	Span 10	Start		0.0000	0.0000
SB12	Span 10	Middle		0.0000	0.0000
SB12	Span 10	End		0.0000	0.0000
SB12	Span 11	Start		0.0000	0.0000
SB12	Span 11	Middle		0.0000	0.0000
SB12	Span 11	End	UDCONU2	28.7493	0.8073
SB12	Span 12	Start	UDCONU2	28.7493	0.8073
SB12	Span 12	Middle		0.0000	0.0000
SB12	Span 12	End	UDCONU2	19.7234	0.5767
SB12	Span 13	Start	UDCONU2	19.7234	0.5767
SB12	Span 13	Middle		0.0000	0.0000
SB12	Span 13	End	UDCONU2	30.6128	0.8459
SB12	Span 14	Start	UDCONU2	30.6128	0.8459
SB12	Span 14	Middle	UDCONU2	41.4450	1.1495
SB12	Span 14	End	UDCONU2	27.3291	0.7543
SB12	Span 15	Start	UDCONU2	27.3291	0.7543
SB12	Span 15	Middle	UDCONU2	24.9368	0.6877
SB12	Span 15	End	UDCONU2	1.3049	0.0364
SB12	Span 16	Start	UDCONU2	1.3049	0.0364
SB12	Span 16	Middle		0.0000	0.0000
SB12	Span 16	End	UDCONU1	2.3335	0.0662
SB12	Span 17	Start	UDCONU1	2.3335	0.0662
SB12	Span 17	Middle		0.0000	0.0000
SB12	Span 17	End	UDCONU1	2.6074	0.0740
SB12	Span 18	Start	UDCONU1	2.6074	0.0740
SB12	Span 18	Middle	UDCONU1	3.2835	0.0932
SB12	Span 18	End	UDCONU1	13.9402	0.3876
SB12	Span 19	Start	UDCONU1	13.9402	0.3876
SB12	Span 19	Middle		0.0000	0.0000
SB12	Span 19	End	UDCONU2	20.1547	0.5550
SB12	Span 20	Start	UDCONU2	20.1547	0.5550
SB12	Span 20	Middle	UDCONU2	23.2328	0.6428
SB12	Span 20	End	UDCONU2	16.3436	0.4505
SB13	Span 1	Start	UDCONU2	7.9393	0.2194
SB13	Span 1	Middle	UDCONU2	2.4757	0.0724
SB13	Span 1	End	UDCONU2	25.2323	0.7059
SB13	Span 2	Start	UDCONU2	25.2323	0.7059
SB13	Span 2	Middle		0.0000	0.0000
SB13	Span 2	End	UDCONU2	8.2362	0.2384
SB13	Span 3	Start	UDCONU2	8.2362	0.2384
SB13	Span 3	Middle		0.0000	0.0000
SB13	Span 3	End	UDCONU2	4.0897	0.1217

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB13	Span 4	Start	UDCONU2	4.0897	0.1217
SB13	Span 4	Middle	UDCONU2	10.5120	0.2885
SB13	Span 4	End	UDCONU2	26.0533	0.7188
SB13	Span 5	Start	UDCONU2	26.0533	0.7188
SB13	Span 5	Middle	UDCONU2	36.2911	1.0048
SB13	Span 5	End	UDCONU1	22.9318	0.6328
SB13	Span 6	Start	UDCONU1	22.9318	0.6328
SB13	Span 6	Middle	UDCONU1	8.5007	0.2365
SB13	Span 6	End	UDCONU2	0.0000	0.0000
SB13	Span 7	Start	UDCONU2	0.0000	0.0000
SB13	Span 7	Middle		0.0000	0.0000
SB13	Span 7	End	UDCONU2	0.0000	0.0000
SB13	Span 8	Start	UDCONU2	0.0000	0.0000
SB13	Span 8	Middle	UDCONU2	29.2010	0.8134
SB13	Span 8	End	UDCONU2	36.5322	1.0147
SB13	Span 9	Start		0.0000	0.0000
SB13	Span 9	Middle		0.0000	0.0000
SB13	Span 9	End		0.0000	0.0000
SB13	Span 10	Start		0.0000	0.0000
SB13	Span 10	Middle		0.0000	0.0000
SB13	Span 10	End		0.0000	0.0000
SB13	Span 11	Start		0.0000	0.0000
SB13	Span 11	Middle	UDCONU1	14.1210	0.3928
SB13	Span 11	End	UDCONU1	31.3499	0.8666
SB13	Span 12	Start	UDCONU1	31.3499	0.8666
SB13	Span 12	Middle	UDCONU2	58.6967	1.6377
SB13	Span 12	End	UDCONU2	50.9989	1.4191
SB13	Span 13	Start	UDCONU2	50.9989	1.4191
SB13	Span 13	Middle	UDCONU2	44.4740	1.2348
SB13	Span 13	End	UDCONU2	12.3979	0.3436
SB13	Span 14	Start	UDCONU2	12.3979	0.3436
SB13	Span 14	Middle		0.0000	0.0000
SB13	Span 14	End	UDCONU1	4.1568	0.1161
SB13	Span 15	Start	UDCONU1	5.4229	0.1515
SB13	Span 15	Middle	UDCONU2	17.5636	0.4843
SB13	Span 15	End	UDCONU2	22.1213	0.6106
SB14	Span 1	Start	UDCONU2	8.9373	0.2509
SB14	Span 1	Middle	UDCONU2	2.8414	0.0778
SB14	Span 1	End	UDCONU2	19.8544	0.5466
SB14	Span 2	Start	UDCONU2	19.8544	0.5466
SB14	Span 2	Middle	UDCONU2	36.9538	1.0233
SB14	Span 2	End	UDCONU2	29.9299	0.8268
SB14	Span 3	Start	UDCONU2	24.1536	0.6660
SB14	Span 3	Middle		0.0000	0.0000
SB14	Span 3	End	UDCONU1	13.4378	0.3692
SB14	Span 4	Start	UDCONU1	13.4378	0.3692
SB14	Span 4	Middle	UDCONU1	0.7874	0.0000
SB14	Span 4	End	UDCONU2	0.0000	0.0000
SB14	Span 5	Start	UDCONU2	0.0000	0.0000
SB14	Span 5	Middle		0.0000	0.0000
SB14	Span 5	End	UDCONU2	0.0000	0.0000
SB14	Span 6	Start	UDCONU2	0.0000	0.0000
SB14	Span 6	Middle	UDCONU2	11.7142	0.3270

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB14	Span 6	End	UDCONU2	3.8525	0.1062
SB14	Span 7	Start		0.0000	0.0000
SB14	Span 7	Middle		0.0000	0.0000
SB14	Span 7	End		0.0000	0.0000
SB14	Span 8	Start		0.0000	0.0000
SB14	Span 8	Middle		0.0000	0.0000
SB14	Span 8	End		0.0000	0.0000
SB14	Span 9	Start		0.0000	0.0000
SB14	Span 9	Middle		0.0000	0.0000
SB14	Span 9	End	UDCONU1	19.5874	0.5428
SB14	Span 10	Start	UDCONU2	43.8183	1.2164
SB14	Span 10	Middle	UDCONU2	72.3708	2.0290
SB14	Span 10	End	UDCONU2	72.3708	2.0290
SB14	Span 11	Start	UDCONU2	68.7397	1.9248
SB14	Span 11	Middle	UDCONU2	60.6424	1.6932
SB14	Span 11	End	UDCONU2	17.5052	0.4922
SB14	Span 12	Start	UDCONU2	17.5052	0.4922
SB14	Span 12	Middle		0.0000	0.0000
SB14	Span 12	End	UDCONU2	0.1527	0.0000
SB14	Span 13	Start	UDCONU2	0.1527	0.0000
SB14	Span 13	Middle	UDCONU2	7.3823	0.2076
SB14	Span 13	End	UDCONU2	17.0987	0.4725
SB15	Span 1	Start	UDCONU2	11.2761	0.3225
SB15	Span 1	Middle		0.0000	0.0000
SB15	Span 1	End	UDCONU2	12.4194	0.3499
SB15	Span 2	Start	UDCONU2	12.4194	0.3499
SB15	Span 2	Middle	UDCONU2	2.7555	0.0944
SB15	Span 2	End	UDCONU2	1.0612	0.0571
SB15	Span 3	Start	UDCONU2	1.0612	0.0571
SB15	Span 3	Middle	UDCONU2	8.4235	0.2310
SB15	Span 3	End	UDCONU2	26.6856	0.7364
SB15	Span 4	Start	UDCONU2	26.6856	0.7364
SB15	Span 4	Middle	UDCONU2	51.9146	1.4451
SB15	Span 4	End	UDCONU2	21.3726	0.5887
SB15	Span 5	Start	UDCONU2	21.3726	0.5887
SB15	Span 5	Middle	UDCONU2	9.1634	0.2524
SB15	Span 5	End	UDCONU2	0.0000	0.0000
SB15	Span 6	Start	UDCONU2	0.0000	0.0000
SB15	Span 6	Middle	UDCONU2	0.0000	0.0000
SB15	Span 6	End	UDCONU2	0.0000	0.0000
SB15	Span 7	Start	UDCONU2	0.0000	0.0000
SB15	Span 7	Middle		0.0000	0.0000
SB15	Span 7	End	UDCONU2	0.0000	0.0000
SB15	Span 8	Start	UDCONU2	0.0000	0.0000
SB15	Span 8	Middle	UDCONU2	22.4008	0.6179
SB15	Span 8	End	UDCONU2	52.7462	1.4702
SB15	Span 9	Start		0.0000	0.0000
SB15	Span 9	Middle		0.0000	0.0000
SB15	Span 9	End		0.0000	0.0000
SB15	Span 10	Start		0.0000	0.0000
SB15	Span 10	Middle		0.0000	0.0000
SB15	Span 10	End		0.0000	0.0000
SB15	Span 11	Start		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB15	Span 11	Middle	UDCONU2	14.7652	0.4058
SB15	Span 11	End	UDCONU1	7.8373	0.2164
SB15	Span 12	Start	UDCONU1	7.8373	0.2164
SB15	Span 12	Middle	UDCONU2	72.6175	2.0361
SB15	Span 12	End	UDCONU2	79.3409	2.2300
SB15	Span 13	Start	UDCONU2	79.3409	2.2300
SB15	Span 13	Middle	UDCONU2	69.1100	1.9354
SB15	Span 13	End	UDCONU2	13.4150	0.3822
SB15	Span 14	Start	UDCONU2	13.4150	0.3822
SB15	Span 14	Middle	UDCONU2	2.0319	0.0681
SB15	Span 14	End	UDCONU2	9.9743	0.2756
SB16	Span 1	Start	UDCONU2	11.9692	0.3320
SB16	Span 1	Middle	UDCONU2	0.4445	0.0579
SB16	Span 1	End	UDCONU2	9.7932	0.2687
SB16	Span 2	Start	UDCONU2	9.7932	0.2687
SB16	Span 2	Middle	UDCONU2	56.0900	1.5636
SB16	Span 2	End	UDCONU2	37.0985	1.0283
SB16	Span 3	Start	UDCONU2	37.0985	1.0283
SB16	Span 3	Middle	UDCONU2	15.8384	0.4432
SB16	Span 3	End	UDCONU2	0.0000	0.0000
SB16	Span 4	Start	UDCONU2	0.0000	0.0000
SB16	Span 4	Middle		0.0000	0.0000
SB16	Span 4	End	UDCONU2	0.0000	0.0000
SB16	Span 5	Start	UDCONU2	0.0000	0.0000
SB16	Span 5	Middle	UDCONU2	31.7571	0.8785
SB16	Span 5	End	UDCONU2	24.5549	0.6822
SB16	Span 6	Start	UDCONU2	24.5549	0.6822
SB16	Span 6	Middle		0.0000	0.0000
SB16	Span 6	End	UDCONU2	26.5032	0.7382
SB16	Span 7	Start	UDCONU2	26.5032	0.7382
SB16	Span 7	Middle	UDCONU2	8.2374	0.2294
SB16	Span 7	End		0.0000	0.0000
SB16	Span 8	Start		0.0000	0.0000
SB16	Span 8	Middle		0.0000	0.0000
SB16	Span 8	End		0.0000	0.0000
SB16	Span 9	Start	UDCONU2	13.4307	0.3742
SB16	Span 9	Middle	UDCONU1	2.1414	0.0683
SB16	Span 9	End	UDCONU1	3.2101	0.0879
SB16	Span 10	Start	UDCONU1	3.2101	0.0879
SB16	Span 10	Middle	UDCONU2	69.5925	1.9492
SB16	Span 10	End	UDCONU2	82.9831	2.3354
SB16	Span 11	Start	UDCONU2	82.9831	2.3354
SB16	Span 11	Middle	UDCONU2	76.0091	2.1338
SB16	Span 11	End	UDCONU2	13.9355	0.3913
SB16	Span 12	Start	UDCONU2	13.9355	0.3913
SB16	Span 12	Middle	UDCONU2	0.0983	0.0000
SB16	Span 12	End	UDCONU2	2.2227	0.0618
SB17	Span 1	Start	UDCONU2	3.4439	0.0979
SB17	Span 1	Middle		0.0000	0.0000
SB17	Span 1	End	UDCONU2	3.5141	0.1455
SB17	Span 2	Start	UDCONU2	3.5141	0.1455
SB17	Span 2	Middle	UDCONU2	2.2828	0.1023
SB17	Span 2	End	UDCONU2	0.0000	0.0525

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB17	Span 3	Start	UDCONU2	0.0000	0.0525
SB17	Span 3	Middle		0.0000	0.0000
SB17	Span 3	End	UDCONU2	0.0000	0.0000
SB17	Span 4	Start	UDCONU2	0.0000	0.0000
SB17	Span 4	Middle	UDCONU2	1.1324	0.0443
SB17	Span 4	End	UDCONU2	8.4229	0.2322
SB17	Span 5	Start	UDCONU2	8.4229	0.2322
SB17	Span 5	Middle	UDCONU2	63.0193	1.7613
SB17	Span 5	End	UDCONU2	51.6035	1.4409
SB17	Span 6	Start	UDCONU2	51.6035	1.4409
SB17	Span 6	Middle	UDCONU2	24.9216	0.7017
SB17	Span 6	End	UDCONU2	0.0000	0.0000
SB17	Span 7	Start	UDCONU2	0.0000	0.0000
SB17	Span 7	Middle		0.0000	0.0000
SB17	Span 7	End	UDCONU2	0.2815	0.0000
SB17	Span 8	Start	UDCONU2	0.2815	0.0000
SB17	Span 8	Middle	UDCONU2	24.7844	0.6874
SB17	Span 8	End	UDCONU2	26.9532	0.7489
SB17	Span 9	Start	UDCONU2	26.9532	0.7489
SB17	Span 9	Middle	UDCONU2	24.4163	0.6817
SB17	Span 9	End	UDCONU2	9.0042	0.2567
SB17	Span 10	Start	UDCONU2	9.0042	0.2567
SB17	Span 10	Middle		0.0000	0.0000
SB17	Span 10	End	UDCONU2	7.5015	0.2128
SB17	Span 11	Start	UDCONU2	7.5015	0.2128
SB17	Span 11	Middle	UDCONU2	13.6902	0.3838
SB17	Span 11	End	UDCONU2	10.4764	0.2952
SB17	Span 12	Start	UDCONU2	10.4764	0.2952
SB17	Span 12	Middle	UDCONU2	5.8052	0.1652
SB17	Span 12	End		0.0000	0.0000
SB17	Span 13	Start		0.0000	0.0000
SB17	Span 13	Middle	UDCONU1	21.1515	0.5896
SB17	Span 13	End	UDCONU1	3.3917	0.1600
SB17	Span 14	Start	UDCONU1	3.3917	0.1600
SB17	Span 14	Middle		0.0000	0.0000
SB17	Span 14	End	UDCONU1	0.0855	0.0744
SB17	Span 15	Start	UDCONU1	0.0855	0.0744
SB17	Span 15	Middle		0.0000	0.0000
SB17	Span 15	End	UDCONU1	1.0125	0.0000
SB17	Span 16	Start	UDCONU1	1.0125	0.0000
SB17	Span 16	Middle	UDCONU2	64.5477	1.8047
SB17	Span 16	End	UDCONU2	79.8262	2.2440
SB17	Span 17	Start	UDCONU2	79.8262	2.2440
SB17	Span 17	Middle	UDCONU2	76.6053	2.1543
SB17	Span 17	End	UDCONU2	15.9532	0.4425
SB17	Span 18	Start	UDCONU2	15.9532	0.4425
SB17	Span 18	Middle	UDCONU2	0.1124	0.0000
SB17	Span 18	End	UDCONU2	2.1641	0.0599
SB18	Span 1	Start	UDCONU2	2.7103	0.0765
SB18	Span 1	Middle	UDCONU2	0.0000	0.0000
SB18	Span 1	End	UDCONU2	0.0000	0.0000
SB18	Span 2	Start	UDCONU2	0.0000	0.0000
SB18	Span 2	Middle	UDCONU2	4.9708	0.1538

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB18	Span 2	End	UDCONU2	12.0604	0.3449
SB18	Span 3	Start	UDCONU2	12.0604	0.3449
SB18	Span 3	Middle	UDCONU2	69.6719	1.9566
SB18	Span 3	End	UDCONU2	58.6453	1.6378
SB18	Span 4	Start	UDCONU2	58.6453	1.6378
SB18	Span 4	Middle	UDCONU2	32.3132	0.8934
SB18	Span 4	End	UDCONU2	0.0000	0.0889
SB18	Span 5	Start	UDCONU2	0.0000	0.0889
SB18	Span 5	Middle		0.0000	0.0000
SB18	Span 5	End	UDCONU2	0.0000	0.0539
SB18	Span 6	Start	UDCONU2	0.0000	0.0539
SB18	Span 6	Middle		0.0000	0.0000
SB18	Span 6	End	UDCONU2	1.5932	0.0622
SB18	Span 7	Start	UDCONU2	1.5932	0.0622
SB18	Span 7	Middle	UDCONU2	9.8892	0.2782
SB18	Span 7	End	UDCONU2	4.3373	0.1220
SB18	Span 8	Start	UDCONU2	4.3373	0.1220
SB18	Span 8	Middle	UDCONU2	1.6782	0.0470
SB18	Span 8	End	UDCONU2	0.0000	0.0000
SB18	Span 9	Start	UDCONU2	0.0000	0.0000
SB18	Span 9	Middle		0.0000	0.0000
SB18	Span 9	End	UDCONU2	0.0000	0.0000
SB18	Span 10	Start	UDCONU2	0.0000	0.0000
SB18	Span 10	Middle	UDCONU2	12.0505	0.3327
SB18	Span 10	End	UDCONU2	10.1921	0.2933
SB18	Span 11	Start	UDCONU2	10.1921	0.2933
SB18	Span 11	Middle	UDCONU2	10.0950	0.2870
SB18	Span 11	End	UDCONU2	12.6312	0.3562
SB18	Span 12	Start	UDCONU2	4.8973	0.1426
SB18	Span 12	Middle		0.0000	0.0000
SB18	Span 12	End	UDCONU2	8.6662	0.2452
SB18	Span 13	Start	UDCONU2	8.6662	0.2452
SB18	Span 13	Middle		0.0000	0.0000
SB18	Span 13	End	UDCONU1	1.5237	0.0594
SB18	Span 14	Start	UDCONU1	1.5237	0.0594
SB18	Span 14	Middle	UDCONU2	62.3024	1.7491
SB18	Span 14	End	UDCONU2	73.2987	2.0653
SB18	Span 15	Start	UDCONU2	73.2987	2.0653
SB18	Span 15	Middle	UDCONU2	72.8051	2.0513
SB18	Span 15	End	UDCONU2	11.7641	0.3237
SB18	Span 16	Start	UDCONU2	11.7641	0.3237
SB18	Span 16	Middle	UDCONU2	2.0759	0.0677
SB18	Span 16	End	UDCONU2	9.7749	0.2706
SB19	Span 1	Start	UDCONU2	5.0788	0.1493
SB19	Span 1	Middle	UDCONU2	0.0000	0.0000
SB19	Span 1	End	UDCONU2	0.0000	0.0000
SB19	Span 2	Start	UDCONU2	0.0000	0.0000
SB19	Span 2	Middle		0.0000	0.0000
SB19	Span 2	End	UDCONU2	0.0000	0.0000
SB19	Span 3	Start	UDCONU2	0.0000	0.0000
SB19	Span 3	Middle	UDCONU2	3.2206	0.0965
SB19	Span 3	End	UDCONU2	11.1969	0.3177
SB19	Span 4	Start	UDCONU2	11.1969	0.3177

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB19	Span 4	Middle	UDCONU2	72.5153	2.0443
SB19	Span 4	End	UDCONU2	60.9943	1.7149
SB19	Span 5	Start	UDCONU2	60.9943	1.7149
SB19	Span 5	Middle	UDCONU2	32.0561	0.8992
SB19	Span 5	End	UDCONU2	21.5504	0.5992
SB19	Span 6	Start	UDCONU2	21.5504	0.5992
SB19	Span 6	Middle	UDCONU2	0.7878	0.0216
SB19	Span 6	End	UDCONU1	0.8157	0.0319
SB19	Span 7	Start	UDCONU1	0.8157	0.0319
SB19	Span 7	Middle		0.0000	0.0000
SB19	Span 7	End	UDCONU2	3.4189	0.0937
SB19	Span 8	Start	UDCONU2	3.4189	0.0937
SB19	Span 8	Middle	UDCONU2	6.2159	0.1704
SB19	Span 8	End	UDCONU2	6.5949	0.1816
SB19	Span 9	Start	UDCONU2	6.5949	0.1816
SB19	Span 9	Middle		0.0000	0.0000
SB19	Span 9	End	UDCONU2	3.7273	0.1033
SB19	Span 10	Start	UDCONU2	3.7273	0.1033
SB19	Span 10	Middle		0.0000	0.0000
SB19	Span 10	End	UDCONU2	3.5752	0.0990
SB19	Span 11	Start	UDCONU2	3.5752	0.0990
SB19	Span 11	Middle		0.0000	0.0000
SB19	Span 11	End	UDCONU2	1.1784	0.0328
SB19	Span 12	Start	UDCONU2	1.1784	0.0328
SB19	Span 12	Middle	UDCONU2	3.7567	0.1029
SB19	Span 12	End	UDCONU2	13.8127	0.3795
SB19	Span 13	Start	UDCONU2	13.8127	0.3795
SB19	Span 13	Middle		0.0000	0.0000
SB19	Span 13	End	UDCONU2	31.8888	0.8815
SB19	Span 14	Start	UDCONU2	30.5670	0.8592
SB19	Span 14	Middle		0.0000	0.0000
SB19	Span 14	End	UDCONU2	19.4234	0.5404
SB19	Span 15	Start	UDCONU2	19.4234	0.5404
SB19	Span 15	Middle		0.0000	0.0000
SB19	Span 15	End	UDCONU2	1.4772	0.0405
SB19	Span 16	Start	UDCONU2	1.4772	0.0405
SB19	Span 16	Middle	UDCONU2	0.0000	0.0000
SB19	Span 16	End	UDCONU2	1.3770	0.0377
SB19	Span 17	Start	UDCONU2	1.3770	0.0377
SB19	Span 17	Middle		0.0000	0.0000
SB19	Span 17	End	UDCONU2	3.4324	0.0942
SB19	Span 18	Start	UDCONU2	3.4324	0.0942
SB19	Span 18	Middle		0.0000	0.0000
SB19	Span 18	End	UDCONU2	13.7779	0.3864
SB19	Span 19	Start	UDCONU2	21.6031	0.6032
SB19	Span 19	Middle	UDCONU2	17.3608	0.4866
SB19	Span 19	End	UDCONU2	25.2189	0.7025
SB19	Span 20	Start	UDCONU2	24.7564	0.6902
SB19	Span 20	Middle		0.0000	0.0000
SB19	Span 20	End	UDCONU2	5.5198	0.1602
SB19	Span 21	Start	UDCONU2	5.5198	0.1602
SB19	Span 21	Middle	UDCONU2	55.1427	1.5566
SB19	Span 21	End	UDCONU2	62.2258	1.7556

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB19	Span 22	Start	UDCONU2	62.2258	1.7556
SB19	Span 22	Middle	UDCONU2	66.7526	1.8799
SB19	Span 22	End	UDCONU2	19.9112	0.5518
SB19	Span 23	Start	UDCONU2	19.9112	0.5518
SB19	Span 23	Middle		0.0000	0.0000
SB19	Span 23	End	UDCONU1	0.1444	0.0000
SB19	Span 24	Start	UDCONU2	0.5656	0.0400
SB19	Span 24	Middle	UDCONU2	3.0713	0.0937
SB19	Span 24	End	UDCONU2	12.5581	0.3472
SB20	Span 1	Start	UDCONU2	2.1183	0.0608
SB20	Span 1	Middle	UDCONU2	0.0000	0.0000
SB20	Span 1	End	UDCONU2	0.0000	0.0000
SB20	Span 2	Start	UDCONU2	0.0000	0.0000
SB20	Span 2	Middle	UDCONU2	0.8787	0.0000
SB20	Span 2	End	UDCONU2	8.3164	0.2306
SB20	Span 3	Start	UDCONU2	8.3164	0.2306
SB20	Span 3	Middle	UDCONU2	70.6505	1.9846
SB20	Span 3	End	UDCONU2	60.9862	1.7129
SB20	Span 4	Start	UDCONU2	60.9862	1.7129
SB20	Span 4	Middle	UDCONU2	35.8345	1.0174
SB20	Span 4	End	UDCONU2	16.2590	0.4655
SB20	Span 5	Start		0.0000	0.0000
SB20	Span 5	Middle	UDCONU2	3.4279	0.0943
SB20	Span 5	End	UDCONU2	7.5053	0.2063
SB20	Span 6	Start	UDCONU2	8.0918	0.2225
SB20	Span 6	Middle	UDCONU2	13.0581	0.3587
SB20	Span 6	End	UDCONU2	9.6634	0.2774
SB20	Span 7	Start	UDCONU2	9.6634	0.2774
SB20	Span 7	Middle		0.0000	0.0000
SB20	Span 7	End	UDCONU2	8.9356	0.2517
SB20	Span 8	Start	UDCONU2	8.9356	0.2517
SB20	Span 8	Middle	UDCONU2	15.9854	0.4492
SB20	Span 8	End		0.0000	0.0000
SB20	Span 9	Start		0.0000	0.0000
SB20	Span 9	Middle	UDCONU2	24.6792	0.6830
SB20	Span 9	End	UDCONU2	21.1771	0.5895
SB20	Span 10	Start	UDCONU2	21.1771	0.5895
SB20	Span 10	Middle		0.0000	0.0000
SB20	Span 10	End	UDCONU2	13.3914	0.3937
SB20	Span 11	Start	UDCONU2	13.3914	0.3937
SB20	Span 11	Middle	UDCONU2	32.8536	0.9378
SB20	Span 11	End	UDCONU2	45.4351	1.2787
SB20	Span 12	Start	UDCONU2	45.4351	1.2787
SB20	Span 12	Middle	UDCONU2	55.4144	1.5520
SB20	Span 12	End	UDCONU2	23.2208	0.6428
SB20	Span 13	Start	UDCONU2	23.2208	0.6428
SB20	Span 13	Middle		0.0000	0.0000
SB20	Span 13	End	UDCONU1	4.1410	0.1144
SB20	Span 14	Start	UDCONU1	4.4615	0.1253
SB20	Span 14	Middle	UDCONU2	10.0895	0.2798
SB20	Span 14	End	UDCONU2	13.4089	0.3694
SB21	Span 1	Start	UDCONU2	3.7135	0.1028
SB21	Span 1	Middle	UDCONU2	0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB21	Span 1	End	UDCONU2	0.0000	0.0000
SB21	Span 2	Start	UDCONU2	0.0000	0.0000
SB21	Span 2	Middle		0.0000	0.0000
SB21	Span 2	End	UDCONU2	0.0000	0.0000
SB21	Span 3	Start	UDCONU2	0.0000	0.0000
SB21	Span 3	Middle	UDCONU2	2.9463	0.0807
SB21	Span 3	End	UDCONU2	11.7878	0.3237
SB21	Span 4	Start	UDCONU2	11.7878	0.3237
SB21	Span 4	Middle	UDCONU2	66.0091	1.8465
SB21	Span 4	End	UDCONU2	62.2017	1.7377
SB21	Span 5	Start	UDCONU2	62.2017	1.7377
SB21	Span 5	Middle	UDCONU2	33.9295	0.9386
SB21	Span 5	End	UDCONU2	2.3328	0.0639
SB21	Span 6	Start	UDCONU2	2.3328	0.0639
SB21	Span 6	Middle		0.0000	0.0000
SB21	Span 6	End	UDCONU2	12.0697	0.4164
SB21	Span 7	Start	UDCONU2	12.0697	0.4164
SB21	Span 7	Middle		0.0000	0.0000
SB21	Span 7	End	UDCONU2	21.7851	0.6449
SB21	Span 8	Start	UDCONU2	26.3566	0.7650
SB21	Span 8	Middle		0.0000	0.0000
SB21	Span 8	End		0.0000	0.0000
SB21	Span 9	Start		0.0000	0.0000
SB21	Span 9	Middle		0.0000	0.0000
SB21	Span 9	End		0.0000	0.0000
SB21	Span 10	Start		0.0000	0.0000
SB21	Span 10	Middle		0.0000	0.0000
SB21	Span 10	End	UDCONU2	6.9515	0.1919
SB21	Span 11	Start	UDCONU2	6.9515	0.1919
SB21	Span 11	Middle		0.0000	0.0000
SB21	Span 11	End	UDCONU2	15.3423	0.4262
SB21	Span 12	Start	UDCONU2	15.3423	0.4262
SB21	Span 12	Middle	UDCONU2	36.8390	1.0333
SB21	Span 12	End	UDCONU2	76.3759	2.1610
SB21	Span 13	Start	UDCONU2	76.3759	2.1610
SB21	Span 13	Middle	UDCONU2	42.0010	1.1689
SB21	Span 13	End	UDCONU2	11.6032	0.3193
SB21	Span 14	Start	UDCONU2	11.6032	0.3193
SB21	Span 14	Middle	UDCONU2	20.9055	0.5758
SB21	Span 14	End	UDCONU2	7.9470	0.2234
SB21	Span 15	Start	UDCONU2	14.1750	0.3895
SB21	Span 15	Middle		0.0000	0.0000
SB21	Span 15	End	UDCONU2	30.0894	0.9086
SB21	Span 16	Start	UDCONU2	30.0894	0.9086
SB21	Span 16	Middle		0.0000	0.0000
SB21	Span 16	End	UDCONU2	8.7729	0.4558
SB21	Span 17	Start	UDCONU2	8.7729	0.4558
SB21	Span 17	Middle	UDCONU2	10.2447	0.4079
SB21	Span 17	End	UDCONU2	3.5480	0.2418
SB21	Span 18	Start	UDCONU2	3.5480	0.2418
SB21	Span 18	Middle		0.0000	0.0000
SB21	Span 18	End	UDCONU2	3.2540	0.1176
SB21	Span 19	Start	UDCONU2	3.2540	0.1176

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB21	Span 19	Middle	UDCONU2	11.7821	0.3266
SB21	Span 19	End	UDCONU2	24.6626	0.6801
SB21	Span 20	Start	UDCONU2	24.6626	0.6801
SB21	Span 20	Middle	UDCONU2	36.2521	1.0037
SB21	Span 20	End	UDCONU2	16.8416	0.4632
SB21	Span 21	Start	UDCONU2	16.8416	0.4632
SB21	Span 21	Middle		0.0000	0.0000
SB21	Span 21	End	UDCONU1	1.8800	0.0514
SB21	Span 22	Start	UDCONU1	2.1699	0.0694
SB21	Span 22	Middle	UDCONU2	7.5888	0.2119
SB21	Span 22	End	UDCONU2	10.2332	0.2816
SB22	Span 1	Start	UDCONU2	2.6779	0.0801
SB22	Span 1	Middle	UDCONU2	0.0000	0.0000
SB22	Span 1	End	UDCONU2	0.0000	0.0000
SB22	Span 2	Start	UDCONU2	0.0000	0.0000
SB22	Span 2	Middle	UDCONU2	3.3517	0.0931
SB22	Span 2	End	UDCONU2	9.0026	0.2470
SB22	Span 3	Start	UDCONU2	9.0026	0.2470
SB22	Span 3	Middle	UDCONU2	52.7084	1.4676
SB22	Span 3	End	UDCONU2	55.7653	1.5544
SB22	Span 4	Start	UDCONU2	55.7653	1.5544
SB22	Span 4	Middle	UDCONU2	33.4533	0.9253
SB22	Span 4	End	UDCONU2	6.9740	0.1987
SB22	Span 5	Start	UDCONU2	6.9740	0.1987
SB22	Span 5	Middle	UDCONU2	8.6269	0.2434
SB22	Span 5	End		0.0000	0.0000
SB22	Span 6	Start		0.0000	0.0000
SB22	Span 6	Middle		0.0000	0.0000
SB22	Span 6	End		0.0000	0.0000
SB22	Span 7	Start		0.0000	0.0000
SB22	Span 7	Middle		0.0000	0.0000
SB22	Span 7	End		0.0000	0.0000
SB22	Span 8	Start		0.0000	0.0000
SB22	Span 8	Middle		0.0000	0.0000
SB22	Span 8	End	UDCONU2	2.1319	0.0670
SB22	Span 9	Start	UDCONU2	2.1319	0.0670
SB22	Span 9	Middle	UDCONU2	3.8405	0.2019
SB22	Span 9	End	UDCONU2	1.0489	0.0962
SB22	Span 10	Start	UDCONU2	1.0489	0.0962
SB22	Span 10	Middle	UDCONU2	0.0000	0.0585
SB22	Span 10	End	UDCONU2	9.1865	0.2622
SB22	Span 11	Start	UDCONU2	14.6865	0.4050
SB22	Span 11	Middle	UDCONU2	16.6415	0.4594
SB22	Span 11	End	UDCONU2	14.9067	0.4111
SB22	Span 12	Start	UDCONU2	8.8806	0.2441
SB22	Span 12	Middle	UDCONU2	1.6697	0.0457
SB22	Span 12	End	UDCONU2	2.3068	0.0637
SB23	Span 1	Start	UDCONU2	1.3668	0.0458
SB23	Span 1	Middle		0.0000	0.0000
SB23	Span 1	End	UDCONU2	1.9293	0.0827
SB23	Span 2	Start	UDCONU2	1.9293	0.0827
SB23	Span 2	Middle	UDCONU2	0.1987	0.0480
SB23	Span 2	End	UDCONU2	7.6510	0.3353

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB23	Span 3	Start	UDCONU2	7.6510	0.3353
SB23	Span 3	Middle		0.0000	0.0000
SB23	Span 3	End	UDCONU2	6.4242	0.2930
SB23	Span 4	Start	UDCONU2	6.4242	0.2930
SB23	Span 4	Middle	UDCONU2	5.9219	0.5651
SB23	Span 4	End	UDCONU2	0.2581	0.0991
SB23	Span 5	Start	UDCONU2	0.2581	0.0991
SB23	Span 5	Middle		0.0000	0.0000
SB23	Span 5	End	UDCONU2	1.6127	0.0442
SB23	Span 6	Start	UDCONU2	1.6127	0.0442
SB23	Span 6	Middle	UDCONU2	16.7153	0.4634
SB23	Span 6	End	UDCONU2	18.4193	0.5114
SB23	Span 7	Start	UDCONU2	18.4193	0.5114
SB23	Span 7	Middle	UDCONU2	11.5050	0.3176
SB23	Span 7	End	UDCONU2	4.7492	0.1304
SB23	Span 8	Start	UDCONU2	4.7492	0.1304
SB23	Span 8	Middle		0.0000	0.0000
SB23	Span 8	End	UDCONU2	6.8672	0.1950
SB23	Span 9	Start	UDCONU2	6.8672	0.1950
SB23	Span 9	Middle		0.0000	0.0000
SB23	Span 9	End	UDCONU2	8.8470	0.2559
SB23	Span 10	Start	UDCONU2	8.8470	0.2559
SB23	Span 10	Middle		0.0000	0.0000
SB23	Span 10	End		0.0000	0.0000
SB23	Span 11	Start		0.0000	0.0000
SB23	Span 11	Middle		0.0000	0.0000
SB23	Span 11	End		0.0000	0.0000
SB23	Span 12	Start		0.0000	0.0000
SB23	Span 12	Middle		0.0000	0.0000
SB23	Span 12	End		0.0000	0.0000
SB23	Span 13	Start		0.0000	0.0000
SB23	Span 13	Middle		0.0000	0.0000
SB23	Span 13	End		0.0000	0.0000
SB23	Span 14	Start		0.0000	0.0000
SB23	Span 14	Middle		0.0000	0.0000
SB23	Span 14	End		0.0000	0.0000
SB23	Span 15	Start		0.0000	0.0000
SB23	Span 15	Middle		0.0000	0.0000
SB23	Span 15	End		0.0000	0.0000
SB23	Span 16	Start		0.0000	0.0000
SB23	Span 16	Middle		0.0000	0.0000
SB23	Span 16	End		0.0000	0.0000
SB23	Span 17	Start		0.0000	0.0000
SB23	Span 17	Middle		0.0000	0.0000
SB23	Span 17	End		0.0000	0.0000
SB24	Span 1	Start	UDCONU2	1.3740	0.0514
SB24	Span 1	Middle		0.0000	0.0000
SB24	Span 1	End	UDCONU2	1.5219	0.0780
SB24	Span 2	Start	UDCONU2	1.5219	0.0780
SB24	Span 2	Middle	UDCONU2	6.7264	0.2199
SB24	Span 2	End	UDCONU2	2.2373	0.0741
SB24	Span 3	Start	UDCONU2	4.4746	0.1352
SB24	Span 3	Middle		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB24	Span 3	End		0.0000	0.0000
SB24	Span 4	Start		0.0000	0.0000
SB24	Span 4	Middle		0.0000	0.0000
SB24	Span 4	End		0.0000	0.0000
SB24	Span 5	Start		0.0000	0.0000
SB24	Span 5	Middle		0.0000	0.0000
SB24	Span 5	End		0.0000	0.0000
SB24	Span 6	Start		0.0000	0.0000
SB24	Span 6	Middle		0.0000	0.0000
SB24	Span 6	End		0.0000	0.0000
SB24	Span 7	Start		0.0000	0.0000
SB24	Span 7	Middle		0.0000	0.0000
SB24	Span 7	End		0.0000	0.0000
SB24	Span 8	Start		0.0000	0.0000
SB24	Span 8	Middle		0.0000	0.0000
SB24	Span 8	End		0.0000	0.0000
SB24	Span 9	Start		0.0000	0.0000
SB24	Span 9	Middle		0.0000	0.0000
SB24	Span 9	End		0.0000	0.0000
SB24	Span 10	Start		0.0000	0.0000
SB24	Span 10	Middle		0.0000	0.0000
SB24	Span 10	End		0.0000	0.0000
SB24	Span 11	Start		0.0000	0.0000
SB24	Span 11	Middle		0.0000	0.0000
SB24	Span 11	End		0.0000	0.0000
SB24	Span 12	Start		0.0000	0.0000
SB24	Span 12	Middle		0.0000	0.0000
SB24	Span 12	End		0.0000	0.0000
SB24	Span 13	Start		0.0000	0.0000
SB24	Span 13	Middle		0.0000	0.0000
SB24	Span 13	End		0.0000	0.0000
SB25	Span 1	Start	UDCONU2	3.2364	0.0954
SB25	Span 1	Middle	UDCONU2	7.8794	0.2161
SB25	Span 1	End	UDCONU1	4.6978	0.1291
SB25	Span 2	Start	UDCONU1	4.6978	0.1291
SB25	Span 2	Middle	UDCONU2	10.1461	0.2802
SB25	Span 2	End	UDCONU2	8.4782	0.2338
SB25	Span 3	Start	UDCONU2	8.4782	0.2338
SB25	Span 3	Middle	UDCONU2	5.5137	0.1516
SB25	Span 3	End	UDCONU2	1.3301	0.0422
SB25	Span 4	Start	UDCONU2	1.3301	0.0422
SB25	Span 4	Middle		0.0000	0.0000
SB25	Span 4	End	UDCONU1	0.0019	0.0000
SB25	Span 5	Start	UDCONU1	0.0019	0.0000
SB25	Span 5	Middle	UDCONU2	0.0378	0.0011
SB25	Span 5	End		0.0000	0.0000
SB25	Span 6	Start		0.0000	0.0000
SB25	Span 6	Middle		0.0000	0.0000
SB25	Span 6	End		0.0000	0.0000
SB25	Span 7	Start		0.0000	0.0000
SB25	Span 7	Middle		0.0000	0.0000
SB25	Span 7	End		0.0000	0.0000
SB25	Span 8	Start		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB25	Span 8	Middle		0.0000	0.0000
SB25	Span 8	End		0.0000	0.0000
SB25	Span 9	Start		0.0000	0.0000
SB25	Span 9	Middle		0.0000	0.0000
SB25	Span 9	End		0.0000	0.0000
SB25	Span 10	Start		0.0000	0.0000
SB25	Span 10	Middle		0.0000	0.0000
SB25	Span 10	End		0.0000	0.0000
SB25	Span 11	Start		0.0000	0.0000
SB25	Span 11	Middle		0.0000	0.0000
SB25	Span 11	End		0.0000	0.0000
SB26	Span 1	Start	UDCONU2	3.5731	0.1127
SB26	Span 1	Middle	UDCONU2	0.0000	0.0000
SB26	Span 1	End	UDCONU2	0.0000	0.0000
SB26	Span 2	Start	UDCONU2	0.0000	0.0000
SB26	Span 2	Middle	UDCONU2	4.3649	0.1211
SB26	Span 2	End	UDCONU2	7.8431	0.2151
SB26	Span 3	Start	UDCONU2	7.8431	0.2151
SB26	Span 3	Middle	UDCONU2	29.1021	0.8037
SB26	Span 3	End	UDCONU2	23.0013	0.6339
SB26	Span 4	Start	UDCONU2	23.0013	0.6339
SB26	Span 4	Middle	UDCONU2	16.0432	0.4411
SB26	Span 4	End	UDCONU2	0.0000	0.0000
SB26	Span 5	Start	UDCONU2	0.0000	0.0000
SB26	Span 5	Middle		0.0000	0.0000
SB26	Span 5	End	UDCONU2	0.0000	0.0000
SB26	Span 6	Start	UDCONU2	0.0000	0.0000
SB26	Span 6	Middle	UDCONU2	1.8596	0.0523
SB26	Span 6	End		0.0000	0.0000
SB26	Span 7	Start		0.0000	0.0000
SB26	Span 7	Middle		0.0000	0.0000
SB26	Span 7	End		0.0000	0.0000
SB26	Span 8	Start		0.0000	0.0000
SB26	Span 8	Middle		0.0000	0.0000
SB26	Span 8	End		0.0000	0.0000
SB26	Span 9	Start		0.0000	0.0000
SB26	Span 9	Middle		0.0000	0.0000
SB26	Span 9	End		0.0000	0.0000
SB26	Span 10	Start		0.0000	0.0000
SB26	Span 10	Middle		0.0000	0.0000
SB26	Span 10	End		0.0000	0.0000
SB26	Span 11	Start		0.0000	0.0000
SB26	Span 11	Middle		0.0000	0.0000
SB26	Span 11	End		0.0000	0.0000
SB26	Span 12	Start		0.0000	0.0000
SB26	Span 12	Middle		0.0000	0.0000
SB26	Span 12	End		0.0000	0.0000
SB27	Span 1	Start	UDCONU2	1.6510	0.0475
SB27	Span 1	Middle	UDCONU2	0.0000	0.0000
SB27	Span 1	End	UDCONU2	5.9605	0.1667
SB27	Span 2	Start	UDCONU2	5.9605	0.1667
SB27	Span 2	Middle	UDCONU2	30.0985	0.8316
SB27	Span 2	End	UDCONU2	25.0020	0.6896

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB27	Span 3	Start	UDCONU2	25.0020	0.6896
SB27	Span 3	Middle	UDCONU2	13.7635	0.3782
SB27	Span 3	End	UDCONU2	0.0000	0.0000
SB27	Span 4	Start	UDCONU2	0.0000	0.0000
SB27	Span 4	Middle		0.0000	0.0000
SB27	Span 4	End	UDCONU2	0.0000	0.0000
SB27	Span 5	Start	UDCONU2	0.0000	0.0000
SB27	Span 5	Middle	UDCONU2	0.6326	0.0000
SB27	Span 5	End		0.0000	0.0000
SB27	Span 6	Start		0.0000	0.0000
SB27	Span 6	Middle		0.0000	0.0000
SB27	Span 6	End		0.0000	0.0000
SB27	Span 7	Start		0.0000	0.0000
SB27	Span 7	Middle		0.0000	0.0000
SB27	Span 7	End		0.0000	0.0000
SB27	Span 8	Start		0.0000	0.0000
SB27	Span 8	Middle		0.0000	0.0000
SB27	Span 8	End		0.0000	0.0000
SB27	Span 9	Start		0.0000	0.0000
SB27	Span 9	Middle		0.0000	0.0000
SB27	Span 9	End		0.0000	0.0000
SB27	Span 10	Start		0.0000	0.0000
SB27	Span 10	Middle		0.0000	0.0000
SB27	Span 10	End		0.0000	0.0000
SB27	Span 11	Start		0.0000	0.0000
SB27	Span 11	Middle		0.0000	0.0000
SB27	Span 11	End		0.0000	0.0000
SB28	Span 1	Start	UDCONU2	3.6104	0.1066
SB28	Span 1	Middle	UDCONU2	0.1123	0.0000
SB28	Span 1	End	UDCONU2	0.0000	0.0000
SB28	Span 2	Start	UDCONU2	0.0000	0.0000
SB28	Span 2	Middle	UDCONU2	3.8462	0.1138
SB28	Span 2	End	UDCONU2	8.2223	0.2272
SB28	Span 3	Start	UDCONU2	8.2223	0.2272
SB28	Span 3	Middle	UDCONU2	28.6361	0.7908
SB28	Span 3	End	UDCONU2	22.5057	0.6202
SB28	Span 4	Start	UDCONU2	22.5057	0.6202
SB28	Span 4	Middle	UDCONU2	15.0143	0.4127
SB28	Span 4	End	UDCONU2	0.1461	0.0000
SB28	Span 5	Start	UDCONU2	0.1461	0.0000
SB28	Span 5	Middle		0.0000	0.0000
SB28	Span 5	End	UDCONU2	0.6878	0.0215
SB28	Span 6	Start	UDCONU2	0.6878	0.0215
SB28	Span 6	Middle	UDCONU2	2.5740	0.0718
SB28	Span 6	End		0.0000	0.0000
SB28	Span 7	Start		0.0000	0.0000
SB28	Span 7	Middle		0.0000	0.0000
SB28	Span 7	End		0.0000	0.0000
SB28	Span 8	Start		0.0000	0.0000
SB28	Span 8	Middle		0.0000	0.0000
SB28	Span 8	End		0.0000	0.0000
SB28	Span 9	Start		0.0000	0.0000
SB28	Span 9	Middle		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB28	Span 9	End		0.0000	0.0000
SB28	Span 10	Start		0.0000	0.0000
SB28	Span 10	Middle		0.0000	0.0000
SB28	Span 10	End		0.0000	0.0000
SB28	Span 11	Start		0.0000	0.0000
SB28	Span 11	Middle		0.0000	0.0000
SB28	Span 11	End		0.0000	0.0000
SB28	Span 12	Start		0.0000	0.0000
SB28	Span 12	Middle		0.0000	0.0000
SB28	Span 12	End		0.0000	0.0000
SB29	Span 1	Start	UDCONU2	1.9256	0.0691
SB29	Span 1	Middle	UDCONU2	2.6143	0.0841
SB29	Span 1	End	UDCONU2	3.6850	0.1220
SB29	Span 2	Start	UDCONU2	3.6850	0.1220
SB29	Span 2	Middle	UDCONU2	9.6389	0.2845
SB29	Span 2	End	UDCONU2	2.9444	0.0806
SB29	Span 3	Start	UDCONU2	9.9903	0.2785
SB29	Span 3	Middle	UDCONU1	0.2041	0.0056
SB29	Span 3	End	UDCONU1	0.4909	0.0136
SB29	Span 4	Start	UDCONU1	0.4909	0.0136
SB29	Span 4	Middle	UDCONU2	2.1452	0.0592
SB29	Span 4	End	UDCONU2	1.5697	0.0432
SB29	Span 5	Start	UDCONU2	1.5697	0.0432
SB29	Span 5	Middle	UDCONU2	0.9214	0.0253
SB29	Span 5	End		0.0000	0.0000
SB29	Span 6	Start		0.0000	0.0000
SB29	Span 6	Middle		0.0000	0.0000
SB29	Span 6	End		0.0000	0.0000
SB29	Span 7	Start		0.0000	0.0000
SB29	Span 7	Middle		0.0000	0.0000
SB29	Span 7	End		0.0000	0.0000
SB29	Span 8	Start		0.0000	0.0000
SB29	Span 8	Middle		0.0000	0.0000
SB29	Span 8	End		0.0000	0.0000
SB29	Span 9	Start		0.0000	0.0000
SB29	Span 9	Middle		0.0000	0.0000
SB29	Span 9	End		0.0000	0.0000
SB29	Span 10	Start		0.0000	0.0000
SB29	Span 10	Middle		0.0000	0.0000
SB29	Span 10	End		0.0000	0.0000
SB29	Span 11	Start		0.0000	0.0000
SB29	Span 11	Middle		0.0000	0.0000
SB29	Span 11	End		0.0000	0.0000
SB29	Span 12	Start		0.0000	0.0000
SB29	Span 12	Middle		0.0000	0.0000
SB29	Span 12	End		0.0000	0.0000
SB30	Span 1	Start	UDCONU2	1.6803	0.0497
SB30	Span 1	Middle	UDCONU2	10.6702	0.2929
SB30	Span 1	End		0.0000	0.0000
SB30	Span 2	Start		0.0000	0.0000
SB30	Span 2	Middle		0.0000	0.0000
SB30	Span 2	End		0.0000	0.0000
SB30	Span 3	Start		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB30	Span 3	Middle		0.0000	0.0000
SB30	Span 3	End		0.0000	0.0000
SB30	Span 4	Start		0.0000	0.0000
SB30	Span 4	Middle		0.0000	0.0000
SB30	Span 4	End		0.0000	0.0000
SB30	Span 5	Start		0.0000	0.0000
SB30	Span 5	Middle		0.0000	0.0000
SB30	Span 5	End		0.0000	0.0000
SB30	Span 6	Start		0.0000	0.0000
SB30	Span 6	Middle		0.0000	0.0000
SB30	Span 6	End		0.0000	0.0000
SB30	Span 7	Start		0.0000	0.0000
SB30	Span 7	Middle		0.0000	0.0000
SB30	Span 7	End		0.0000	0.0000
SB30	Span 8	Start		0.0000	0.0000
SB30	Span 8	Middle		0.0000	0.0000
SB30	Span 8	End		0.0000	0.0000
SB30	Span 9	Start		0.0000	0.0000
SB30	Span 9	Middle		0.0000	0.0000
SB30	Span 9	End		0.0000	0.0000
SB30	Span 10	Start		0.0000	0.0000
SB30	Span 10	Middle		0.0000	0.0000
SB30	Span 10	End		0.0000	0.0000
SB31	Span 1	Start	UDCONU2	1.6875	0.0462
SB31	Span 1	Middle	UDCONU2	2.6996	0.0740
SB31	Span 1	End	UDCONU2	1.1513	0.0332
SB31	Span 2	Start	UDCONU2	1.1513	0.0332
SB31	Span 2	Middle		0.0000	0.0000
SB31	Span 2	End		0.0000	0.0000
SB31	Span 3	Start		0.0000	0.0000
SB31	Span 3	Middle		0.0000	0.0000
SB31	Span 3	End		0.0000	0.0000
SB31	Span 4	Start		0.0000	0.0000
SB31	Span 4	Middle		0.0000	0.0000
SB31	Span 4	End		0.0000	0.0000
SB31	Span 5	Start		0.0000	0.0000
SB31	Span 5	Middle		0.0000	0.0000
SB31	Span 5	End		0.0000	0.0000
SB31	Span 6	Start		0.0000	0.0000
SB31	Span 6	Middle		0.0000	0.0000
SB31	Span 6	End		0.0000	0.0000
SB31	Span 7	Start		0.0000	0.0000
SB31	Span 7	Middle		0.0000	0.0000
SB31	Span 7	End		0.0000	0.0000
SB31	Span 8	Start		0.0000	0.0000
SB31	Span 8	Middle		0.0000	0.0000
SB31	Span 8	End		0.0000	0.0000
SB31	Span 9	Start		0.0000	0.0000
SB31	Span 9	Middle		0.0000	0.0000
SB31	Span 9	End		0.0000	0.0000
SB31	Span 10	Start		0.0000	0.0000
SB31	Span 10	Middle		0.0000	0.0000
SB31	Span 10	End		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 2 of 3

Strip	SpanID	Location	FBotCombo	FBotMoment	FBotArea
				kip-ft	in2
SB31	Span 11	Start		0.0000	0.0000
SB31	Span 11	Middle		0.0000	0.0000
SB31	Span 11	End		0.0000	0.0000

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA1	Span 1	Start		0.000	0.0000	OK	A
SA1	Span 1	Middle	UDCONU2	1.329	0.0000	OK	A
SA1	Span 1	End	UDCONU2	3.253	0.0000	OK	A
SA1	Span 2	Start	UDCONU2	3.253	0.0000	OK	A
SA1	Span 2	Middle		0.000	0.0000	OK	A
SA1	Span 2	End	UDCONU2	1.961	0.0000	OK	A
SA1	Span 3	Start	UDCONU2	1.961	0.0000	OK	A
SA1	Span 3	Middle	UDCONU2	0.495	0.0000	OK	A
SA1	Span 3	End	UDCONU2	0.170	0.0000	OK	A
SA1	Span 4	Start	UDCONU2	0.170	0.0000	OK	A
SA1	Span 4	Middle	UDCONU2	0.382	0.0000	OK	A
SA1	Span 4	End	UDCONU2	1.684	0.0000	OK	A
SA1	Span 5	Start	UDCONU2	1.684	0.0000	OK	A
SA1	Span 5	Middle		0.000	0.0000	OK	A
SA1	Span 5	End	UDCONU1	0.700	0.0000	OK	A
SA1	Span 6	Start	UDCONU1	0.700	0.0000	OK	A
SA1	Span 6	Middle	UDCONU2	0.191	0.0000	OK	A
SA1	Span 6	End	UDCONU2	0.677	0.0000	OK	A
SA1	Span 7	Start	UDCONU2	0.677	0.0000	OK	A
SA1	Span 7	Middle		0.000	0.0000	OK	A
SA1	Span 7	End	UDCONU2	3.416	0.0000	OK	A
SA1	Span 8	Start	UDCONU2	3.416	0.0000	OK	A
SA1	Span 8	Middle	UDCONU2	2.037	0.0000	OK	A
SA1	Span 8	End	UDCONU2	1.329	0.0000	OK	A
SA1	Span 9	Start	UDCONU2	1.329	0.0000	OK	A
SA1	Span 9	Middle		0.000	0.0000	OK	A
SA1	Span 9	End	UDCONU2	0.434	0.0000	OK	A
SA1	Span 10	Start	UDCONU2	0.434	0.0000	OK	A
SA1	Span 10	Middle		0.000	0.0000	OK	A
SA1	Span 10	End	UDCONU2	0.802	0.0000	OK	A
SA1	Span 11	Start	UDCONU2	0.802	0.0000	OK	A
SA1	Span 11	Middle		0.000	0.0000	OK	A
SA1	Span 11	End	UDCONU2	0.941	0.0000	OK	A
SA1	Span 12	Start	UDCONU2	0.941	0.0000	OK	A
SA1	Span 12	Middle	UDCONU2	0.057	0.0000	OK	A
SA1	Span 12	End	UDCONU2	2.988	0.0000	OK	A
SA1	Span 13	Start	UDCONU2	2.988	0.0000	OK	A
SA1	Span 13	Middle		0.000	0.0000	OK	A
SA1	Span 13	End	UDCONU2	2.988	0.0000	OK	A
SA1	Span 14	Start	UDCONU2	2.988	0.0000	OK	A
SA1	Span 14	Middle	UDCONU2	2.015	0.0000	OK	A
SA1	Span 14	End	UDCONU2	0.649	0.0000	OK	A
SA1	Span 15	Start	UDCONU2	0.649	0.0000	OK	A
SA1	Span 15	Middle	UDCONU2	0.361	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA1	Span 15	End	UDCONU2	3.400	0.0000	OK	A
SA1	Span 16	Start	UDCONU2	3.400	0.0000	OK	A
SA1	Span 16	Middle		0.000	0.0000	OK	A
SA1	Span 16	End	UDCONU2	3.400	0.0000	OK	A
SA1	Span 17	Start	UDCONU2	3.400	0.0000	OK	A
SA1	Span 17	Middle	UDCONU2	2.135	0.0000	OK	A
SA1	Span 17	End	UDCONU2	0.818	0.0000	OK	A
SA1	Span 18	Start	UDCONU2	0.818	0.0000	OK	A
SA1	Span 18	Middle		0.000	0.0000	OK	A
SA1	Span 18	End	UDCONU2	0.299	0.0000	OK	A
SA1	Span 19	Start	UDCONU2	0.299	0.0000	OK	A
SA1	Span 19	Middle	UDCONU2	0.014	0.0000	OK	A
SA1	Span 19	End	UDCONU2	1.694	0.0000	OK	A
SA1	Span 20	Start	UDCONU2	1.694	0.0000	OK	A
SA1	Span 20	Middle	UDCONU2	0.561	0.0000	OK	A
SA1	Span 20	End	UDCONU2	0.344	0.0000	OK	A
SA1	Span 21	Start	UDCONU2	0.344	0.0000	OK	A
SA1	Span 21	Middle	UDCONU2	0.337	0.0000	OK	A
SA1	Span 21	End	UDCONU2	1.681	0.0000	OK	A
SA1	Span 22	Start	UDCONU2	1.681	0.0000	OK	A
SA1	Span 22	Middle		0.000	0.0000	OK	A
SA1	Span 22	End	UDCONU2	1.260	0.0000	OK	A
SA1	Span 23	Start	UDCONU2	1.260	0.0000	OK	A
SA1	Span 23	Middle	UDCONU2	1.075	0.0000	OK	A
SA1	Span 23	End	UDCONU2	1.075	0.0000	OK	A
SA1	Span 24	Start	UDCONU2	1.075	0.0000	OK	A
SA1	Span 24	Middle	UDCONU2	0.679	0.0000	OK	A
SA1	Span 24	End	UDCONU2	0.225	0.0000	OK	A
SA1	Span 25	Start	UDCONU2	0.225	0.0000	OK	A
SA1	Span 25	Middle	UDCONU2	0.639	0.0000	OK	A
SA1	Span 25	End	UDCONU1	0.044	0.0000	OK	A
SA1	Span 26	Start	UDCONU1	0.044	0.0000	OK	A
SA1	Span 26	Middle	UDCONU2	0.040	0.0000	OK	A
SA1	Span 26	End	UDCONU1	0.710	0.0000	OK	A
SA1	Span 27	Start	UDCONU1	0.710	0.0000	OK	A
SA1	Span 27	Middle		0.000	0.0000	OK	A
SA1	Span 27	End	UDCONU1	0.621	0.0000	OK	A
SA1	Span 28	Start	UDCONU1	0.621	0.0000	OK	A
SA1	Span 28	Middle	UDCONU2	0.397	0.0000	OK	A
SA1	Span 28	End	UDCONU2	0.657	0.0000	OK	A
SA2	Cantilever Start	Start		0.000	0.0000	OK	A
SA2	Cantilever Start	Middle		0.000	0.0000	OK	A
SA2	Cantilever Start	End	UDCONU2	4.338	0.0000	OK	A
SA2	Span 1	Start	UDCONU2	4.338	0.0000	OK	A
SA2	Span 1	Middle	UDCONU2	1.425	0.0000	OK	A
SA2	Span 1	End	UDCONU2	4.392	0.0000	OK	A
SA2	Span 2	Start	UDCONU2	4.392	0.0000	OK	A
SA2	Span 2	Middle	UDCONU2	2.230	0.0000	OK	A
SA2	Span 2	End	UDCONU2	2.320	0.0000	OK	A
SA2	Span 3	Start	UDCONU2	2.320	0.0000	OK	A
SA2	Span 3	Middle		0.000	0.0000	OK	A
SA2	Span 3	End	UDCONU1	4.698	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA2	Span 4	Start	UDCONU1	4.698	0.0000	OK	A
SA2	Span 4	Middle		0.000	0.0000	OK	A
SA2	Span 4	End	UDCONU1	1.190	0.0000	OK	A
SA2	Span 5	Start	UDCONU1	1.190	0.0000	OK	A
SA2	Span 5	Middle	UDCONU2	1.040	0.0000	OK	A
SA2	Span 5	End	UDCONU2	6.613	0.0000	OK	A
SA2	Span 6	Start	UDCONU2	6.613	0.0000	OK	A
SA2	Span 6	Middle		0.000	0.0000	OK	A
SA2	Span 6	End	UDCONU2	1.083	0.0000	OK	A
SA2	Span 7	Start	UDCONU2	1.396	0.0000	OK	A
SA2	Span 7	Middle		0.000	0.0000	OK	A
SA2	Span 7	End	UDCONU2	0.960	0.0000	OK	A
SA2	Span 8	Start	UDCONU2	0.960	0.0000	OK	A
SA2	Span 8	Middle		0.000	0.0000	OK	A
SA2	Span 8	End	UDCONU2	2.025	0.0000	OK	A
SA2	Span 9	Start	UDCONU2	2.025	0.0000	OK	A
SA2	Span 9	Middle		0.000	0.0000	OK	A
SA2	Span 9	End	UDCONU2	1.804	0.0000	OK	A
SA2	Span 10	Start	UDCONU2	1.804	0.0000	OK	A
SA2	Span 10	Middle	UDCONU1	0.117	0.0000	OK	A
SA2	Span 10	End	UDCONU2	2.147	0.0000	OK	A
SA2	Span 11	Start	UDCONU2	2.147	0.0000	OK	A
SA2	Span 11	Middle		0.000	0.0000	OK	A
SA2	Span 11	End	UDCONU2	0.399	0.0000	OK	A
SA2	Span 12	Start	UDCONU2	0.399	0.0000	OK	A
SA2	Span 12	Middle		0.000	0.0000	OK	A
SA2	Span 12	End	UDCONU2	0.753	0.0000	OK	A
SA2	Span 13	Start	UDCONU2	0.753	0.0000	OK	A
SA2	Span 13	Middle		0.000	0.0000	OK	A
SA2	Span 13	End	UDCONU2	3.738	0.0000	OK	A
SA2	Span 14	Start	UDCONU2	3.738	0.0000	OK	A
SA2	Span 14	Middle	UDCONU2	6.350	0.0000	OK	A
SA2	Span 14	End	UDCONU2	4.106	0.0000	OK	A
SA2	Span 15	Start	UDCONU2	4.106	0.0000	OK	A
SA2	Span 15	Middle		0.000	0.0000	OK	A
SA2	Span 15	End	UDCONU2	5.947	0.0000	OK	A
SA2	Span 16	Start	UDCONU2	5.947	0.0000	OK	A
SA2	Span 16	Middle	UDCONU2	3.875	0.0000	OK	A
SA2	Span 16	End	UDCONU2	6.476	0.0000	OK	A
SA2	Span 17	Start	UDCONU2	6.476	0.0000	OK	A
SA2	Span 17	Middle		0.000	0.0000	OK	A
SA2	Span 17	End	UDCONU2	7.034	0.0000	OK	A
SA2	Span 18	Start	UDCONU2	7.034	0.0000	OK	A
SA2	Span 18	Middle		0.000	0.0000	OK	A
SA2	Span 18	End	UDCONU1	2.575	0.0000	OK	A
SA2	Span 19	Start	UDCONU1	2.575	0.0000	OK	A
SA2	Span 19	Middle	UDCONU1	1.834	0.0000	OK	A
SA2	Span 19	End	UDCONU2	3.843	0.0000	OK	A
SA2	Span 20	Start	UDCONU2	3.843	0.0000	OK	A
SA2	Span 20	Middle	UDCONU2	3.433	0.0000	OK	A
SA2	Span 20	End	UDCONU2	11.512	0.0000	OK	A
SA2	Span 21	Start	UDCONU2	11.512	0.0000	OK	A
SA2	Span 21	Middle		0.000	0.0000	OK	A
SA2	Span 21	End	UDCONU2	11.500	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA2	Span 22	Start	UDCONU2	11.500	0.0000	OK	A
SA2	Span 22	Middle	UDCONU2	1.258	0.0000	OK	A
SA2	Span 22	End	UDCONU2	2.563	0.0000	OK	A
SA2	Span 23	Start	UDCONU2	2.563	0.0000	OK	A
SA2	Span 23	Middle		0.000	0.0000	OK	A
SA2	Span 23	End	UDCONU2	0.816	0.0000	OK	A
SA2	Span 24	Start	UDCONU2	0.816	0.0000	OK	A
SA2	Span 24	Middle		0.000	0.0000	OK	A
SA2	Span 24	End	UDCONU2	6.272	0.0000	OK	A
SA2	Span 25	Start	UDCONU2	6.272	0.0000	OK	A
SA2	Span 25	Middle	UDCONU2	2.299	0.0000	OK	A
SA2	Span 25	End	UDCONU2	11.494	0.0000	OK	A
SA2	Span 26	Start	UDCONU2	11.494	0.0000	OK	A
SA2	Span 26	Middle		0.000	0.0000	OK	A
SA2	Span 26	End	UDCONU2	9.405	0.0000	OK	A
SA2	Span 27	Start	UDCONU2	9.405	0.0000	OK	A
SA2	Span 27	Middle	UDCONU2	3.352	0.0000	OK	A
SA2	Span 27	End	UDCONU2	8.313	0.0000	OK	A
SA2	Span 28	Start	UDCONU2	8.313	0.0000	OK	A
SA2	Span 28	Middle		0.000	0.0000	OK	A
SA2	Span 28	End	UDCONU2	8.313	0.0000	OK	A
SA2	Span 29	Start	UDCONU2	8.313	0.0000	OK	A
SA2	Span 29	Middle	UDCONU2	9.752	0.0000	OK	A
SA2	Span 29	End	UDCONU2	3.551	0.0000	OK	A
SA2	Span 30	Start	UDCONU2	3.551	0.0000	OK	A
SA2	Span 30	Middle		0.000	0.0000	OK	A
SA2	Span 30	End	UDCONU2	4.073	0.0000	OK	A
SA2	Span 31	Start	UDCONU2	4.073	0.0000	OK	A
SA2	Span 31	Middle	UDCONU2	0.804	0.0000	OK	A
SA2	Span 31	End	UDCONU2	0.628	0.0000	OK	A
SA2	Span 32	Start	UDCONU2	0.628	0.0000	OK	A
SA2	Span 32	Middle	UDCONU2	0.442	0.0000	OK	A
SA2	Span 32	End	UDCONU2	1.338	0.0000	OK	A
SA2	Span 33	Start	UDCONU2	1.338	0.0000	OK	A
SA2	Span 33	Middle		0.000	0.0000	OK	A
SA2	Span 33	End	UDCONU2	1.393	0.0000	OK	A
SA2	Span 34	Start	UDCONU2	1.393	0.0000	OK	A
SA2	Span 34	Middle		0.000	0.0000	OK	A
SA2	Span 34	End	UDCONU1	0.968	0.0000	OK	A
SA2	Span 35	Start	UDCONU1	0.968	0.0000	OK	A
SA2	Span 35	Middle	UDCONU2	0.717	0.0000	OK	A
SA2	Span 35	End	UDCONU1	0.181	0.0000	OK	A
SA2	Span 36	Start	UDCONU1	0.181	0.0000	OK	A
SA2	Span 36	Middle	UDCONU2	0.210	0.0000	OK	A
SA2	Span 36	End	UDCONU1	2.247	0.0000	OK	A
SA2	Span 37	Start	UDCONU1	2.247	0.0000	OK	A
SA2	Span 37	Middle		0.000	0.0000	OK	A
SA2	Span 37	End	UDCONU1	1.537	0.0000	OK	A
SA2	Span 38	Start	UDCONU1	1.537	0.0000	OK	A
SA2	Span 38	Middle		0.000	0.0000	OK	A
SA2	Span 38	End	UDCONU2	0.442	0.0000	OK	A
SA2	Span 39	Start	UDCONU2	0.442	0.0000	OK	A
SA2	Span 39	Middle		0.000	0.0000	OK	A
SA2	Span 39	End	UDCONU2	1.180	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA2	Span 40	Start	UDCONU2	1.180	0.0000	OK	A
SA2	Span 40	Middle	UDCONU1	0.234	0.0000	OK	A
SA2	Span 40	End	UDCONU2	0.744	0.0000	OK	A
SA3	Span 1	Start		0.000	0.0000	OK	A
SA3	Span 1	Middle		0.000	0.0000	OK	A
SA3	Span 1	End	UDCONU2	2.092	0.0000	OK	A
SA3	Span 2	Start	UDCONU2	3.281	0.0000	OK	A
SA3	Span 2	Middle		0.000	0.0000	OK	A
SA3	Span 2	End	UDCONU2	1.539	0.0000	OK	A
SA3	Span 3	Start	UDCONU2	1.539	0.0000	OK	A
SA3	Span 3	Middle		0.000	0.0000	OK	A
SA3	Span 3	End	UDCONU2	12.741	0.0000	OK	A
SA3	Span 4	Start	UDCONU2	12.741	0.0000	OK	A
SA3	Span 4	Middle		0.000	0.0000	OK	A
SA3	Span 4	End	UDCONU2	3.629	0.0000	OK	A
SA3	Span 5	Start	UDCONU2	3.629	0.0000	OK	A
SA3	Span 5	Middle		0.000	0.0000	OK	A
SA3	Span 5	End	UDCONU2	14.830	0.0000	OK	A
SA3	Span 6	Start	UDCONU2	14.830	0.0000	OK	A
SA3	Span 6	Middle	UDCONU2	4.882	0.0000	OK	A
SA3	Span 6	End	UDCONU2	6.323	0.0000	OK	A
SA3	Span 7	Start	UDCONU2	6.323	0.0000	OK	A
SA3	Span 7	Middle	UDCONU2	5.744	0.0000	OK	A
SA3	Span 7	End	UDCONU2	3.119	0.0000	OK	A
SA3	Span 8	Start	UDCONU2	3.119	0.0000	OK	A
SA3	Span 8	Middle	UDCONU2	3.580	0.0000	OK	A
SA3	Span 8	End	UDCONU2	6.877	0.0000	OK	A
SA3	Span 9	Start	UDCONU2	6.877	0.0000	OK	A
SA3	Span 9	Middle		0.000	0.0000	OK	A
SA3	Span 9	End	UDCONU2	0.357	0.0000	OK	A
SA3	Span 10	Start	UDCONU2	0.357	0.0000	OK	A
SA3	Span 10	Middle		0.000	0.0000	OK	A
SA3	Span 10	End	UDCONU2	4.265	0.0000	OK	A
SA3	Span 11	Start	UDCONU2	4.265	0.0000	OK	A
SA3	Span 11	Middle	UDCONU2	5.382	0.0000	OK	A
SA3	Span 11	End	UDCONU2	3.715	0.0000	OK	A
SA3	Span 12	Start	UDCONU2	3.715	0.0000	OK	A
SA3	Span 12	Middle	UDCONU2	3.159	0.0000	OK	A
SA3	Span 12	End	UDCONU2	0.239	0.0000	OK	A
SA3	Span 13	Start	UDCONU2	0.239	0.0000	OK	A
SA3	Span 13	Middle	UDCONU2	0.773	0.0000	OK	A
SA3	Span 13	End	UDCONU2	2.479	0.0000	OK	A
SA3	Span 14	Start	UDCONU2	2.479	0.0000	OK	A
SA3	Span 14	Middle		0.000	0.0000	OK	A
SA3	Span 14	End	UDCONU1	2.095	0.0000	OK	A
SA3	Span 15	Start	UDCONU1	2.095	0.0000	OK	A
SA3	Span 15	Middle	UDCONU2	6.792	0.0000	OK	A
SA3	Span 15	End	UDCONU2	5.583	0.0000	OK	A
SA3	Span 16	Start	UDCONU2	5.583	0.0000	OK	A
SA3	Span 16	Middle	UDCONU2	0.764	0.0000	OK	A
SA3	Span 16	End	UDCONU2	1.844	0.0000	OK	A
SA3	Span 17	Start	UDCONU2	3.993	0.0000	OK	A
SA3	Span 17	Middle	UDCONU2	1.307	0.0000	OK	A
SA3	Span 17	End	UDCONU2	6.732	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA3	Span 18	Start	UDCONU2	6.732	0.0000	OK	A
SA3	Span 18	Middle	UDCONU2	2.184	0.0000	OK	A
SA3	Span 18	End	UDCONU2	4.904	0.0000	OK	A
SA3	Span 19	Start	UDCONU2	12.218	0.0000	OK	A
SA3	Span 19	Middle	UDCONU2	3.769	0.0000	OK	A
SA3	Span 19	End	UDCONU2	11.319	0.0000	OK	A
SA3	Span 20	Start	UDCONU2	11.319	0.0000	OK	A
SA3	Span 20	Middle	UDCONU2	2.387	0.0000	OK	A
SA3	Span 20	End		0.000	0.0000	OK	A
SA3	Span 21	Start		0.000	0.0000	OK	A
SA3	Span 21	Middle		0.000	0.0000	OK	A
SA3	Span 21	End		0.000	0.0000	OK	A
SA3	Span 22	Start		0.000	0.0000	OK	A
SA3	Span 22	Middle		0.000	0.0000	OK	A
SA3	Span 22	End	UDCONU2	2.595	0.0000	OK	A
SA3	Span 23	Start	UDCONU2	2.595	0.0000	OK	A
SA3	Span 23	Middle	UDCONU2	1.096	0.0000	OK	A
SA3	Span 23	End	UDCONU2	0.432	0.0000	OK	A
SA3	Span 24	Start	UDCONU2	0.432	0.0000	OK	A
SA3	Span 24	Middle	UDCONU2	0.246	0.0000	OK	A
SA3	Span 24	End	UDCONU2	3.244	0.0000	OK	A
SA3	Span 25	Start	UDCONU2	3.244	0.0000	OK	A
SA3	Span 25	Middle	UDCONU1	2.435	0.0000	OK	A
SA3	Span 25	End		0.000	0.0000	OK	A
SA3	Span 26	Start		0.000	0.0000	OK	A
SA3	Span 26	Middle		0.000	0.0000	OK	A
SA3	Span 26	End		0.000	0.0000	OK	A
SA4	Span 1	Start		0.000	0.0000	OK	A
SA4	Span 1	Middle		0.000	0.0000	OK	A
SA4	Span 1	End	UDCONU2	0.821	0.0000	OK	A
SA4	Span 2	Start	UDCONU2	0.379	0.0000	OK	A
SA4	Span 2	Middle		0.000	0.0000	OK	A
SA4	Span 2	End	UDCONU2	2.148	0.0000	OK	A
SA4	Span 3	Start	UDCONU2	2.148	0.0000	OK	A
SA4	Span 3	Middle	UDCONU2	7.041	0.0000	OK	A
SA4	Span 3	End	UDCONU2	2.259	0.0000	OK	A
SA4	Span 4	Start	UDCONU2	2.259	0.0000	OK	A
SA4	Span 4	Middle		0.000	0.0000	OK	A
SA4	Span 4	End	UDCONU2	5.661	0.0000	OK	A
SA4	Span 5	Start	UDCONU2	4.500	0.0000	OK	A
SA4	Span 5	Middle	UDCONU2	8.678	0.0000	OK	A
SA4	Span 5	End	UDCONU2	8.680	0.0000	OK	A
SA4	Span 6	Start	UDCONU2	9.244	0.0000	OK	A
SA4	Span 6	Middle	UDCONU2	7.209	0.0000	OK	A
SA4	Span 6	End	UDCONU2	11.173	0.0000	OK	A
SA4	Span 7	Start	UDCONU2	7.609	0.0000	OK	A
SA4	Span 7	Middle	UDCONU2	4.338	0.0000	OK	A
SA4	Span 7	End	UDCONU2	1.049	0.0000	OK	A
SA4	Span 8	Start	UDCONU2	1.049	0.0000	OK	A
SA4	Span 8	Middle	UDCONU1	0.019	0.0000	OK	A
SA4	Span 8	End	UDCONU2	2.948	0.0000	OK	A
SA4	Span 9	Start	UDCONU2	2.948	0.0000	OK	A
SA4	Span 9	Middle	UDCONU2	4.111	0.0000	OK	A
SA4	Span 9	End	UDCONU1	2.580	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA4	Span 10	Start	UDCONU1	2.580	0.0000	OK	A
SA4	Span 10	Middle	UDCONU2	5.980	0.0000	OK	A
SA4	Span 10	End	UDCONU2	2.632	0.0000	OK	A
SA4	Span 11	Start	UDCONU2	2.632	0.0000	OK	A
SA4	Span 11	Middle	UDCONU2	2.758	0.0000	OK	A
SA4	Span 11	End	UDCONU2	1.350	0.0000	OK	A
SA4	Span 12	Start	UDCONU2	1.132	0.0000	OK	A
SA4	Span 12	Middle	UDCONU1	0.119	0.0000	OK	A
SA4	Span 12	End	UDCONU2	1.865	0.0000	OK	A
SA4	Span 13	Start	UDCONU2	1.865	0.0000	OK	A
SA4	Span 13	Middle	UDCONU2	0.612	0.0000	OK	A
SA4	Span 13	End	UDCONU2	1.837	0.0000	OK	A
SA4	Span 14	Start	UDCONU2	0.817	0.0000	OK	A
SA4	Span 14	Middle	UDCONU2	2.139	0.0000	OK	A
SA4	Span 14	End	UDCONU2	6.208	0.0000	OK	A
SA4	Span 15	Start	UDCONU2	6.208	0.0000	OK	A
SA4	Span 15	Middle	UDCONU2	0.338	0.0000	OK	A
SA4	Span 15	End		0.000	0.0000	OK	A
SA4	Span 16	Start		0.000	0.0000	OK	A
SA4	Span 16	Middle		0.000	0.0000	OK	A
SA4	Span 16	End		0.000	0.0000	OK	A
SA4	Span 17	Start		0.000	0.0000	OK	A
SA4	Span 17	Middle		0.000	0.0000	OK	A
SA4	Span 17	End	UDCONU2	2.596	0.0000	OK	A
SA4	Span 18	Start	UDCONU2	2.596	0.0000	OK	A
SA4	Span 18	Middle	UDCONU2	3.404	0.0000	OK	A
SA4	Span 18	End	UDCONU2	1.150	0.0000	OK	A
SA4	Span 19	Start	UDCONU2	1.150	0.0000	OK	A
SA4	Span 19	Middle	UDCONU2	1.057	0.0000	OK	A
SA4	Span 19	End	UDCONU2	3.894	0.0000	OK	A
SA4	Span 20	Start	UDCONU2	3.894	0.0000	OK	A
SA4	Span 20	Middle	UDCONU1	0.218	0.0000	OK	A
SA4	Span 20	End		0.000	0.0000	OK	A
SA4	Span 21	Start		0.000	0.0000	OK	A
SA4	Span 21	Middle		0.000	0.0000	OK	A
SA4	Span 21	End		0.000	0.0000	OK	A
SA5	Span 1	Start		0.000	0.0000	OK	A
SA5	Span 1	Middle		0.000	0.0000	OK	A
SA5	Span 1	End	UDCONU2	1.783	0.0000	OK	A
SA5	Span 2	Start	UDCONU2	1.783	0.0000	OK	A
SA5	Span 2	Middle	UDCONU2	3.232	0.0000	OK	A
SA5	Span 2	End	UDCONU2	3.485	0.0000	OK	A
SA5	Span 3	Start	UDCONU2	3.485	0.0000	OK	A
SA5	Span 3	Middle		0.000	0.0000	OK	A
SA5	Span 3	End	UDCONU2	0.516	0.0000	OK	A
SA5	Span 4	Start	UDCONU2	0.516	0.0000	OK	A
SA5	Span 4	Middle		0.000	0.0000	OK	A
SA5	Span 4	End	UDCONU2	12.206	0.0000	OK	A
SA5	Span 5	Start	UDCONU2	12.206	0.0000	OK	A
SA5	Span 5	Middle		0.000	0.0000	OK	A
SA5	Span 5	End	UDCONU2	4.948	0.0000	OK	A
SA5	Span 6	Start	UDCONU2	4.948	0.0000	OK	A
SA5	Span 6	Middle	UDCONU2	9.335	0.0000	OK	A
SA5	Span 6	End	UDCONU2	1.504	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA5	Span 7	Start	UDCONU2	1.504	0.0000	OK	A
SA5	Span 7	Middle	UDCONU2	4.930	0.0000	OK	A
SA5	Span 7	End	UDCONU2	14.352	0.0000	OK	A
SA5	Span 8	Start	UDCONU2	14.352	0.0000	OK	A
SA5	Span 8	Middle	UDCONU2	9.844	0.0000	OK	A
SA5	Span 8	End	UDCONU2	11.956	0.0000	OK	A
SA5	Span 9	Start	UDCONU2	11.956	0.0000	OK	A
SA5	Span 9	Middle	UDCONU2	4.145	0.0000	OK	A
SA5	Span 9	End	UDCONU2	8.905	0.0000	OK	A
SA5	Span 10	Start	UDCONU2	8.905	0.0000	OK	A
SA5	Span 10	Middle		0.000	0.0000	OK	A
SA5	Span 10	End	UDCONU2	5.927	0.0000	OK	A
SA5	Span 11	Start	UDCONU2	5.927	0.0000	OK	A
SA5	Span 11	Middle		0.000	0.0000	OK	A
SA5	Span 11	End	UDCONU2	0.452	0.0000	OK	A
SA5	Span 12	Start	UDCONU2	0.452	0.0000	OK	A
SA5	Span 12	Middle		0.000	0.0000	OK	A
SA5	Span 12	End	UDCONU2	4.718	0.0000	OK	A
SA5	Span 13	Start	UDCONU2	4.718	0.0000	OK	A
SA5	Span 13	Middle	UDCONU2	3.387	0.0000	OK	A
SA5	Span 13	End	UDCONU2	2.782	0.0000	OK	A
SA5	Span 14	Start	UDCONU2	2.782	0.0000	OK	A
SA5	Span 14	Middle		0.000	0.0000	OK	A
SA5	Span 14	End	UDCONU2	0.804	0.0000	OK	A
SA5	Span 15	Start	UDCONU2	0.804	0.0000	OK	A
SA5	Span 15	Middle		0.000	0.0000	OK	A
SA5	Span 15	End	UDCONU2	5.822	0.0000	OK	A
SA5	Span 16	Start	UDCONU2	5.822	0.0000	OK	A
SA5	Span 16	Middle	UDCONU2	14.518	0.0000	OK	A
SA5	Span 16	End	UDCONU2	9.115	0.0000	OK	A
SA5	Span 17	Start	UDCONU2	9.115	0.0000	OK	A
SA5	Span 17	Middle	UDCONU2	4.100	0.0000	OK	A
SA5	Span 17	End	UDCONU1	1.199	0.0000	OK	A
SA5	Span 18	Start	UDCONU1	1.199	0.0000	OK	A
SA5	Span 18	Middle	UDCONU2	0.143	0.0000	OK	A
SA5	Span 18	End	UDCONU2	1.614	0.0000	OK	A
SA5	Span 19	Start	UDCONU2	1.614	0.0000	OK	A
SA5	Span 19	Middle	UDCONU1	2.827	0.0000	OK	A
SA5	Span 19	End	UDCONU2	3.222	0.0000	OK	A
SA5	Span 20	Start	UDCONU2	3.222	0.0000	OK	A
SA5	Span 20	Middle	UDCONU2	5.699	0.0000	OK	A
SA5	Span 20	End	UDCONU2	5.424	0.0000	OK	A
SA5	Span 21	Start	UDCONU2	5.424	0.0000	OK	A
SA5	Span 21	Middle	UDCONU2	5.056	0.0000	OK	A
SA5	Span 21	End	UDCONU2	2.290	0.0000	OK	A
SA5	Span 22	Start	UDCONU2	1.257	0.0000	OK	A
SA5	Span 22	Middle	UDCONU2	1.138	0.0000	OK	A
SA5	Span 22	End	UDCONU2	2.198	0.0000	OK	A
SA5	Span 23	Start	UDCONU2	2.198	0.0000	OK	A
SA5	Span 23	Middle	UDCONU2	1.080	0.0000	OK	A
SA5	Span 23	End	UDCONU2	1.710	0.0000	OK	A
SA5	Span 24	Start	UDCONU2	0.106	0.0000	OK	A
SA5	Span 24	Middle	UDCONU2	1.233	0.0000	OK	A
SA5	Span 24	End	UDCONU2	5.403	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA5	Span 25	Start	UDCONU2	5.403	0.0000	OK	A
SA5	Span 25	Middle	UDCONU2	0.111	0.0000	OK	A
SA5	Span 25	End		0.000	0.0000	OK	A
SA5	Span 26	Start		0.000	0.0000	OK	A
SA5	Span 26	Middle		0.000	0.0000	OK	A
SA5	Span 26	End		0.000	0.0000	OK	A
SA5	Span 27	Start		0.000	0.0000	OK	A
SA5	Span 27	Middle		0.000	0.0000	OK	A
SA5	Span 27	End	UDCONU2	2.472	0.0000	OK	A
SA5	Span 28	Start	UDCONU2	2.472	0.0000	OK	A
SA5	Span 28	Middle	UDCONU2	4.438	0.0000	OK	A
SA5	Span 28	End	UDCONU2	1.573	0.0000	OK	A
SA5	Span 29	Start	UDCONU2	1.573	0.0000	OK	A
SA5	Span 29	Middle	UDCONU2	1.422	0.0000	OK	A
SA5	Span 29	End	UDCONU2	3.896	0.0000	OK	A
SA5	Span 30	Start	UDCONU2	3.896	0.0000	OK	A
SA5	Span 30	Middle	UDCONU2	0.139	0.0000	OK	A
SA5	Span 30	End		0.000	0.0000	OK	A
SA5	Span 31	Start		0.000	0.0000	OK	A
SA5	Span 31	Middle		0.000	0.0000	OK	A
SA5	Span 31	End		0.000	0.0000	OK	A
SA6	Span 1	Start		0.000	0.0000	OK	A
SA6	Span 1	Middle		0.000	0.0000	OK	A
SA6	Span 1	End	UDCONU2	0.042	0.0000	OK	A
SA6	Span 2	Start	UDCONU2	0.042	0.0000	OK	A
SA6	Span 2	Middle	UDCONU2	0.632	0.0000	OK	A
SA6	Span 2	End	UDCONU1	0.158	0.0000	OK	A
SA6	Span 3	Start	UDCONU1	0.158	0.0000	OK	A
SA6	Span 3	Middle	UDCONU2	3.437	0.0000	OK	A
SA6	Span 3	End	UDCONU2	0.633	0.0000	OK	A
SA6	Span 4	Start	UDCONU2	0.633	0.0000	OK	A
SA6	Span 4	Middle	UDCONU2	1.661	0.0000	OK	A
SA6	Span 4	End	UDCONU2	0.903	0.0000	OK	A
SA6	Span 5	Start	UDCONU2	0.551	0.0000	OK	A
SA6	Span 5	Middle		0.000	0.0000	OK	A
SA6	Span 5	End	UDCONU2	1.214	0.0000	OK	A
SA6	Span 6	Start	UDCONU2	1.214	0.0000	OK	A
SA6	Span 6	Middle	UDCONU2	1.512	0.0000	OK	A
SA6	Span 6	End	UDCONU2	2.953	0.0000	OK	A
SA6	Span 7	Start	UDCONU2	2.953	0.0000	OK	A
SA6	Span 7	Middle	UDCONU2	4.001	0.0000	OK	A
SA6	Span 7	End	UDCONU2	1.667	0.0000	OK	A
SA6	Span 8	Start	UDCONU2	1.667	0.0000	OK	A
SA6	Span 8	Middle	UDCONU2	0.355	0.0000	OK	A
SA6	Span 8	End	UDCONU1	1.768	0.0000	OK	A
SA6	Span 9	Start	UDCONU1	1.768	0.0000	OK	A
SA6	Span 9	Middle	UDCONU2	1.648	0.0000	OK	A
SA6	Span 9	End	UDCONU2	4.352	0.0000	OK	A
SA6	Span 10	Start	UDCONU2	4.352	0.0000	OK	A
SA6	Span 10	Middle		0.000	0.0000	OK	A
SA6	Span 10	End	UDCONU2	6.823	0.0000	OK	A
SA6	Span 11	Start	UDCONU2	6.823	0.0000	OK	A
SA6	Span 11	Middle	UDCONU1	0.138	0.0000	OK	A
SA6	Span 11	End	UDCONU2	2.160	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA6	Span 12	Start	UDCONU2	2.160	0.0000	OK	A
SA6	Span 12	Middle		0.000	0.0000	OK	A
SA6	Span 12	End	UDCONU2	7.474	0.0000	OK	A
SA6	Span 13	Start	UDCONU2	7.474	0.0000	OK	A
SA6	Span 13	Middle	UDCONU2	7.254	0.0000	OK	A
SA6	Span 13	End	UDCONU2	1.588	0.0000	OK	A
SA6	Span 14	Start	UDCONU2	1.168	0.0000	OK	A
SA6	Span 14	Middle	UDCONU2	4.763	0.0000	OK	A
SA6	Span 14	End	UDCONU2	15.306	0.0000	OK	A
SA6	Span 15	Start	UDCONU2	15.306	0.0000	OK	A
SA6	Span 15	Middle	UDCONU2	13.489	0.0000	OK	A
SA6	Span 15	End	UDCONU2	5.254	0.0000	OK	A
SA6	Span 16	Start	UDCONU2	5.254	0.0000	OK	A
SA6	Span 16	Middle	UDCONU2	2.670	0.0000	OK	A
SA6	Span 16	End	UDCONU2	8.253	0.0000	OK	A
SA6	Span 17	Start	UDCONU2	16.931	0.0000	OK	A
SA6	Span 17	Middle		0.000	0.0000	OK	A
SA6	Span 17	End	UDCONU2	16.942	0.0000	OK	A
SA6	Span 18	Start	UDCONU2	16.942	0.0000	OK	A
SA6	Span 18	Middle	UDCONU2	5.395	0.0000	OK	A
SA6	Span 18	End	UDCONU2	6.349	0.0000	OK	A
SA6	Span 19	Start	UDCONU2	6.349	0.0000	OK	A
SA6	Span 19	Middle	UDCONU2	1.864	0.0000	OK	A
SA6	Span 19	End		0.000	0.0000	OK	A
SA6	Span 20	Start		0.000	0.0000	OK	A
SA6	Span 20	Middle		0.000	0.0000	OK	A
SA6	Span 20	End		0.000	0.0000	OK	A
SA6	Span 21	Start		0.000	0.0000	OK	A
SA6	Span 21	Middle		0.000	0.0000	OK	A
SA6	Span 21	End	UDCONU1	5.044	0.0000	OK	A
SA6	Span 22	Start	UDCONU1	5.044	0.0000	OK	A
SA6	Span 22	Middle	UDCONU2	6.571	0.0000	OK	A
SA6	Span 22	End	UDCONU2	2.362	0.0000	OK	A
SA6	Span 23	Start	UDCONU2	2.362	0.0000	OK	A
SA6	Span 23	Middle	UDCONU2	1.772	0.0000	OK	A
SA6	Span 23	End	UDCONU2	13.399	0.0000	OK	A
SA6	Span 24	Start	UDCONU2	13.399	0.0000	OK	A
SA6	Span 24	Middle	UDCONU2	0.613	0.0000	OK	A
SA6	Span 24	End		0.000	0.0000	OK	A
SA6	Span 25	Start		0.000	0.0000	OK	A
SA6	Span 25	Middle		0.000	0.0000	OK	A
SA6	Span 25	End		0.000	0.0000	OK	A
SA7	Span 1	Start		0.000	0.0000	OK	A
SA7	Span 1	Middle		0.000	0.0000	OK	A
SA7	Span 1	End		0.000	0.0000	OK	A
SA7	Span 2	Start		0.000	0.0000	OK	A
SA7	Span 2	Middle		0.000	0.0000	OK	A
SA7	Span 2	End	UDCONU2	0.519	0.0000	OK	A
SA7	Span 3	Start	UDCONU2	0.519	0.0000	OK	A
SA7	Span 3	Middle	UDCONU1	0.089	0.0000	OK	A
SA7	Span 3	End	UDCONU2	0.037	0.0000	OK	A
SA7	Span 4	Start	UDCONU2	0.037	0.0000	OK	A
SA7	Span 4	Middle	UDCONU2	0.041	0.0000	OK	A
SA7	Span 4	End	UDCONU2	0.078	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA7	Span 5	Start	UDCONU2	0.078	0.0000	OK	A
SA7	Span 5	Middle		0.000	0.0000	OK	A
SA7	Span 5	End	UDCONU2	0.681	0.0000	OK	A
SA7	Span 6	Start	UDCONU2	0.681	0.0000	OK	A
SA7	Span 6	Middle		0.000	0.0000	OK	A
SA7	Span 6	End	UDCONU2	0.380	0.0000	OK	A
SA7	Span 7	Start	UDCONU2	0.380	0.0000	OK	A
SA7	Span 7	Middle		0.000	0.0000	OK	A
SA7	Span 7	End	UDCONU2	0.073	0.0000	OK	A
SA7	Span 8	Start	UDCONU2	0.073	0.0000	OK	A
SA7	Span 8	Middle	UDCONU2	0.071	0.0000	OK	A
SA7	Span 8	End	UDCONU2	0.339	0.0000	OK	A
SA7	Span 9	Start	UDCONU2	0.313	0.0000	OK	A
SA7	Span 9	Middle	UDCONU2	0.180	0.0000	OK	A
SA7	Span 9	End	UDCONU2	0.967	0.0000	OK	A
SA7	Span 10	Start	UDCONU2	0.967	0.0000	OK	A
SA7	Span 10	Middle		0.000	0.0000	OK	A
SA7	Span 10	End	UDCONU2	2.126	0.0000	OK	A
SA7	Span 11	Start	UDCONU2	2.126	0.0000	OK	A
SA7	Span 11	Middle		0.000	0.0000	OK	A
SA7	Span 11	End	UDCONU1	0.709	0.0000	OK	A
SA7	Span 12	Start	UDCONU1	0.709	0.0000	OK	A
SA7	Span 12	Middle	UDCONU2	1.048	0.0000	OK	A
SA7	Span 12	End	UDCONU2	7.764	0.0000	OK	A
SA7	Span 13	Start	UDCONU2	7.764	0.0000	OK	A
SA7	Span 13	Middle	UDCONU2	0.069	0.0000	OK	A
SA7	Span 13	End	UDCONU2	8.461	0.0000	OK	A
SA7	Span 14	Start	UDCONU2	8.461	0.0000	OK	A
SA7	Span 14	Middle		0.000	0.0000	OK	A
SA7	Span 14	End	UDCONU1	0.788	0.0000	OK	A
SA7	Span 15	Start	UDCONU1	0.788	0.0000	OK	A
SA7	Span 15	Middle		0.000	0.0000	OK	A
SA7	Span 15	End	UDCONU2	2.170	0.0000	OK	A
SA7	Span 16	Start	UDCONU2	2.170	0.0000	OK	A
SA7	Span 16	Middle	UDCONU2	11.107	0.0000	OK	A
SA7	Span 16	End	UDCONU2	2.157	0.0000	OK	A
SA7	Span 17	Start	UDCONU2	2.157	0.0000	OK	A
SA7	Span 17	Middle	UDCONU2	5.240	0.0000	OK	A
SA7	Span 17	End	UDCONU2	8.580	0.0000	OK	A
SA7	Span 18	Start	UDCONU2	8.580	0.0000	OK	A
SA7	Span 18	Middle		0.000	0.0000	OK	A
SA7	Span 18	End	UDCONU2	29.706	0.0000	OK	A
SA7	Span 19	Start	UDCONU2	29.706	0.0000	OK	A
SA7	Span 19	Middle		0.000	0.0000	OK	A
SA7	Span 19	End	UDCONU2	9.727	0.0000	OK	A
SA7	Span 20	Start	UDCONU2	9.727	0.0000	OK	A
SA7	Span 20	Middle		0.000	0.0000	OK	A
SA7	Span 20	End	UDCONU2	5.811	0.0000	OK	A
SA7	Span 21	Start	UDCONU2	5.811	0.0000	OK	A
SA7	Span 21	Middle	UDCONU2	0.188	0.0000	OK	A
SA7	Span 21	End	UDCONU2	0.826	0.0000	OK	A
SA7	Span 22	Start	UDCONU2	0.826	0.0000	OK	A
SA7	Span 22	Middle		0.000	0.0000	OK	A
SA7	Span 22	End	UDCONU2	5.556	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA7	Span 23	Start	UDCONU2	5.556	0.0000	OK	A
SA7	Span 23	Middle	UDCONU2	0.723	0.0000	OK	A
SA7	Span 23	End	UDCONU2	0.497	0.0000	OK	A
SA7	Span 24	Start	UDCONU2	0.497	0.0000	OK	A
SA7	Span 24	Middle		0.000	0.0000	OK	A
SA7	Span 24	End		0.000	0.0000	OK	A
SA7	Span 25	Start		0.000	0.0000	OK	A
SA7	Span 25	Middle		0.000	0.0000	OK	A
SA7	Span 25	End		0.000	0.0000	OK	A
SA7	Span 26	Start		0.000	0.0000	OK	A
SA7	Span 26	Middle		0.000	0.0000	OK	A
SA7	Span 26	End	UDCONU2	0.524	0.0000	OK	A
SA7	Span 27	Start	UDCONU2	0.524	0.0000	OK	A
SA7	Span 27	Middle	UDCONU2	4.782	0.0000	OK	A
SA7	Span 27	End	UDCONU2	2.421	0.0000	OK	A
SA7	Span 28	Start	UDCONU2	2.421	0.0000	OK	A
SA7	Span 28	Middle	UDCONU2	1.184	0.0000	OK	A
SA7	Span 28	End		0.000	0.0000	OK	A
SA7	Span 29	Start		0.000	0.0000	OK	A
SA7	Span 29	Middle		0.000	0.0000	OK	A
SA7	Span 29	End		0.000	0.0000	OK	A
SA8	Span 1	Start		0.000	0.0000	OK	A
SA8	Span 1	Middle		0.000	0.0000	OK	A
SA8	Span 1	End		0.000	0.0000	OK	A
SA8	Span 2	Start		0.000	0.0000	OK	A
SA8	Span 2	Middle		0.000	0.0000	OK	A
SA8	Span 2	End		0.000	0.0000	OK	A
SA8	Span 3	Start		0.000	0.0000	OK	A
SA8	Span 3	Middle		0.000	0.0000	OK	A
SA8	Span 3	End		0.000	0.0000	OK	A
SA8	Span 4	Start		0.000	0.0000	OK	A
SA8	Span 4	Middle		0.000	0.0000	OK	A
SA8	Span 4	End		0.000	0.0000	OK	A
SA8	Span 5	Start		0.000	0.0000	OK	A
SA8	Span 5	Middle		0.000	0.0000	OK	A
SA8	Span 5	End		0.000	0.0000	OK	A
SA8	Span 6	Start	UDCONU2	0.822	0.0000	OK	A
SA8	Span 6	Middle		0.000	0.0000	OK	A
SA8	Span 6	End	UDCONU2	15.354	0.0000	OK	A
SA8	Span 7	Start	UDCONU2	15.354	0.0000	OK	A
SA8	Span 7	Middle		0.000	0.0000	OK	A
SA8	Span 7	End	UDCONU2	9.151	0.0000	OK	A
SA8	Span 8	Start	UDCONU2	9.151	0.0000	OK	A
SA8	Span 8	Middle	UDCONU2	6.938	0.0000	OK	A
SA8	Span 8	End	UDCONU2	3.930	0.0000	OK	A
SA8	Span 9	Start	UDCONU2	3.930	0.0000	OK	A
SA8	Span 9	Middle	UDCONU2	5.341	0.0000	OK	A
SA8	Span 9	End	UDCONU2	16.880	0.0000	OK	A
SA8	Span 10	Start	UDCONU2	16.880	0.0000	OK	A
SA8	Span 10	Middle		0.000	0.0000	OK	A
SA8	Span 10	End	UDCONU2	13.527	0.0000	OK	A
SA8	Span 11	Start	UDCONU2	13.527	0.0000	OK	A
SA8	Span 11	Middle	UDCONU2	2.365	0.0000	OK	A
SA8	Span 11	End	UDCONU2	1.963	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA8	Span 12	Start	UDCONU2	1.963	0.0000	OK	A
SA8	Span 12	Middle	UDCONU2	4.919	0.0000	OK	A
SA8	Span 12	End	UDCONU2	7.545	0.0000	OK	A
SA8	Span 13	Start	UDCONU2	7.545	0.0000	OK	A
SA8	Span 13	Middle		0.000	0.0000	OK	A
SA8	Span 13	End	UDCONU2	7.325	0.0000	OK	A
SA8	Span 14	Start	UDCONU2	7.325	0.0000	OK	A
SA8	Span 14	Middle	UDCONU2	2.281	0.0000	OK	A
SA8	Span 14	End		0.000	0.0000	OK	A
SA8	Span 15	Start		0.000	0.0000	OK	A
SA8	Span 15	Middle		0.000	0.0000	OK	A
SA8	Span 15	End		0.000	0.0000	OK	A
SA8	Span 16	Start		0.000	0.0000	OK	A
SA8	Span 16	Middle		0.000	0.0000	OK	A
SA8	Span 16	End		0.000	0.0000	OK	A
SA8	Span 17	Start		0.000	0.0000	OK	A
SA8	Span 17	Middle	UDCONU2	0.058	0.0000	OK	A
SA8	Span 17	End	UDCONU2	0.333	0.0000	OK	A
SA8	Span 18	Start	UDCONU2	0.333	0.0000	OK	A
SA8	Span 18	Middle	UDCONU2	0.102	0.0000	OK	A
SA8	Span 18	End		0.000	0.0000	OK	A
SA8	Span 19	Start		0.000	0.0000	OK	A
SA8	Span 19	Middle		0.000	0.0000	OK	A
SA8	Span 19	End		0.000	0.0000	OK	A
SA9	Span 1	Start		0.000	0.0000	OK	A
SA9	Span 1	Middle		0.000	0.0000	OK	A
SA9	Span 1	End		0.000	0.0000	OK	A
SA9	Span 2	Start		0.000	0.0000	OK	A
SA9	Span 2	Middle		0.000	0.0000	OK	A
SA9	Span 2	End		0.000	0.0000	OK	A
SA9	Span 3	Start		0.000	0.0000	OK	A
SA9	Span 3	Middle		0.000	0.0000	OK	A
SA9	Span 3	End		0.000	0.0000	OK	A
SA9	Span 4	Start		0.000	0.0000	OK	A
SA9	Span 4	Middle		0.000	0.0000	OK	A
SA9	Span 4	End		0.000	0.0000	OK	A
SA9	Span 5	Start		0.000	0.0000	OK	A
SA9	Span 5	Middle		0.000	0.0000	OK	A
SA9	Span 5	End		0.000	0.0000	OK	A
SA9	Span 6	Start	UDCONU2	5.262	0.0000	OK	A
SA9	Span 6	Middle		0.000	0.0000	OK	A
SA9	Span 6	End	UDCONU2	3.923	0.0000	OK	A
SA9	Span 7	Start	UDCONU2	3.923	0.0000	OK	A
SA9	Span 7	Middle	UDCONU2	2.469	0.0000	OK	A
SA9	Span 7	End	UDCONU2	1.270	0.0000	OK	A
SA9	Span 8	Start	UDCONU2	1.270	0.0000	OK	A
SA9	Span 8	Middle	UDCONU2	2.214	0.0000	OK	A
SA9	Span 8	End	UDCONU2	2.857	0.0000	OK	A
SA9	Span 9	Start	UDCONU2	4.395	0.0000	OK	A
SA9	Span 9	Middle	UDCONU2	3.852	0.0000	OK	A
SA9	Span 9	End	UDCONU2	2.395	0.0000	OK	A
SA9	Span 10	Start	UDCONU2	2.480	0.0000	OK	A
SA9	Span 10	Middle	UDCONU2	1.359	0.0000	OK	A
SA9	Span 10	End	UDCONU2	2.928	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA9	Span 11	Start	UDCONU2	2.928	0.0000	OK	A
SA9	Span 11	Middle	UDCONU2	7.340	0.0000	OK	A
SA9	Span 11	End	UDCONU2	16.924	0.0000	OK	A
SA9	Span 12	Start	UDCONU2	16.924	0.0000	OK	A
SA9	Span 12	Middle		0.000	0.0000	OK	A
SA9	Span 12	End	UDCONU2	12.595	0.0000	OK	A
SA9	Span 13	Start	UDCONU2	12.595	0.0000	OK	A
SA9	Span 13	Middle	UDCONU2	1.902	0.0000	OK	A
SA9	Span 13	End		0.000	0.0000	OK	A
SA9	Span 14	Start		0.000	0.0000	OK	A
SA9	Span 14	Middle		0.000	0.0000	OK	A
SA9	Span 14	End		0.000	0.0000	OK	A
SA9	Span 15	Start		0.000	0.0000	OK	A
SA9	Span 15	Middle		0.000	0.0000	OK	A
SA9	Span 15	End		0.000	0.0000	OK	A
SA9	Span 16	Start		0.000	0.0000	OK	A
SA9	Span 16	Middle		0.000	0.0000	OK	A
SA9	Span 16	End		0.000	0.0000	OK	A
SA9	Span 17	Start		0.000	0.0000	OK	A
SA9	Span 17	Middle		0.000	0.0000	OK	A
SA9	Span 17	End		0.000	0.0000	OK	A
SA9	Span 18	Start		0.000	0.0000	OK	A
SA9	Span 18	Middle		0.000	0.0000	OK	A
SA9	Span 18	End		0.000	0.0000	OK	A
SB1	Span 1	Start		0.000	0.0000	OK	B
SB1	Span 1	Middle	UDCONU2	0.473	0.0000	OK	B
SB1	Span 1	End		0.000	0.0000	OK	B
SB1	Span 2	Start		0.000	0.0000	OK	B
SB1	Span 2	Middle		0.000	0.0000	OK	B
SB1	Span 2	End		0.000	0.0000	OK	B
SB1	Span 3	Start		0.000	0.0000	OK	B
SB1	Span 3	Middle		0.000	0.0000	OK	B
SB1	Span 3	End		0.000	0.0000	OK	B
SB1	Span 4	Start		0.000	0.0000	OK	B
SB1	Span 4	Middle		0.000	0.0000	OK	B
SB1	Span 4	End		0.000	0.0000	OK	B
SB1	Span 5	Start		0.000	0.0000	OK	B
SB1	Span 5	Middle		0.000	0.0000	OK	B
SB1	Span 5	End		0.000	0.0000	OK	B
SB1	Span 6	Start		0.000	0.0000	OK	B
SB1	Span 6	Middle		0.000	0.0000	OK	B
SB1	Span 6	End		0.000	0.0000	OK	B
SB1	Span 7	Start		0.000	0.0000	OK	B
SB1	Span 7	Middle		0.000	0.0000	OK	B
SB1	Span 7	End		0.000	0.0000	OK	B
SB1	Span 8	Start		0.000	0.0000	OK	B
SB1	Span 8	Middle		0.000	0.0000	OK	B
SB1	Span 8	End		0.000	0.0000	OK	B
SB1	Span 9	Start		0.000	0.0000	OK	B
SB1	Span 9	Middle		0.000	0.0000	OK	B
SB1	Span 9	End		0.000	0.0000	OK	B
SB1	Span 10	Start		0.000	0.0000	OK	B
SB1	Span 10	Middle		0.000	0.0000	OK	B
SB1	Span 10	End		0.000	0.0000	OK	B

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB2	Span 1	Start		0.000	0.0000	OK	B
SB2	Span 1	Middle		0.000	0.0000	OK	B
SB2	Span 1	End	UDCONU2	0.508	0.0000	OK	B
SB2	Span 2	Start	UDCONU2	0.508	0.0000	OK	B
SB2	Span 2	Middle		0.000	0.0000	OK	B
SB2	Span 2	End	UDCONU2	1.195	0.0000	OK	B
SB2	Span 3	Start	UDCONU2	1.195	0.0000	OK	B
SB2	Span 3	Middle	UDCONU2	1.177	0.0000	OK	B
SB2	Span 3	End	UDCONU2	5.214	0.0000	OK	B
SB2	Span 4	Start	UDCONU2	5.214	0.0000	OK	B
SB2	Span 4	Middle		0.000	0.0000	OK	B
SB2	Span 4	End	UDCONU2	6.814	0.0000	OK	B
SB2	Span 5	Start	UDCONU2	6.814	0.0000	OK	B
SB2	Span 5	Middle		0.000	0.0000	OK	B
SB2	Span 5	End	UDCONU2	2.356	0.0000	OK	B
SB2	Span 6	Start	UDCONU2	2.356	0.0000	OK	B
SB2	Span 6	Middle	UDCONU2	0.318	0.0000	OK	B
SB2	Span 6	End		0.000	0.0000	OK	B
SB2	Span 7	Start		0.000	0.0000	OK	B
SB2	Span 7	Middle		0.000	0.0000	OK	B
SB2	Span 7	End		0.000	0.0000	OK	B
SB2	Span 8	Start		0.000	0.0000	OK	B
SB2	Span 8	Middle		0.000	0.0000	OK	B
SB2	Span 8	End		0.000	0.0000	OK	B
SB2	Span 9	Start		0.000	0.0000	OK	B
SB2	Span 9	Middle		0.000	0.0000	OK	B
SB2	Span 9	End		0.000	0.0000	OK	B
SB2	Span 10	Start		0.000	0.0000	OK	B
SB2	Span 10	Middle		0.000	0.0000	OK	B
SB2	Span 10	End		0.000	0.0000	OK	B
SB2	Span 11	Start		0.000	0.0000	OK	B
SB2	Span 11	Middle		0.000	0.0000	OK	B
SB2	Span 11	End		0.000	0.0000	OK	B
SB2	Span 12	Start		0.000	0.0000	OK	B
SB2	Span 12	Middle		0.000	0.0000	OK	B
SB2	Span 12	End		0.000	0.0000	OK	B
SB2	Span 13	Start		0.000	0.0000	OK	B
SB2	Span 13	Middle		0.000	0.0000	OK	B
SB2	Span 13	End		0.000	0.0000	OK	B
SB2	Span 14	Start		0.000	0.0000	OK	B
SB2	Span 14	Middle		0.000	0.0000	OK	B
SB2	Span 14	End		0.000	0.0000	OK	B
SB3	Span 1	Start		0.000	0.0000	OK	B
SB3	Span 1	Middle	UDCONU2	0.662	0.0000	OK	B
SB3	Span 1	End	UDCONU2	17.716	0.0000	OK	B
SB3	Span 2	Start	UDCONU2	17.716	0.0000	OK	B
SB3	Span 2	Middle		0.000	0.0000	OK	B
SB3	Span 2	End	UDCONU2	7.796	0.0000	OK	B
SB3	Span 3	Start	UDCONU2	7.796	0.0000	OK	B
SB3	Span 3	Middle		0.000	0.0000	OK	B
SB3	Span 3	End	UDCONU2	3.896	0.0000	OK	B
SB3	Span 4	Start	UDCONU2	3.896	0.0000	OK	B
SB3	Span 4	Middle	UDCONU2	1.330	0.0000	OK	B
SB3	Span 4	End	UDCONU2	6.950	0.0000	OK	B

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB3	Span 5	Start	UDCONU2	6.950	0.0000	OK	B
SB3	Span 5	Middle		0.000	0.0000	OK	B
SB3	Span 5	End	UDCONU2	15.108	0.0000	OK	B
SB3	Span 6	Start	UDCONU2	15.108	0.0000	OK	B
SB3	Span 6	Middle		0.000	0.0000	OK	B
SB3	Span 6	End	UDCONU2	0.589	0.0000	OK	B
SB3	Span 7	Start	UDCONU2	0.589	0.0000	OK	B
SB3	Span 7	Middle		0.000	0.0000	OK	B
SB3	Span 7	End	UDCONU2	5.071	0.0000	OK	B
SB3	Span 8	Start	UDCONU2	5.071	0.0000	OK	B
SB3	Span 8	Middle	UDCONU2	1.377	0.0000	OK	B
SB3	Span 8	End	UDCONU2	0.218	0.0000	OK	B
SB3	Span 9	Start	UDCONU2	0.218	0.0000	OK	B
SB3	Span 9	Middle		0.000	0.0000	OK	B
SB3	Span 9	End	UDCONU2	0.829	0.0000	OK	B
SB3	Span 10	Start	UDCONU2	0.829	0.0000	OK	B
SB3	Span 10	Middle		0.000	0.0000	OK	B
SB3	Span 10	End		0.000	0.0000	OK	B
SB3	Span 11	Start		0.000	0.0000	OK	B
SB3	Span 11	Middle		0.000	0.0000	OK	B
SB3	Span 11	End		0.000	0.0000	OK	B
SB3	Span 12	Start		0.000	0.0000	OK	B
SB3	Span 12	Middle		0.000	0.0000	OK	B
SB3	Span 12	End		0.000	0.0000	OK	B
SB3	Span 13	Start		0.000	0.0000	OK	B
SB3	Span 13	Middle		0.000	0.0000	OK	B
SB3	Span 13	End		0.000	0.0000	OK	B
SB3	Span 14	Start		0.000	0.0000	OK	B
SB3	Span 14	Middle		0.000	0.0000	OK	B
SB3	Span 14	End		0.000	0.0000	OK	B
SB3	Span 15	Start		0.000	0.0000	OK	B
SB3	Span 15	Middle		0.000	0.0000	OK	B
SB3	Span 15	End		0.000	0.0000	OK	B
SB3	Span 16	Start		0.000	0.0000	OK	B
SB3	Span 16	Middle		0.000	0.0000	OK	B
SB3	Span 16	End		0.000	0.0000	OK	B
SB3	Span 17	Start		0.000	0.0000	OK	B
SB3	Span 17	Middle		0.000	0.0000	OK	B
SB3	Span 17	End		0.000	0.0000	OK	B
SB4	Span 1	Start		0.000	0.0000	OK	B
SB4	Span 1	Middle		0.000	0.0000	OK	B
SB4	Span 1	End	UDCONU2	0.330	0.0000	OK	B
SB4	Span 2	Start	UDCONU2	0.330	0.0000	OK	B
SB4	Span 2	Middle	UDCONU2	2.404	0.0000	OK	B
SB4	Span 2	End	UDCONU2	0.954	0.0000	OK	B
SB4	Span 3	Start	UDCONU2	0.954	0.0000	OK	B
SB4	Span 3	Middle	UDCONU2	1.803	0.0000	OK	B
SB4	Span 3	End	UDCONU2	4.732	0.0000	OK	B
SB4	Span 4	Start	UDCONU2	4.732	0.0000	OK	B
SB4	Span 4	Middle	UDCONU2	0.501	0.0000	OK	B
SB4	Span 4	End	UDCONU2	5.594	0.0000	OK	B
SB4	Span 5	Start		0.000	0.0000	OK	B
SB4	Span 5	Middle		0.000	0.0000	OK	B
SB4	Span 5	End		0.000	0.0000	OK	B

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB4	Span 6	Start		0.000	0.0000	OK	B
SB4	Span 6	Middle		0.000	0.0000	OK	B
SB4	Span 6	End		0.000	0.0000	OK	B
SB4	Span 7	Start		0.000	0.0000	OK	B
SB4	Span 7	Middle		0.000	0.0000	OK	B
SB4	Span 7	End		0.000	0.0000	OK	B
SB4	Span 8	Start		0.000	0.0000	OK	B
SB4	Span 8	Middle		0.000	0.0000	OK	B
SB4	Span 8	End		0.000	0.0000	OK	B
SB4	Span 9	Start		0.000	0.0000	OK	B
SB4	Span 9	Middle		0.000	0.0000	OK	B
SB4	Span 9	End		0.000	0.0000	OK	B
SB4	Span 10	Start		0.000	0.0000	OK	B
SB4	Span 10	Middle		0.000	0.0000	OK	B
SB4	Span 10	End		0.000	0.0000	OK	B
SB4	Span 11	Start		0.000	0.0000	OK	B
SB4	Span 11	Middle		0.000	0.0000	OK	B
SB4	Span 11	End		0.000	0.0000	OK	B
SB5	Span 1	Start		0.000	0.0000	OK	B
SB5	Span 1	Middle	UDCONU2	4.369	0.0000	OK	B
SB5	Span 1	End	UDCONU2	2.349	0.0000	OK	B
SB5	Span 2	Start	UDCONU2	3.000	0.0000	OK	B
SB5	Span 2	Middle		0.000	0.0000	OK	B
SB5	Span 2	End	UDCONU2	0.644	0.0000	OK	B
SB5	Span 3	Start	UDCONU2	0.644	0.0000	OK	B
SB5	Span 3	Middle	UDCONU2	0.702	0.0000	OK	B
SB5	Span 3	End	UDCONU2	1.165	0.0000	OK	B
SB5	Span 4	Start	UDCONU2	1.165	0.0000	OK	B
SB5	Span 4	Middle	UDCONU1	0.823	0.0000	OK	B
SB5	Span 4	End	UDCONU2	1.794	0.0000	OK	B
SB5	Span 5	Start	UDCONU2	1.794	0.0000	OK	B
SB5	Span 5	Middle	UDCONU2	1.160	0.0000	OK	B
SB5	Span 5	End	UDCONU2	6.599	0.0000	OK	B
SB5	Span 6	Start		0.000	0.0000	OK	B
SB5	Span 6	Middle		0.000	0.0000	OK	B
SB5	Span 6	End		0.000	0.0000	OK	B
SB5	Span 7	Start		0.000	0.0000	OK	B
SB5	Span 7	Middle		0.000	0.0000	OK	B
SB5	Span 7	End		0.000	0.0000	OK	B
SB5	Span 8	Start		0.000	0.0000	OK	B
SB5	Span 8	Middle		0.000	0.0000	OK	B
SB5	Span 8	End		0.000	0.0000	OK	B
SB5	Span 9	Start		0.000	0.0000	OK	B
SB5	Span 9	Middle		0.000	0.0000	OK	B
SB5	Span 9	End		0.000	0.0000	OK	B
SB5	Span 10	Start		0.000	0.0000	OK	B
SB5	Span 10	Middle		0.000	0.0000	OK	B
SB5	Span 10	End		0.000	0.0000	OK	B
SB5	Span 11	Start		0.000	0.0000	OK	B
SB5	Span 11	Middle		0.000	0.0000	OK	B
SB5	Span 11	End		0.000	0.0000	OK	B
SB5	Span 12	Start		0.000	0.0000	OK	B
SB5	Span 12	Middle		0.000	0.0000	OK	B
SB5	Span 12	End		0.000	0.0000	OK	B

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB6	Span 1	Start		0.000	0.0000	OK	B
SB6	Span 1	Middle	UDCONU2	2.151	0.0000	OK	B
SB6	Span 1	End	UDCONU2	2.666	0.0000	OK	B
SB6	Span 2	Start	UDCONU2	3.762	0.0000	OK	B
SB6	Span 2	Middle	UDCONU2	1.488	0.0000	OK	B
SB6	Span 2	End	UDCONU2	0.411	0.0000	OK	B
SB6	Span 3	Start	UDCONU2	0.411	0.0000	OK	B
SB6	Span 3	Middle	UDCONU2	2.317	0.0000	OK	B
SB6	Span 3	End	UDCONU2	2.705	0.0000	OK	B
SB6	Span 4	Start	UDCONU2	2.705	0.0000	OK	B
SB6	Span 4	Middle		0.000	0.0000	OK	B
SB6	Span 4	End	UDCONU2	3.927	0.0000	OK	B
SB6	Span 5	Start	UDCONU2	3.927	0.0000	OK	B
SB6	Span 5	Middle		0.000	0.0000	OK	B
SB6	Span 5	End	UDCONU2	3.997	0.0000	OK	B
SB6	Span 6	Start	UDCONU2	3.997	0.0000	OK	B
SB6	Span 6	Middle	UDCONU2	0.855	0.0000	OK	B
SB6	Span 6	End	UDCONU2	2.174	0.0000	OK	B
SB6	Span 7	Start		0.000	0.0000	OK	B
SB6	Span 7	Middle		0.000	0.0000	OK	B
SB6	Span 7	End		0.000	0.0000	OK	B
SB6	Span 8	Start		0.000	0.0000	OK	B
SB6	Span 8	Middle		0.000	0.0000	OK	B
SB6	Span 8	End		0.000	0.0000	OK	B
SB6	Span 9	Start		0.000	0.0000	OK	B
SB6	Span 9	Middle		0.000	0.0000	OK	B
SB6	Span 9	End		0.000	0.0000	OK	B
SB6	Span 10	Start		0.000	0.0000	OK	B
SB6	Span 10	Middle		0.000	0.0000	OK	B
SB6	Span 10	End		0.000	0.0000	OK	B
SB6	Span 11	Start		0.000	0.0000	OK	B
SB6	Span 11	Middle		0.000	0.0000	OK	B
SB6	Span 11	End		0.000	0.0000	OK	B
SB6	Span 12	Start		0.000	0.0000	OK	B
SB6	Span 12	Middle		0.000	0.0000	OK	B
SB6	Span 12	End		0.000	0.0000	OK	B
SB6	Span 13	Start		0.000	0.0000	OK	B
SB6	Span 13	Middle		0.000	0.0000	OK	B
SB6	Span 13	End		0.000	0.0000	OK	B
SB7	Span 1	Start		0.000	0.0000	OK	B
SB7	Span 1	Middle		0.000	0.0000	OK	B
SB7	Span 1	End	UDCONU2	0.716	0.0000	OK	B
SB7	Span 2	Start	UDCONU2	0.716	0.0000	OK	B
SB7	Span 2	Middle	UDCONU2	1.189	0.0000	OK	B
SB7	Span 2	End	UDCONU2	0.154	0.0000	OK	B
SB7	Span 3	Start	UDCONU2	0.154	0.0000	OK	B
SB7	Span 3	Middle	UDCONU2	2.974	0.0000	OK	B
SB7	Span 3	End	UDCONU2	4.406	0.0000	OK	B
SB7	Span 4	Start	UDCONU2	4.406	0.0000	OK	B
SB7	Span 4	Middle		0.000	0.0000	OK	B
SB7	Span 4	End	UDCONU2	5.963	0.0000	OK	B
SB7	Span 5	Start	UDCONU2	5.963	0.0000	OK	B
SB7	Span 5	Middle		0.000	0.0000	OK	B
SB7	Span 5	End	UDCONU2	5.886	0.0000	OK	B

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB7	Span 6	Start	UDCONU2	5.886	0.0000	OK	B
SB7	Span 6	Middle		0.000	0.0000	OK	B
SB7	Span 6	End	UDCONU2	4.797	0.0000	OK	B
SB7	Span 7	Start	UDCONU2	4.797	0.0000	OK	B
SB7	Span 7	Middle	UDCONU2	3.884	0.0000	OK	B
SB7	Span 7	End	UDCONU2	3.341	0.0000	OK	B
SB7	Span 8	Start	UDCONU2	3.341	0.0000	OK	B
SB7	Span 8	Middle		0.000	0.0000	OK	B
SB7	Span 8	End	UDCONU2	0.101	0.0000	OK	B
SB7	Span 9	Start	UDCONU2	0.101	0.0000	OK	B
SB7	Span 9	Middle	UDCONU2	0.098	0.0000	OK	B
SB7	Span 9	End	UDCONU2	3.529	0.0000	OK	B
SB7	Span 10	Start	UDCONU2	3.529	0.0000	OK	B
SB7	Span 10	Middle		0.000	0.0000	OK	B
SB7	Span 10	End	UDCONU1	0.333	0.0000	OK	B
SB7	Span 11	Start	UDCONU1	0.333	0.0000	OK	B
SB7	Span 11	Middle		0.000	0.0000	OK	B
SB7	Span 11	End	UDCONU2	6.268	0.0000	OK	B
SB7	Span 12	Start	UDCONU2	6.268	0.0000	OK	B
SB7	Span 12	Middle	UDCONU2	2.408	0.0000	OK	B
SB7	Span 12	End	UDCONU2	0.491	0.0000	OK	B
SB7	Span 13	Start	UDCONU2	0.491	0.0000	OK	B
SB7	Span 13	Middle		0.000	0.0000	OK	B
SB7	Span 13	End		0.000	0.0000	OK	B
SB7	Span 14	Start		0.000	0.0000	OK	B
SB7	Span 14	Middle		0.000	0.0000	OK	B
SB7	Span 14	End		0.000	0.0000	OK	B
SB7	Span 15	Start		0.000	0.0000	OK	B
SB7	Span 15	Middle		0.000	0.0000	OK	B
SB7	Span 15	End		0.000	0.0000	OK	B
SB7	Span 16	Start		0.000	0.0000	OK	B
SB7	Span 16	Middle		0.000	0.0000	OK	B
SB7	Span 16	End		0.000	0.0000	OK	B
SB7	Span 17	Start		0.000	0.0000	OK	B
SB7	Span 17	Middle		0.000	0.0000	OK	B
SB7	Span 17	End		0.000	0.0000	OK	B
SB7	Span 18	Start		0.000	0.0000	OK	B
SB7	Span 18	Middle		0.000	0.0000	OK	B
SB7	Span 18	End		0.000	0.0000	OK	B
SB7	Span 19	Start		0.000	0.0000	OK	B
SB7	Span 19	Middle		0.000	0.0000	OK	B
SB7	Span 19	End		0.000	0.0000	OK	B
SB7	Span 20	Start		0.000	0.0000	OK	B
SB7	Span 20	Middle		0.000	0.0000	OK	B
SB7	Span 20	End		0.000	0.0000	OK	B
SB8	Span 1	Start		0.000	0.0000	OK	B
SB8	Span 1	Middle	UDCONU2	0.492	0.0000	OK	B
SB8	Span 1	End	UDCONU2	5.183	0.0000	OK	B
SB8	Span 2	Start	UDCONU2	3.781	0.0000	OK	B
SB8	Span 2	Middle	UDCONU2	3.905	0.0000	OK	B
SB8	Span 2	End	UDCONU2	3.489	0.0000	OK	B
SB8	Span 3	Start	UDCONU2	3.489	0.0000	OK	B
SB8	Span 3	Middle		0.000	0.0000	OK	B
SB8	Span 3	End	UDCONU2	3.644	0.0000	OK	B

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB8	Span 4	Start	UDCONU2	3.644	0.0000	OK	B
SB8	Span 4	Middle	UDCONU2	1.773	0.0000	OK	B
SB8	Span 4	End	UDCONU2	8.256	0.0000	OK	B
SB8	Span 5	Start	UDCONU2	8.256	0.0000	OK	B
SB8	Span 5	Middle		0.000	0.0000	OK	B
SB8	Span 5	End	UDCONU2	3.436	0.0000	OK	B
SB8	Span 6	Start	UDCONU2	3.436	0.0000	OK	B
SB8	Span 6	Middle		0.000	0.0000	OK	B
SB8	Span 6	End	UDCONU2	6.460	0.0000	OK	B
SB8	Span 7	Start	UDCONU2	6.460	0.0000	OK	B
SB8	Span 7	Middle	UDCONU2	2.040	0.0000	OK	B
SB8	Span 7	End	UDCONU1	0.488	0.0000	OK	B
SB8	Span 8	Start	UDCONU1	0.488	0.0000	OK	B
SB8	Span 8	Middle		0.000	0.0000	OK	B
SB8	Span 8	End		0.000	0.0000	OK	B
SB8	Span 9	Start		0.000	0.0000	OK	B
SB8	Span 9	Middle		0.000	0.0000	OK	B
SB8	Span 9	End		0.000	0.0000	OK	B
SB8	Span 10	Start		0.000	0.0000	OK	B
SB8	Span 10	Middle		0.000	0.0000	OK	B
SB8	Span 10	End		0.000	0.0000	OK	B
SB8	Span 11	Start		0.000	0.0000	OK	B
SB8	Span 11	Middle		0.000	0.0000	OK	B
SB8	Span 11	End		0.000	0.0000	OK	B
SB8	Span 12	Start		0.000	0.0000	OK	B
SB8	Span 12	Middle		0.000	0.0000	OK	B
SB8	Span 12	End		0.000	0.0000	OK	B
SB8	Span 13	Start		0.000	0.0000	OK	B
SB8	Span 13	Middle	UDCONU2	1.191	0.0000	OK	B
SB8	Span 13	End	UDCONU2	0.143	0.0000	OK	B
SB8	Span 14	Start	UDCONU2	0.143	0.0000	OK	B
SB8	Span 14	Middle	UDCONU1	7.297E-003	0.0000	OK	B
SB8	Span 14	End	UDCONU2	0.376	0.0000	OK	B
SB8	Span 15	Start	UDCONU2	0.376	0.0000	OK	B
SB8	Span 15	Middle	UDCONU2	0.895	0.0000	OK	B
SB8	Span 15	End	UDCONU2	6.706E-003	0.0000	OK	B
SB9	Span 1	Start		0.000	0.0000	OK	B
SB9	Span 1	Middle		0.000	0.0000	OK	B
SB9	Span 1	End	UDCONU2	0.065	0.0000	OK	B
SB9	Span 2	Start	UDCONU2	0.065	0.0000	OK	B
SB9	Span 2	Middle	UDCONU2	0.650	0.0000	OK	B
SB9	Span 2	End	UDCONU1	0.163	0.0000	OK	B
SB9	Span 3	Start	UDCONU1	0.163	0.0000	OK	B
SB9	Span 3	Middle	UDCONU2	1.237	0.0000	OK	B
SB9	Span 3	End	UDCONU2	2.141	0.0000	OK	B
SB9	Span 4	Start	UDCONU2	2.141	0.0000	OK	B
SB9	Span 4	Middle	UDCONU2	4.365	0.0000	OK	B
SB9	Span 4	End	UDCONU2	3.013	0.0000	OK	B
SB9	Span 5	Start	UDCONU2	3.013	0.0000	OK	B
SB9	Span 5	Middle	UDCONU2	0.989	0.0000	OK	B
SB9	Span 5	End	UDCONU1	0.373	0.0000	OK	B
SB9	Span 6	Start	UDCONU1	0.373	0.0000	OK	B
SB9	Span 6	Middle	UDCONU2	1.578	0.0000	OK	B
SB9	Span 6	End	UDCONU2	3.509	0.0000	OK	B

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB9	Span 7	Start		0.000	0.0000	OK	B
SB9	Span 7	Middle		0.000	0.0000	OK	B
SB9	Span 7	End		0.000	0.0000	OK	B
SB9	Span 8	Start		0.000	0.0000	OK	B
SB9	Span 8	Middle		0.000	0.0000	OK	B
SB9	Span 8	End		0.000	0.0000	OK	B
SB9	Span 9	Start		0.000	0.0000	OK	B
SB9	Span 9	Middle		0.000	0.0000	OK	B
SB9	Span 9	End		0.000	0.0000	OK	B
SB9	Span 10	Start		0.000	0.0000	OK	B
SB9	Span 10	Middle		0.000	0.0000	OK	B
SB9	Span 10	End	UDCONU2	0.787	0.0000	OK	B
SB9	Span 11	Start	UDCONU2	0.922	0.0000	OK	B
SB9	Span 11	Middle	UDCONU2	8.926	0.0000	OK	B
SB9	Span 11	End	UDCONU2	2.563	0.0000	OK	B
SB9	Span 12	Start	UDCONU2	2.563	0.0000	OK	B
SB9	Span 12	Middle	UDCONU2	2.832	0.0000	OK	B
SB9	Span 12	End	UDCONU2	0.801	0.0000	OK	B
SB9	Span 13	Start	UDCONU2	0.801	0.0000	OK	B
SB9	Span 13	Middle	UDCONU2	0.038	0.0000	OK	B
SB9	Span 13	End	UDCONU2	2.994	0.0000	OK	B
SB9	Span 14	Start	UDCONU2	2.994	0.0000	OK	B
SB9	Span 14	Middle		0.000	0.0000	OK	B
SB9	Span 14	End	UDCONU2	2.994	0.0000	OK	B
SB9	Span 15	Start	UDCONU2	2.994	0.0000	OK	B
SB9	Span 15	Middle	UDCONU2	9.613	0.0000	OK	B
SB9	Span 15	End	UDCONU2	0.359	0.0000	OK	B
SA10	Span 1	Start		0.000	0.0000	OK	A
SA10	Span 1	Middle		0.000	0.0000	OK	A
SA10	Span 1	End		0.000	0.0000	OK	A
SA10	Span 2	Start		0.000	0.0000	OK	A
SA10	Span 2	Middle		0.000	0.0000	OK	A
SA10	Span 2	End		0.000	0.0000	OK	A
SA10	Span 3	Start		0.000	0.0000	OK	A
SA10	Span 3	Middle		0.000	0.0000	OK	A
SA10	Span 3	End		0.000	0.0000	OK	A
SA10	Span 4	Start		0.000	0.0000	OK	A
SA10	Span 4	Middle		0.000	0.0000	OK	A
SA10	Span 4	End		0.000	0.0000	OK	A
SA10	Span 5	Start		0.000	0.0000	OK	A
SA10	Span 5	Middle		0.000	0.0000	OK	A
SA10	Span 5	End		0.000	0.0000	OK	A
SA10	Span 6	Start	UDCONU2	0.251	0.0000	OK	A
SA10	Span 6	Middle		0.000	0.0000	OK	A
SA10	Span 6	End		0.000	0.0000	OK	A
SA10	Span 7	Start		0.000	0.0000	OK	A
SA10	Span 7	Middle		0.000	0.0000	OK	A
SA10	Span 7	End		0.000	0.0000	OK	A
SA10	Span 8	Start		0.000	0.0000	OK	A
SA10	Span 8	Middle		0.000	0.0000	OK	A
SA10	Span 8	End		0.000	0.0000	OK	A
SA10	Span 9	Start		0.000	0.0000	OK	A
SA10	Span 9	Middle		0.000	0.0000	OK	A
SA10	Span 9	End	UDCONU2	0.521	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA10	Span 10	Start	UDCONU2	1.958	0.0000	OK	A
SA10	Span 10	Middle		0.000	0.0000	OK	A
SA10	Span 10	End	UDCONU2	5.322	0.0000	OK	A
SA10	Span 11	Start	UDCONU2	5.322	0.0000	OK	A
SA10	Span 11	Middle	UDCONU2	8.424	0.0000	OK	A
SA10	Span 11	End	UDCONU2	17.239	0.0000	OK	A
SA10	Span 12	Start	UDCONU2	17.239	0.0000	OK	A
SA10	Span 12	Middle		0.000	0.0000	OK	A
SA10	Span 12	End	UDCONU2	15.960	0.0000	OK	A
SA10	Span 13	Start	UDCONU2	15.960	0.0000	OK	A
SA10	Span 13	Middle	UDCONU2	10.191	0.0000	OK	A
SA10	Span 13	End		0.000	0.0000	OK	A
SA10	Span 14	Start		0.000	0.0000	OK	A
SA10	Span 14	Middle		0.000	0.0000	OK	A
SA10	Span 14	End		0.000	0.0000	OK	A
SA10	Span 15	Start		0.000	0.0000	OK	A
SA10	Span 15	Middle		0.000	0.0000	OK	A
SA10	Span 15	End		0.000	0.0000	OK	A
SA10	Span 16	Start		0.000	0.0000	OK	A
SA10	Span 16	Middle		0.000	0.0000	OK	A
SA10	Span 16	End		0.000	0.0000	OK	A
SA10	Span 17	Start		0.000	0.0000	OK	A
SA10	Span 17	Middle		0.000	0.0000	OK	A
SA10	Span 17	End		0.000	0.0000	OK	A
SA10	Span 18	Start		0.000	0.0000	OK	A
SA10	Span 18	Middle		0.000	0.0000	OK	A
SA10	Span 18	End		0.000	0.0000	OK	A
SA11	Span 1	Start		0.000	0.0000	OK	A
SA11	Span 1	Middle		0.000	0.0000	OK	A
SA11	Span 1	End		0.000	0.0000	OK	A
SA11	Span 2	Start		0.000	0.0000	OK	A
SA11	Span 2	Middle		0.000	0.0000	OK	A
SA11	Span 2	End		0.000	0.0000	OK	A
SA11	Span 3	Start		0.000	0.0000	OK	A
SA11	Span 3	Middle		0.000	0.0000	OK	A
SA11	Span 3	End		0.000	0.0000	OK	A
SA11	Span 4	Start		0.000	0.0000	OK	A
SA11	Span 4	Middle		0.000	0.0000	OK	A
SA11	Span 4	End		0.000	0.0000	OK	A
SA11	Span 5	Start		0.000	0.0000	OK	A
SA11	Span 5	Middle		0.000	0.0000	OK	A
SA11	Span 5	End		0.000	0.0000	OK	A
SA11	Span 6	Start	UDCONU2	0.262	0.0000	OK	A
SA11	Span 6	Middle		0.000	0.0000	OK	A
SA11	Span 6	End		0.000	0.0000	OK	A
SA11	Span 7	Start		0.000	0.0000	OK	A
SA11	Span 7	Middle		0.000	0.0000	OK	A
SA11	Span 7	End		0.000	0.0000	OK	A
SA11	Span 8	Start		0.000	0.0000	OK	A
SA11	Span 8	Middle		0.000	0.0000	OK	A
SA11	Span 8	End		0.000	0.0000	OK	A
SA11	Span 9	Start		0.000	0.0000	OK	A
SA11	Span 9	Middle		0.000	0.0000	OK	A
SA11	Span 9	End	UDCONU2	5.899	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA11	Span 10	Start	UDCONU2	7.454	0.0000	OK	A
SA11	Span 10	Middle		0.000	0.0000	OK	A
SA11	Span 10	End	UDCONU2	2.351	0.0000	OK	A
SA11	Span 11	Start	UDCONU2	2.351	0.0000	OK	A
SA11	Span 11	Middle	UDCONU2	3.029	0.0000	OK	A
SA11	Span 11	End	UDCONU2	5.155	0.0000	OK	A
SA11	Span 12	Start	UDCONU2	5.155	0.0000	OK	A
SA11	Span 12	Middle	UDCONU2	8.410	0.0000	OK	A
SA11	Span 12	End	UDCONU2	16.298	0.0000	OK	A
SA11	Span 13	Start	UDCONU2	16.298	0.0000	OK	A
SA11	Span 13	Middle		0.000	0.0000	OK	A
SA11	Span 13	End	UDCONU2	18.784	0.0000	OK	A
SA11	Span 14	Start	UDCONU2	5.370	0.0000	OK	A
SA11	Span 14	Middle	UDCONU2	4.066	0.0000	OK	A
SA11	Span 14	End	UDCONU2	7.677	0.0000	OK	A
SA11	Span 15	Start	UDCONU2	3.227	0.0000	OK	A
SA11	Span 15	Middle	UDCONU2	2.831	0.0000	OK	A
SA11	Span 15	End	UDCONU2	0.110	0.0000	OK	A
SA11	Span 16	Start	UDCONU2	0.110	0.0000	OK	A
SA11	Span 16	Middle		0.000	0.0000	OK	A
SA11	Span 16	End		0.000	0.0000	OK	A
SA11	Span 17	Start		0.000	0.0000	OK	A
SA11	Span 17	Middle		0.000	0.0000	OK	A
SA11	Span 17	End		0.000	0.0000	OK	A
SA11	Span 18	Start		0.000	0.0000	OK	A
SA11	Span 18	Middle		0.000	0.0000	OK	A
SA11	Span 18	End		0.000	0.0000	OK	A
SA11	Span 19	Start		0.000	0.0000	OK	A
SA11	Span 19	Middle		0.000	0.0000	OK	A
SA11	Span 19	End		0.000	0.0000	OK	A
SA11	Span 20	Start		0.000	0.0000	OK	A
SA11	Span 20	Middle		0.000	0.0000	OK	A
SA11	Span 20	End		0.000	0.0000	OK	A
SA12	Span 1	Start		0.000	0.0000	OK	A
SA12	Span 1	Middle		0.000	0.0000	OK	A
SA12	Span 1	End		0.000	0.0000	OK	A
SA12	Span 2	Start		0.000	0.0000	OK	A
SA12	Span 2	Middle		0.000	0.0000	OK	A
SA12	Span 2	End		0.000	0.0000	OK	A
SA12	Span 3	Start		0.000	0.0000	OK	A
SA12	Span 3	Middle		0.000	0.0000	OK	A
SA12	Span 3	End		0.000	0.0000	OK	A
SA12	Span 4	Start		0.000	0.0000	OK	A
SA12	Span 4	Middle		0.000	0.0000	OK	A
SA12	Span 4	End		0.000	0.0000	OK	A
SA12	Span 5	Start		0.000	0.0000	OK	A
SA12	Span 5	Middle		0.000	0.0000	OK	A
SA12	Span 5	End		0.000	0.0000	OK	A
SA12	Span 6	Start	UDCONU2	0.298	0.0000	OK	A
SA12	Span 6	Middle		0.000	0.0000	OK	A
SA12	Span 6	End		0.000	0.0000	OK	A
SA12	Span 7	Start		0.000	0.0000	OK	A
SA12	Span 7	Middle		0.000	0.0000	OK	A
SA12	Span 7	End		0.000	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA12	Span 8	Start		0.000	0.0000	OK	A
SA12	Span 8	Middle		0.000	0.0000	OK	A
SA12	Span 8	End		0.000	0.0000	OK	A
SA12	Span 9	Start		0.000	0.0000	OK	A
SA12	Span 9	Middle		0.000	0.0000	OK	A
SA12	Span 9	End		0.000	0.0000	OK	A
SA12	Span 10	Start	UDCONU2	0.051	0.0000	OK	A
SA12	Span 10	Middle	UDCONU2	2.553	0.0000	OK	A
SA12	Span 10	End	UDCONU2	3.878	0.0000	OK	A
SA12	Span 11	Start	UDCONU2	3.878	0.0000	OK	A
SA12	Span 11	Middle	UDCONU2	9.290	0.0000	OK	A
SA12	Span 11	End	UDCONU2	15.085	0.0000	OK	A
SA12	Span 12	Start	UDCONU2	15.085	0.0000	OK	A
SA12	Span 12	Middle		0.000	0.0000	OK	A
SA12	Span 12	End	UDCONU2	2.008	0.0000	OK	A
SA12	Span 13	Start		0.000	0.0000	OK	A
SA12	Span 13	Middle		0.000	0.0000	OK	A
SA12	Span 13	End	UDCONU2	2.089	0.0000	OK	A
SA12	Span 14	Start	UDCONU2	2.089	0.0000	OK	A
SA12	Span 14	Middle		0.000	0.0000	OK	A
SA12	Span 14	End	UDCONU2	0.847	0.0000	OK	A
SA12	Span 15	Start	UDCONU2	2.645	0.0000	OK	A
SA12	Span 15	Middle		0.000	0.0000	OK	A
SA12	Span 15	End		0.000	0.0000	OK	A
SA12	Span 16	Start		0.000	0.0000	OK	A
SA12	Span 16	Middle		0.000	0.0000	OK	A
SA12	Span 16	End		0.000	0.0000	OK	A
SA12	Span 17	Start		0.000	0.0000	OK	A
SA12	Span 17	Middle		0.000	0.0000	OK	A
SA12	Span 17	End		0.000	0.0000	OK	A
SA12	Span 18	Start		0.000	0.0000	OK	A
SA12	Span 18	Middle		0.000	0.0000	OK	A
SA12	Span 18	End		0.000	0.0000	OK	A
SA12	Span 19	Start		0.000	0.0000	OK	A
SA12	Span 19	Middle		0.000	0.0000	OK	A
SA12	Span 19	End		0.000	0.0000	OK	A
SA13	Span 1	Start		0.000	0.0000	OK	A
SA13	Span 1	Middle		0.000	0.0000	OK	A
SA13	Span 1	End		0.000	0.0000	OK	A
SA13	Span 2	Start		0.000	0.0000	OK	A
SA13	Span 2	Middle		0.000	0.0000	OK	A
SA13	Span 2	End		0.000	0.0000	OK	A
SA13	Span 3	Start		0.000	0.0000	OK	A
SA13	Span 3	Middle		0.000	0.0000	OK	A
SA13	Span 3	End		0.000	0.0000	OK	A
SA13	Span 4	Start		0.000	0.0000	OK	A
SA13	Span 4	Middle		0.000	0.0000	OK	A
SA13	Span 4	End		0.000	0.0000	OK	A
SA13	Span 5	Start		0.000	0.0000	OK	A
SA13	Span 5	Middle		0.000	0.0000	OK	A
SA13	Span 5	End		0.000	0.0000	OK	A
SA13	Span 6	Start	UDCONU2	0.210	0.0000	OK	A
SA13	Span 6	Middle		0.000	0.0000	OK	A
SA13	Span 6	End		0.000	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA13	Span 7	Start		0.000	0.0000	OK	A
SA13	Span 7	Middle		0.000	0.0000	OK	A
SA13	Span 7	End		0.000	0.0000	OK	A
SA13	Span 8	Start		0.000	0.0000	OK	A
SA13	Span 8	Middle		0.000	0.0000	OK	A
SA13	Span 8	End		0.000	0.0000	OK	A
SA13	Span 9	Start		0.000	0.0000	OK	A
SA13	Span 9	Middle		0.000	0.0000	OK	A
SA13	Span 9	End	UDCONU2	0.051	0.0000	OK	A
SA13	Span 10	Start		0.000	0.0000	OK	A
SA13	Span 10	Middle		0.000	0.0000	OK	A
SA13	Span 10	End	UDCONU2	2.290	0.0000	OK	A
SA13	Span 11	Start	UDCONU2	4.039	0.0000	OK	A
SA13	Span 11	Middle	UDCONU2	11.495	0.0000	OK	A
SA13	Span 11	End	UDCONU2	11.495	0.0000	OK	A
SA13	Span 12	Start	UDCONU2	11.495	0.0000	OK	A
SA13	Span 12	Middle		0.000	0.0000	OK	A
SA13	Span 12	End	UDCONU2	2.211	0.0000	OK	A
SA13	Span 13	Start		0.000	0.0000	OK	A
SA13	Span 13	Middle		0.000	0.0000	OK	A
SA13	Span 13	End	UDCONU2	2.439	0.0000	OK	A
SA13	Span 14	Start	UDCONU2	4.843	0.0000	OK	A
SA13	Span 14	Middle		0.000	0.0000	OK	A
SA13	Span 14	End	UDCONU2	5.663	0.0000	OK	A
SA13	Span 15	Start	UDCONU2	5.663	0.0000	OK	A
SA13	Span 15	Middle		0.000	0.0000	OK	A
SA13	Span 15	End		0.000	0.0000	OK	A
SA13	Span 16	Start		0.000	0.0000	OK	A
SA13	Span 16	Middle		0.000	0.0000	OK	A
SA13	Span 16	End		0.000	0.0000	OK	A
SA13	Span 17	Start		0.000	0.0000	OK	A
SA13	Span 17	Middle		0.000	0.0000	OK	A
SA13	Span 17	End		0.000	0.0000	OK	A
SA13	Span 18	Start		0.000	0.0000	OK	A
SA13	Span 18	Middle		0.000	0.0000	OK	A
SA13	Span 18	End		0.000	0.0000	OK	A
SA13	Span 19	Start		0.000	0.0000	OK	A
SA13	Span 19	Middle		0.000	0.0000	OK	A
SA13	Span 19	End		0.000	0.0000	OK	A
SA14	Span 1	Start		0.000	0.0000	OK	A
SA14	Span 1	Middle		0.000	0.0000	OK	A
SA14	Span 1	End		0.000	0.0000	OK	A
SA14	Span 2	Start		0.000	0.0000	OK	A
SA14	Span 2	Middle		0.000	0.0000	OK	A
SA14	Span 2	End		0.000	0.0000	OK	A
SA14	Span 3	Start		0.000	0.0000	OK	A
SA14	Span 3	Middle		0.000	0.0000	OK	A
SA14	Span 3	End		0.000	0.0000	OK	A
SA14	Span 4	Start		0.000	0.0000	OK	A
SA14	Span 4	Middle		0.000	0.0000	OK	A
SA14	Span 4	End		0.000	0.0000	OK	A
SA14	Span 5	Start		0.000	0.0000	OK	A
SA14	Span 5	Middle		0.000	0.0000	OK	A
SA14	Span 5	End	UDCONU2	1.945	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA14	Span 6	Start	UDCONU2	1.890	0.0000	OK	A
SA14	Span 6	Middle		0.000	0.0000	OK	A
SA14	Span 6	End	UDCONU2	0.532	0.0000	OK	A
SA14	Span 7	Start	UDCONU2	0.532	0.0000	OK	A
SA14	Span 7	Middle	UDCONU2	0.257	0.0000	OK	A
SA14	Span 7	End	UDCONU2	0.378	0.0000	OK	A
SA14	Span 8	Start	UDCONU2	0.378	0.0000	OK	A
SA14	Span 8	Middle	UDCONU2	0.756	0.0000	OK	A
SA14	Span 8	End	UDCONU2	0.361	0.0000	OK	A
SA14	Span 9	Start	UDCONU2	0.361	0.0000	OK	A
SA14	Span 9	Middle	UDCONU2	3.172	0.0000	OK	A
SA14	Span 9	End	UDCONU1	1.001	0.0000	OK	A
SA14	Span 10	Start	UDCONU1	1.001	0.0000	OK	A
SA14	Span 10	Middle	UDCONU2	3.273	0.0000	OK	A
SA14	Span 10	End	UDCONU1	4.738	0.0000	OK	A
SA14	Span 11	Start	UDCONU1	4.738	0.0000	OK	A
SA14	Span 11	Middle		0.000	0.0000	OK	A
SA14	Span 11	End	UDCONU2	6.561	0.0000	OK	A
SA14	Span 12	Start	UDCONU2	6.561	0.0000	OK	A
SA14	Span 12	Middle		0.000	0.0000	OK	A
SA14	Span 12	End	UDCONU1	3.469	0.0000	OK	A
SA14	Span 13	Start	UDCONU1	3.469	0.0000	OK	A
SA14	Span 13	Middle	UDCONU1	6.212	0.0000	OK	A
SA14	Span 13	End	UDCONU2	4.093	0.0000	OK	A
SA14	Span 14	Start	UDCONU2	4.093	0.0000	OK	A
SA14	Span 14	Middle	UDCONU2	6.483	0.0000	OK	A
SA14	Span 14	End	UDCONU2	8.441	0.0000	OK	A
SA14	Span 15	Start	UDCONU2	8.441	0.0000	OK	A
SA14	Span 15	Middle		0.000	0.0000	OK	A
SA14	Span 15	End	UDCONU2	9.450	0.0000	OK	A
SA14	Span 16	Start	UDCONU2	9.450	0.0000	OK	A
SA14	Span 16	Middle		0.000	0.0000	OK	A
SA14	Span 16	End	UDCONU2	12.245	0.0000	OK	A
SA14	Span 17	Start	UDCONU2	12.245	0.0000	OK	A
SA14	Span 17	Middle	UDCONU2	5.620	0.0000	OK	A
SA14	Span 17	End	UDCONU2	0.201	0.0000	OK	A
SA14	Span 18	Start	UDCONU2	0.201	0.0000	OK	A
SA14	Span 18	Middle		0.000	0.0000	OK	A
SA14	Span 18	End	UDCONU2	1.698	0.0000	OK	A
SA14	Span 19	Start	UDCONU2	1.698	0.0000	OK	A
SA14	Span 19	Middle		0.000	0.0000	OK	A
SA14	Span 19	End	UDCONU2	10.388	0.0000	OK	A
SA14	Span 20	Start	UDCONU2	10.388	0.0000	OK	A
SA14	Span 20	Middle		0.000	0.0000	OK	A
SA14	Span 20	End	UDCONU2	11.589	0.0000	OK	A
SA14	Span 21	Start	UDCONU2	11.589	0.0000	OK	A
SA14	Span 21	Middle		0.000	0.0000	OK	A
SA14	Span 21	End	UDCONU2	13.732	0.0000	OK	A
SA14	Span 22	Start	UDCONU2	13.732	0.0000	OK	A
SA14	Span 22	Middle	UDCONU2	1.994	0.0000	OK	A
SA14	Span 22	End		0.000	0.0000	OK	A
SA14	Span 23	Start		0.000	0.0000	OK	A
SA14	Span 23	Middle		0.000	0.0000	OK	A
SA14	Span 23	End		0.000	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA14	Span 24	Start		0.000	0.0000	OK	A
SA14	Span 24	Middle		0.000	0.0000	OK	A
SA14	Span 24	End		0.000	0.0000	OK	A
SA14	Span 25	Start		0.000	0.0000	OK	A
SA14	Span 25	Middle		0.000	0.0000	OK	A
SA14	Span 25	End		0.000	0.0000	OK	A
SA14	Span 26	Start		0.000	0.0000	OK	A
SA14	Span 26	Middle		0.000	0.0000	OK	A
SA14	Span 26	End		0.000	0.0000	OK	A
SA15	Span 1	Start		0.000	0.0000	OK	A
SA15	Span 1	Middle		0.000	0.0000	OK	A
SA15	Span 1	End		0.000	0.0000	OK	A
SA15	Span 2	Start		0.000	0.0000	OK	A
SA15	Span 2	Middle		0.000	0.0000	OK	A
SA15	Span 2	End		0.000	0.0000	OK	A
SA15	Span 3	Start		0.000	0.0000	OK	A
SA15	Span 3	Middle		0.000	0.0000	OK	A
SA15	Span 3	End		0.000	0.0000	OK	A
SA15	Span 4	Start		0.000	0.0000	OK	A
SA15	Span 4	Middle		0.000	0.0000	OK	A
SA15	Span 4	End	UDCONU2	2.682	0.0000	OK	A
SA15	Span 5	Start	UDCONU2	2.764	0.0000	OK	A
SA15	Span 5	Middle	UDCONU2	0.856	0.0000	OK	A
SA15	Span 5	End	UDCONU2	8.230	0.0000	OK	A
SA15	Span 6	Start	UDCONU2	8.236	0.0000	OK	A
SA15	Span 6	Middle		0.000	0.0000	OK	A
SA15	Span 6	End	UDCONU2	5.082	0.0000	OK	A
SA15	Span 7	Start	UDCONU2	5.082	0.0000	OK	A
SA15	Span 7	Middle		0.000	0.0000	OK	A
SA15	Span 7	End	UDCONU2	4.382	0.0000	OK	A
SA15	Span 8	Start	UDCONU2	4.382	0.0000	OK	A
SA15	Span 8	Middle	UDCONU2	1.154	0.0000	OK	A
SA15	Span 8	End	UDCONU2	1.154	0.0000	OK	A
SA15	Span 9	Start	UDCONU2	1.154	0.0000	OK	A
SA15	Span 9	Middle	UDCONU2	1.426	0.0000	OK	A
SA15	Span 9	End	UDCONU2	0.403	0.0000	OK	A
SA15	Span 10	Start	UDCONU2	0.403	0.0000	OK	A
SA15	Span 10	Middle	UDCONU2	0.663	0.0000	OK	A
SA15	Span 10	End	UDCONU2	2.506	0.0000	OK	A
SA15	Span 11	Start	UDCONU2	2.506	0.0000	OK	A
SA15	Span 11	Middle	UDCONU2	2.829	0.0000	OK	A
SA15	Span 11	End	UDCONU1	8.095	0.0000	OK	A
SA15	Span 12	Start	UDCONU2	8.297	0.0000	OK	A
SA15	Span 12	Middle	UDCONU2	2.732	0.0000	OK	A
SA15	Span 12	End	UDCONU2	5.065	0.0000	OK	A
SA15	Span 13	Start	UDCONU2	5.065	0.0000	OK	A
SA15	Span 13	Middle	UDCONU2	1.689	0.0000	OK	A
SA15	Span 13	End	UDCONU2	4.717	0.0000	OK	A
SA15	Span 14	Start	UDCONU2	2.685	0.0000	OK	A
SA15	Span 14	Middle		0.000	0.0000	OK	A
SA15	Span 14	End	UDCONU2	2.685	0.0000	OK	A
SA15	Span 15	Start	UDCONU2	2.685	0.0000	OK	A
SA15	Span 15	Middle	UDCONU2	5.034	0.0000	OK	A
SA15	Span 15	End		0.000	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA15	Span 16	Start		0.000	0.0000	OK	A
SA15	Span 16	Middle		0.000	0.0000	OK	A
SA15	Span 16	End		0.000	0.0000	OK	A
SA15	Span 17	Start		0.000	0.0000	OK	A
SA15	Span 17	Middle		0.000	0.0000	OK	A
SA15	Span 17	End		0.000	0.0000	OK	A
SA15	Span 18	Start		0.000	0.0000	OK	A
SA15	Span 18	Middle		0.000	0.0000	OK	A
SA15	Span 18	End		0.000	0.0000	OK	A
SA15	Span 19	Start		0.000	0.0000	OK	A
SA15	Span 19	Middle		0.000	0.0000	OK	A
SA15	Span 19	End		0.000	0.0000	OK	A
SA16	Span 1	Start		0.000	0.0000	OK	A
SA16	Span 1	Middle		0.000	0.0000	OK	A
SA16	Span 1	End		0.000	0.0000	OK	A
SA16	Span 2	Start		0.000	0.0000	OK	A
SA16	Span 2	Middle		0.000	0.0000	OK	A
SA16	Span 2	End		0.000	0.0000	OK	A
SA16	Span 3	Start		0.000	0.0000	OK	A
SA16	Span 3	Middle		0.000	0.0000	OK	A
SA16	Span 3	End		0.000	0.0000	OK	A
SA16	Span 4	Start		0.000	0.0000	OK	A
SA16	Span 4	Middle		0.000	0.0000	OK	A
SA16	Span 4	End	UDCONU2	4.638	0.0000	OK	A
SA16	Span 5	Start	UDCONU2	11.229	0.0000	OK	A
SA16	Span 5	Middle		0.000	0.0000	OK	A
SA16	Span 5	End	UDCONU2	11.190	0.0000	OK	A
SA16	Span 6	Start	UDCONU2	11.190	0.0000	OK	A
SA16	Span 6	Middle	UDCONU2	2.636	0.0000	OK	A
SA16	Span 6	End	UDCONU2	2.183	0.0000	OK	A
SA16	Span 7	Start	UDCONU1	4.659	0.0000	OK	A
SA16	Span 7	Middle		0.000	0.0000	OK	A
SA16	Span 7	End	UDCONU2	1.115	0.0000	OK	A
SA16	Span 8	Start	UDCONU2	1.115	0.0000	OK	A
SA16	Span 8	Middle		0.000	0.0000	OK	A
SA16	Span 8	End	UDCONU2	11.610	0.0000	OK	A
SA16	Span 9	Start	UDCONU2	11.610	0.0000	OK	A
SA16	Span 9	Middle	UDCONU2	8.744	0.0000	OK	A
SA16	Span 9	End	UDCONU2	3.299	0.0000	OK	A
SA16	Span 10	Start	UDCONU2	3.299	0.0000	OK	A
SA16	Span 10	Middle	UDCONU1	1.528	0.0000	OK	A
SA16	Span 10	End	UDCONU2	0.675	0.0000	OK	A
SA16	Span 11	Start	UDCONU2	0.675	0.0000	OK	A
SA16	Span 11	Middle	UDCONU2	1.055	0.0000	OK	A
SA16	Span 11	End	UDCONU1	1.065	0.0000	OK	A
SA16	Span 12	Start	UDCONU2	0.805	0.0000	OK	A
SA16	Span 12	Middle	UDCONU1	0.109	0.0000	OK	A
SA16	Span 12	End	UDCONU2	1.256	0.0000	OK	A
SA16	Span 13	Start	UDCONU2	1.256	0.0000	OK	A
SA16	Span 13	Middle	UDCONU2	3.060	0.0000	OK	A
SA16	Span 13	End	UDCONU2	10.289	0.0000	OK	A
SA16	Span 14	Start	UDCONU2	15.899	0.0000	OK	A
SA16	Span 14	Middle		0.000	0.0000	OK	A
SA16	Span 14	End	UDCONU2	15.899	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA16	Span 15	Start	UDCONU2	15.899	0.0000	OK	A
SA16	Span 15	Middle	UDCONU2	5.064	0.0000	OK	A
SA16	Span 15	End		0.000	0.0000	OK	A
SA16	Span 16	Start		0.000	0.0000	OK	A
SA16	Span 16	Middle		0.000	0.0000	OK	A
SA16	Span 16	End		0.000	0.0000	OK	A
SA16	Span 17	Start		0.000	0.0000	OK	A
SA16	Span 17	Middle		0.000	0.0000	OK	A
SA16	Span 17	End		0.000	0.0000	OK	A
SA16	Span 18	Start		0.000	0.0000	OK	A
SA16	Span 18	Middle		0.000	0.0000	OK	A
SA16	Span 18	End		0.000	0.0000	OK	A
SA16	Span 19	Start		0.000	0.0000	OK	A
SA16	Span 19	Middle		0.000	0.0000	OK	A
SA16	Span 19	End		0.000	0.0000	OK	A
SA17	Span 1	Start		0.000	0.0000	OK	A
SA17	Span 1	Middle		0.000	0.0000	OK	A
SA17	Span 1	End		0.000	0.0000	OK	A
SA17	Span 2	Start		0.000	0.0000	OK	A
SA17	Span 2	Middle		0.000	0.0000	OK	A
SA17	Span 2	End		0.000	0.0000	OK	A
SA17	Span 3	Start		0.000	0.0000	OK	A
SA17	Span 3	Middle		0.000	0.0000	OK	A
SA17	Span 3	End		0.000	0.0000	OK	A
SA17	Span 4	Start		0.000	0.0000	OK	A
SA17	Span 4	Middle		0.000	0.0000	OK	A
SA17	Span 4	End	UDCONU2	4.160	0.0000	OK	A
SA17	Span 5	Start	UDCONU2	8.880	0.0000	OK	A
SA17	Span 5	Middle	UDCONU2	1.899	0.0000	OK	A
SA17	Span 5	End	UDCONU2	7.689	0.0000	OK	A
SA17	Span 6	Start	UDCONU2	7.689	0.0000	OK	A
SA17	Span 6	Middle		0.000	0.0000	OK	A
SA17	Span 6	End	UDCONU1	3.258	0.0000	OK	A
SA17	Span 7	Start	UDCONU1	3.258	0.0000	OK	A
SA17	Span 7	Middle	UDCONU2	8.102	0.0000	OK	A
SA17	Span 7	End	UDCONU2	11.542	0.0000	OK	A
SA17	Span 8	Start	UDCONU2	11.542	0.0000	OK	A
SA17	Span 8	Middle	UDCONU2	6.970	0.0000	OK	A
SA17	Span 8	End	UDCONU2	4.851	0.0000	OK	A
SA17	Span 9	Start	UDCONU2	4.851	0.0000	OK	A
SA17	Span 9	Middle	UDCONU2	2.750	0.0000	OK	A
SA17	Span 9	End	UDCONU2	0.385	0.0000	OK	A
SA17	Span 10	Start	UDCONU2	0.385	0.0000	OK	A
SA17	Span 10	Middle	UDCONU1	0.079	0.0000	OK	A
SA17	Span 10	End	UDCONU1	0.301	0.0000	OK	A
SA17	Span 11	Start	UDCONU1	0.259	0.0000	OK	A
SA17	Span 11	Middle	UDCONU1	0.498	0.0000	OK	A
SA17	Span 11	End	UDCONU2	1.417	0.0000	OK	A
SA17	Span 12	Start	UDCONU2	1.417	0.0000	OK	A
SA17	Span 12	Middle	UDCONU2	3.149	0.0000	OK	A
SA17	Span 12	End	UDCONU2	8.069	0.0000	OK	A
SA17	Span 13	Start	UDCONU2	9.797	0.0000	OK	A
SA17	Span 13	Middle		0.000	0.0000	OK	A
SA17	Span 13	End	UDCONU2	9.797	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA17	Span 14	Start	UDCONU2	9.797	0.0000	OK	A
SA17	Span 14	Middle	UDCONU2	2.499	0.0000	OK	A
SA17	Span 14	End		0.000	0.0000	OK	A
SA17	Span 15	Start		0.000	0.0000	OK	A
SA17	Span 15	Middle		0.000	0.0000	OK	A
SA17	Span 15	End		0.000	0.0000	OK	A
SA17	Span 16	Start		0.000	0.0000	OK	A
SA17	Span 16	Middle		0.000	0.0000	OK	A
SA17	Span 16	End		0.000	0.0000	OK	A
SA17	Span 17	Start		0.000	0.0000	OK	A
SA17	Span 17	Middle		0.000	0.0000	OK	A
SA17	Span 17	End		0.000	0.0000	OK	A
SA17	Span 18	Start		0.000	0.0000	OK	A
SA17	Span 18	Middle		0.000	0.0000	OK	A
SA17	Span 18	End		0.000	0.0000	OK	A
SA18	Span 1	Start		0.000	0.0000	OK	A
SA18	Span 1	Middle		0.000	0.0000	OK	A
SA18	Span 1	End		0.000	0.0000	OK	A
SA18	Span 2	Start		0.000	0.0000	OK	A
SA18	Span 2	Middle		0.000	0.0000	OK	A
SA18	Span 2	End		0.000	0.0000	OK	A
SA18	Span 3	Start		0.000	0.0000	OK	A
SA18	Span 3	Middle		0.000	0.0000	OK	A
SA18	Span 3	End		0.000	0.0000	OK	A
SA18	Span 4	Start		0.000	0.0000	OK	A
SA18	Span 4	Middle		0.000	0.0000	OK	A
SA18	Span 4	End	UDCONU2	4.108	0.0000	OK	A
SA18	Span 5	Start	UDCONU2	8.976	0.0000	OK	A
SA18	Span 5	Middle	UDCONU2	3.061	0.0000	OK	A
SA18	Span 5	End	UDCONU2	2.518	0.0000	OK	A
SA18	Span 6	Start	UDCONU2	2.518	0.0000	OK	A
SA18	Span 6	Middle		0.000	0.0000	OK	A
SA18	Span 6	End	UDCONU2	3.812	0.0000	OK	A
SA18	Span 7	Start	UDCONU2	3.493	0.0000	OK	A
SA18	Span 7	Middle	UDCONU2	10.273	0.0000	OK	A
SA18	Span 7	End	UDCONU2	5.598	0.0000	OK	A
SA18	Span 8	Start	UDCONU2	5.598	0.0000	OK	A
SA18	Span 8	Middle	UDCONU2	2.581	0.0000	OK	A
SA18	Span 8	End	UDCONU2	0.692	0.0000	OK	A
SA18	Span 9	Start	UDCONU2	0.692	0.0000	OK	A
SA18	Span 9	Middle	UDCONU2	0.271	0.0000	OK	A
SA18	Span 9	End	UDCONU2	0.129	0.0000	OK	A
SA18	Span 10	Start	UDCONU2	0.279	0.0000	OK	A
SA18	Span 10	Middle	UDCONU2	0.553	0.0000	OK	A
SA18	Span 10	End	UDCONU2	0.907	0.0000	OK	A
SA18	Span 11	Start	UDCONU2	0.907	0.0000	OK	A
SA18	Span 11	Middle	UDCONU2	1.077	0.0000	OK	A
SA18	Span 11	End	UDCONU2	5.556	0.0000	OK	A
SA18	Span 12	Start	UDCONU2	7.245	0.0000	OK	A
SA18	Span 12	Middle		0.000	0.0000	OK	A
SA18	Span 12	End	UDCONU2	7.245	0.0000	OK	A
SA18	Span 13	Start	UDCONU2	7.245	0.0000	OK	A
SA18	Span 13	Middle	UDCONU2	2.239	0.0000	OK	A
SA18	Span 13	End		0.000	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA18	Span 14	Start		0.000	0.0000	OK	A
SA18	Span 14	Middle		0.000	0.0000	OK	A
SA18	Span 14	End		0.000	0.0000	OK	A
SA18	Span 15	Start		0.000	0.0000	OK	A
SA18	Span 15	Middle		0.000	0.0000	OK	A
SA18	Span 15	End		0.000	0.0000	OK	A
SA18	Span 16	Start		0.000	0.0000	OK	A
SA18	Span 16	Middle		0.000	0.0000	OK	A
SA18	Span 16	End		0.000	0.0000	OK	A
SA18	Span 17	Start		0.000	0.0000	OK	A
SA18	Span 17	Middle		0.000	0.0000	OK	A
SA18	Span 17	End		0.000	0.0000	OK	A
SA19	Span 1	Start		0.000	0.0000	OK	A
SA19	Span 1	Middle		0.000	0.0000	OK	A
SA19	Span 1	End		0.000	0.0000	OK	A
SA19	Span 2	Start		0.000	0.0000	OK	A
SA19	Span 2	Middle		0.000	0.0000	OK	A
SA19	Span 2	End		0.000	0.0000	OK	A
SA19	Span 3	Start		0.000	0.0000	OK	A
SA19	Span 3	Middle		0.000	0.0000	OK	A
SA19	Span 3	End		0.000	0.0000	OK	A
SA19	Span 4	Start		0.000	0.0000	OK	A
SA19	Span 4	Middle		0.000	0.0000	OK	A
SA19	Span 4	End	UDCONU2	4.714	0.0000	OK	A
SA19	Span 5	Start	UDCONU2	11.238	0.0000	OK	A
SA19	Span 5	Middle		0.000	0.0000	OK	A
SA19	Span 5	End	UDCONU2	11.238	0.0000	OK	A
SA19	Span 6	Start	UDCONU2	11.238	0.0000	OK	A
SA19	Span 6	Middle		0.000	0.0000	OK	A
SA19	Span 6	End	UDCONU2	10.703	0.0000	OK	A
SA19	Span 7	Start	UDCONU2	10.703	0.0000	OK	A
SA19	Span 7	Middle		0.000	0.0000	OK	A
SA19	Span 7	End	UDCONU2	9.106	0.0000	OK	A
SA19	Span 8	Start	UDCONU2	9.106	0.0000	OK	A
SA19	Span 8	Middle	UDCONU2	0.566	0.0000	OK	A
SA19	Span 8	End	UDCONU2	3.135	0.0000	OK	A
SA19	Span 9	Start	UDCONU2	3.135	0.0000	OK	A
SA19	Span 9	Middle		0.000	0.0000	OK	A
SA19	Span 9	End	UDCONU2	12.320	0.0000	OK	A
SA19	Span 10	Start	UDCONU2	12.320	0.0000	OK	A
SA19	Span 10	Middle		0.000	0.0000	OK	A
SA19	Span 10	End	UDCONU2	18.933	0.0000	OK	A
SA19	Span 11	Start	UDCONU2	18.933	0.0000	OK	A
SA19	Span 11	Middle		0.000	0.0000	OK	A
SA19	Span 11	End	UDCONU2	11.665	0.0000	OK	A
SA19	Span 12	Start	UDCONU2	11.665	0.0000	OK	A
SA19	Span 12	Middle	UDCONU2	3.629	0.0000	OK	A
SA19	Span 12	End	UDCONU2	3.190	0.0000	OK	A
SA19	Span 13	Start	UDCONU2	3.190	0.0000	OK	A
SA19	Span 13	Middle		0.000	0.0000	OK	A
SA19	Span 13	End	UDCONU2	0.525	0.0000	OK	A
SA19	Span 14	Start	UDCONU2	0.525	0.0000	OK	A
SA19	Span 14	Middle		0.000	0.0000	OK	A
SA19	Span 14	End	UDCONU2	5.335	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA19	Span 15	Start	UDCONU2	5.335	0.0000	OK	A
SA19	Span 15	Middle		0.000	0.0000	OK	A
SA19	Span 15	End	UDCONU2	9.558	0.0000	OK	A
SA19	Span 16	Start	UDCONU2	9.558	0.0000	OK	A
SA19	Span 16	Middle		0.000	0.0000	OK	A
SA19	Span 16	End	UDCONU2	6.235	0.0000	OK	A
SA19	Span 17	Start	UDCONU2	6.235	0.0000	OK	A
SA19	Span 17	Middle	UDCONU2	1.886	0.0000	OK	A
SA19	Span 17	End	UDCONU2	5.465	0.0000	OK	A
SA19	Span 18	Start	UDCONU2	5.465	0.0000	OK	A
SA19	Span 18	Middle		0.000	0.0000	OK	A
SA19	Span 18	End	UDCONU2	3.394	0.0000	OK	A
SA19	Span 19	Start	UDCONU2	3.394	0.0000	OK	A
SA19	Span 19	Middle		0.000	0.0000	OK	A
SA19	Span 19	End	UDCONU2	0.685	0.0000	OK	A
SA19	Span 20	Start	UDCONU2	0.685	0.0000	OK	A
SA19	Span 20	Middle	UDCONU2	0.257	0.0000	OK	A
SA19	Span 20	End	UDCONU2	0.039	0.0000	OK	A
SA19	Span 21	Start	UDCONU1	0.111	0.0000	OK	A
SA19	Span 21	Middle	UDCONU2	0.209	0.0000	OK	A
SA19	Span 21	End	UDCONU2	1.995	0.0000	OK	A
SA19	Span 22	Start	UDCONU2	1.995	0.0000	OK	A
SA19	Span 22	Middle		0.000	0.0000	OK	A
SA19	Span 22	End	UDCONU2	4.031	0.0000	OK	A
SA19	Span 23	Start	UDCONU2	4.031	0.0000	OK	A
SA19	Span 23	Middle		0.000	0.0000	OK	A
SA19	Span 23	End	UDCONU2	7.435	0.0000	OK	A
SA19	Span 24	Start	UDCONU2	7.435	0.0000	OK	A
SA19	Span 24	Middle	UDCONU2	3.005	0.0000	OK	A
SA19	Span 24	End	UDCONU2	4.514	0.0000	OK	A
SA19	Span 25	Start	UDCONU2	7.915	0.0000	OK	A
SA19	Span 25	Middle		0.000	0.0000	OK	A
SA19	Span 25	End	UDCONU2	7.915	0.0000	OK	A
SA19	Span 26	Start	UDCONU2	7.915	0.0000	OK	A
SA19	Span 26	Middle	UDCONU2	3.940	0.0000	OK	A
SA19	Span 26	End		0.000	0.0000	OK	A
SA19	Span 27	Start		0.000	0.0000	OK	A
SA19	Span 27	Middle		0.000	0.0000	OK	A
SA19	Span 27	End		0.000	0.0000	OK	A
SA19	Span 28	Start		0.000	0.0000	OK	A
SA19	Span 28	Middle		0.000	0.0000	OK	A
SA19	Span 28	End		0.000	0.0000	OK	A
SA19	Span 29	Start		0.000	0.0000	OK	A
SA19	Span 29	Middle		0.000	0.0000	OK	A
SA19	Span 29	End		0.000	0.0000	OK	A
SA19	Span 30	Start		0.000	0.0000	OK	A
SA19	Span 30	Middle		0.000	0.0000	OK	A
SA19	Span 30	End		0.000	0.0000	OK	A
SA20	Span 1	Start		0.000	0.0000	OK	A
SA20	Span 1	Middle		0.000	0.0000	OK	A
SA20	Span 1	End		0.000	0.0000	OK	A
SA20	Span 2	Start		0.000	0.0000	OK	A
SA20	Span 2	Middle		0.000	0.0000	OK	A
SA20	Span 2	End		0.000	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA20	Span 3	Start		0.000	0.0000	OK	A
SA20	Span 3	Middle		0.000	0.0000	OK	A
SA20	Span 3	End		0.000	0.0000	OK	A
SA20	Span 4	Start		0.000	0.0000	OK	A
SA20	Span 4	Middle		0.000	0.0000	OK	A
SA20	Span 4	End	UDCONU2	4.521	0.0000	OK	A
SA20	Span 5	Start	UDCONU2	10.060	0.0000	OK	A
SA20	Span 5	Middle		0.000	0.0000	OK	A
SA20	Span 5	End	UDCONU2	10.060	0.0000	OK	A
SA20	Span 6	Start	UDCONU2	10.060	0.0000	OK	A
SA20	Span 6	Middle		0.000	0.0000	OK	A
SA20	Span 6	End	UDCONU2	8.840	0.0000	OK	A
SA20	Span 7	Start	UDCONU2	8.840	0.0000	OK	A
SA20	Span 7	Middle		0.000	0.0000	OK	A
SA20	Span 7	End	UDCONU2	8.201	0.0000	OK	A
SA20	Span 8	Start	UDCONU2	8.201	0.0000	OK	A
SA20	Span 8	Middle	UDCONU2	1.921	0.0000	OK	A
SA20	Span 8	End	UDCONU2	1.036	0.0000	OK	A
SA20	Span 9	Start	UDCONU2	1.036	0.0000	OK	A
SA20	Span 9	Middle		0.000	0.0000	OK	A
SA20	Span 9	End	UDCONU2	3.973	0.0000	OK	A
SA20	Span 10	Start	UDCONU2	3.973	0.0000	OK	A
SA20	Span 10	Middle		0.000	0.0000	OK	A
SA20	Span 10	End	UDCONU2	6.116	0.0000	OK	A
SA20	Span 11	Start	UDCONU2	6.116	0.0000	OK	A
SA20	Span 11	Middle		0.000	0.0000	OK	A
SA20	Span 11	End	UDCONU2	4.069	0.0000	OK	A
SA20	Span 12	Start	UDCONU2	4.069	0.0000	OK	A
SA20	Span 12	Middle		0.000	0.0000	OK	A
SA20	Span 12	End	UDCONU1	1.330	0.0000	OK	A
SA20	Span 13	Start	UDCONU1	1.330	0.0000	OK	A
SA20	Span 13	Middle		0.000	0.0000	OK	A
SA20	Span 13	End	UDCONU2	1.562	0.0000	OK	A
SA20	Span 14	Start	UDCONU2	1.562	0.0000	OK	A
SA20	Span 14	Middle		0.000	0.0000	OK	A
SA20	Span 14	End	UDCONU2	3.568	0.0000	OK	A
SA20	Span 15	Start	UDCONU2	3.568	0.0000	OK	A
SA20	Span 15	Middle	UDCONU1	0.440	0.0000	OK	A
SA20	Span 15	End	UDCONU2	7.858	0.0000	OK	A
SA20	Span 16	Start	UDCONU2	7.858	0.0000	OK	A
SA20	Span 16	Middle		0.000	0.0000	OK	A
SA20	Span 16	End	UDCONU2	5.614	0.0000	OK	A
SA20	Span 17	Start	UDCONU2	5.614	0.0000	OK	A
SA20	Span 17	Middle		0.000	0.0000	OK	A
SA20	Span 17	End	UDCONU2	0.386	0.0000	OK	A
SA20	Span 18	Start	UDCONU2	0.386	0.0000	OK	A
SA20	Span 18	Middle	UDCONU2	0.185	0.0000	OK	A
SA20	Span 18	End	UDCONU1	0.149	0.0000	OK	A
SA20	Span 19	Start	UDCONU1	0.433	0.0000	OK	A
SA20	Span 19	Middle	UDCONU1	0.314	0.0000	OK	A
SA20	Span 19	End	UDCONU2	2.843	0.0000	OK	A
SA20	Span 20	Start	UDCONU2	2.843	0.0000	OK	A
SA20	Span 20	Middle		0.000	0.0000	OK	A
SA20	Span 20	End	UDCONU2	4.340	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA20	Span 21	Start	UDCONU2	4.340	0.0000	OK	A
SA20	Span 21	Middle		0.000	0.0000	OK	A
SA20	Span 21	End	UDCONU2	8.607	0.0000	OK	A
SA20	Span 22	Start	UDCONU2	8.607	0.0000	OK	A
SA20	Span 22	Middle	UDCONU2	4.442	0.0000	OK	A
SA20	Span 22	End	UDCONU2	3.592	0.0000	OK	A
SA20	Span 23	Start	UDCONU2	6.427	0.0000	OK	A
SA20	Span 23	Middle		0.000	0.0000	OK	A
SA20	Span 23	End	UDCONU2	6.427	0.0000	OK	A
SA20	Span 24	Start	UDCONU2	6.427	0.0000	OK	A
SA20	Span 24	Middle	UDCONU2	0.372	0.0000	OK	A
SA20	Span 24	End		0.000	0.0000	OK	A
SA20	Span 25	Start		0.000	0.0000	OK	A
SA20	Span 25	Middle		0.000	0.0000	OK	A
SA20	Span 25	End		0.000	0.0000	OK	A
SA20	Span 26	Start		0.000	0.0000	OK	A
SA20	Span 26	Middle		0.000	0.0000	OK	A
SA20	Span 26	End		0.000	0.0000	OK	A
SA20	Span 27	Start		0.000	0.0000	OK	A
SA20	Span 27	Middle		0.000	0.0000	OK	A
SA20	Span 27	End		0.000	0.0000	OK	A
SA20	Span 28	Start		0.000	0.0000	OK	A
SA20	Span 28	Middle		0.000	0.0000	OK	A
SA20	Span 28	End		0.000	0.0000	OK	A
SA21	Span 1	Start		0.000	0.0000	OK	A
SA21	Span 1	Middle		0.000	0.0000	OK	A
SA21	Span 1	End		0.000	0.0000	OK	A
SA21	Span 2	Start		0.000	0.0000	OK	A
SA21	Span 2	Middle		0.000	0.0000	OK	A
SA21	Span 2	End		0.000	0.0000	OK	A
SA21	Span 3	Start		0.000	0.0000	OK	A
SA21	Span 3	Middle		0.000	0.0000	OK	A
SA21	Span 3	End		0.000	0.0000	OK	A
SA21	Span 4	Start		0.000	0.0000	OK	A
SA21	Span 4	Middle		0.000	0.0000	OK	A
SA21	Span 4	End	UDCONU2	1.917	0.0000	OK	A
SA21	Span 5	Start	UDCONU2	3.612	0.0000	OK	A
SA21	Span 5	Middle	UDCONU2	1.831	0.0000	OK	A
SA21	Span 5	End	UDCONU2	0.089	0.0000	OK	A
SA21	Span 6	Start	UDCONU2	0.089	0.0000	OK	A
SA21	Span 6	Middle	UDCONU2	0.560	0.0000	OK	A
SA21	Span 6	End	UDCONU2	0.747	0.0000	OK	A
SA21	Span 7	Start	UDCONU2	0.747	0.0000	OK	A
SA21	Span 7	Middle	UDCONU2	2.271	0.0000	OK	A
SA21	Span 7	End	UDCONU1	0.011	0.0000	OK	A
SA21	Span 8	Start	UDCONU1	0.011	0.0000	OK	A
SA21	Span 8	Middle	UDCONU2	0.050	0.0000	OK	A
SA21	Span 8	End	UDCONU1	0.114	0.0000	OK	A
SA21	Span 9	Start	UDCONU1	0.241	0.0000	OK	A
SA21	Span 9	Middle	UDCONU1	0.170	0.0000	OK	A
SA21	Span 9	End	UDCONU2	3.560	0.0000	OK	A
SA21	Span 10	Start	UDCONU2	3.560	0.0000	OK	A
SA21	Span 10	Middle	UDCONU2	2.232	0.0000	OK	A
SA21	Span 10	End	UDCONU2	1.344	0.0000	OK	A

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SA21	Span 11	Start	UDCONU2	1.993	0.0000	OK	A
SA21	Span 11	Middle	UDCONU2	1.770	0.0000	OK	A
SA21	Span 11	End		0.000	0.0000	OK	A
SA21	Span 12	Start		0.000	0.0000	OK	A
SA21	Span 12	Middle		0.000	0.0000	OK	A
SA21	Span 12	End		0.000	0.0000	OK	A
SA21	Span 13	Start		0.000	0.0000	OK	A
SA21	Span 13	Middle		0.000	0.0000	OK	A
SA21	Span 13	End		0.000	0.0000	OK	A
SA21	Span 14	Start		0.000	0.0000	OK	A
SA21	Span 14	Middle		0.000	0.0000	OK	A
SA21	Span 14	End		0.000	0.0000	OK	A
SA21	Span 15	Start		0.000	0.0000	OK	A
SA21	Span 15	Middle		0.000	0.0000	OK	A
SA21	Span 15	End		0.000	0.0000	OK	A
SB10	Span 1	Start		0.000	0.0000	OK	B
SB10	Span 1	Middle		0.000	0.0000	OK	B
SB10	Span 1	End	UDCONU2	0.397	0.0000	OK	B
SB10	Span 2	Start	UDCONU2	0.397	0.0000	OK	B
SB10	Span 2	Middle		0.000	0.0000	OK	B
SB10	Span 2	End	UDCONU1	0.101	0.0000	OK	B
SB10	Span 3	Start	UDCONU1	0.101	0.0000	OK	B
SB10	Span 3	Middle	UDCONU1	0.191	0.0000	OK	B
SB10	Span 3	End	UDCONU1	0.268	0.0000	OK	B
SB10	Span 4	Start	UDCONU1	0.268	0.0000	OK	B
SB10	Span 4	Middle	UDCONU2	2.843	0.0000	OK	B
SB10	Span 4	End	UDCONU1	1.829	0.0000	OK	B
SB10	Span 5	Start	UDCONU1	1.829	0.0000	OK	B
SB10	Span 5	Middle	UDCONU2	7.940	0.0000	OK	B
SB10	Span 5	End	UDCONU2	4.344	0.0000	OK	B
SB10	Span 6	Start	UDCONU2	4.344	0.0000	OK	B
SB10	Span 6	Middle	UDCONU1	0.674	0.0000	OK	B
SB10	Span 6	End	UDCONU2	4.260	0.0000	OK	B
SB10	Span 7	Start	UDCONU2	4.260	0.0000	OK	B
SB10	Span 7	Middle	UDCONU2	6.858	0.0000	OK	B
SB10	Span 7	End	UDCONU2	10.578	0.0000	OK	B
SB10	Span 8	Start		0.000	0.0000	OK	B
SB10	Span 8	Middle		0.000	0.0000	OK	B
SB10	Span 8	End		0.000	0.0000	OK	B
SB10	Span 9	Start		0.000	0.0000	OK	B
SB10	Span 9	Middle		0.000	0.0000	OK	B
SB10	Span 9	End		0.000	0.0000	OK	B
SB10	Span 10	Start		0.000	0.0000	OK	B
SB10	Span 10	Middle		0.000	0.0000	OK	B
SB10	Span 10	End		0.000	0.0000	OK	B
SB10	Span 11	Start		0.000	0.0000	OK	B
SB10	Span 11	Middle		0.000	0.0000	OK	B
SB10	Span 11	End	UDCONU2	0.444	0.0000	OK	B
SB10	Span 12	Start	UDCONU2	1.457	0.0000	OK	B
SB10	Span 12	Middle	UDCONU2	3.913	0.0000	OK	B
SB10	Span 12	End	UDCONU1	0.507	0.0000	OK	B
SB10	Span 13	Start	UDCONU2	2.860	0.0000	OK	B
SB10	Span 13	Middle	UDCONU2	1.262	0.0000	OK	B
SB10	Span 13	End	UDCONU2	1.050	0.0000	OK	B

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB10	Span 14	Start	UDCONU2	1.050	0.0000	OK	B
SB10	Span 14	Middle		0.000	0.0000	OK	B
SB10	Span 14	End	UDCONU2	1.050	0.0000	OK	B
SB10	Span 15	Start	UDCONU2	1.050	0.0000	OK	B
SB10	Span 15	Middle	UDCONU2	7.127	0.0000	OK	B
SB10	Span 15	End	UDCONU2	0.295	0.0000	OK	B
SB11	Span 1	Start		0.000	0.0000	OK	B
SB11	Span 1	Middle	UDCONU2	0.178	0.0000	OK	B
SB11	Span 1	End	UDCONU1	1.682	0.0000	OK	B
SB11	Span 2	Start	UDCONU2	0.819	0.0000	OK	B
SB11	Span 2	Middle	UDCONU2	8.644	0.0000	OK	B
SB11	Span 2	End	UDCONU2	4.913	0.0000	OK	B
SB11	Span 3	Start	UDCONU2	4.913	0.0000	OK	B
SB11	Span 3	Middle	UDCONU1	1.058	0.0000	OK	B
SB11	Span 3	End	UDCONU2	5.934	0.0000	OK	B
SB11	Span 4	Start	UDCONU2	5.934	0.0000	OK	B
SB11	Span 4	Middle	UDCONU2	9.972	0.0000	OK	B
SB11	Span 4	End	UDCONU2	4.890	0.0000	OK	B
SB11	Span 5	Start	UDCONU2	4.890	0.0000	OK	B
SB11	Span 5	Middle		0.000	0.0000	OK	B
SB11	Span 5	End	UDCONU2	9.653	0.0000	OK	B
SB11	Span 6	Start	UDCONU2	9.653	0.0000	OK	B
SB11	Span 6	Middle		0.000	0.0000	OK	B
SB11	Span 6	End	UDCONU2	14.498	0.0000	OK	B
SB11	Span 7	Start	UDCONU2	14.498	0.0000	OK	B
SB11	Span 7	Middle		0.000	0.0000	OK	B
SB11	Span 7	End	UDCONU1	2.974	0.0000	OK	B
SB11	Span 8	Start	UDCONU1	2.974	0.0000	OK	B
SB11	Span 8	Middle		0.000	0.0000	OK	B
SB11	Span 8	End	UDCONU2	15.485	0.0000	OK	B
SB11	Span 9	Start	UDCONU2	15.485	0.0000	OK	B
SB11	Span 9	Middle	UDCONU1	0.537	0.0000	OK	B
SB11	Span 9	End	UDCONU2	4.677	0.0000	OK	B
SB11	Span 10	Start	UDCONU2	4.677	0.0000	OK	B
SB11	Span 10	Middle		0.000	0.0000	OK	B
SB11	Span 10	End	UDCONU2	12.013	0.0000	OK	B
SB11	Span 11	Start	UDCONU2	12.013	0.0000	OK	B
SB11	Span 11	Middle	UDCONU1	1.069	0.0000	OK	B
SB11	Span 11	End	UDCONU2	0.118	0.0000	OK	B
SB11	Span 12	Start	UDCONU2	0.118	0.0000	OK	B
SB11	Span 12	Middle	UDCONU2	0.101	0.0000	OK	B
SB11	Span 12	End	UDCONU2	0.206	0.0000	OK	B
SB11	Span 13	Start	UDCONU2	0.206	0.0000	OK	B
SB11	Span 13	Middle		0.000	0.0000	OK	B
SB11	Span 13	End	UDCONU2	0.139	0.0000	OK	B
SB11	Span 14	Start	UDCONU2	0.139	0.0000	OK	B
SB11	Span 14	Middle	UDCONU2	0.174	0.0000	OK	B
SB11	Span 14	End	UDCONU2	0.174	0.0000	OK	B
SB11	Span 15	Start	UDCONU2	0.174	0.0000	OK	B
SB11	Span 15	Middle	UDCONU2	0.048	0.0000	OK	B
SB11	Span 15	End	UDCONU1	0.020	0.0000	OK	B
SB11	Span 16	Start	UDCONU1	0.020	0.0000	OK	B
SB11	Span 16	Middle	UDCONU1	0.255	0.0000	OK	B
SB11	Span 16	End	UDCONU1	0.165	0.0000	OK	B

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB11	Span 17	Start	UDCONU1	0.165	0.0000	OK	B
SB11	Span 17	Middle	UDCONU2	2.379	0.0000	OK	B
SB11	Span 17	End	UDCONU2	0.928	0.0000	OK	B
SB11	Span 18	Start	UDCONU2	0.928	0.0000	OK	B
SB11	Span 18	Middle		0.000	0.0000	OK	B
SB11	Span 18	End	UDCONU2	0.976	0.0000	OK	B
SB11	Span 19	Start	UDCONU2	0.976	0.0000	OK	B
SB11	Span 19	Middle		0.000	0.0000	OK	B
SB11	Span 19	End	UDCONU2	1.814	0.0000	OK	B
SB11	Span 20	Start	UDCONU2	1.814	0.0000	OK	B
SB11	Span 20	Middle		0.000	0.0000	OK	B
SB11	Span 20	End	UDCONU2	5.029	0.0000	OK	B
SB11	Span 21	Start	UDCONU2	5.029	0.0000	OK	B
SB11	Span 21	Middle		0.000	0.0000	OK	B
SB11	Span 21	End	UDCONU2	8.458	0.0000	OK	B
SB11	Span 22	Start	UDCONU2	8.458	0.0000	OK	B
SB11	Span 22	Middle		0.000	0.0000	OK	B
SB11	Span 22	End	UDCONU2	4.627	0.0000	OK	B
SB11	Span 23	Start	UDCONU2	4.627	0.0000	OK	B
SB11	Span 23	Middle	UDCONU2	4.860	0.0000	OK	B
SB11	Span 23	End	UDCONU2	5.188	0.0000	OK	B
SB11	Span 24	Start	UDCONU2	5.188	0.0000	OK	B
SB11	Span 24	Middle	UDCONU2	3.007	0.0000	OK	B
SB11	Span 24	End	UDCONU2	4.134	0.0000	OK	B
SB11	Span 25	Start	UDCONU2	4.134	0.0000	OK	B
SB11	Span 25	Middle		0.000	0.0000	OK	B
SB11	Span 25	End	UDCONU2	4.134	0.0000	OK	B
SB11	Span 26	Start	UDCONU2	4.134	0.0000	OK	B
SB11	Span 26	Middle	UDCONU2	5.225	0.0000	OK	B
SB11	Span 26	End	UDCONU2	0.184	0.0000	OK	B
SB12	Span 1	Start		0.000	0.0000	OK	B
SB12	Span 1	Middle		0.000	0.0000	OK	B
SB12	Span 1	End	UDCONU2	0.291	0.0000	OK	B
SB12	Span 2	Start	UDCONU2	0.291	0.0000	OK	B
SB12	Span 2	Middle	UDCONU1	0.622	0.0000	OK	B
SB12	Span 2	End	UDCONU2	0.903	0.0000	OK	B
SB12	Span 3	Start	UDCONU2	0.903	0.0000	OK	B
SB12	Span 3	Middle	UDCONU1	1.308	0.0000	OK	B
SB12	Span 3	End	UDCONU1	2.999	0.0000	OK	B
SB12	Span 4	Start	UDCONU1	2.999	0.0000	OK	B
SB12	Span 4	Middle	UDCONU2	9.578	0.0000	OK	B
SB12	Span 4	End	UDCONU2	5.003	0.0000	OK	B
SB12	Span 5	Start	UDCONU2	5.003	0.0000	OK	B
SB12	Span 5	Middle	UDCONU1	1.254	0.0000	OK	B
SB12	Span 5	End	UDCONU2	6.754	0.0000	OK	B
SB12	Span 6	Start	UDCONU2	6.754	0.0000	OK	B
SB12	Span 6	Middle	UDCONU2	11.292	0.0000	OK	B
SB12	Span 6	End	UDCONU2	6.057	0.0000	OK	B
SB12	Span 7	Start	UDCONU2	6.057	0.0000	OK	B
SB12	Span 7	Middle		0.000	0.0000	OK	B
SB12	Span 7	End	UDCONU2	14.893	0.0000	OK	B
SB12	Span 8	Start	UDCONU2	14.893	0.0000	OK	B
SB12	Span 8	Middle	UDCONU2	8.168	0.0000	OK	B
SB12	Span 8	End	UDCONU2	4.561	0.0000	OK	B

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB12	Span 9	Start		0.000	0.0000	OK	B
SB12	Span 9	Middle		0.000	0.0000	OK	B
SB12	Span 9	End		0.000	0.0000	OK	B
SB12	Span 10	Start		0.000	0.0000	OK	B
SB12	Span 10	Middle		0.000	0.0000	OK	B
SB12	Span 10	End		0.000	0.0000	OK	B
SB12	Span 11	Start		0.000	0.0000	OK	B
SB12	Span 11	Middle		0.000	0.0000	OK	B
SB12	Span 11	End		0.000	0.0000	OK	B
SB12	Span 12	Start	UDCONU2	0.323	0.0000	OK	B
SB12	Span 12	Middle		0.000	0.0000	OK	B
SB12	Span 12	End	UDCONU2	4.123	0.0000	OK	B
SB12	Span 13	Start	UDCONU2	4.123	0.0000	OK	B
SB12	Span 13	Middle		0.000	0.0000	OK	B
SB12	Span 13	End	UDCONU2	4.506	0.0000	OK	B
SB12	Span 14	Start	UDCONU2	4.506	0.0000	OK	B
SB12	Span 14	Middle	UDCONU2	2.857	0.0000	OK	B
SB12	Span 14	End	UDCONU1	0.492	0.0000	OK	B
SB12	Span 15	Start	UDCONU1	0.492	0.0000	OK	B
SB12	Span 15	Middle	UDCONU2	2.367	0.0000	OK	B
SB12	Span 15	End	UDCONU2	9.637	0.0000	OK	B
SB12	Span 16	Start	UDCONU2	9.637	0.0000	OK	B
SB12	Span 16	Middle		0.000	0.0000	OK	B
SB12	Span 16	End	UDCONU2	5.691	0.0000	OK	B
SB12	Span 17	Start	UDCONU2	5.691	0.0000	OK	B
SB12	Span 17	Middle		0.000	0.0000	OK	B
SB12	Span 17	End	UDCONU2	6.019	0.0000	OK	B
SB12	Span 18	Start	UDCONU2	6.019	0.0000	OK	B
SB12	Span 18	Middle	UDCONU2	6.842	0.0000	OK	B
SB12	Span 18	End	UDCONU2	8.255	0.0000	OK	B
SB12	Span 19	Start	UDCONU2	8.255	0.0000	OK	B
SB12	Span 19	Middle		0.000	0.0000	OK	B
SB12	Span 19	End	UDCONU2	7.390	0.0000	OK	B
SB12	Span 20	Start	UDCONU2	7.390	0.0000	OK	B
SB12	Span 20	Middle	UDCONU2	6.669	0.0000	OK	B
SB12	Span 20	End	UDCONU2	0.309	0.0000	OK	B
SB13	Span 1	Start		0.000	0.0000	OK	B
SB13	Span 1	Middle	UDCONU1	2.005	0.0000	OK	B
SB13	Span 1	End	UDCONU2	3.685	0.0000	OK	B
SB13	Span 2	Start	UDCONU2	10.884	0.0000	OK	B
SB13	Span 2	Middle		0.000	0.0000	OK	B
SB13	Span 2	End	UDCONU1	1.764	0.0000	OK	B
SB13	Span 3	Start	UDCONU1	1.764	0.0000	OK	B
SB13	Span 3	Middle		0.000	0.0000	OK	B
SB13	Span 3	End	UDCONU2	1.272	0.0000	OK	B
SB13	Span 4	Start	UDCONU2	1.272	0.0000	OK	B
SB13	Span 4	Middle	UDCONU2	8.842	0.0000	OK	B
SB13	Span 4	End	UDCONU2	4.187	0.0000	OK	B
SB13	Span 5	Start	UDCONU2	4.187	0.0000	OK	B
SB13	Span 5	Middle	UDCONU1	1.096	0.0000	OK	B
SB13	Span 5	End	UDCONU2	6.894	0.0000	OK	B
SB13	Span 6	Start	UDCONU2	6.894	0.0000	OK	B
SB13	Span 6	Middle	UDCONU2	11.069	0.0000	OK	B
SB13	Span 6	End	UDCONU2	17.917	0.0000	OK	B

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB13	Span 7	Start	UDCONU2	17.917	0.0000	OK	B
SB13	Span 7	Middle		0.000	0.0000	OK	B
SB13	Span 7	End	UDCONU2	17.265	0.0000	OK	B
SB13	Span 8	Start	UDCONU2	17.265	0.0000	OK	B
SB13	Span 8	Middle	UDCONU2	11.602	0.0000	OK	B
SB13	Span 8	End	UDCONU2	1.657	0.0000	OK	B
SB13	Span 9	Start		0.000	0.0000	OK	B
SB13	Span 9	Middle		0.000	0.0000	OK	B
SB13	Span 9	End		0.000	0.0000	OK	B
SB13	Span 10	Start		0.000	0.0000	OK	B
SB13	Span 10	Middle		0.000	0.0000	OK	B
SB13	Span 10	End		0.000	0.0000	OK	B
SB13	Span 11	Start		0.000	0.0000	OK	B
SB13	Span 11	Middle	UDCONU2	0.544	0.0000	OK	B
SB13	Span 11	End	UDCONU2	9.283	0.0000	OK	B
SB13	Span 12	Start	UDCONU2	9.283	0.0000	OK	B
SB13	Span 12	Middle	UDCONU2	6.539	0.0000	OK	B
SB13	Span 12	End	UDCONU1	0.418	0.0000	OK	B
SB13	Span 13	Start	UDCONU1	0.418	0.0000	OK	B
SB13	Span 13	Middle	UDCONU2	2.986	0.0000	OK	B
SB13	Span 13	End	UDCONU2	7.088	0.0000	OK	B
SB13	Span 14	Start	UDCONU2	7.088	0.0000	OK	B
SB13	Span 14	Middle		0.000	0.0000	OK	B
SB13	Span 14	End	UDCONU2	7.113	0.0000	OK	B
SB13	Span 15	Start	UDCONU2	7.113	0.0000	OK	B
SB13	Span 15	Middle	UDCONU2	2.916	0.0000	OK	B
SB13	Span 15	End	UDCONU2	0.194	0.0000	OK	B
SB14	Span 1	Start		0.000	0.0000	OK	B
SB14	Span 1	Middle	UDCONU1	1.217	0.0000	OK	B
SB14	Span 1	End	UDCONU2	1.332	0.0000	OK	B
SB14	Span 2	Start	UDCONU2	1.332	0.0000	OK	B
SB14	Span 2	Middle	UDCONU2	1.476	0.0000	OK	B
SB14	Span 2	End	UDCONU2	4.379	0.0000	OK	B
SB14	Span 3	Start	UDCONU2	4.185	0.0000	OK	B
SB14	Span 3	Middle		0.000	0.0000	OK	B
SB14	Span 3	End	UDCONU2	7.394	0.0000	OK	B
SB14	Span 4	Start	UDCONU2	7.394	0.0000	OK	B
SB14	Span 4	Middle	UDCONU2	6.060	0.0000	OK	B
SB14	Span 4	End	UDCONU2	15.419	0.0000	OK	B
SB14	Span 5	Start	UDCONU2	15.419	0.0000	OK	B
SB14	Span 5	Middle		0.000	0.0000	OK	B
SB14	Span 5	End	UDCONU2	13.821	0.0000	OK	B
SB14	Span 6	Start	UDCONU2	13.821	0.0000	OK	B
SB14	Span 6	Middle	UDCONU2	12.916	0.0000	OK	B
SB14	Span 6	End	UDCONU2	0.043	0.0000	OK	B
SB14	Span 7	Start		0.000	0.0000	OK	B
SB14	Span 7	Middle		0.000	0.0000	OK	B
SB14	Span 7	End		0.000	0.0000	OK	B
SB14	Span 8	Start		0.000	0.0000	OK	B
SB14	Span 8	Middle		0.000	0.0000	OK	B
SB14	Span 8	End		0.000	0.0000	OK	B
SB14	Span 9	Start		0.000	0.0000	OK	B
SB14	Span 9	Middle		0.000	0.0000	OK	B
SB14	Span 9	End	UDCONU2	13.008	0.0000	OK	B

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB14	Span 10	Start	UDCONU2	13.008	0.0000	OK	B
SB14	Span 10	Middle	UDCONU2	9.266	0.0000	OK	B
SB14	Span 10	End	UDCONU2	3.804	0.0000	OK	B
SB14	Span 11	Start	UDCONU2	0.693	0.0000	OK	B
SB14	Span 11	Middle	UDCONU2	2.920	0.0000	OK	B
SB14	Span 11	End	UDCONU2	13.602	0.0000	OK	B
SB14	Span 12	Start	UDCONU2	13.602	0.0000	OK	B
SB14	Span 12	Middle		0.000	0.0000	OK	B
SB14	Span 12	End	UDCONU2	13.602	0.0000	OK	B
SB14	Span 13	Start	UDCONU2	13.602	0.0000	OK	B
SB14	Span 13	Middle	UDCONU2	3.312	0.0000	OK	B
SB14	Span 13	End	UDCONU2	0.277	0.0000	OK	B
SB15	Span 1	Start		0.000	0.0000	OK	B
SB15	Span 1	Middle		0.000	0.0000	OK	B
SB15	Span 1	End	UDCONU2	0.404	0.0000	OK	B
SB15	Span 2	Start	UDCONU2	0.404	0.0000	OK	B
SB15	Span 2	Middle	UDCONU1	0.145	0.0000	OK	B
SB15	Span 2	End	UDCONU2	0.204	0.0000	OK	B
SB15	Span 3	Start	UDCONU2	0.204	0.0000	OK	B
SB15	Span 3	Middle	UDCONU2	14.673	0.0000	OK	B
SB15	Span 3	End	UDCONU2	8.713	0.0000	OK	B
SB15	Span 4	Start	UDCONU2	8.713	0.0000	OK	B
SB15	Span 4	Middle	UDCONU2	2.193	0.0000	OK	B
SB15	Span 4	End	UDCONU2	8.454	0.0000	OK	B
SB15	Span 5	Start	UDCONU2	8.454	0.0000	OK	B
SB15	Span 5	Middle	UDCONU2	14.102	0.0000	OK	B
SB15	Span 5	End	UDCONU2	15.698	0.0000	OK	B
SB15	Span 6	Start	UDCONU2	15.698	0.0000	OK	B
SB15	Span 6	Middle	UDCONU2	20.680	0.0000	OK	B
SB15	Span 6	End	UDCONU2	19.128	0.0000	OK	B
SB15	Span 7	Start	UDCONU2	19.128	0.0000	OK	B
SB15	Span 7	Middle		0.000	0.0000	OK	B
SB15	Span 7	End	UDCONU2	9.777	0.0000	OK	B
SB15	Span 8	Start	UDCONU2	9.777	0.0000	OK	B
SB15	Span 8	Middle	UDCONU2	10.712	0.0000	OK	B
SB15	Span 8	End	UDCONU2	13.747	0.0000	OK	B
SB15	Span 9	Start		0.000	0.0000	OK	B
SB15	Span 9	Middle		0.000	0.0000	OK	B
SB15	Span 9	End		0.000	0.0000	OK	B
SB15	Span 10	Start		0.000	0.0000	OK	B
SB15	Span 10	Middle		0.000	0.0000	OK	B
SB15	Span 10	End		0.000	0.0000	OK	B
SB15	Span 11	Start		0.000	0.0000	OK	B
SB15	Span 11	Middle		0.000	0.0000	OK	B
SB15	Span 11	End	UDCONU2	14.946	0.0000	OK	B
SB15	Span 12	Start	UDCONU2	14.946	0.0000	OK	B
SB15	Span 12	Middle	UDCONU2	11.018	0.0000	OK	B
SB15	Span 12	End	UDCONU2	1.418	0.0000	OK	B
SB15	Span 13	Start	UDCONU2	1.418	0.0000	OK	B
SB15	Span 13	Middle	UDCONU2	2.822	0.0000	OK	B
SB15	Span 13	End	UDCONU2	16.575	0.0000	OK	B
SB15	Span 14	Start	UDCONU2	16.575	0.0000	OK	B
SB15	Span 14	Middle	UDCONU2	16.575	0.0000	OK	B
SB15	Span 14	End	UDCONU2	0.362	0.0000	OK	B

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB16	Span 1	Start		0.000	0.0000	OK	B
SB16	Span 1	Middle	UDCONU2	4.746	0.0000	OK	B
SB16	Span 1	End	UDCONU2	21.571	0.0000	OK	B
SB16	Span 2	Start	UDCONU2	12.681	0.0000	OK	B
SB16	Span 2	Middle	UDCONU2	5.777	0.0000	OK	B
SB16	Span 2	End	UDCONU2	6.912	0.0000	OK	B
SB16	Span 3	Start	UDCONU2	6.912	0.0000	OK	B
SB16	Span 3	Middle	UDCONU2	11.889	0.0000	OK	B
SB16	Span 3	End	UDCONU2	8.537	0.0000	OK	B
SB16	Span 4	Start	UDCONU2	8.537	0.0000	OK	B
SB16	Span 4	Middle		0.000	0.0000	OK	B
SB16	Span 4	End	UDCONU2	9.166	0.0000	OK	B
SB16	Span 5	Start	UDCONU2	9.166	0.0000	OK	B
SB16	Span 5	Middle	UDCONU2	10.190	0.0000	OK	B
SB16	Span 5	End	UDCONU2	6.122	0.0000	OK	B
SB16	Span 6	Start	UDCONU2	5.979	0.0000	OK	B
SB16	Span 6	Middle		0.000	0.0000	OK	B
SB16	Span 6	End	UDCONU2	5.014	0.0000	OK	B
SB16	Span 7	Start	UDCONU2	5.014	0.0000	OK	B
SB16	Span 7	Middle	UDCONU2	0.598	0.0000	OK	B
SB16	Span 7	End		0.000	0.0000	OK	B
SB16	Span 8	Start		0.000	0.0000	OK	B
SB16	Span 8	Middle		0.000	0.0000	OK	B
SB16	Span 8	End		0.000	0.0000	OK	B
SB16	Span 9	Start		0.000	0.0000	OK	B
SB16	Span 9	Middle	UDCONU2	17.797	0.0000	OK	B
SB16	Span 9	End	UDCONU2	17.783	0.0000	OK	B
SB16	Span 10	Start	UDCONU2	17.783	0.0000	OK	B
SB16	Span 10	Middle	UDCONU2	13.016	0.0000	OK	B
SB16	Span 10	End	UDCONU2	2.283	0.0000	OK	B
SB16	Span 11	Start	UDCONU2	2.283	0.0000	OK	B
SB16	Span 11	Middle	UDCONU2	2.276	0.0000	OK	B
SB16	Span 11	End	UDCONU2	16.052	0.0000	OK	B
SB16	Span 12	Start	UDCONU2	16.052	0.0000	OK	B
SB16	Span 12	Middle	UDCONU2	16.052	0.0000	OK	B
SB16	Span 12	End	UDCONU2	0.300	0.0000	OK	B
SB17	Span 1	Start		0.000	0.0000	OK	B
SB17	Span 1	Middle		0.000	0.0000	OK	B
SB17	Span 1	End	UDCONU2	0.365	0.0000	OK	B
SB17	Span 2	Start	UDCONU2	0.365	0.0000	OK	B
SB17	Span 2	Middle	UDCONU2	2.607	0.0000	OK	B
SB17	Span 2	End	UDCONU2	13.090	0.0000	OK	B
SB17	Span 3	Start	UDCONU2	13.090	0.0000	OK	B
SB17	Span 3	Middle		0.000	0.0000	OK	B
SB17	Span 3	End	UDCONU2	8.032	0.0000	OK	B
SB17	Span 4	Start	UDCONU2	8.032	0.0000	OK	B
SB17	Span 4	Middle	UDCONU2	20.825	0.0000	OK	B
SB17	Span 4	End	UDCONU2	13.549	0.0000	OK	B
SB17	Span 5	Start	UDCONU2	13.549	0.0000	OK	B
SB17	Span 5	Middle	UDCONU2	6.161	0.0000	OK	B
SB17	Span 5	End	UDCONU2	7.239	0.0000	OK	B
SB17	Span 6	Start	UDCONU2	7.239	0.0000	OK	B
SB17	Span 6	Middle	UDCONU2	13.112	0.0000	OK	B
SB17	Span 6	End	UDCONU2	12.296	0.0000	OK	B

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB17	Span 7	Start	UDCONU2	12.296	0.0000	OK	B
SB17	Span 7	Middle		0.000	0.0000	OK	B
SB17	Span 7	End	UDCONU2	11.638	0.0000	OK	B
SB17	Span 8	Start	UDCONU2	11.638	0.0000	OK	B
SB17	Span 8	Middle	UDCONU2	8.567	0.0000	OK	B
SB17	Span 8	End	UDCONU2	2.393	0.0000	OK	B
SB17	Span 9	Start	UDCONU2	2.393	0.0000	OK	B
SB17	Span 9	Middle	UDCONU2	1.347	0.0000	OK	B
SB17	Span 9	End	UDCONU2	1.281	0.0000	OK	B
SB17	Span 10	Start	UDCONU2	1.281	0.0000	OK	B
SB17	Span 10	Middle		0.000	0.0000	OK	B
SB17	Span 10	End	UDCONU2	0.788	0.0000	OK	B
SB17	Span 11	Start	UDCONU2	0.788	0.0000	OK	B
SB17	Span 11	Middle	UDCONU2	0.451	0.0000	OK	B
SB17	Span 11	End	UDCONU2	0.423	0.0000	OK	B
SB17	Span 12	Start	UDCONU2	0.423	0.0000	OK	B
SB17	Span 12	Middle	UDCONU2	0.842	0.0000	OK	B
SB17	Span 12	End		0.000	0.0000	OK	B
SB17	Span 13	Start		0.000	0.0000	OK	B
SB17	Span 13	Middle	UDCONU2	4.387	0.0000	OK	B
SB17	Span 13	End	UDCONU2	25.313	0.0000	OK	B
SB17	Span 14	Start	UDCONU2	25.313	0.0000	OK	B
SB17	Span 14	Middle		0.000	0.0000	OK	B
SB17	Span 14	End	UDCONU2	29.892	0.0000	OK	B
SB17	Span 15	Start	UDCONU2	29.892	0.0000	OK	B
SB17	Span 15	Middle		0.000	0.0000	OK	B
SB17	Span 15	End	UDCONU2	18.835	0.0000	OK	B
SB17	Span 16	Start	UDCONU2	18.835	0.0000	OK	B
SB17	Span 16	Middle	UDCONU2	13.494	0.0000	OK	B
SB17	Span 16	End	UDCONU2	2.612	0.0000	OK	B
SB17	Span 17	Start	UDCONU2	2.612	0.0000	OK	B
SB17	Span 17	Middle	UDCONU2	2.033	0.0000	OK	B
SB17	Span 17	End	UDCONU2	15.399	0.0000	OK	B
SB17	Span 18	Start	UDCONU2	15.399	0.0000	OK	B
SB17	Span 18	Middle	UDCONU2	15.399	0.0000	OK	B
SB17	Span 18	End	UDCONU2	0.282	0.0000	OK	B
SB18	Span 1	Start		0.000	0.0000	OK	B
SB18	Span 1	Middle	UDCONU2	12.246	0.0000	OK	B
SB18	Span 1	End	UDCONU2	9.364	0.0000	OK	B
SB18	Span 2	Start	UDCONU2	9.364	0.0000	OK	B
SB18	Span 2	Middle	UDCONU2	20.172	0.0000	OK	B
SB18	Span 2	End	UDCONU2	13.639	0.0000	OK	B
SB18	Span 3	Start	UDCONU2	13.639	0.0000	OK	B
SB18	Span 3	Middle	UDCONU2	6.593	0.0000	OK	B
SB18	Span 3	End	UDCONU2	8.341	0.0000	OK	B
SB18	Span 4	Start	UDCONU2	8.341	0.0000	OK	B
SB18	Span 4	Middle	UDCONU2	17.064	0.0000	OK	B
SB18	Span 4	End	UDCONU2	27.946	0.0000	OK	B
SB18	Span 5	Start	UDCONU2	27.946	0.0000	OK	B
SB18	Span 5	Middle		0.000	0.0000	OK	B
SB18	Span 5	End	UDCONU2	37.278	0.0000	OK	B
SB18	Span 6	Start	UDCONU2	37.278	0.0000	OK	B
SB18	Span 6	Middle		0.000	0.0000	OK	B
SB18	Span 6	End	UDCONU2	16.646	0.0000	OK	B

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB18	Span 7	Start	UDCONU2	16.646	0.0000	OK	B
SB18	Span 7	Middle	UDCONU2	7.030	0.0000	OK	B
SB18	Span 7	End	UDCONU2	1.254	0.0000	OK	B
SB18	Span 8	Start	UDCONU2	1.254	0.0000	OK	B
SB18	Span 8	Middle	UDCONU2	2.104	0.0000	OK	B
SB18	Span 8	End	UDCONU2	0.317	0.0000	OK	B
SB18	Span 9	Start	UDCONU2	0.317	0.0000	OK	B
SB18	Span 9	Middle		0.000	0.0000	OK	B
SB18	Span 9	End	UDCONU2	1.141	0.0000	OK	B
SB18	Span 10	Start	UDCONU2	1.141	0.0000	OK	B
SB18	Span 10	Middle	UDCONU2	2.043	0.0000	OK	B
SB18	Span 10	End	UDCONU2	0.327	0.0000	OK	B
SB18	Span 11	Start	UDCONU2	0.327	0.0000	OK	B
SB18	Span 11	Middle	UDCONU1	3.183	0.0000	OK	B
SB18	Span 11	End	UDCONU1	2.379	0.0000	OK	B
SB18	Span 12	Start	UDCONU2	7.782	0.0000	OK	B
SB18	Span 12	Middle		0.000	0.0000	OK	B
SB18	Span 12	End	UDCONU2	18.744	0.0000	OK	B
SB18	Span 13	Start	UDCONU2	18.744	0.0000	OK	B
SB18	Span 13	Middle		0.000	0.0000	OK	B
SB18	Span 13	End	UDCONU2	15.753	0.0000	OK	B
SB18	Span 14	Start	UDCONU2	15.753	0.0000	OK	B
SB18	Span 14	Middle	UDCONU2	11.808	0.0000	OK	B
SB18	Span 14	End	UDCONU2	2.408	0.0000	OK	B
SB18	Span 15	Start	UDCONU2	2.408	0.0000	OK	B
SB18	Span 15	Middle	UDCONU2	2.051	0.0000	OK	B
SB18	Span 15	End	UDCONU2	14.695	0.0000	OK	B
SB18	Span 16	Start	UDCONU2	14.695	0.0000	OK	B
SB18	Span 16	Middle	UDCONU2	15.858	0.0000	OK	B
SB18	Span 16	End	UDCONU2	0.295	0.0000	OK	B
SB19	Span 1	Start		0.000	0.0000	OK	B
SB19	Span 1	Middle	UDCONU2	16.415	0.0000	OK	B
SB19	Span 1	End	UDCONU2	80.577	0.7637	OK	B
SB19	Span 2	Start	UDCONU2	80.577	0.7637	OK	B
SB19	Span 2	Middle		0.000	0.0000	OK	B
SB19	Span 2	End	UDCONU2	80.575	0.7637	OK	B
SB19	Span 3	Start	UDCONU2	80.575	0.7637	OK	B
SB19	Span 3	Middle	UDCONU2	24.762	0.0000	OK	B
SB19	Span 3	End	UDCONU2	15.423	0.0000	OK	B
SB19	Span 4	Start	UDCONU2	15.423	0.0000	OK	B
SB19	Span 4	Middle	UDCONU2	7.347	0.0000	OK	B
SB19	Span 4	End	UDCONU2	6.805	0.0000	OK	B
SB19	Span 5	Start	UDCONU2	6.805	0.0000	OK	B
SB19	Span 5	Middle	UDCONU2	13.857	0.0000	OK	B
SB19	Span 5	End	UDCONU2	4.496	0.0000	OK	B
SB19	Span 6	Start	UDCONU2	4.496	0.0000	OK	B
SB19	Span 6	Middle	UDCONU2	1.217	0.0000	OK	B
SB19	Span 6	End	UDCONU2	2.795	0.0000	OK	B
SB19	Span 7	Start	UDCONU2	2.795	0.0000	OK	B
SB19	Span 7	Middle		0.000	0.0000	OK	B
SB19	Span 7	End	UDCONU2	3.250	0.0000	OK	B
SB19	Span 8	Start	UDCONU2	3.250	0.0000	OK	B
SB19	Span 8	Middle	UDCONU2	0.546	0.0000	OK	B
SB19	Span 8	End	UDCONU2	1.824	0.0000	OK	B

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB19	Span 9	Start	UDCONU2	1.824	0.0000	OK	B
SB19	Span 9	Middle		0.000	0.0000	OK	B
SB19	Span 9	End	UDCONU2	1.546	0.0000	OK	B
SB19	Span 10	Start	UDCONU2	1.546	0.0000	OK	B
SB19	Span 10	Middle		0.000	0.0000	OK	B
SB19	Span 10	End	UDCONU2	0.967	0.0000	OK	B
SB19	Span 11	Start	UDCONU2	0.967	0.0000	OK	B
SB19	Span 11	Middle		0.000	0.0000	OK	B
SB19	Span 11	End	UDCONU2	1.208	0.0000	OK	B
SB19	Span 12	Start	UDCONU2	1.208	0.0000	OK	B
SB19	Span 12	Middle	UDCONU2	0.613	0.0000	OK	B
SB19	Span 12	End	UDCONU2	3.568	0.0000	OK	B
SB19	Span 13	Start	UDCONU2	3.568	0.0000	OK	B
SB19	Span 13	Middle		0.000	0.0000	OK	B
SB19	Span 13	End	UDCONU2	11.761	0.0000	OK	B
SB19	Span 14	Start	UDCONU2	4.299	0.0000	OK	B
SB19	Span 14	Middle		0.000	0.0000	OK	B
SB19	Span 14	End	UDCONU2	3.032	0.0000	OK	B
SB19	Span 15	Start	UDCONU2	3.032	0.0000	OK	B
SB19	Span 15	Middle		0.000	0.0000	OK	B
SB19	Span 15	End	UDCONU2	1.605	0.0000	OK	B
SB19	Span 16	Start	UDCONU2	1.605	0.0000	OK	B
SB19	Span 16	Middle	UDCONU1	0.150	0.0000	OK	B
SB19	Span 16	End	UDCONU2	0.163	0.0000	OK	B
SB19	Span 17	Start	UDCONU2	0.163	0.0000	OK	B
SB19	Span 17	Middle		0.000	0.0000	OK	B
SB19	Span 17	End	UDCONU2	0.395	0.0000	OK	B
SB19	Span 18	Start	UDCONU2	0.395	0.0000	OK	B
SB19	Span 18	Middle		0.000	0.0000	OK	B
SB19	Span 18	End	UDCONU2	0.300	0.0000	OK	B
SB19	Span 19	Start	UDCONU2	0.300	0.0000	OK	B
SB19	Span 19	Middle	UDCONU2	2.138	0.0000	OK	B
SB19	Span 19	End	UDCONU2	5.255	0.0000	OK	B
SB19	Span 20	Start	UDCONU2	4.330	0.0000	OK	B
SB19	Span 20	Middle		0.000	0.0000	OK	B
SB19	Span 20	End	UDCONU2	13.414	0.0000	OK	B
SB19	Span 21	Start	UDCONU2	13.414	0.0000	OK	B
SB19	Span 21	Middle	UDCONU2	9.844	0.0000	OK	B
SB19	Span 21	End	UDCONU2	2.268	0.0000	OK	B
SB19	Span 22	Start	UDCONU2	2.268	0.0000	OK	B
SB19	Span 22	Middle	UDCONU2	1.427	0.0000	OK	B
SB19	Span 22	End	UDCONU2	13.397	0.0000	OK	B
SB19	Span 23	Start	UDCONU2	13.397	0.0000	OK	B
SB19	Span 23	Middle		0.000	0.0000	OK	B
SB19	Span 23	End	UDCONU2	22.737	0.0000	OK	B
SB19	Span 24	Start	UDCONU2	22.737	0.0000	OK	B
SB19	Span 24	Middle	UDCONU2	3.906	0.0000	OK	B
SB19	Span 24	End	UDCONU2	0.379	0.0000	OK	B
SB20	Span 1	Start		0.000	0.0000	OK	B
SB20	Span 1	Middle	UDCONU2	10.472	0.0000	OK	B
SB20	Span 1	End	UDCONU2	10.472	0.0000	OK	B
SB20	Span 2	Start	UDCONU2	10.472	0.0000	OK	B
SB20	Span 2	Middle	UDCONU2	18.451	0.0000	OK	B
SB20	Span 2	End	UDCONU2	12.972	0.0000	OK	B

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB20	Span 3	Start	UDCONU2	12.972	0.0000	OK	B
SB20	Span 3	Middle	UDCONU2	6.774	0.0000	OK	B
SB20	Span 3	End	UDCONU2	5.815	0.0000	OK	B
SB20	Span 4	Start	UDCONU2	5.815	0.0000	OK	B
SB20	Span 4	Middle	UDCONU2	12.016	0.0000	OK	B
SB20	Span 4	End	UDCONU2	11.241	0.0000	OK	B
SB20	Span 5	Start		0.000	0.0000	OK	B
SB20	Span 5	Middle		0.000	0.0000	OK	B
SB20	Span 5	End	UDCONU2	3.095	0.0000	OK	B
SB20	Span 6	Start	UDCONU2	3.095	0.0000	OK	B
SB20	Span 6	Middle	UDCONU2	2.761	0.0000	OK	B
SB20	Span 6	End	UDCONU2	10.553	0.0000	OK	B
SB20	Span 7	Start	UDCONU2	10.553	0.0000	OK	B
SB20	Span 7	Middle		0.000	0.0000	OK	B
SB20	Span 7	End	UDCONU2	8.457	0.0000	OK	B
SB20	Span 8	Start	UDCONU2	8.108	0.0000	OK	B
SB20	Span 8	Middle	UDCONU2	1.109	0.0000	OK	B
SB20	Span 8	End		0.000	0.0000	OK	B
SB20	Span 9	Start		0.000	0.0000	OK	B
SB20	Span 9	Middle		0.000	0.0000	OK	B
SB20	Span 9	End	UDCONU2	4.869	0.0000	OK	B
SB20	Span 10	Start	UDCONU2	0.728	0.0000	OK	B
SB20	Span 10	Middle		0.000	0.0000	OK	B
SB20	Span 10	End	UDCONU2	8.054	0.0000	OK	B
SB20	Span 11	Start	UDCONU2	8.054	0.0000	OK	B
SB20	Span 11	Middle	UDCONU2	4.939	0.0000	OK	B
SB20	Span 11	End	UDCONU2	3.160	0.0000	OK	B
SB20	Span 12	Start	UDCONU2	3.160	0.0000	OK	B
SB20	Span 12	Middle	UDCONU2	0.221	0.0000	OK	B
SB20	Span 12	End	UDCONU2	8.073	0.0000	OK	B
SB20	Span 13	Start	UDCONU2	8.073	0.0000	OK	B
SB20	Span 13	Middle		0.000	0.0000	OK	B
SB20	Span 13	End	UDCONU2	7.304	0.0000	OK	B
SB20	Span 14	Start	UDCONU2	7.304	0.0000	OK	B
SB20	Span 14	Middle	UDCONU2	2.224	0.0000	OK	B
SB20	Span 14	End	UDCONU2	0.110	0.0000	OK	B
SB21	Span 1	Start		0.000	0.0000	OK	B
SB21	Span 1	Middle	UDCONU2	14.134	0.0000	OK	B
SB21	Span 1	End	UDCONU2	63.803	0.7637	OK	B
SB21	Span 2	Start	UDCONU2	63.803	0.7637	OK	B
SB21	Span 2	Middle		0.000	0.0000	OK	B
SB21	Span 2	End	UDCONU2	55.855	0.0000	OK	B
SB21	Span 3	Start	UDCONU2	55.855	0.0000	OK	B
SB21	Span 3	Middle	UDCONU2	22.396	0.0000	OK	B
SB21	Span 3	End	UDCONU2	14.336	0.0000	OK	B
SB21	Span 4	Start	UDCONU2	14.336	0.0000	OK	B
SB21	Span 4	Middle	UDCONU2	7.075	0.0000	OK	B
SB21	Span 4	End	UDCONU2	5.566	0.0000	OK	B
SB21	Span 5	Start	UDCONU2	5.566	0.0000	OK	B
SB21	Span 5	Middle	UDCONU2	12.913	0.0000	OK	B
SB21	Span 5	End	UDCONU2	23.676	0.0000	OK	B
SB21	Span 6	Start	UDCONU2	23.676	0.0000	OK	B
SB21	Span 6	Middle		0.000	0.0000	OK	B
SB21	Span 6	End	UDCONU2	36.836	0.0000	OK	B

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB21	Span 7	Start	UDCONU2	36.836	0.0000	OK	B
SB21	Span 7	Middle		0.000	0.0000	OK	B
SB21	Span 7	End	UDCONU2	8.904	0.0000	OK	B
SB21	Span 8	Start	UDCONU2	8.904	0.0000	OK	B
SB21	Span 8	Middle		0.000	0.0000	OK	B
SB21	Span 8	End		0.000	0.0000	OK	B
SB21	Span 9	Start		0.000	0.0000	OK	B
SB21	Span 9	Middle		0.000	0.0000	OK	B
SB21	Span 9	End		0.000	0.0000	OK	B
SB21	Span 10	Start		0.000	0.0000	OK	B
SB21	Span 10	Middle		0.000	0.0000	OK	B
SB21	Span 10	End	UDCONU2	1.394	0.0000	OK	B
SB21	Span 11	Start	UDCONU2	1.394	0.0000	OK	B
SB21	Span 11	Middle		0.000	0.0000	OK	B
SB21	Span 11	End	UDCONU2	2.572	0.0000	OK	B
SB21	Span 12	Start	UDCONU2	2.572	0.0000	OK	B
SB21	Span 12	Middle	UDCONU2	4.631	0.0000	OK	B
SB21	Span 12	End	UDCONU2	4.497	0.0000	OK	B
SB21	Span 13	Start	UDCONU2	4.497	0.0000	OK	B
SB21	Span 13	Middle	UDCONU2	5.700	0.0000	OK	B
SB21	Span 13	End	UDCONU2	3.650	0.0000	OK	B
SB21	Span 14	Start	UDCONU2	3.650	0.0000	OK	B
SB21	Span 14	Middle	UDCONU2	3.170	0.0000	OK	B
SB21	Span 14	End	UDCONU2	6.957	0.0000	OK	B
SB21	Span 15	Start	UDCONU2	6.957	0.0000	OK	B
SB21	Span 15	Middle		0.000	0.0000	OK	B
SB21	Span 15	End	UDCONU2	0.375	0.0000	OK	B
SB21	Span 16	Start	UDCONU2	0.375	0.0000	OK	B
SB21	Span 16	Middle		0.000	0.0000	OK	B
SB21	Span 16	End	UDCONU2	1.484	0.0000	OK	B
SB21	Span 17	Start	UDCONU2	1.484	0.0000	OK	B
SB21	Span 17	Middle	UDCONU2	5.548	0.0000	OK	B
SB21	Span 17	End	UDCONU2	5.548	0.0000	OK	B
SB21	Span 18	Start	UDCONU2	5.548	0.0000	OK	B
SB21	Span 18	Middle		0.000	0.0000	OK	B
SB21	Span 18	End	UDCONU2	4.313	0.0000	OK	B
SB21	Span 19	Start	UDCONU2	4.313	0.0000	OK	B
SB21	Span 19	Middle	UDCONU2	10.215	0.0000	OK	B
SB21	Span 19	End	UDCONU2	5.812	0.0000	OK	B
SB21	Span 20	Start	UDCONU2	5.812	0.0000	OK	B
SB21	Span 20	Middle	UDCONU2	2.195	0.0000	OK	B
SB21	Span 20	End	UDCONU2	7.514	0.0000	OK	B
SB21	Span 21	Start	UDCONU2	7.514	0.0000	OK	B
SB21	Span 21	Middle		0.000	0.0000	OK	B
SB21	Span 21	End	UDCONU2	7.514	0.0000	OK	B
SB21	Span 22	Start	UDCONU2	7.514	0.0000	OK	B
SB21	Span 22	Middle	UDCONU2	3.846	0.0000	OK	B
SB21	Span 22	End	UDCONU2	0.346	0.0000	OK	B
SB22	Span 1	Start		0.000	0.0000	OK	B
SB22	Span 1	Middle	UDCONU2	8.192	0.0000	OK	B
SB22	Span 1	End	UDCONU2	3.616	0.0000	OK	B
SB22	Span 2	Start	UDCONU2	3.616	0.0000	OK	B
SB22	Span 2	Middle	UDCONU2	13.174	0.0000	OK	B
SB22	Span 2	End	UDCONU2	12.473	0.0000	OK	B

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB22	Span 3	Start	UDCONU2	12.473	0.0000	OK	B
SB22	Span 3	Middle	UDCONU2	7.343	0.0000	OK	B
SB22	Span 3	End	UDCONU2	4.684	0.0000	OK	B
SB22	Span 4	Start	UDCONU2	4.684	0.0000	OK	B
SB22	Span 4	Middle	UDCONU2	9.934	0.0000	OK	B
SB22	Span 4	End	UDCONU2	12.269	0.0000	OK	B
SB22	Span 5	Start	UDCONU2	12.269	0.0000	OK	B
SB22	Span 5	Middle	UDCONU2	0.326	0.0000	OK	B
SB22	Span 5	End		0.000	0.0000	OK	B
SB22	Span 6	Start		0.000	0.0000	OK	B
SB22	Span 6	Middle		0.000	0.0000	OK	B
SB22	Span 6	End		0.000	0.0000	OK	B
SB22	Span 7	Start		0.000	0.0000	OK	B
SB22	Span 7	Middle		0.000	0.0000	OK	B
SB22	Span 7	End		0.000	0.0000	OK	B
SB22	Span 8	Start		0.000	0.0000	OK	B
SB22	Span 8	Middle		0.000	0.0000	OK	B
SB22	Span 8	End	UDCONU2	1.766	0.0000	OK	B
SB22	Span 9	Start	UDCONU2	1.766	0.0000	OK	B
SB22	Span 9	Middle	UDCONU2	3.467	0.0000	OK	B
SB22	Span 9	End	UDCONU2	5.166	0.0000	OK	B
SB22	Span 10	Start	UDCONU1	1.197	0.0000	OK	B
SB22	Span 10	Middle	UDCONU2	3.826	0.0000	OK	B
SB22	Span 10	End	UDCONU2	3.748	0.0000	OK	B
SB22	Span 11	Start	UDCONU2	3.748	0.0000	OK	B
SB22	Span 11	Middle	UDCONU2	1.630	0.0000	OK	B
SB22	Span 11	End	UDCONU2	2.342	0.0000	OK	B
SB22	Span 12	Start	UDCONU2	3.384	0.0000	OK	B
SB22	Span 12	Middle	UDCONU2	2.412	0.0000	OK	B
SB22	Span 12	End	UDCONU2	0.123	0.0000	OK	B
SB23	Span 1	Start		0.000	0.0000	OK	B
SB23	Span 1	Middle		0.000	0.0000	OK	B
SB23	Span 1	End	UDCONU2	0.444	0.0000	OK	B
SB23	Span 2	Start	UDCONU2	0.444	0.0000	OK	B
SB23	Span 2	Middle	UDCONU2	1.145	0.0000	OK	B
SB23	Span 2	End	UDCONU2	4.506	0.0000	OK	B
SB23	Span 3	Start	UDCONU2	4.506	0.0000	OK	B
SB23	Span 3	Middle		0.000	0.0000	OK	B
SB23	Span 3	End	UDCONU2	4.390	0.0000	OK	B
SB23	Span 4	Start	UDCONU2	4.390	0.0000	OK	B
SB23	Span 4	Middle	UDCONU2	25.502	0.0000	OK	B
SB23	Span 4	End	UDCONU2	11.411	0.0000	OK	B
SB23	Span 5	Start	UDCONU2	11.411	0.0000	OK	B
SB23	Span 5	Middle		0.000	0.0000	OK	B
SB23	Span 5	End	UDCONU2	5.132	0.0000	OK	B
SB23	Span 6	Start	UDCONU2	5.132	0.0000	OK	B
SB23	Span 6	Middle	UDCONU2	2.844	0.0000	OK	B
SB23	Span 6	End	UDCONU2	1.554	0.0000	OK	B
SB23	Span 7	Start	UDCONU2	1.554	0.0000	OK	B
SB23	Span 7	Middle	UDCONU2	3.524	0.0000	OK	B
SB23	Span 7	End	UDCONU2	5.853	0.0000	OK	B
SB23	Span 8	Start	UDCONU2	5.853	0.0000	OK	B
SB23	Span 8	Middle		0.000	0.0000	OK	B
SB23	Span 8	End	UDCONU2	1.335	0.0000	OK	B

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB23	Span 9	Start	UDCONU2	1.335	0.0000	OK	B
SB23	Span 9	Middle		0.000	0.0000	OK	B
SB23	Span 9	End	UDCONU1	3.505	0.0000	OK	B
SB23	Span 10	Start	UDCONU1	3.505	0.0000	OK	B
SB23	Span 10	Middle		0.000	0.0000	OK	B
SB23	Span 10	End		0.000	0.0000	OK	B
SB23	Span 11	Start		0.000	0.0000	OK	B
SB23	Span 11	Middle		0.000	0.0000	OK	B
SB23	Span 11	End		0.000	0.0000	OK	B
SB23	Span 12	Start		0.000	0.0000	OK	B
SB23	Span 12	Middle		0.000	0.0000	OK	B
SB23	Span 12	End		0.000	0.0000	OK	B
SB23	Span 13	Start		0.000	0.0000	OK	B
SB23	Span 13	Middle		0.000	0.0000	OK	B
SB23	Span 13	End		0.000	0.0000	OK	B
SB23	Span 14	Start		0.000	0.0000	OK	B
SB23	Span 14	Middle		0.000	0.0000	OK	B
SB23	Span 14	End		0.000	0.0000	OK	B
SB23	Span 15	Start		0.000	0.0000	OK	B
SB23	Span 15	Middle		0.000	0.0000	OK	B
SB23	Span 15	End		0.000	0.0000	OK	B
SB23	Span 16	Start		0.000	0.0000	OK	B
SB23	Span 16	Middle		0.000	0.0000	OK	B
SB23	Span 16	End		0.000	0.0000	OK	B
SB23	Span 17	Start		0.000	0.0000	OK	B
SB23	Span 17	Middle		0.000	0.0000	OK	B
SB23	Span 17	End		0.000	0.0000	OK	B
SB24	Span 1	Start		0.000	0.0000	OK	B
SB24	Span 1	Middle		0.000	0.0000	OK	B
SB24	Span 1	End	UDCONU2	0.411	0.0000	OK	B
SB24	Span 2	Start	UDCONU2	0.411	0.0000	OK	B
SB24	Span 2	Middle	UDCONU2	2.587	0.0000	OK	B
SB24	Span 2	End	UDCONU1	1.440	0.0000	OK	B
SB24	Span 3	Start	UDCONU1	1.440	0.0000	OK	B
SB24	Span 3	Middle		0.000	0.0000	OK	B
SB24	Span 3	End		0.000	0.0000	OK	B
SB24	Span 4	Start		0.000	0.0000	OK	B
SB24	Span 4	Middle		0.000	0.0000	OK	B
SB24	Span 4	End		0.000	0.0000	OK	B
SB24	Span 5	Start		0.000	0.0000	OK	B
SB24	Span 5	Middle		0.000	0.0000	OK	B
SB24	Span 5	End		0.000	0.0000	OK	B
SB24	Span 6	Start		0.000	0.0000	OK	B
SB24	Span 6	Middle		0.000	0.0000	OK	B
SB24	Span 6	End		0.000	0.0000	OK	B
SB24	Span 7	Start		0.000	0.0000	OK	B
SB24	Span 7	Middle		0.000	0.0000	OK	B
SB24	Span 7	End		0.000	0.0000	OK	B
SB24	Span 8	Start		0.000	0.0000	OK	B
SB24	Span 8	Middle		0.000	0.0000	OK	B
SB24	Span 8	End		0.000	0.0000	OK	B
SB24	Span 9	Start		0.000	0.0000	OK	B
SB24	Span 9	Middle		0.000	0.0000	OK	B
SB24	Span 9	End		0.000	0.0000	OK	B

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB24	Span 10	Start		0.000	0.0000	OK	B
SB24	Span 10	Middle		0.000	0.0000	OK	B
SB24	Span 10	End		0.000	0.0000	OK	B
SB24	Span 11	Start		0.000	0.0000	OK	B
SB24	Span 11	Middle		0.000	0.0000	OK	B
SB24	Span 11	End		0.000	0.0000	OK	B
SB24	Span 12	Start		0.000	0.0000	OK	B
SB24	Span 12	Middle		0.000	0.0000	OK	B
SB24	Span 12	End		0.000	0.0000	OK	B
SB24	Span 13	Start		0.000	0.0000	OK	B
SB24	Span 13	Middle		0.000	0.0000	OK	B
SB24	Span 13	End		0.000	0.0000	OK	B
SB25	Span 1	Start		0.000	0.0000	OK	B
SB25	Span 1	Middle	UDCONU2	8.916	0.0000	OK	B
SB25	Span 1	End	UDCONU2	2.839	0.0000	OK	B
SB25	Span 2	Start	UDCONU2	2.039	0.0000	OK	B
SB25	Span 2	Middle	UDCONU2	0.876	0.0000	OK	B
SB25	Span 2	End	UDCONU2	1.335	0.0000	OK	B
SB25	Span 3	Start	UDCONU2	1.335	0.0000	OK	B
SB25	Span 3	Middle	UDCONU2	2.511	0.0000	OK	B
SB25	Span 3	End	UDCONU1	2.863	0.0000	OK	B
SB25	Span 4	Start	UDCONU1	2.863	0.0000	OK	B
SB25	Span 4	Middle		0.000	0.0000	OK	B
SB25	Span 4	End	UDCONU2	0.158	0.0000	OK	B
SB25	Span 5	Start	UDCONU2	0.158	0.0000	OK	B
SB25	Span 5	Middle	UDCONU2	0.045	0.0000	OK	B
SB25	Span 5	End		0.000	0.0000	OK	B
SB25	Span 6	Start		0.000	0.0000	OK	B
SB25	Span 6	Middle		0.000	0.0000	OK	B
SB25	Span 6	End		0.000	0.0000	OK	B
SB25	Span 7	Start		0.000	0.0000	OK	B
SB25	Span 7	Middle		0.000	0.0000	OK	B
SB25	Span 7	End		0.000	0.0000	OK	B
SB25	Span 8	Start		0.000	0.0000	OK	B
SB25	Span 8	Middle		0.000	0.0000	OK	B
SB25	Span 8	End		0.000	0.0000	OK	B
SB25	Span 9	Start		0.000	0.0000	OK	B
SB25	Span 9	Middle		0.000	0.0000	OK	B
SB25	Span 9	End		0.000	0.0000	OK	B
SB25	Span 10	Start		0.000	0.0000	OK	B
SB25	Span 10	Middle		0.000	0.0000	OK	B
SB25	Span 10	End		0.000	0.0000	OK	B
SB25	Span 11	Start		0.000	0.0000	OK	B
SB25	Span 11	Middle		0.000	0.0000	OK	B
SB25	Span 11	End		0.000	0.0000	OK	B
SB26	Span 1	Start		0.000	0.0000	OK	B
SB26	Span 1	Middle	UDCONU2	9.574	0.0000	OK	B
SB26	Span 1	End	UDCONU2	9.571	0.0000	OK	B
SB26	Span 2	Start	UDCONU2	9.571	0.0000	OK	B
SB26	Span 2	Middle	UDCONU2	9.914	0.0000	OK	B
SB26	Span 2	End	UDCONU2	6.277	0.0000	OK	B
SB26	Span 3	Start	UDCONU2	6.277	0.0000	OK	B
SB26	Span 3	Middle	UDCONU2	3.072	0.0000	OK	B
SB26	Span 3	End	UDCONU2	3.429	0.0000	OK	B

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB26	Span 4	Start	UDCONU2	3.429	0.0000	OK	B
SB26	Span 4	Middle	UDCONU2	6.633	0.0000	OK	B
SB26	Span 4	End	UDCONU2	10.177	0.0000	OK	B
SB26	Span 5	Start	UDCONU2	10.177	0.0000	OK	B
SB26	Span 5	Middle		0.000	0.0000	OK	B
SB26	Span 5	End	UDCONU2	7.515	0.0000	OK	B
SB26	Span 6	Start	UDCONU2	7.515	0.0000	OK	B
SB26	Span 6	Middle	UDCONU2	2.630	0.0000	OK	B
SB26	Span 6	End		0.000	0.0000	OK	B
SB26	Span 7	Start		0.000	0.0000	OK	B
SB26	Span 7	Middle		0.000	0.0000	OK	B
SB26	Span 7	End		0.000	0.0000	OK	B
SB26	Span 8	Start		0.000	0.0000	OK	B
SB26	Span 8	Middle		0.000	0.0000	OK	B
SB26	Span 8	End		0.000	0.0000	OK	B
SB26	Span 9	Start		0.000	0.0000	OK	B
SB26	Span 9	Middle		0.000	0.0000	OK	B
SB26	Span 9	End		0.000	0.0000	OK	B
SB26	Span 10	Start		0.000	0.0000	OK	B
SB26	Span 10	Middle		0.000	0.0000	OK	B
SB26	Span 10	End		0.000	0.0000	OK	B
SB26	Span 11	Start		0.000	0.0000	OK	B
SB26	Span 11	Middle		0.000	0.0000	OK	B
SB26	Span 11	End		0.000	0.0000	OK	B
SB26	Span 12	Start		0.000	0.0000	OK	B
SB26	Span 12	Middle		0.000	0.0000	OK	B
SB26	Span 12	End		0.000	0.0000	OK	B
SB27	Span 1	Start		0.000	0.0000	OK	B
SB27	Span 1	Middle	UDCONU2	9.734	0.0000	OK	B
SB27	Span 1	End	UDCONU2	10.877	0.0000	OK	B
SB27	Span 2	Start	UDCONU2	7.434	0.0000	OK	B
SB27	Span 2	Middle	UDCONU2	3.850	0.0000	OK	B
SB27	Span 2	End	UDCONU2	2.655	0.0000	OK	B
SB27	Span 3	Start	UDCONU2	2.655	0.0000	OK	B
SB27	Span 3	Middle	UDCONU2	5.189	0.0000	OK	B
SB27	Span 3	End	UDCONU2	6.189	0.0000	OK	B
SB27	Span 4	Start	UDCONU2	6.189	0.0000	OK	B
SB27	Span 4	Middle		0.000	0.0000	OK	B
SB27	Span 4	End	UDCONU2	3.916	0.0000	OK	B
SB27	Span 5	Start	UDCONU2	3.916	0.0000	OK	B
SB27	Span 5	Middle	UDCONU2	1.653	0.0000	OK	B
SB27	Span 5	End		0.000	0.0000	OK	B
SB27	Span 6	Start		0.000	0.0000	OK	B
SB27	Span 6	Middle		0.000	0.0000	OK	B
SB27	Span 6	End		0.000	0.0000	OK	B
SB27	Span 7	Start		0.000	0.0000	OK	B
SB27	Span 7	Middle		0.000	0.0000	OK	B
SB27	Span 7	End		0.000	0.0000	OK	B
SB27	Span 8	Start		0.000	0.0000	OK	B
SB27	Span 8	Middle		0.000	0.0000	OK	B
SB27	Span 8	End		0.000	0.0000	OK	B
SB27	Span 9	Start		0.000	0.0000	OK	B
SB27	Span 9	Middle		0.000	0.0000	OK	B
SB27	Span 9	End		0.000	0.0000	OK	B

Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

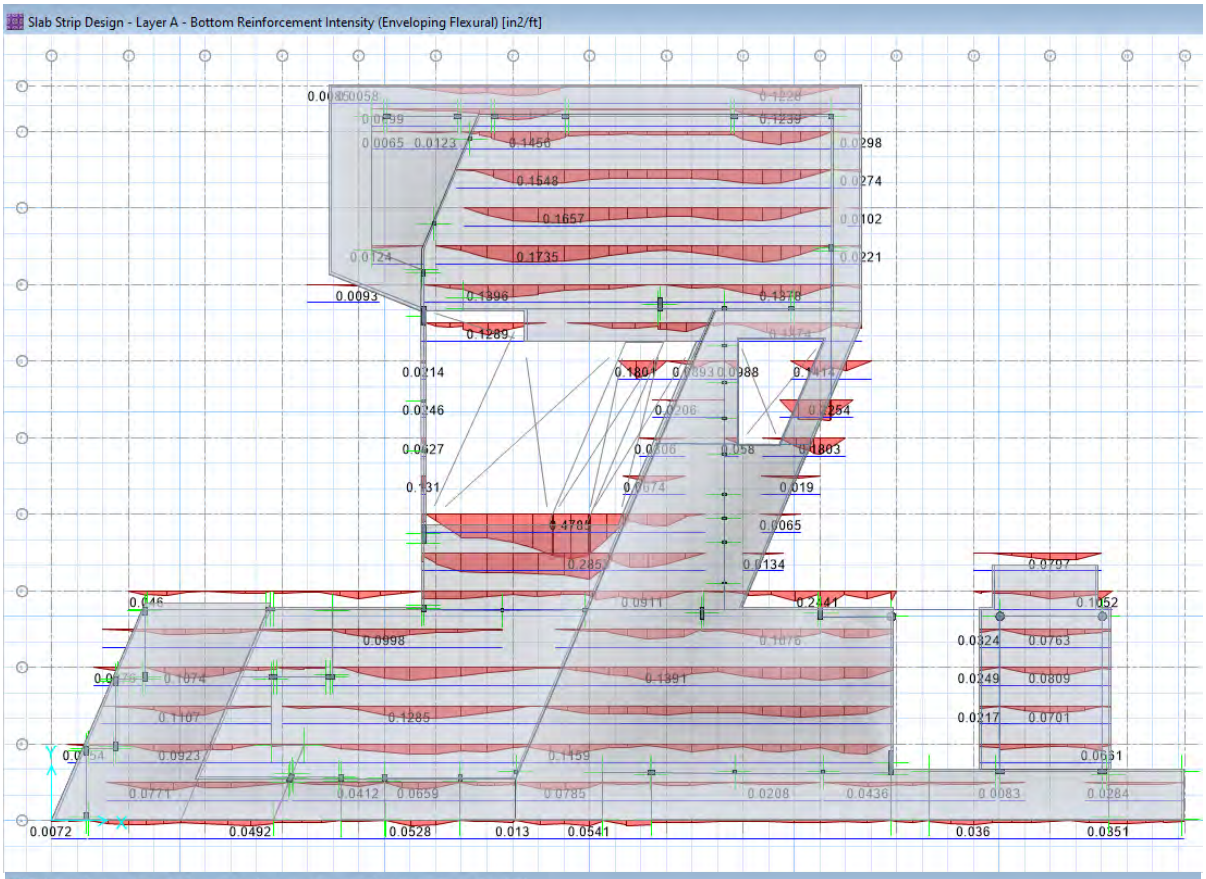
Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB27	Span 10	Start		0.000	0.0000	OK	B
SB27	Span 10	Middle		0.000	0.0000	OK	B
SB27	Span 10	End		0.000	0.0000	OK	B
SB27	Span 11	Start		0.000	0.0000	OK	B
SB27	Span 11	Middle		0.000	0.0000	OK	B
SB27	Span 11	End		0.000	0.0000	OK	B
SB28	Span 1	Start		0.000	0.0000	OK	B
SB28	Span 1	Middle	UDCONU2	9.786	0.0000	OK	B
SB28	Span 1	End	UDCONU2	14.749	0.0000	OK	B
SB28	Span 2	Start	UDCONU2	14.749	0.0000	OK	B
SB28	Span 2	Middle	UDCONU2	9.502	0.0000	OK	B
SB28	Span 2	End	UDCONU2	6.330	0.0000	OK	B
SB28	Span 3	Start	UDCONU2	6.330	0.0000	OK	B
SB28	Span 3	Middle	UDCONU2	3.271	0.0000	OK	B
SB28	Span 3	End	UDCONU2	3.138	0.0000	OK	B
SB28	Span 4	Start	UDCONU2	3.138	0.0000	OK	B
SB28	Span 4	Middle	UDCONU2	5.183	0.0000	OK	B
SB28	Span 4	End	UDCONU2	8.958	0.0000	OK	B
SB28	Span 5	Start	UDCONU2	8.958	0.0000	OK	B
SB28	Span 5	Middle		0.000	0.0000	OK	B
SB28	Span 5	End	UDCONU2	4.940	0.0000	OK	B
SB28	Span 6	Start	UDCONU2	4.940	0.0000	OK	B
SB28	Span 6	Middle	UDCONU2	1.695	0.0000	OK	B
SB28	Span 6	End		0.000	0.0000	OK	B
SB28	Span 7	Start		0.000	0.0000	OK	B
SB28	Span 7	Middle		0.000	0.0000	OK	B
SB28	Span 7	End		0.000	0.0000	OK	B
SB28	Span 8	Start		0.000	0.0000	OK	B
SB28	Span 8	Middle		0.000	0.0000	OK	B
SB28	Span 8	End		0.000	0.0000	OK	B
SB28	Span 9	Start		0.000	0.0000	OK	B
SB28	Span 9	Middle		0.000	0.0000	OK	B
SB28	Span 9	End		0.000	0.0000	OK	B
SB28	Span 10	Start		0.000	0.0000	OK	B
SB28	Span 10	Middle		0.000	0.0000	OK	B
SB28	Span 10	End		0.000	0.0000	OK	B
SB28	Span 11	Start		0.000	0.0000	OK	B
SB28	Span 11	Middle		0.000	0.0000	OK	B
SB28	Span 11	End		0.000	0.0000	OK	B
SB28	Span 12	Start		0.000	0.0000	OK	B
SB28	Span 12	Middle		0.000	0.0000	OK	B
SB28	Span 12	End		0.000	0.0000	OK	B
SB29	Span 1	Start		0.000	0.0000	OK	B
SB29	Span 1	Middle	UDCONU2	0.342	0.0000	OK	B
SB29	Span 1	End	UDCONU2	2.817	0.0000	OK	B
SB29	Span 2	Start	UDCONU2	2.817	0.0000	OK	B
SB29	Span 2	Middle	UDCONU2	8.238	0.0000	OK	B
SB29	Span 2	End	UDCONU2	8.238	0.0000	OK	B
SB29	Span 3	Start	UDCONU2	8.238	0.0000	OK	B
SB29	Span 3	Middle	UDCONU2	0.613	0.0000	OK	B
SB29	Span 3	End	UDCONU2	0.474	0.0000	OK	B
SB29	Span 4	Start	UDCONU2	0.474	0.0000	OK	B
SB29	Span 4	Middle	UDCONU2	0.188	0.0000	OK	B
SB29	Span 4	End	UDCONU2	0.260	0.0000	OK	B

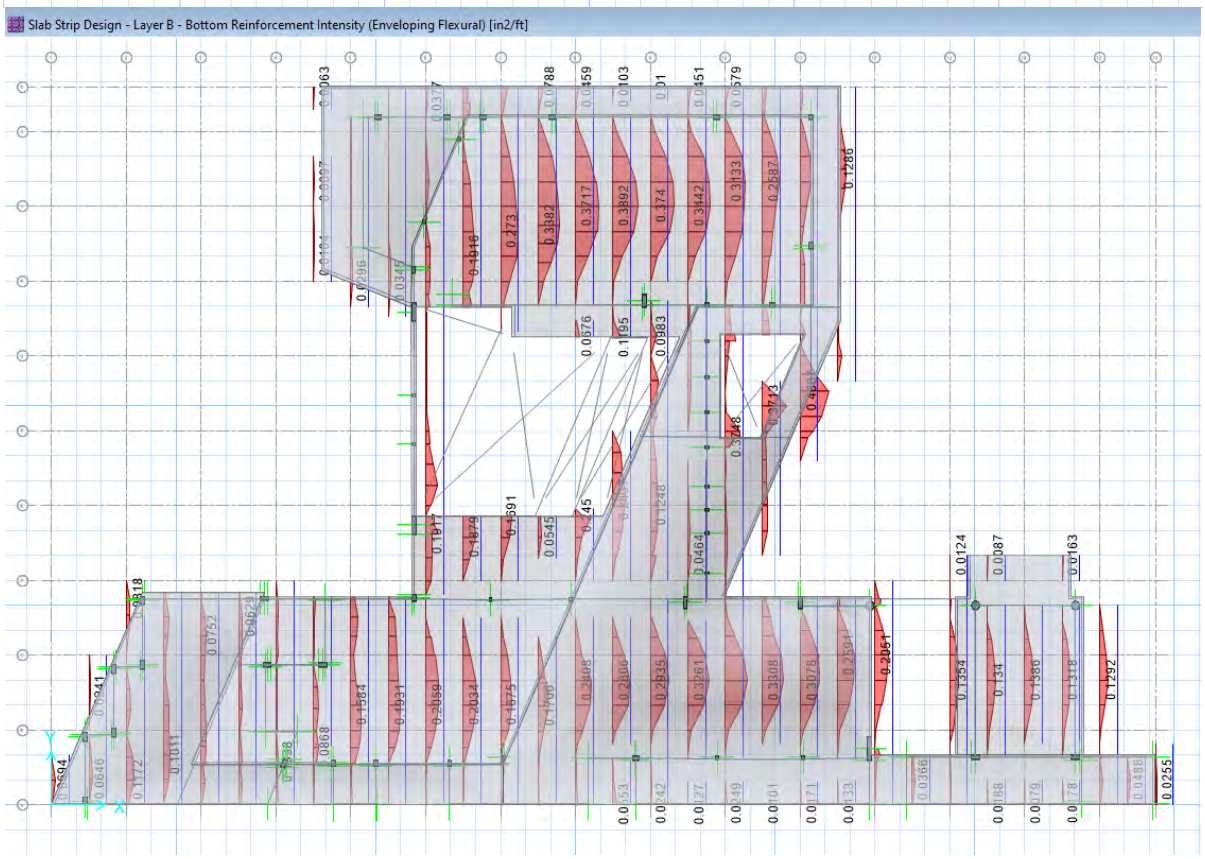
Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

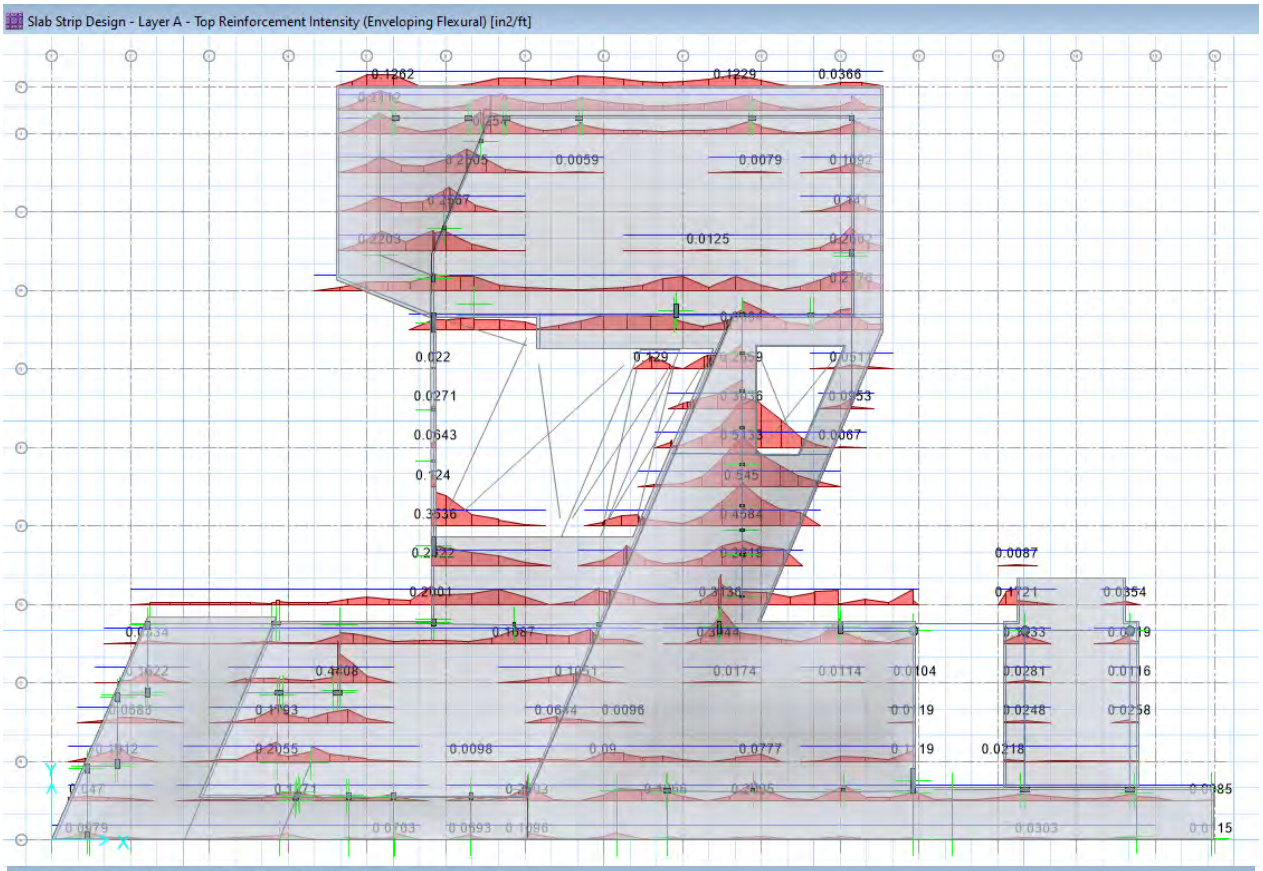
Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB29	Span 5	Start	UDCONU2	0.260	0.0000	OK	B
SB29	Span 5	Middle	UDCONU2	0.378	0.0000	OK	B
SB29	Span 5	End		0.000	0.0000	OK	B
SB29	Span 6	Start		0.000	0.0000	OK	B
SB29	Span 6	Middle		0.000	0.0000	OK	B
SB29	Span 6	End		0.000	0.0000	OK	B
SB29	Span 7	Start		0.000	0.0000	OK	B
SB29	Span 7	Middle		0.000	0.0000	OK	B
SB29	Span 7	End		0.000	0.0000	OK	B
SB29	Span 8	Start		0.000	0.0000	OK	B
SB29	Span 8	Middle		0.000	0.0000	OK	B
SB29	Span 8	End		0.000	0.0000	OK	B
SB29	Span 9	Start		0.000	0.0000	OK	B
SB29	Span 9	Middle		0.000	0.0000	OK	B
SB29	Span 9	End		0.000	0.0000	OK	B
SB29	Span 10	Start		0.000	0.0000	OK	B
SB29	Span 10	Middle		0.000	0.0000	OK	B
SB29	Span 10	End		0.000	0.0000	OK	B
SB29	Span 11	Start		0.000	0.0000	OK	B
SB29	Span 11	Middle		0.000	0.0000	OK	B
SB29	Span 11	End		0.000	0.0000	OK	B
SB29	Span 12	Start		0.000	0.0000	OK	B
SB29	Span 12	Middle		0.000	0.0000	OK	B
SB29	Span 12	End		0.000	0.0000	OK	B
SB30	Span 1	Start		0.000	0.0000	OK	B
SB30	Span 1	Middle	UDCONU2	3.715	0.0000	OK	B
SB30	Span 1	End		0.000	0.0000	OK	B
SB30	Span 2	Start		0.000	0.0000	OK	B
SB30	Span 2	Middle		0.000	0.0000	OK	B
SB30	Span 2	End		0.000	0.0000	OK	B
SB30	Span 3	Start		0.000	0.0000	OK	B
SB30	Span 3	Middle		0.000	0.0000	OK	B
SB30	Span 3	End		0.000	0.0000	OK	B
SB30	Span 4	Start		0.000	0.0000	OK	B
SB30	Span 4	Middle		0.000	0.0000	OK	B
SB30	Span 4	End		0.000	0.0000	OK	B
SB30	Span 5	Start		0.000	0.0000	OK	B
SB30	Span 5	Middle		0.000	0.0000	OK	B
SB30	Span 5	End		0.000	0.0000	OK	B
SB30	Span 6	Start		0.000	0.0000	OK	B
SB30	Span 6	Middle		0.000	0.0000	OK	B
SB30	Span 6	End		0.000	0.0000	OK	B
SB30	Span 7	Start		0.000	0.0000	OK	B
SB30	Span 7	Middle		0.000	0.0000	OK	B
SB30	Span 7	End		0.000	0.0000	OK	B
SB30	Span 8	Start		0.000	0.0000	OK	B
SB30	Span 8	Middle		0.000	0.0000	OK	B
SB30	Span 8	End		0.000	0.0000	OK	B
SB30	Span 9	Start		0.000	0.0000	OK	B
SB30	Span 9	Middle		0.000	0.0000	OK	B
SB30	Span 9	End		0.000	0.0000	OK	B
SB30	Span 10	Start		0.000	0.0000	OK	B
SB30	Span 10	Middle		0.000	0.0000	OK	B
SB30	Span 10	End		0.000	0.0000	OK	B

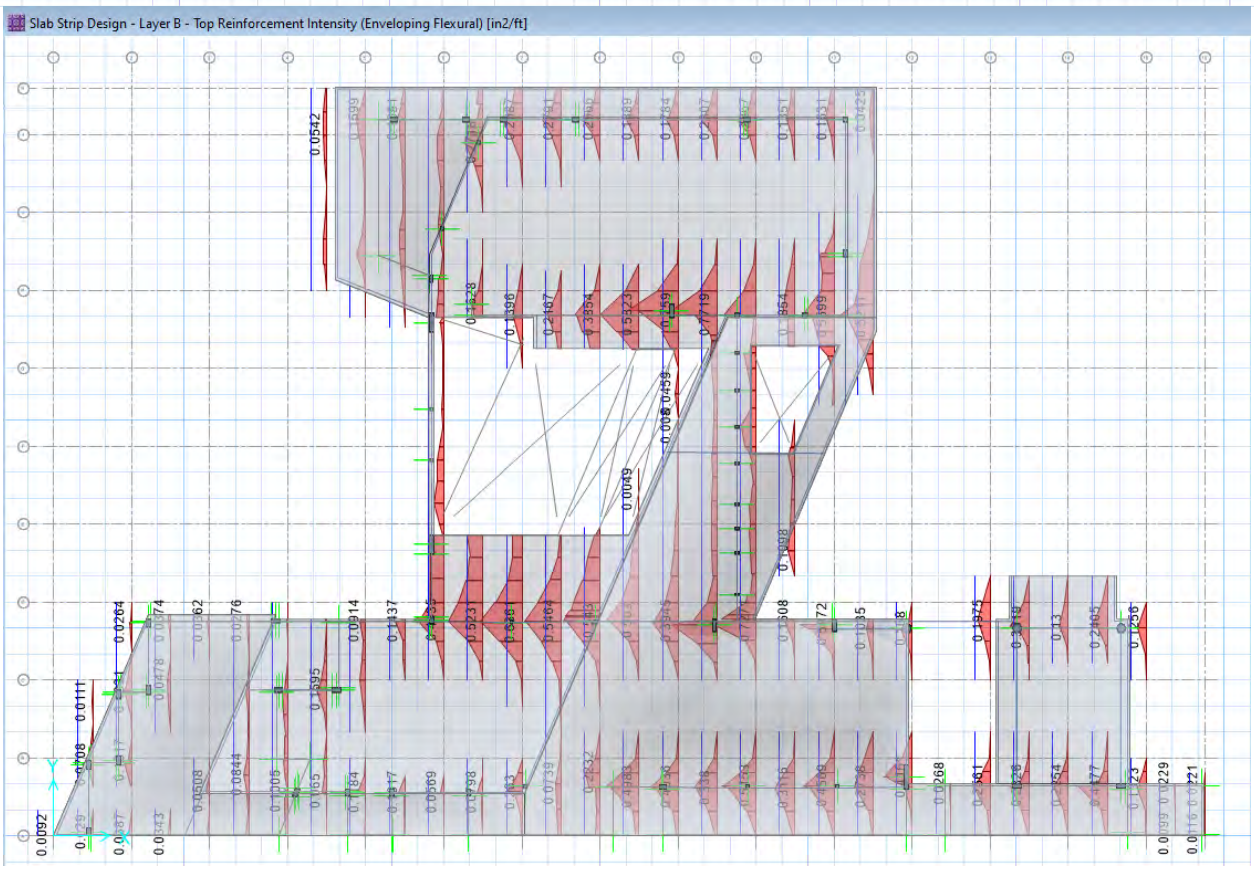
Table 42: Concrete Slab Design Summary 01 - Flexural And Shear Data, Part 3 of 3

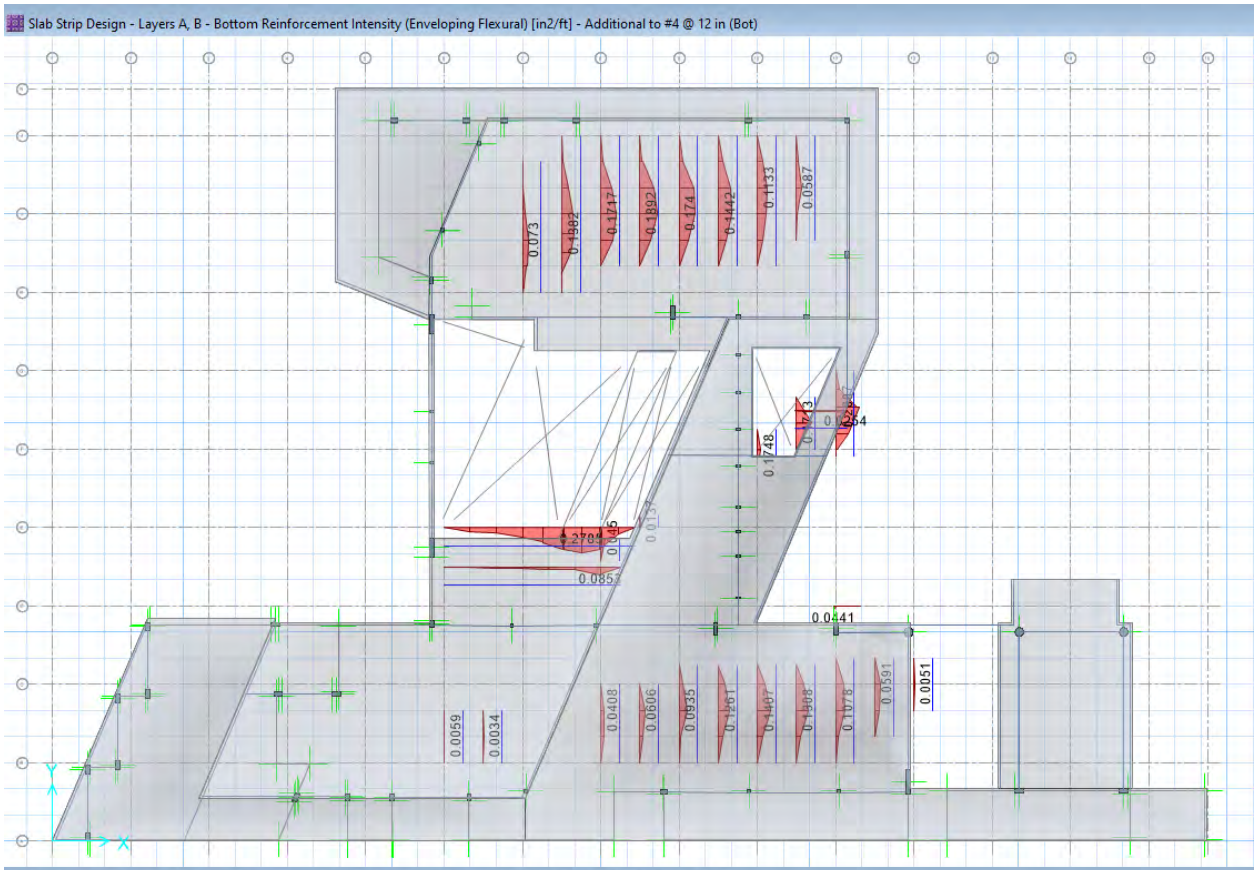
Strip	SpanID	Location	VCombo	VForce	VArea	Status	Layer
				kip	in2/ft		
SB31	Span 1	Start	UDCONU2	0.171	0.0000	OK	B
SB31	Span 1	Middle	UDCONU2	1.027	0.0000	OK	B
SB31	Span 1	End	UDCONU2	1.027	0.0000	OK	B
SB31	Span 2	Start	UDCONU2	1.027	0.0000	OK	B
SB31	Span 2	Middle		0.000	0.0000	OK	B
SB31	Span 2	End		0.000	0.0000	OK	B
SB31	Span 3	Start		0.000	0.0000	OK	B
SB31	Span 3	Middle		0.000	0.0000	OK	B
SB31	Span 3	End		0.000	0.0000	OK	B
SB31	Span 4	Start		0.000	0.0000	OK	B
SB31	Span 4	Middle		0.000	0.0000	OK	B
SB31	Span 4	End		0.000	0.0000	OK	B
SB31	Span 5	Start		0.000	0.0000	OK	B
SB31	Span 5	Middle		0.000	0.0000	OK	B
SB31	Span 5	End		0.000	0.0000	OK	B
SB31	Span 6	Start		0.000	0.0000	OK	B
SB31	Span 6	Middle		0.000	0.0000	OK	B
SB31	Span 6	End		0.000	0.0000	OK	B
SB31	Span 7	Start		0.000	0.0000	OK	B
SB31	Span 7	Middle		0.000	0.0000	OK	B
SB31	Span 7	End		0.000	0.0000	OK	B
SB31	Span 8	Start		0.000	0.0000	OK	B
SB31	Span 8	Middle		0.000	0.0000	OK	B
SB31	Span 8	End		0.000	0.0000	OK	B
SB31	Span 9	Start		0.000	0.0000	OK	B
SB31	Span 9	Middle		0.000	0.0000	OK	B
SB31	Span 9	End		0.000	0.0000	OK	B
SB31	Span 10	Start		0.000	0.0000	OK	B
SB31	Span 10	Middle		0.000	0.0000	OK	B
SB31	Span 10	End		0.000	0.0000	OK	B
SB31	Span 11	Start		0.000	0.0000	OK	B
SB31	Span 11	Middle		0.000	0.0000	OK	B
SB31	Span 11	End		0.000	0.0000	OK	B

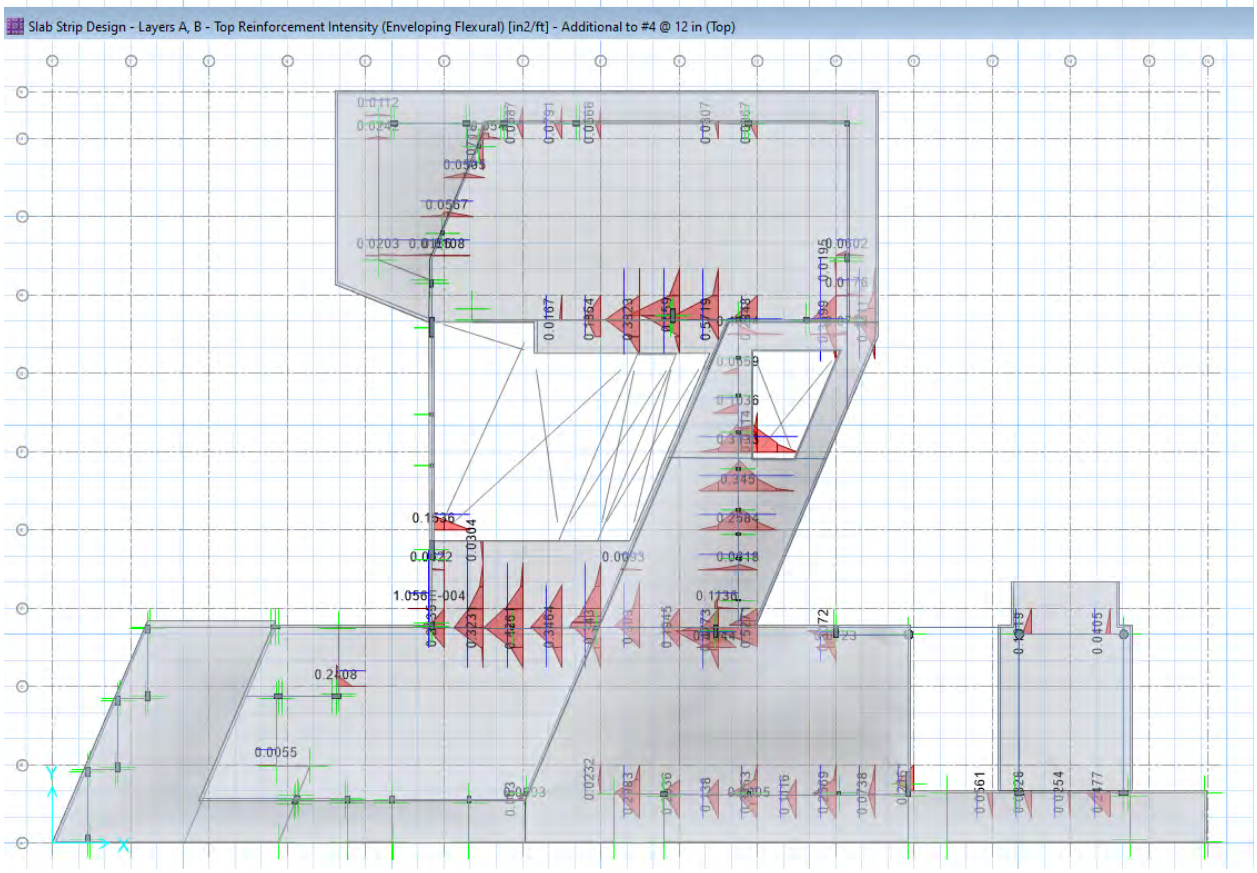

Slab Strip Design - Layer A - Bottom Reinforcement Intensity (Enveloping Flexural) [in2/ft]


Slab Strip Design - Layer B - Bottom Reinforcement Intensity (Enveloping Flexural) [in2/ft]


Slab Strip Design - Layer A - Top Reinforcement Intensity (Enveloping Flexural) [in2/ft]


Slab Strip Design - Layer B - Top Reinforcement Intensity (Enveloping Flexural) [in2/ft]


Slab Strip Design - Layers A, B - Bottom Reinforcement Intensity (Enveloping Flexural) [in2/ft] - Additional to #4 @ 12 in (Bot)


Slab Strip Design - Layers A, B - Top Reinforcement Intensity (Enveloping Flexural) [in2/ft] - Additional to #4 @ 12 in (Top)

6.4. Punching check/design

Table 44: Concrete Slab Design 02 - Punching Shear Data, Part 1 of 3

Table 44: Concrete Slab Design 02 - Punching Shear Data, Part 1 of 3

Point	GlobalX	GlobalY	Location	Perimeter	Depth	Status	Ratio
	ft	ft		in	in		
2	131.1655	32.0041				Not Calculated	
4	148.1512	32.0278				Not Calculated	
6	164.1512	32.0278				Not Calculated	
16	164.1136	7.6803				Not Calculated	
18	148.1512	7.6803				Not Calculated	
20	131.1594	9.1381				Not Calculated	
24	120.0760	32.5041				Not Calculated	
26	101.5960	32.5041				Not Calculated	
30	93.6655	7.5041				Not Calculated	
32	63.8260	6.6667				Not Calculated	
34	51.9927	6.6667				Not Calculated	
36	45.1594	6.6945				Not Calculated	
40	37.2471	6.7082				Not Calculated	
42	43.5007	22.5041				Not Calculated	
44	34.5741	22.5041				Not Calculated	
46	14.5760	32.8333				Not Calculated	
48	14.5760	22.5437				Not Calculated	
50	9.9927	21.9187				Not Calculated	
52	9.9927	11.6270				Not Calculated	
54	5.3260	11.0020				Not Calculated	
56	5.3260	0.6667				Not Calculated	
58	34.0927	33.1667				Not Calculated	
60	58.0729	33.3314				Not Calculated	
66	58.0729	44.9179				Not Calculated	
68	58.0729	79.1010				Not Calculated	
70	58.0524	85.7861				Not Calculated	

Table 44: Concrete Slab Design 02 - Punching Shear Data, Part 1 of 3

Point	GlobalX	GlobalY	Location	Perimeter	Depth	Status	Ratio
	ft	ft		in	in		
72	52.4113	110.3612				Not Calculated	
74	63.4075	110.3565				Not Calculated	
76	69.2446	110.3510				Not Calculated	
78	80.2408	110.3510				Not Calculated	
80	106.5780	110.3374				Not Calculated	
82	121.7427	110.3374				Not Calculated	
84	121.7427	89.7519				Not Calculated	
86	115.5394	80.2500				Not Calculated	
92	94.9932	80.9335				Not Calculated	
94	105.0760	80.2526				Not Calculated	
96	59.7080	93.5237				Not Calculated	
98	65.3260	106.7793				Not Calculated	
100	58.0729	65.7093				Not Calculated	
102	58.0760	57.9599				Not Calculated	
104	105.0760	68.6707				Not Calculated	
106	105.0760	57.4445				Not Calculated	
108	105.0760	51.0874				Not Calculated	
110	105.0760	43.5874				Not Calculated	
112	105.0760	37.2541				Not Calculated	
114	70.3808	33.0000				Not Calculated	
116	83.2741	33.0041				Not Calculated	
118	106.7603	7.6945				Not Calculated	
120	120.4927	7.6945				Not Calculated	
122	72.5177	7.6660				Not Calculated	
309	105.0760	47.3374				Not Calculated	
315	105.0760	74.4617				Not Calculated	
317	105.0760	63.0576				Not Calculated	

Table 44: Concrete Slab Design 02 - Punching Shear Data, Part 2 of 3

Table 44: Concrete Slab Design 02 - Punching Shear Data, Part 2 of 3

Point	Combo	Vu	Gamma_v2	Gamma_v3	ShrStrMax	Mu2	ShrStrCap
		kip			kip/in2	kip-ft	kip/in2
2							
4							
6							
16							
18							
20							
24							
26							
30							
32							
34							
36							
40							
42							
44							
46							
48							
50							
52							
54							
56							
58							
60							
66							
68							
70							
72							
74							
76							
78							
80							
82							
84							
86							
92							
94							
96							
98							
100							
102							
104							
106							
108							
110							
112							
114							
116							
118							
120							
122							
309							
315							

Table 44: Concrete Slab Design 02 - Punching Shear Data, Part 2 of 3

Point	Combo	Vu	Gamma_v2	Gamma_v3	ShrStrMax	Mu2	ShrStrCap
		kip			kip/in2	kip-ft	kip/in2
317							

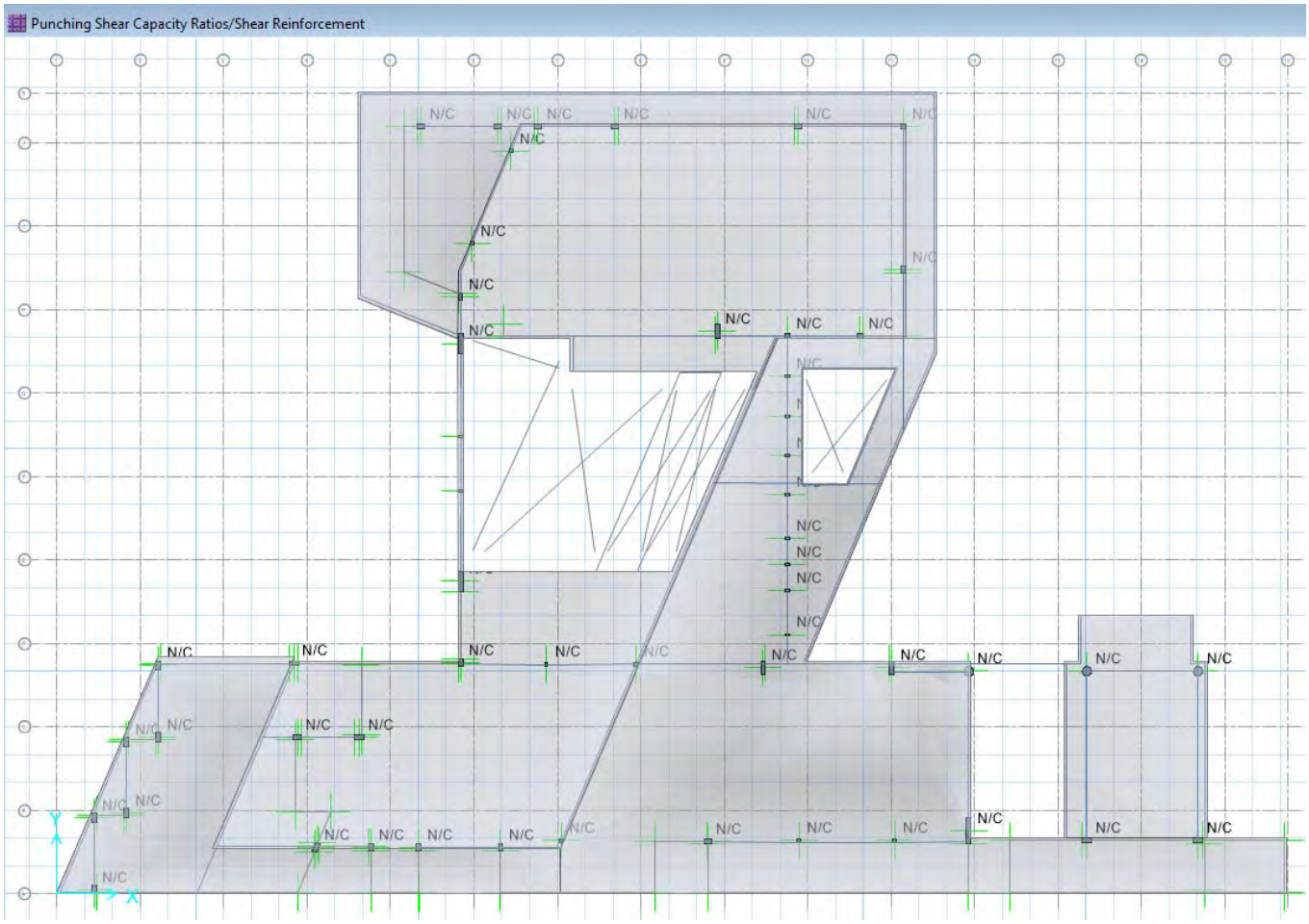
Table 44: Concrete Slab Design 02 - Punching Shear Data, Part 3 of 3

Table 44: Concrete Slab Design 02 - Punching Shear Data, Part 3 of 3

Point	Mu3	ReinfType	UnbalMu2	UnbalMu3	NumRails	StudPerRail
	kip-ft		kip-ft	kip-ft		
2		None				
4		None				
6		None				
16		None				
18		None				
20		None				
24		None				
26		None				
30		None				
32		None				
34		None				
36		None				
40		None				
42		None				
44		None				
46		None				
48		None				
50		None				
52		None				
54		None				
56		None				
58		None				
60		None				
66		None				
68		None				
70		None				
72		None				
74		None				
76		None				
78		None				
80		None				
82		None				
84		None				
86		None				
92		None				
94		None				
96		None				
98		None				
100		None				
102		None				
104		None				
106		None				
108		None				
110		None				
112		None				
114		None				

Table 44: Concrete Slab Design 02 - Punching Shear Data, Part 3 of 3

Point	Mu3	ReinfType	UnbalMu2	UnbalMu3	NumRails	StudPerRail
	kip-ft		kip-ft	kip-ft		
116		None				
118		None				
120		None				
122		None				
309		None				
315		None				
317		None				


Punching Shear Capacity Ratios/Shear Reinforcement
N/C

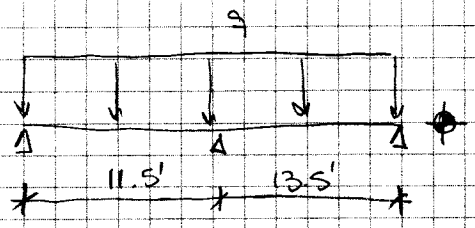
CONCRETE BEAMS DESIGN

(ACI 318-14)

Ph: (305) 710-2987 Email: 3aengdrafting@gmail.com

Concrete Beams Design:

→ RB-1 (8" x 24")

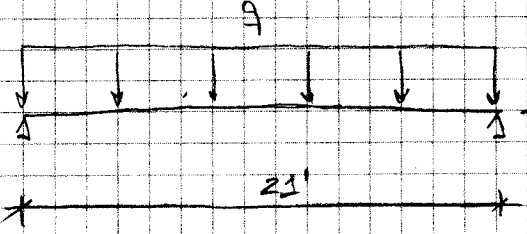


$q \begin{cases} DL_{Roof} = 150\ psf \times 0.83' = 125\ psf \\ DL_{SDL} = 25\ psf \\ LL_{Roof} = 30\ psf \end{cases}$ 180 psf

Trib Width = 17'.

B (2#6), T (2#6), I (1#5),
□ #3 - 5@8" E.S. & Bal 10" *

→ RB-2 (8" x 36")



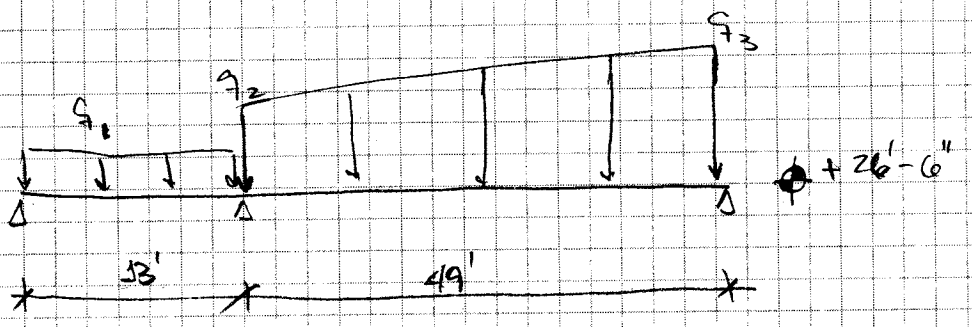
$q \begin{cases} DL_{Roof} + DL_{SDL} = 150\ psf \\ LL_{Roof} = 30\ psf \end{cases}$

Trib. Width = 21.4'

B (2#7, 2#7), T (2#7)
I (2#5), □ #3 - 12@6" E.S. & Bal 12" *

* See calculation report

2. RB-3 (12" x 42") Up-turned Beam.



q_1 $\begin{cases} DL_{roof} + ph = 150\ psf \times 10.5' + 150\ pcf \times 0.67' \times 1.2' = 1696\ plf \\ LL_{Roof} = 30\ psf \times 10.5' = 406\ plf \end{cases}$

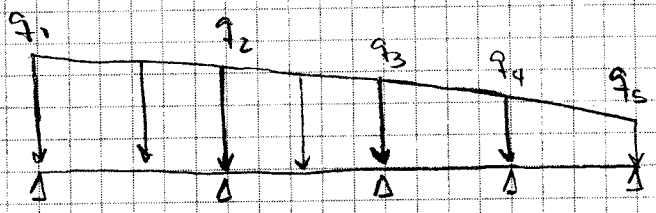
q_2 $\begin{cases} DL_{roof} + ph = 150\ pcf \times 9.2' + 323\ plf = 1503\ plf \\ LL_{Roof} = 30\ psf \times 9.2' = 276\ plf \end{cases}$

q_3 $\begin{cases} DL_{roof} + ph = 150\ pcf \times 12.83' + 323\ plf = 2046\ plf \\ LL_{Roof} = 30\ psf \times 12.83' = 385\ plf \end{cases}$

B(4 #8), T(4 #8),
I(3#8), "□ #3@10" *

* See calculation report

→ RB-4 (8" x 20")



q_1 { $DL_{Roof+ph}$ = 150 psf x 28.67' + 121 plf = 4422 plf
LL_{Roof} = 30 psf x 28.67' = 861 plf

q_2 { $DL_{Roof+ph}$ = 150 psf x 25.5' + 121 plf = 3946 plf
LL_{Roof} = 30 psf x 25.5' = 765 plf

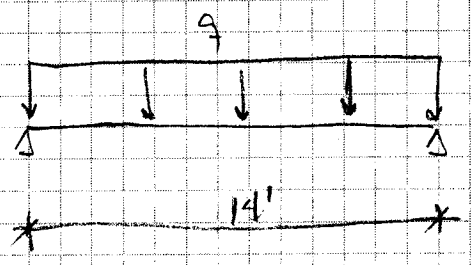
q_3 { $DL_{Roof+ph}$ = 150 psf x 23.2' + 121 plf = 3601 plf
LL_{Roof} = 30 psf x 23.2' = 696 plf

q_4 { $DL_{Roof+ph}$ = 150 psf x 19.5' + 121 plf = 3046 plf
LL_{Roof} = 30 psf x 19.5' = 585 plf

q_5 { $DL_{Roof+ph}$ = 150 psf x 15.58' + 121 plf = 2458 plf
LL_{Roof} = 30 psf x 15.58' = 468 plf

B(2#6), T(2#6)
□#3@8" *

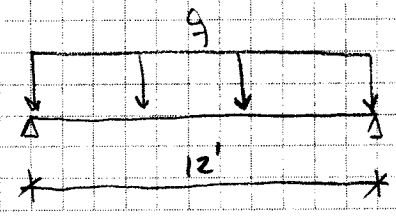
→ RB-5 (8" x 18")



q { $DL_{Roof+SDL}$ = 150 psf x 18' + 121 plf = 2821 plf
LL_{Roof} = 30 psf x 18' = 540 plf

B(2#7, 2#7), T(2#7), □#3@7" *

→ RB-6 (8" x 18")

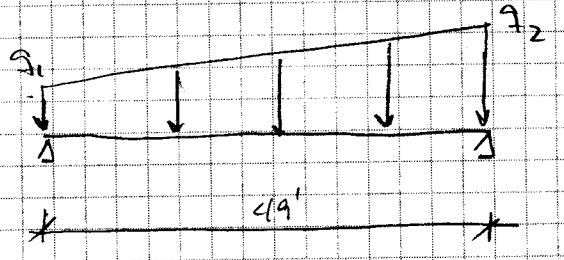


q { $DL_{Roof+SDL}$ = 150 psf x 13.2' + 121 plf = 2101 plf
LL_{Roof} = 30 psf x 13.2' = 396 plf

B(2#7), T(2#7), □#3@7" *

* See calculation report

→ RB-7 (16" x <12") Up-turned Beam



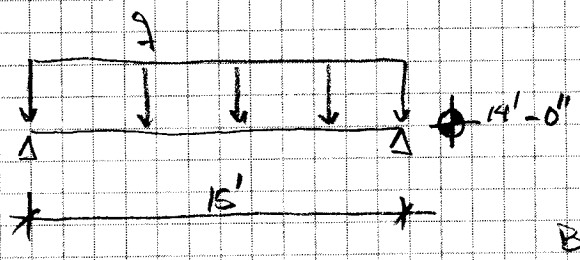
q_1 { DL roof +SDL = 150 psf x 12.33' = 1850 plf
LL roof = 30 psf x 12.33' = 370 plf

q_2 { DL roof +SDL = 150 psf x 22.33' = 3350 plf
LL roof = 30 psf x 22.33' = 670 plf

B (6 #8, 6 #8) , T (6 #8)
I (3 #5)
[] #3 @ 12"

*

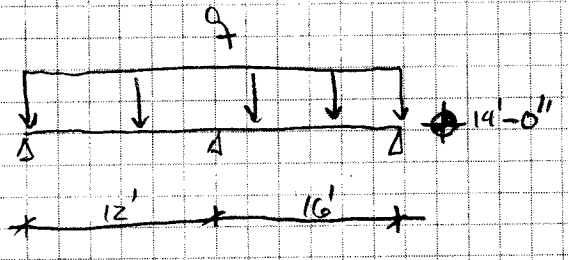
→ RB-8 (8" x 20")



$$\begin{cases} DL_{Roof} + SDL = 101\ psf \times (8.33' + 1') + 25\ psf \times 8.33' \\ \qquad = 1172\ plf \\ LL_{Roof} = 30\ psf \times 8.33' = 250\ plf \end{cases}$$

B(2#6), T(2#6), □ #3@8" *

→ RB-9 (8" x 20")



$$9 \begin{cases} DL_{Roof} + SDL = 101\ psf \times (11' + 1') + 25\ psf \times 11' \\ \qquad = 1487\ plf \\ LL_{Roof} = 30\ psf \times 11' = 330\ plf \end{cases}$$

B(2#7), T(2#7), □ #3@8" *

* See calculation report

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

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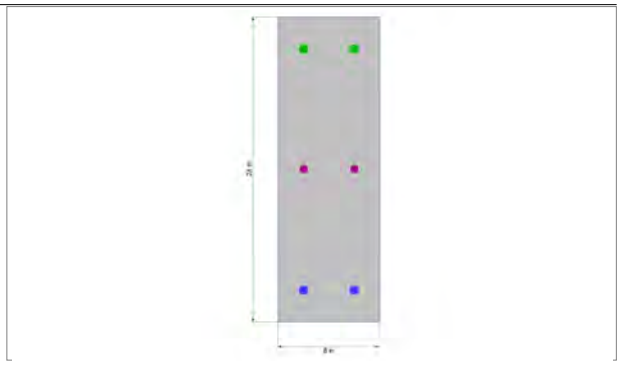
DESCRIPTION: RB-1 (8"x24")

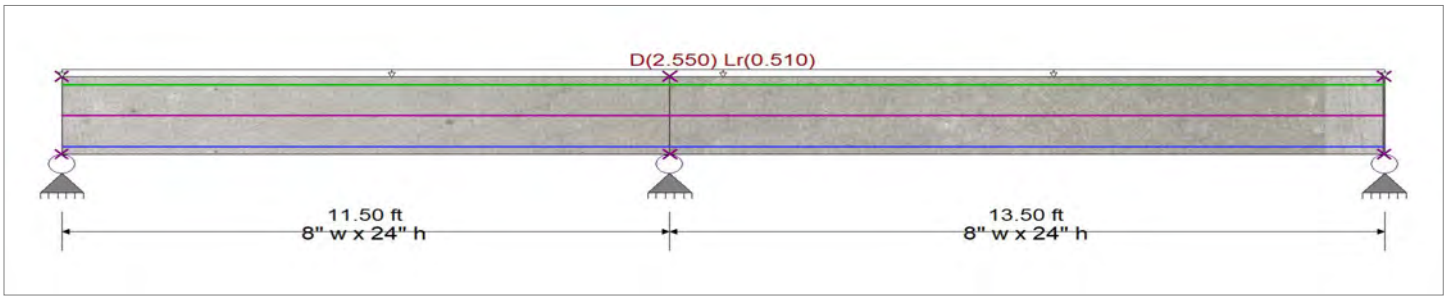
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ *7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2





Cross Section & Reinforcing Details

Rectangular Section, Width = 8.0 in, Height = 24.0 in

Span #1 Reinforcing....
- 2-#6 at 2.50 in from Bottom, from 0.0 to 11.50 ft in this span
- 2-#6 at 2.50 in from Top, from 0.0 to 11.50 ft in this spa
- 2-#5 at 12.0 in from Bottom, from 0.0 to 11.50 ft in this span

Span #2 Reinforcing....
- 2-#6 at 2.50 in from Bottom, from 0.0 to 13.50 ft in this span
- 2-#6 at 2.50 in from Top, from 0.0 to 13.50 ft in this spa
- 2-#5 at 12.0 in from Bottom, from 0.0 to 13.50 ft in this span

Beam self weight calculated and added to loads
Loads on all spans...
D = 0.150, Lr = 0.030

Uniform Load on ALL spans : D = 0.150, Lr = 0.030 ksf, Tributary Width = 17.0 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio = **0.746** : 1

Section used for this span	**Typical Section**	
Mu : Applied	-81.775	k-ft
Mn * Phi : Allowable	109.676	k-ft
Location of maximum on span	0.000	ft
Span # where maximum occurs	Span # 2	

Maximum Deflection

Max Downward Transient Deflection	0.005 in	Ratio =	32482	>=360.0	Span: 2 : Lr Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 2 : Lr Only
Max Downward Total Deflection	0.058 in	Ratio =	2805	>=180.0	Span: 2 : +D+Lr
Max Upward Total Deflection	0.000 in	Ratio =	0	<180.0	Span: 2 : +D+Lr

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2	Support 3
Overall MAXimum	13.874	49.615	17.844
Overall MINimum	2.050	8.010	2.690

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: RB-1 (8"x24")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2	Support 3
D Only	11.641	41.945	14.997
+D+Lr	13.874	49.615	17.844
+D+0.750Lr	13.331	47.671	17.144
+0.60D	6.615	25.851	8.683
Lr Only	2.050	8.010	2.690

Shear Stirrup Requirements

Between 0.00 to 1.60 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 1.64 to 6.38 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 6.43 to 8.92 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 8.97 to 14.81 ft, PhiVc < Vu, Req'd Vs = 0.06116, use #3 stirrups spaced at 9.000 in
Between 14.86 to 17.45 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 17.51 to 21.91 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 21.97 to 24.94 ft, PhiVc < Vu, Req'd Vs = 1.528, use #3 stirrups spaced at 10.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	11.500	-80.34	109.68	0.73
Span # 2	2	13.500	-81.77	109.68	0.75
+1.40D					
Span # 1	1	11.500	-75.11	109.68	0.68
Span # 2	2	13.500	-76.45	109.68	0.70
+1.20D+0.50Lr					
Span # 1	1	11.500	-69.37	109.68	0.63
Span # 2	2	13.500	-70.61	109.68	0.64
+1.20D					
Span # 1	1	11.500	-64.38	109.68	0.59
Span # 2	2	13.500	-65.53	109.68	0.60
+1.20D+1.60Lr					
Span # 1	1	11.500	-80.34	109.68	0.73
Span # 2	2	13.500	-81.77	109.68	0.75
+0.90D					
Span # 1	1	11.500	-48.28	109.68	0.44
Span # 2	2	13.500	-49.15	109.68	0.45

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+Lr	1	0.0130	4.764	+D+Lr	-0.0009	11.007
+D+Lr	2	0.0577	7.521		0.0000	11.007

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

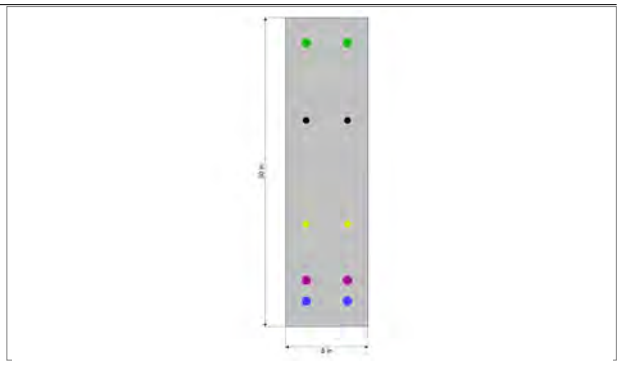
DESCRIPTION: RB-2 (8"x30")

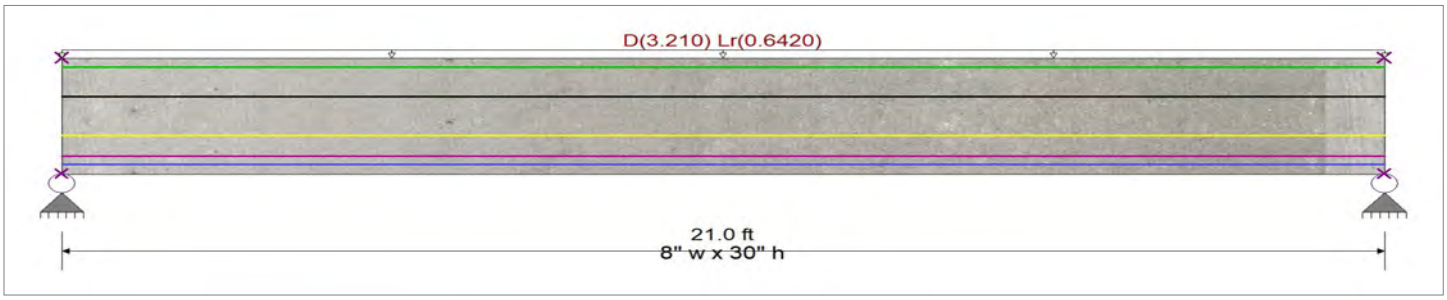
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ *7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2





Cross Section & Reinforcing Details

Rectangular Section, Width = 8.0 in, Height = 30.0 in
Span #1 Reinforcing....
2-#7 at 2.50 in from Bottom, from 0.0 to 21.0 ft in this span
2-#7 at 4.50 in from Bottom, from 0.0 to 21.0 ft in this span
2-#5 at 10.0 in from Bottom, from 0.0 to 21.0 ft in this span
2-#7 at 2.50 in from Top, from 0.0 to 21.0 ft in this span
2-#5 at 10.0 in from Top, from 0.0 to 21.0 ft in this span

Beam self weight calculated and added to loads
Loads on all spans...
D = 0.150, Lr = 0.030

Uniform Load on ALL spans : D = 0.150, Lr = 0.030 ksf, Tributary Width = 21.40 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.863**	: 1
Section used for this span	**Typical Section**	
Mu : Applied	284.951	k-ft
Mn * Phi : Allowable	330.180	k-ft
Location of maximum on span	10.481	ft
Span # where maximum occurs	Span # 1	

Maximum Deflection

Max Downward Transient Deflection	0.039 in	Ratio =	6509	>=360.0	Span: 1 : Lr Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 1 : Lr Only
Max Downward Total Deflection	0.541 in	Ratio =	466	>=180.0	Span: 1 : +D+Lr
Max Upward Total Deflection	0.000 in	Ratio =	0	<180.0	Span: 1 : +D+Lr

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
Overall MAXimum	42.984	42.983
Overall MINimum	6.741	6.741
D Only	36.242	36.243
+D+Lr	42.984	42.983

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: RB-2 (8"x30")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
+D+0.750Lr	41.298	41.298
+0.60D	21.745	21.745
Lr Only	6.741	6.741

Shear Stirrup Requirements

Between 0.00 to 5.93 ft, PhiVc < Vu, Req'd Vs = 0.1332, use #3 stirrups spaced at 6.000 in
Between 5.97 to 8.30 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 13.000 in
Between 8.34 to 12.66 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 12.70 to 15.03 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 13.000 in
Between 15.07 to 20.96 ft, PhiVc < Vu, Req'd Vs = 26.446, use #3 stirrups spaced at 6.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	21.000	284.95	330.18	0.86
+1.40D					
Span # 1	1	21.000	266.38	330.18	0.81
+1.20D+0.50Lr					
Span # 1	1	21.000	246.02	330.18	0.75
+1.20D					
Span # 1	1	21.000	228.33	330.18	0.69
+1.20D+1.60Lr					
Span # 1	1	21.000	284.95	330.18	0.86
+0.90D					
Span # 1	1	21.000	171.25	330.18	0.52

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+Lr	1	0.5405	10.500		0.0000	0.000

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

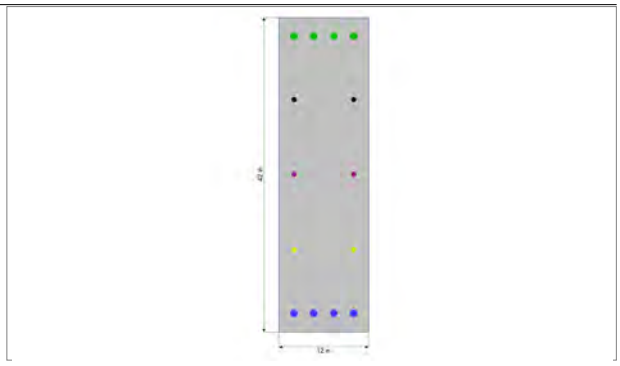
DESCRIPTION: RB-3 (12"x42")

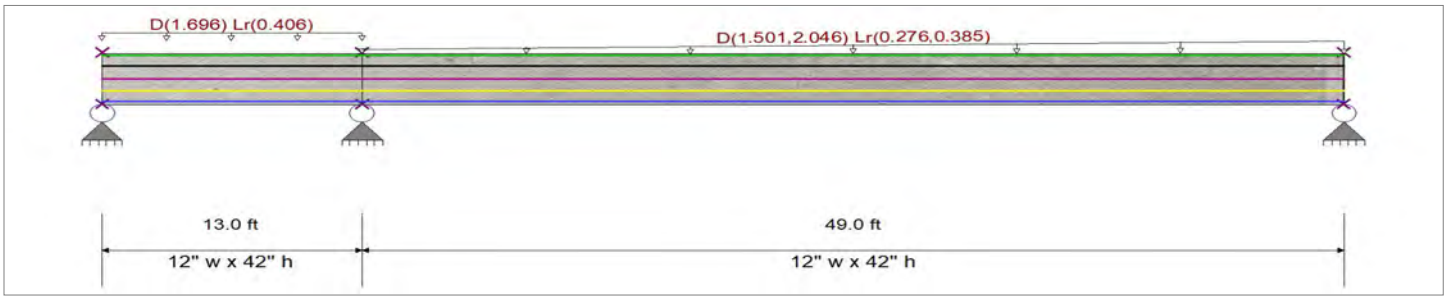
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ *7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2





Cross Section & Reinforcing Details

Rectangular Section, Width = 12.0 in, Height = 42.0 in
Span #1 Reinforcing....
4-#8 at 2.50 in from Bottom, from 0.0 to 13.0 ft in this span
2-#5 at 21.0 in from Bottom, from 0.0 to 13.0 ft in this span
2-#5 at 11.0 in from Bottom, from 0.0 to 13.0 ft in this span
4-#8 at 2.50 in from Top, from 0.0 to 13.0 ft in this span
2-#5 at 11.0 in from Top, from 0.0 to 13.0 ft in this span
Span #2 Reinforcing....
4-#8 at 2.50 in from Bottom, from 0.0 to 49.0 ft in this span
2-#5 at 21.0 in from Bottom, from 0.0 to 49.0 ft in this span
2-#5 at 11.0 in from Bottom, from 0.0 to 49.0 ft in this span
4-#8 at 2.50 in from Top, from 0.0 to 49.0 ft in this span
2-#5 at 11.0 in from Top, from 0.0 to 49.0 ft in this span

Load for Span Number 1
Uniform Load : D = 1.696, Lr = 0.4060 k/ft, Tributary Width = 1.0 ft

Load for Span Number 2
Varying Uniform Load : D= 1.501->2.046, Lr= 0.2760->0.3850 k/ft, Extent = 0.0 -->> 49.0 ft, Trib Width = 1.0 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.921** : 1					
Section used for this span	**Typical Section**					
Mu : Applied	-635.57 k-ft					
Mn * Phi : Allowable	690.43 k-ft					
Location of maximum on span	0.000 ft					
Span # where maximum occurs	Span # 2					
Maximum Deflection						
Max Downward Transient Deflection	0.076 in	Ratio =	7729	>=360.0	Span: 2 : Lr Only	
Max Upward Transient Deflection	-0.004 in	Ratio =	37786	>=360.0	Span: 2 : Lr Only	
Max Downward Total Deflection	1.195 in	Ratio =	492	>=180.0	Span: 2 : +D+Lr	
Max Upward Total Deflection	-0.058 in	Ratio =	2705	>=180.0	Span: 2 : +D+Lr	

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: RB-3 (12"x42")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2	Support 3
Overall MAXimum	-25.196	111.709	43.909
Overall MINimum	-3.463	18.012	6.923
D Only	-21.608	93.539	37.019
+D+Lr	-25.196	111.709	43.909
+D+0.750Lr	-24.314	107.186	42.182
+0.60D	-12.287	55.266	22.391
Lr Only	-3.463	18.012	6.923

Shear Stirrup Requirements

Between 0.00 to 7.69 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 11.5.6.3, use #3 stirrups spaced at 13.000 in
Between 7.75 to 21.00 ft, PhiVc < Vu, Req'd Vs = 0.1898, use #3 stirrups spaced at 10.000 in
Between 21.20 to 33.60 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 11.5.6.3, use #3 stirrups spaced at 13.000 in
Between 33.80 to 51.60 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 51.80 to 61.80 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 11.5.6.3, use #3 stirrups spaced at 13.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	13.000	-632.05	690.43	0.92
Span # 2	2	49.000	-635.57	690.43	0.92
+1.40D					
Span # 1	1	13.000	-590.18	690.43	0.85
Span # 2	2	49.000	-593.42	690.43	0.86
+1.20D+0.50Lr					
Span # 1	1	13.000	-545.30	690.43	0.79
Span # 2	2	49.000	-548.31	690.43	0.79
+1.20D					
Span # 1	1	13.000	-505.87	690.43	0.73
Span # 2	2	49.000	-508.64	690.43	0.74
+1.20D+1.60Lr					
Span # 1	1	13.000	-632.05	690.43	0.92
Span # 2	2	49.000	-635.57	690.43	0.92
+0.90D					
Span # 1	1	13.000	-379.40	690.43	0.55
Span # 2	2	49.000	-381.48	690.43	0.55

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+Lr	1	0.0228	13.700	+D+Lr	-0.0577	8.357
+D+Lr	2	1.1951	27.300		0.0000	8.357

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

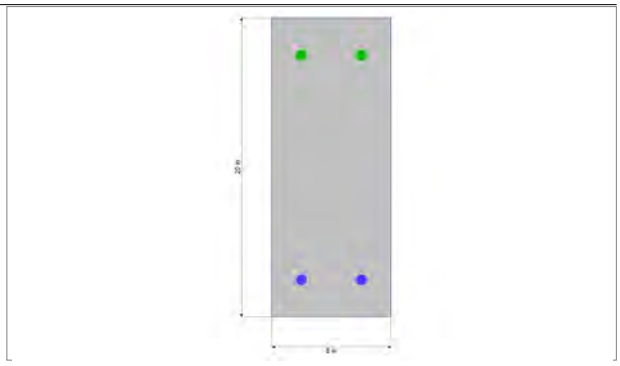
DESCRIPTION: RB-4 (8"x20")

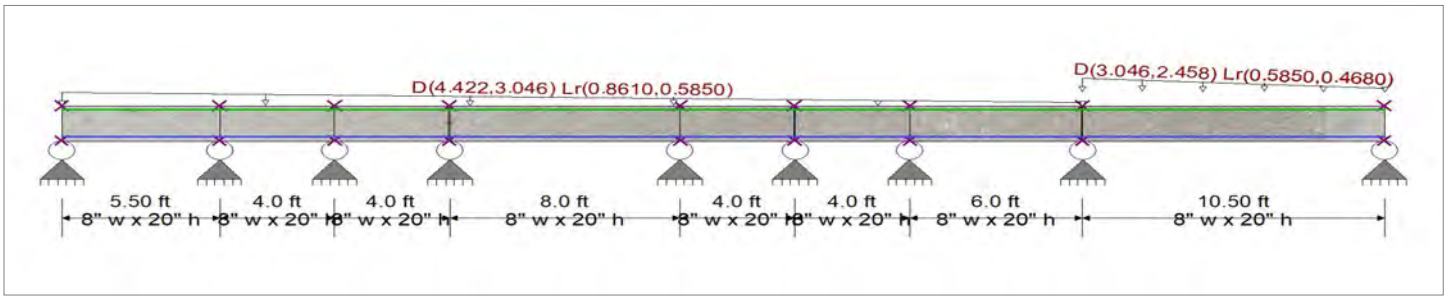
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ *7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2





Cross Section & Reinforcing Details

Rectangular Section, Width = 8.0 in, Height = 20.0 in

Span #1 Reinforcing....
2-#6 at 2.50 in from Bottom, from 0.0 to 5.50 ft in this span
2-#6 at 2.50 in from Top, from 0.0 to 5.50 ft in this span

Span #2 Reinforcing....
2-#6 at 2.50 in from Bottom, from 0.0 to 4.0 ft in this span
2-#6 at 2.50 in from Top, from 0.0 to 4.0 ft in this span

Span #3 Reinforcing....
2-#6 at 2.50 in from Bottom, from 0.0 to 4.0 ft in this span
2-#6 at 2.50 in from Top, from 0.0 to 4.0 ft in this span

Span #4 Reinforcing....
2-#6 at 2.50 in from Bottom, from 0.0 to 8.0 ft in this span
2-#6 at 2.50 in from Top, from 0.0 to 8.0 ft in this span

Span #5 Reinforcing....
2-#6 at 2.50 in from Bottom, from 0.0 to 4.0 ft in this span
2-#6 at 2.50 in from Top, from 0.0 to 4.0 ft in this span

Span #6 Reinforcing....
2-#6 at 2.50 in from Bottom, from 0.0 to 4.0 ft in this span
2-#6 at 2.50 in from Top, from 0.0 to 4.0 ft in this span

Span #7 Reinforcing....
2-#6 at 2.50 in from Bottom, from 0.0 to 6.0 ft in this span
2-#6 at 2.50 in from Top, from 0.0 to 6.0 ft in this span

Span #8 Reinforcing....
2-#6 at 2.50 in from Bottom, from 0.0 to 10.50 ft in this span
2-#6 at 2.50 in from Top, from 0.0 to 10.50 ft in this spa

Beam self weight calculated and added to loads
Loads on all spans...
Varying Uniform Load : D= 4.422->3.046, Lr= 0.8610->0.5850 k/ft, Extent = 0.0 -->> 35.50 ft
Varying Uniform Load : D= 3.046->2.458, Lr= 0.5850->0.4680 k/ft, Extent = 35.50 -->> 46.0 ft

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 | Arch-Deco Design & Construction Inc. | (c) ENERCALC INC 1983-2021

DESCRIPTION: RB-4 (8"x20")

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.687**	: 1
Section used for this span	**Typical Section**	
Mu : Applied	-45.834	k-ft
Mn * Phi : Allowable	66.746	k-ft
Location of maximum on span	0.000	ft
Span # where maximum occurs	Span # 8	

Maximum Deflection

Max Downward Transient Deflection	0.004 in	Ratio =	35231	>=360.0	Span: 8 : Lr Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 8 : Lr Only
Max Downward Total Deflection	0.035 in	Ratio =	3597	>=180.0	Span: 8 : +D+Lr
Max Upward Total Deflection	-0.002 in	Ratio =	29377	>=180.0	Span: 8 : +D+Lr

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2	Support 3	Support 4	Support 5	Support 6	Support 7	Support 8	Support 9
Overall MAXimum	11.872	31.370	11.132	33.810	31.115	12.977	14.317	39.487	13.978
Overall MINimum	1.876	4.949	1.755	5.302	4.870	1.991	2.261	6.060	2.131
D Only	9.995	26.421	9.379	28.504	26.269	10.878	12.241	33.299	11.876
+D+Lr	11.872	31.370	11.132	33.810	31.115	12.977	14.317	39.487	13.978
+D+0.750Lr	11.403	30.133	10.694	32.484	29.906	12.444	13.817	37.923	13.457
+0.60D	5.997	15.853	5.628	17.103	15.762	6.527	7.316	20.024	7.110
Lr Only	1.876	4.949	1.755	5.302	4.870	1.991	2.261	6.060	2.131

Shear Stirrup Requirements

Between 0.00 to 1.00 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 8.000 in
Between 1.13 to 3.25 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 3.38 to 6.95 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 8.000 in
Between 7.05 to 11.68 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 11.77 to 16.05 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 8.000 in
Between 16.23 to 18.77 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 18.95 to 22.95 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 8.000 in
Between 23.05 to 25.41 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 25.50 to 25.95 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 8.000 in
Between 26.05 to 28.95 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 29.05 to 29.50 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 8.000 in
Between 29.64 to 32.50 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 32.64 to 39.80 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 8.000 in
Between 40.03 to 43.38 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 43.61 to 45.76 ft, PhiVc < Vu, Req'd Vs = 1.872, use #3 stirrups spaced at 8.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	5.500	-17.21	66.75	0.26
Span # 2	2	4.000	-19.90	66.75	0.30
Span # 3	3	4.000	-25.14	66.75	0.38
Span # 4	4	8.000	-26.83	66.75	0.40
Span # 5	5	4.000	-25.21	66.75	0.38
Span # 6	6	4.000	7.11	66.75	0.11
Span # 7	7	6.000	-42.90	66.75	0.64
Span # 8	8	10.500	-45.83	66.75	0.69
+1.40D					
Span # 1	1	5.500	-16.07	66.75	0.24
Span # 2	2	4.000	-18.58	66.75	0.28
Span # 3	3	4.000	-23.51	66.75	0.35
Span # 4	4	8.000	-25.08	66.75	0.38
Span # 5	5	4.000	-23.58	66.75	0.35
Span # 6	6	4.000	6.67	66.75	0.10
Span # 7	7	6.000	-40.32	66.75	0.60
Span # 8	8	10.500	-43.07	66.75	0.65
+1.20D+0.50Lr					
Span # 1	1	5.500	-14.85	66.75	0.22
Span # 2	2	4.000	-17.17	66.75	0.26
Span # 3	3	4.000	-21.71	66.75	0.33
Span # 4	4	8.000	-23.16	66.75	0.35

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: RB-4 (8"x20")

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
Span # 5	5	4.000	-21.77	66.75	0.33
Span # 6	6	4.000	6.15	66.75	0.09
Span # 7	7	6.000	-37.17	66.75	0.56
Span # 8	8	10.500	-39.71	66.75	0.59
+1.20D					
Span # 1	1	5.500	-13.77	66.75	0.21
Span # 2	2	4.000	-15.92	66.75	0.24
Span # 3	3	4.000	-20.15	66.75	0.30
Span # 4	4	8.000	-21.50	66.75	0.32
Span # 5	5	4.000	-20.21	66.75	0.30
Span # 6	6	4.000	5.72	66.75	0.09
Span # 7	7	6.000	-34.56	66.75	0.52
Span # 8	8	10.500	-36.92	66.75	0.55
+1.20D+1.60Lr					
Span # 1	1	5.500	-17.21	66.75	0.26
Span # 2	2	4.000	-19.90	66.75	0.30
Span # 3	3	4.000	-25.14	66.75	0.38
Span # 4	4	8.000	-26.83	66.75	0.40
Span # 5	5	4.000	-25.21	66.75	0.38
Span # 6	6	4.000	7.11	66.75	0.11
Span # 7	7	6.000	-42.90	66.75	0.64
Span # 8	8	10.500	-45.83	66.75	0.69
+0.90D					
Span # 1	1	5.500	-10.33	66.75	0.15
Span # 2	2	4.000	-11.94	66.75	0.18
Span # 3	3	4.000	-15.11	66.75	0.23
Span # 4	4	8.000	-16.12	66.75	0.24
Span # 5	5	4.000	-15.16	66.75	0.23
Span # 6	6	4.000	4.29	66.75	0.06
Span # 7	7	6.000	-25.92	66.75	0.39
Span # 8	8	10.500	-27.69	66.75	0.41

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+Lr	1	0.0027	2.250	+D+Lr	-0.0001	5.682
+D+Lr	2	0.0001	2.727	+D+Lr	-0.0002	0.909
+D+Lr	3	0.0006	4.364	+D+Lr	-0.0006	2.727
+D+Lr	4	0.0065	4.000	+D+Lr	-0.0002	8.182
+D+Lr	5	0.0001	4.182	+D+Lr	-0.0007	1.273
+D+Lr	6	0.0007	2.000	+D+Lr	-0.0001	4.273
+D+Lr	7	0.0030	6.477	+D+Lr	-0.0025	4.091
+D+Lr	8	0.0350	6.205		0.0000	4.091

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 | Arch-Deco Design & Construction Inc. | (c) ENERCALC INC 1983-2021

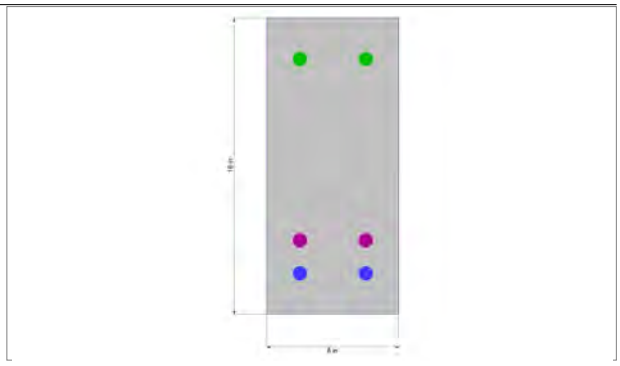
DESCRIPTION: RB-5 (8"x18")

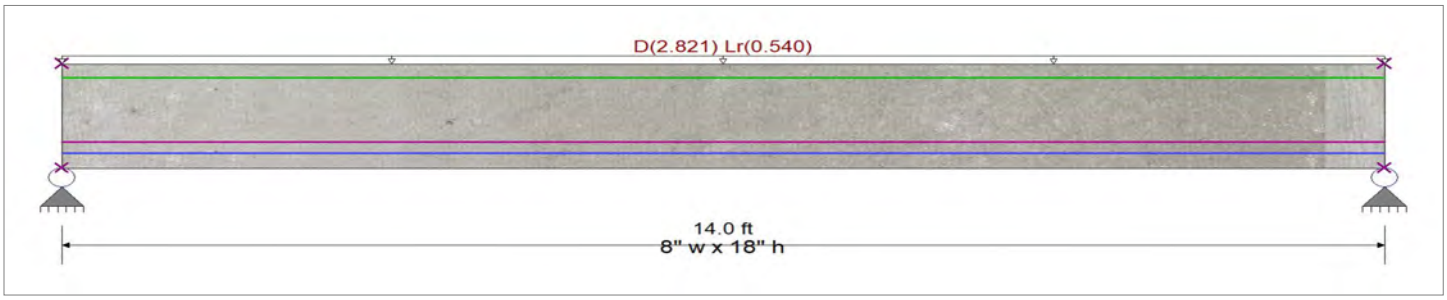
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ *7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2





Cross Section & Reinforcing Details

Rectangular Section, Width = 8.0 in, Height = 18.0 in
Span #1 Reinforcing....
2-#7 at 2.50 in from Bottom, from 0.0 to 14.0 ft in this span
2-#7 at 4.50 in from Bottom, from 0.0 to 14.0 ft in this span
2-#7 at 2.50 in from Top, from 0.0 to 14.0 ft in this span

Beam self weight calculated and added to loads
Loads on all spans...
D = 2.821, Lr = 0.540

Uniform Load on ALL spans : D = 2.821, Lr = 0.540 k/ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.791**	: 1
Section used for this span	**Typical Section**	
Mu : Applied	108.368	k-ft
Mn * Phi : Allowable	136.983	k-ft
Location of maximum on span	7.013	ft
Span # where maximum occurs	Span # 1	

Maximum Deflection

Max Downward Transient Deflection	0.030 in	Ratio =	5642	>=360.0	Span: 1 : Lr Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 1 : Lr Only
Max Downward Total Deflection	0.387 in	Ratio =	434	>=180.0	Span: 1 : +D+Lr
Max Upward Total Deflection	0.000 in	Ratio =	0	<180.0	Span: 1 : +D+Lr

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
Overall MAXimum	24.542	24.542
Overall MINimum	3.780	3.780
D Only	20.762	20.762
+D+Lr	24.542	24.542
+D+0.750Lr	23.597	23.597

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: RB-5 (8"x18")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
+0.60D	12.457	12.457
Lr Only	3.780	3.780

Shear Stirrup Requirements

Between 0.00 to 5.53 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 7.000 in
Between 5.56 to 8.44 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 8.47 to 13.97 ft, PhiVc < Vu, Req'd Vs = 13.968, use #3 stirrups spaced at 7.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	14.000	108.37	136.98	0.79
+1.40D					
Span # 1	1	14.000	101.73	136.98	0.74
+1.20D+0.50Lr					
Span # 1	1	14.000	93.82	136.98	0.68
+1.20D					
Span # 1	1	14.000	87.20	136.98	0.64
+1.20D+1.60Lr					
Span # 1	1	14.000	108.37	136.98	0.79
+0.90D					
Span # 1	1	14.000	65.40	136.98	0.48

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+Lr	1	0.3866	7.000		0.0000	0.000

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

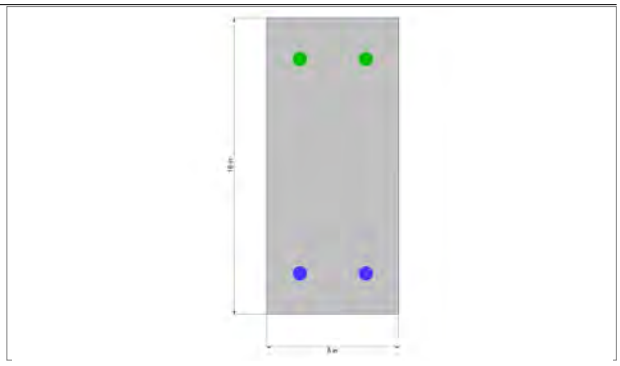
DESCRIPTION: RB-6 (8"x18")

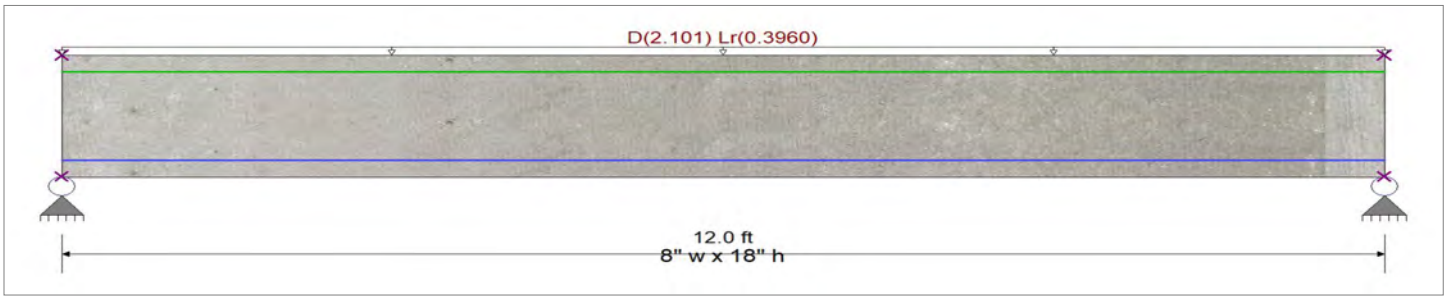
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ *7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2





Cross Section & Reinforcing Details

Rectangular Section, Width = 8.0 in, Height = 18.0 in
Span #1 Reinforcing....
2-#7 at 2.50 in from Bottom, from 0.0 to 12.0 ft in this span
2-#7 at 2.50 in from Top, from 0.0 to 12.0 ft in this span

Beam self weight calculated and added to loads
Loads on all spans...
D = 2.101, Lr = 0.3960
Uniform Load on ALL spans : D = 2.101, Lr = 0.3960 k/ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.769**	: 1
Section used for this span	**Typical Section**	
Mu : Applied	59.918	k-ft
Mn * Phi : Allowable	77.907	k-ft
Location of maximum on span	6.011	ft
Span # where maximum occurs	Span # 1	

Maximum Deflection

Max Downward Transient Deflection	0.012 in	Ratio =	12217	>=360.0	Span: 1 : Lr Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 1 : Lr Only
Max Downward Total Deflection	0.208 in	Ratio =	693	>=180.0	Span: 1 : +D+Lr
Max Upward Total Deflection	0.000 in	Ratio =	0	<180.0	Span: 1 : +D+Lr

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
Overall MAXimum	15.852	15.852
Overall MINimum	2.376	2.376
D Only	13.476	13.476
+D+Lr	15.852	15.852
+D+0.750Lr	15.258	15.258
+0.60D	8.086	8.086

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: RB-6 (8"x18")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
Lr Only	2.376	2.376

Shear Stirrup Requirements

Between 0.00 to 4.07 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 7.000 in
Between 4.09 to 7.91 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 7.93 to 11.98 ft, PhiVc < Vu, Req'd Vs = 5.228, use #3 stirrups spaced at 7.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	12.000	59.92	77.91	0.77
+1.40D					
Span # 1	1	12.000	56.60	77.91	0.73
+1.20D+0.50Lr					
Span # 1	1	12.000	52.08	77.91	0.67
+1.20D					
Span # 1	1	12.000	48.51	77.91	0.62
+1.20D+1.60Lr					
Span # 1	1	12.000	59.92	77.91	0.77
+0.90D					
Span # 1	1	12.000	36.39	77.91	0.47

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+Lr	1	0.2075	6.000		0.0000	0.000

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

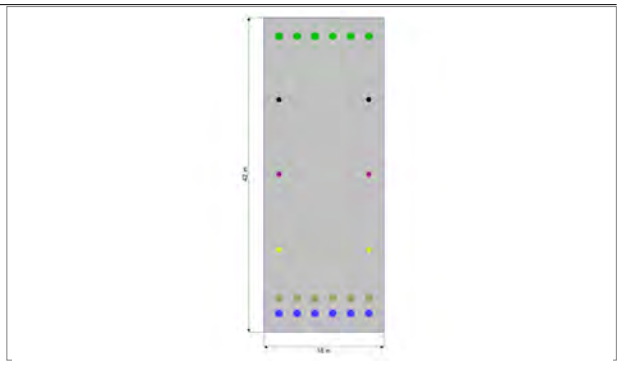
DESCRIPTION: RB-7 (16"x42")

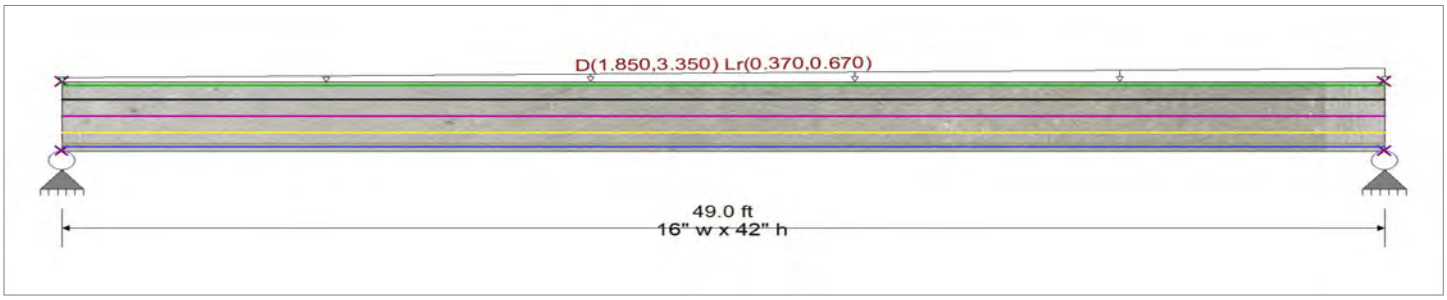
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ *7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	6.0





Cross Section & Reinforcing Details

Rectangular Section, Width = 16.0 in, Height = 42.0 in
Span #1 Reinforcing....

6-#8 at 2.50 in from Bottom, from 0.0 to 49.0 ft in this span
2-#5 at 21.0 in from Bottom, from 0.0 to 49.0 ft in this span
2-#5 at 11.0 in from Bottom, from 0.0 to 49.0 ft in this span
6-#8 at 2.50 in from Top, from 0.0 to 49.0 ft in this span
2-#5 at 11.0 in from Top, from 0.0 to 49.0 ft in this span
6-#8 at 4.50 in from Bottom, from 0.0 to 49.0 ft in this sp

Beam self weight calculated and added to loads
Loads on all spans...
Varying Uniform Load : D= 1.850->3.350, Lr= 0.370->0.670 k/ft, Extent = 0.0 -->> 50.0 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.855** : 1	
Section used for this span	**Typical Section**	
Mu : Applied	1,425.14	k-ft
Mn * Phi : Allowable	1,666.37	k-ft
Location of maximum on span	25.437	ft
Span # where maximum occurs	Span # 1	

Maximum Deflection

Max Downward Transient Deflection	0.168 in	Ratio =	3492	>=360.0	Span: 1 : Lr Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 1 : Lr Only
Max Downward Total Deflection	1.985 in	Ratio =	296	>=180.0	Span: 1 : +D+Lr
Max Upward Total Deflection	0.000 in	Ratio =	0	<180.0	Span: 1 : +D+Lr

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
Overall MAXimum	85.374	99.780
Overall MINimum	11.466	13.867
D Only	73.908	85.913
+D+Lr	85.374	99.780
+D+0.750Lr	82.508	96.314

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: RB-7 (16"x42")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
+0.60D	44.345	51.548
Lr Only	11.466	13.867

Shear Stirrup Requirements

Between 0.00 to 18.39 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 19.000 in
Between 18.48 to 32.04 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 32.13 to 38.83 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 19.000 in
Between 38.91 to 48.91 ft, PhiVc < Vu, Req'd Vs = 41.501, use #3 stirrups spaced at 18.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	49.000	1,425.14	1,666.37	0.86
+1.40D					
Span # 1	1	49.000	1,372.40	1,666.37	0.82
+1.20D+0.50Lr					
Span # 1	1	49.000	1,254.09	1,666.37	0.75
+1.20D					
Span # 1	1	49.000	1,176.34	1,666.37	0.71
+1.20D+1.60Lr					
Span # 1	1	49.000	1,425.14	1,666.37	0.86
+0.90D					
Span # 1	1	49.000	882.26	1,666.37	0.53

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+Lr	1	1.9853	24.500		0.0000	0.000

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

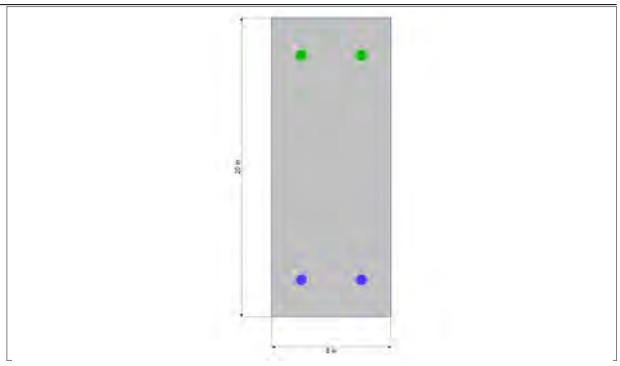
DESCRIPTION: RB-8 (8"x20")

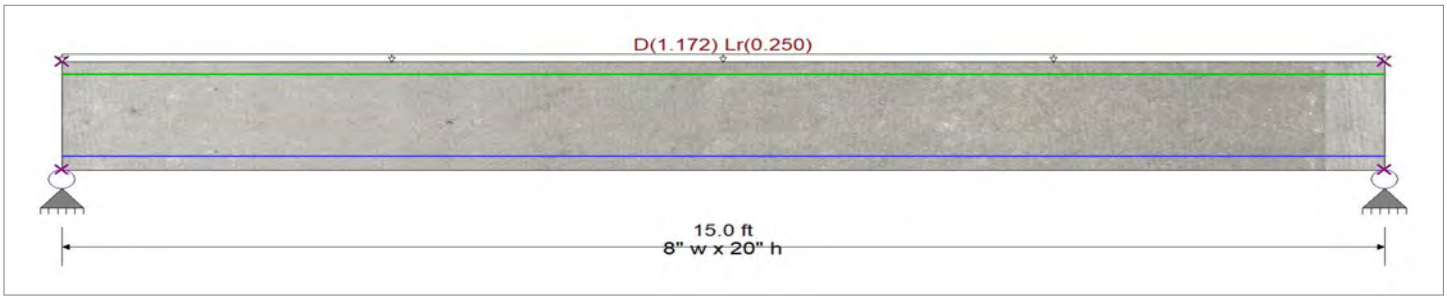
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = $f'c^{1/2}$ * 7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2





Cross Section & Reinforcing Details

Rectangular Section, Width = 8.0 in, Height = 20.0 in
Span #1 Reinforcing....
2-#6 at 2.50 in from Bottom, from 0.0 to 15.0 ft in this span
2-#6 at 2.50 in from Top, from 0.0 to 15.0 ft in this span

Beam self weight calculated and added to loads
Loads on all spans...
D = 1.172, Lr = 0.250

Uniform Load on ALL spans : D = 1.172, Lr = 0.250 k/ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio = **0.843** : 1

Section used for this span	**Typical Section**	
Mu : Applied	56.242	k-ft
Mn * Phi : Allowable	66.746	k-ft
Location of maximum on span	7.514	ft
Span # where maximum occurs	Span # 1	

Maximum Deflection

Max Downward Transient Deflection	0.013 in	Ratio =	13592	>=360.0	Span: 1 : Lr Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 1 : Lr Only
Max Downward Total Deflection	0.234 in	Ratio =	767	>=180.0	Span: 1 : +D+Lr
Max Upward Total Deflection	0.000 in	Ratio =	0	<180.0	Span: 1 : +D+Lr

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
Overall MAXimum	11.873	11.873
Overall MINimum	1.875	1.875
D Only	9.998	9.998
+D+Lr	11.873	11.873
+D+0.750Lr	11.405	11.405
+0.60D	5.999	5.999

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: RB-8 (8"x20")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
Lr Only	1.875	1.875

Shear Stirrup Requirements

Between 0.00 to 3.85 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 8.000 in
Between 3.88 to 11.12 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 11.15 to 14.97 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 8.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	15.000	56.24	66.75	0.84
+1.40D					
Span # 1	1	15.000	52.49	66.75	0.79
+1.20D+0.50Lr					
Span # 1	1	15.000	48.51	66.75	0.73
+1.20D					
Span # 1	1	15.000	44.99	66.75	0.67
+1.20D+1.60Lr					
Span # 1	1	15.000	56.24	66.75	0.84
+0.90D					
Span # 1	1	15.000	33.74	66.75	0.51

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+Lr	1	0.2345	7.500		0.0000	0.000

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

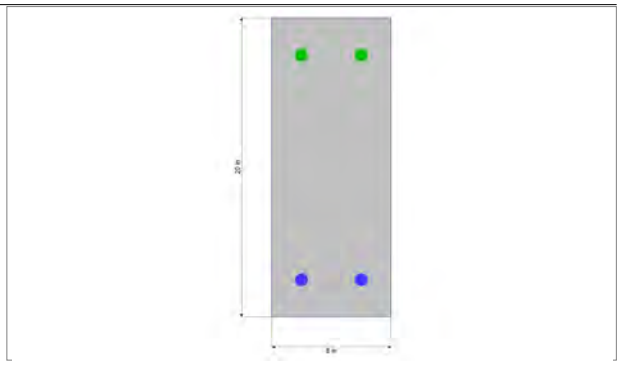
DESCRIPTION: RB-9 (8"x20")

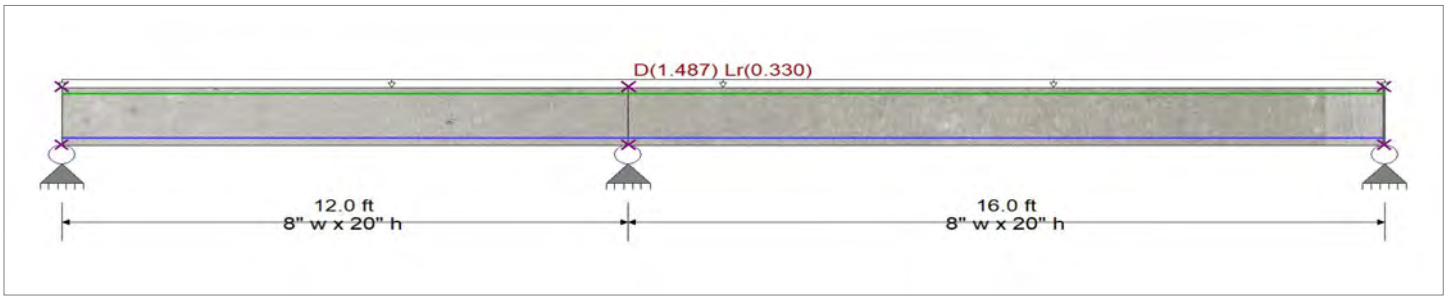
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ *7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2





Cross Section & Reinforcing Details

Rectangular Section, Width = 8.0 in, Height = 20.0 in
Span #1 Reinforcing....
2-#7 at 2.50 in from Bottom, from 0.0 to 12.0 ft in this span
2-#7 at 2.50 in from Top, from 0.0 to 12.0 ft in this span
Span #2 Reinforcing....
2-#7 at 2.50 in from Bottom, from 0.0 to 16.0 ft in this span
2-#7 at 2.50 in from Top, from 0.0 to 16.0 ft in this span

Beam self weight calculated and added to loads
Loads on all spans...
D = 1.487, Lr = 0.330
Uniform Load on ALL spans : D = 1.487, Lr = 0.330 k/ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.734**	: 1
Section used for this span	**Typical Section**	
Mu : Applied	-65.149	k-ft
Mn * Phi : Allowable	88.707	k-ft
Location of maximum on span	0.000	ft
Span # where maximum occurs	Span # 2	

Maximum Deflection

Max Downward Transient Deflection	0.012 in	Ratio =	16281	>=360.0	Span: 2 : Lr Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 2 : Lr Only
Max Downward Total Deflection	0.153 in	Ratio =	1251	>=180.0	Span: 2 : +D+Lr
Max Upward Total Deflection	-0.007 in	Ratio =	21776	>=180.0	Span: 2 : +D+Lr

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2	Support 3
Overall MAXimum	7.754	34.895	12.739
Overall MINimum	1.265	5.871	2.104
D Only	6.509	28.988	10.650
+D+Lr	7.754	34.895	12.739

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: RB-9 (8"x20")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2	Support 3
+D+0.750Lr	7.444	33.416	12.217
+0.60D	3.797	17.582	6.309
Lr Only	1.265	5.871	2.104

Shear Stirrup Requirements

Between 0.00 to 0.54 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 8.000 in
Between 0.59 to 7.05 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 7.10 to 18.66 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 8.000 in
Between 18.73 to 24.54 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 24.60 to 27.93 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 8.000 in

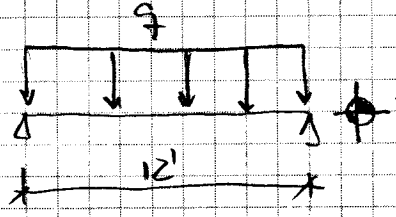
Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	12.000	-64.15	88.71	0.72
Span # 2	2	16.000	-65.15	88.71	0.73
+1.40D					
Span # 1	1	12.000	-59.07	88.71	0.67
Span # 2	2	16.000	-59.99	88.71	0.68
+1.20D+0.50Lr					
Span # 1	1	12.000	-54.86	88.71	0.62
Span # 2	2	16.000	-55.71	88.71	0.63
+1.20D					
Span # 1	1	12.000	-50.63	88.71	0.57
Span # 2	2	16.000	-51.42	88.71	0.58
+1.20D+1.60Lr					
Span # 1	1	12.000	-64.15	88.71	0.72
Span # 2	2	16.000	-65.15	88.71	0.73
+0.90D					
Span # 1	1	12.000	-37.97	88.71	0.43
Span # 2	2	16.000	-38.57	88.71	0.43

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+Lr	1	0.0076	3.943	+D+Lr	-0.0066	10.457
+D+Lr	2	0.1534	8.914		0.0000	10.457

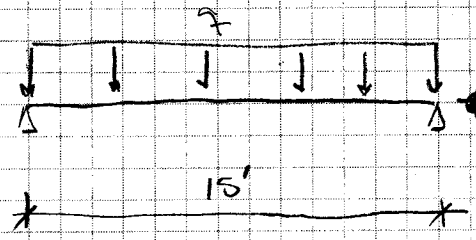
→ ZB-1 (8" x 20")



B(2#6), T(2#6)
□ #3 @ 8" *

q { DL slab = 150 pcf x 0.83' x 9' = 1121 pf
DL SDL = 28 psf x 9' = 225 pf
DL TB+wall = 150 pcf x 0.67' x 3' + 60 psf x 1' = 341 pf
1687 pf
LL slab = 60 psf x 9' = 540 pf

→ ZB-2 (8" x 28") - Upturned Beam

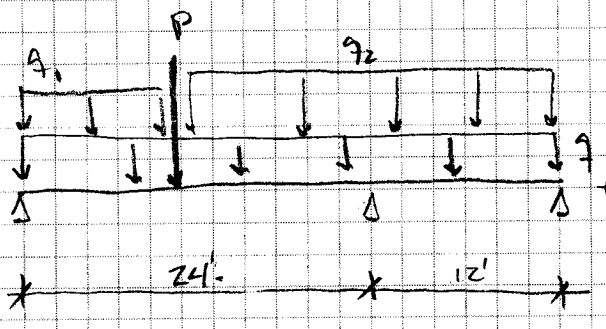


B(2#6, 2#6), T(2#6), I(1#5)
□ #3 - 9 @ 6" E.S. & Bal 12" *

q { DL slab+SDL = 150 psf x 7.67' = 1151 pf
DL Roof = 150 psf x 16.5' + 121 pf = 2596 pf
DL TB+wall = 303 pf + 60 psf x 10' = 703 pf
4448 pf
LL Roof = 30 psf x 16.5' = 495 pf
LL slab = 40 psf x 4.67' + 60 psf x 3' = 367 pf

* See calculation report

→ ZB-3 (12" X 36") Supp. ZB-2



q { DLslab+SDL = 150 psf × 8.5' = 1275 plf ; LLslab = 60 psf × 8.5' = 510 plf

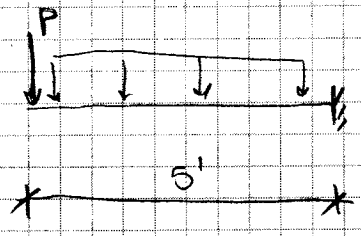
q_1 { DLslab+SDL = 150 psf × 3' = 450 plf ; LLslab = 60 psf × 3' = 180 plf

q_2 { DLroof+TB+Wall = 1696 plf + 701 plf = 2397 plf ; DLslab+SDL = 150 psf × 9.33' = 1400 plf ; 3797 plf ; LLRoof = 406 plf ; LLslab = 40 psf × 9.33' = 373 plf

P (Reacc ZB-2) { D = 35.06 ; Lr = 3.71 @ = 9.67' ; L = 2.75

B(4#8) , T(4#8) , I(3#5) , □ #3 @ 4" *

→ ZB-4 (12" x 48") Supp. ZB-3



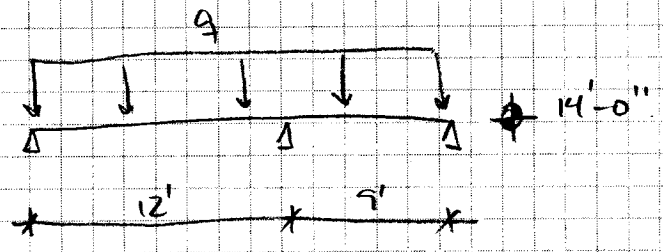
q { DLslab+SDL = 150 psf × 7' = 1050 plf ; LLslab = 40 psf × 7' = 280 plf

P (Reacc ZB-3) { D = 150.31 ; Lr = 11.28 @ = 0.33' ; L = 22.96

B(4#8) , T(4#8, 4#8)
I(3#5) , □ #3 @ 3" > *

* See calculation report

→ ZB-5 (8" x 24")

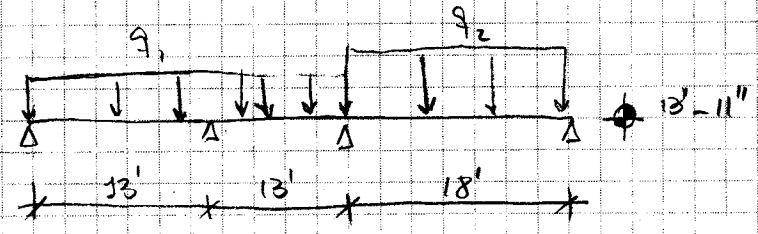


q { DL roof = 150 psf x 16.5' + 123 pf = 2596 pf
DL TB + wall = 103 pf + 60 psf x 10' = 703 pf
DL slab = 130 psf x (13' + 6.5') = 2735 pf
6222 pf

LL roof = 30 psf x 16.5' = 495 pf
LL slab = 60 psf x 6.5' + 40 psf x 13' = 910 pf

B(2#9), T(2#7), C(2#7), I(1#5)
□ #3 - 21@3" E.S & Bal 10" *

→ ZB-7 (12" x 30")



q_1 { DL slab + SDL = 150 psf x 20' = 3000 pf
LL slab = 40 psf x 20' = 800 pf

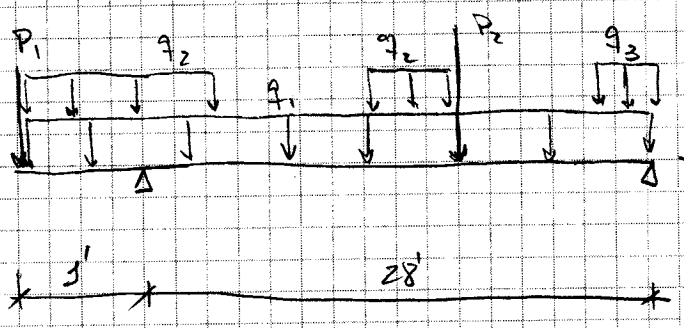
q_2 { DL slab + SDL = 150 psf x 22.83' = 3425 pf
LL slab = 100 psf x 22.83' = 2283 pf

B(3#7, 2#7), T(3#7)
C(2#7), I(2#5)
□ #3 - 15@5" E.S.
& Bal 12" *

* See calculation report

2nd FB - G (16" x 30")

Supp. of FB-5.



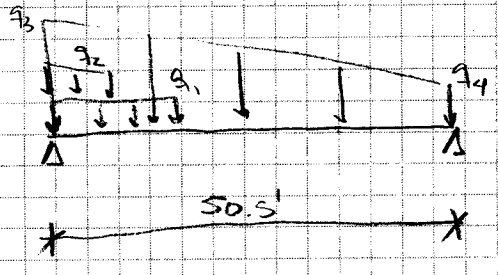
q_1 { DL slab+SDL = 150 psf (9.5+9.33') = 2825 pf
LL slab = 100 psf x 9.5' + 40 psf x 9.33' = 1323 pf

q_2 { DL Roof+FB+Wall = 1696 pf + 101 pf + 60 psf x 10' = 2397 pf
LL Roof = 906 pf @ = 0'-2.5', 85'-19.5'

P_1 (2B-5) { D = 21.43 L = 29
Lr = 1.58 @ = 0.33'

P_2 (RB-3) { D = -21.51 + 1.01 = -20.5
Lr = -3.96
@ = 19.75'

q_3 { DL TC = 150 pcf x 0.67' x 10' = 1005 pf
@ = 26.25'-28'



q_1 { DL slab+SDL = 150 psf x 2.5' = 375 pf
LL slab = 40 psf x 2.5' = 100 pf
@ = 0'-14'

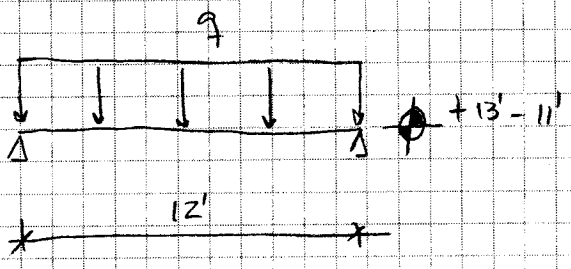
q_2 { DL = 1005 pf
@ = 2'

B (S#8), T (S#8, S#8)
I (2#5), [illegible] #3 - 15@7" E.S. & Bal 12" *

q_3 { DL slab+SDL = 150 psf x 10.5' = 1575 pf
LL slab = 100 psf x 10.5' = 1050 pf

q_4 { DL slab+SDL = 150 psf x 1' = 150 pf
LL slab = 100 psf x 1' = 100 pf

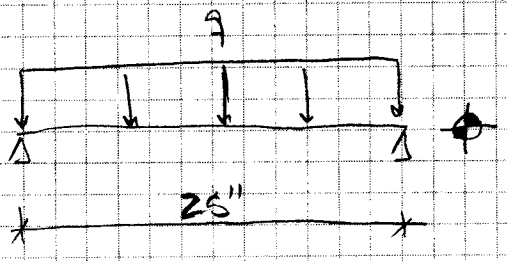
→ ZB-9 (8" x 22")



B(2#7), T(2#7), □ #3@9" *

$$q \begin{cases} DL_{slab} = 150\ psf \times 13' = 1950\ pf \\ DL_{ph} = 121\ pf \quad \Big/ \quad 2071\ pf \\ LL_{slab} = 100\ psf \times 13' = 1300\ pf \end{cases}$$

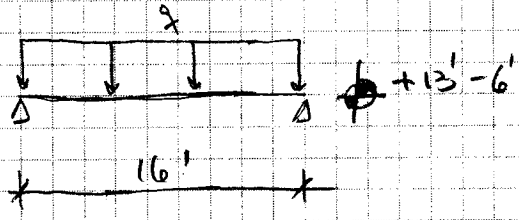
→ ZB-10 (16" x 24")



B(1#8), T(1#8), □ #3@10" *
I(1#5)

$$q \begin{cases} DL_{slab+SDL} = 126\ psf \times 11.25' = 1418\ pf \\ DL_{water} = 63\ pcf \times 11.25' \times 0.8' = 355\ pf \\ DL_{walls} = 30\ psf \times 6.75' = 67.5'\ pf \\ \qquad\qquad\qquad\qquad\qquad\qquad 1841\ pf \\ LL_{slab} = 30\ psf \times (11.25' + 6.75') = 540\ pf \end{cases}$$

→ ZB-11 (16" x 24")

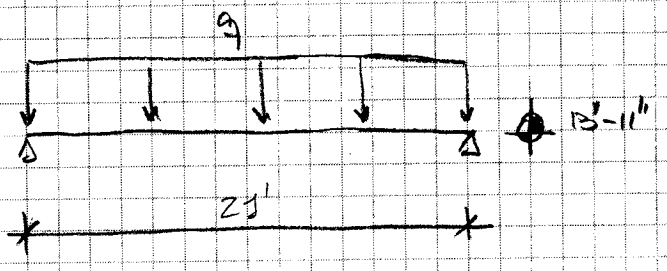


B(3#6), T(3#6), □ #3@10" *
I(1#5)

$$q \begin{cases} DL_{slab+SDL} = 126\ psf \times 12.5' = 1575\ pf \\ DL_{water} = 63\ pcf \times 12.5' \times 0.5' = 394\ pf \\ \qquad\qquad\qquad\qquad\qquad\qquad 1969\ pf \\ LL_{slab} = 30\ psf \times 12.5' = 375\ pf \end{cases}$$

* See calculation report

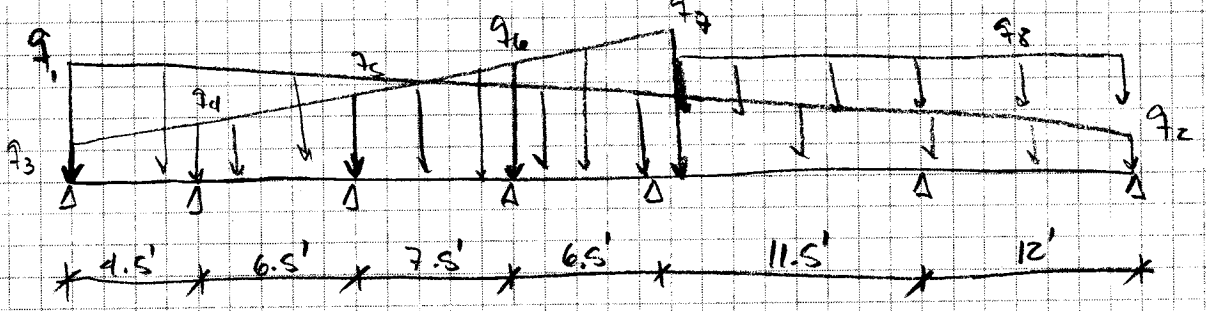
→ ZB-12 (8" x 22")



q { DL slab + SDL = 180 psf
LL slab = 100 psf
Trib. width = 8'
q_1 = DL ph = 121 pf

B(2#8, 2#8), T(2#8), □ #3 @9" *

→ ZB-13 (8" x 22")



q_1 { DL slab + SDL = 1575 pf
LL slab = 1050 pf

q_2 { DL slab + SDL = 150 pf
LL slab = 100 pf

q_3 { DL slab = 150 psf x 2.33' = 350 pf
DL ph = 121 pf ——— 471 pf
LL slab = 100 psf x 2.33' = 233 pf

q_4 { DL slab + ph = 150 psf x 4' + 121 pf = 721 pf
LL slab = 100 psf x 4' = 400 pf

q_5 { DL slab + ph = 150 psf x 6.75' + 121 pf = 1134 pf
LL slab = 100 psf x 6.75' = 675 pf

q_6 { DL slab + ph = 150 psf x 10' + 121 pf = 1621 pf
LL slab = 100 psf x 10' = 1000 pf

q_7 { DL slab + ph = 150 psf x 12.58' + 121 pf = 2008 pf
LL slab = 100 psf x 12.58' = 1258 pf

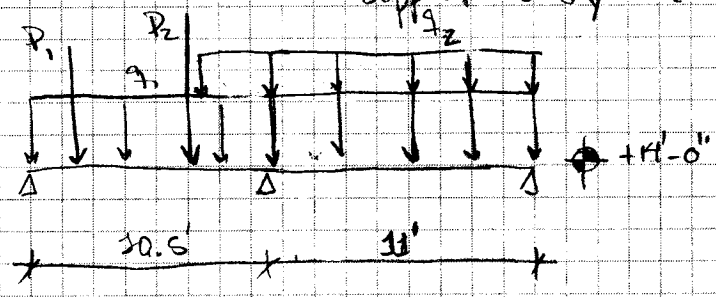
q_8 { DL slab + SDL = 150 psf x 2' = 300 pf
LL slab = 100 psf x 2' = 200 pf

B(2#6), T(2#6), □ #3 @9" *

* See calculation report

→ ZB-14 (8" x 30")

Supp of ZB-8 y ZB-6



q_1
- DL slab + sdl = 150 psf
- LL slab = 40 psf

Trib Width = 14.67'

DL TC = 150 pcf × 0.67 × 2.25' × 10' = 2261 lb

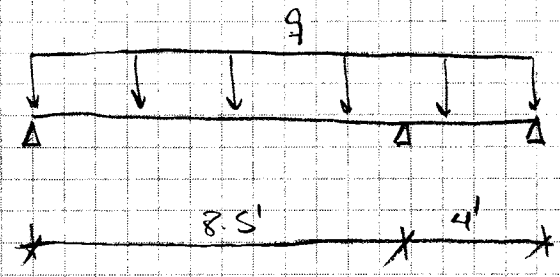
q_2
- DL Roof + slab = 150 psf × (18' + 4.5') = 3375 pf
- DL ph = 323 pf × 2 = 2412 pf
- DL TB + wall = 701 pf / 4318 pf
- LL Roof = 20 psf × 18' = 540 pf
- LL slab = 100 psf × 4.5' = 450 pf

P_2 (Reacc RB-2(2) + RB-3(3) + ZB-6(3))
- D = 93.27 + 2.26 = 95.53
- Lr = 13.71
- L = 7.44
- @ = 8.5'

P_1 (Reacc ZB-8(2))
- D = 16.41
- L = 2.75
- @ = 2.5'

B(2#8), T(2#8), C(2#8), I(2#5), □ #4 - 13@3" f.s. & Bal 7" *

→ ZB-15 (8" X 22")

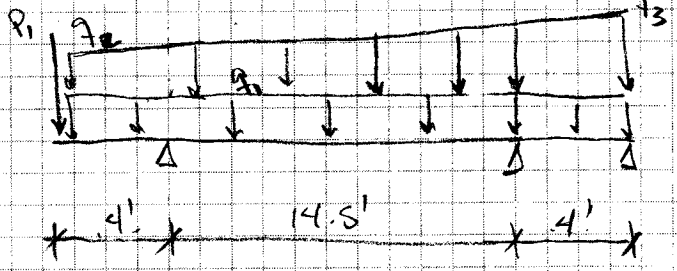


q
- DL slab + sdl = 150 psf × 8.5 = 1275 pf
- LL slab = 60 psf × 4' + 40 psf × 4.5' = 420 pf

B(2#6), T(2#6), □ #3@9" *

* See calculation report

↳ ZB-16 (8" x 24") Support ZB-15.



q_1
- DL slab + SDL = 150 psf x 7' = 1050 plf
- DL TB + wall = 701 plf
- DL Roof = 150 psf x 11' = 1650 plf / 3401 plf
- LL Roof = 30 psf x 11' = 330 plf
- LL slab = 40 psf x 7' = 280 plf

q_2
- DL slab + SDL = 150 psf x 3.75' = 563 plf
- LL slab = 60 psf x 3.75' = 225

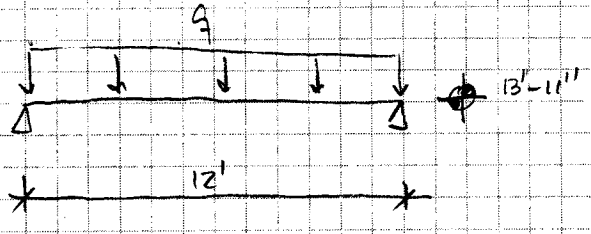
q_3
- DL slab + SDL = 150 psf x 7.83' = 1175 plf
- LL slab = 60 psf x 7.83' = 470 plf

P_1 (Reac ZB-15) + RB-4
- D = 12.32 + 1.51 = 13.83
- @ = 0.33' L_r = 2.13 L = 0.13

DL TC = 150 psf x 0.67 x 1.5' x 10'
= 1508 lbs

B(2 #7), T(2 #7), I(1 #5), □ #3 @ 4" *

↳ ZB-17 (8" x 22")



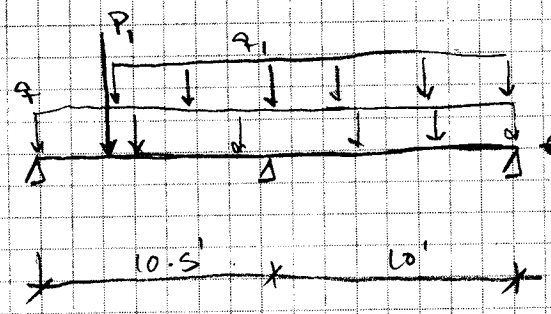
q
- DL slab + SDL = 150 psf
- LL slab = 60 psf

T. Width = 10'

B(2 #6), T(2 #6), □ #3 @ 9" *

* See calculation report

→ ZB-18 (8" x 22")



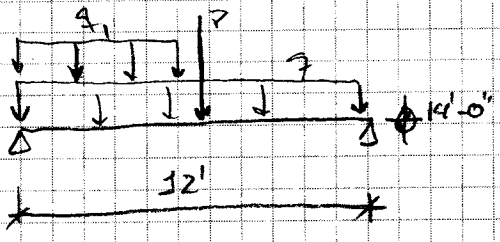
P_1 (Roof RB-16) { D = 13.48 + 1.01 = 14.49 ; L_r = 2.38 @ = 2.75' }

q { DL slab + SDL = 150 psf x 14.28' + 121 plf = 2263 plf ; LL slab = 40 psf x 9.78' + 60 psf x 4.5' = 662 plf }

q_1 { DL Roof + TB + Wall = 2101 + 701 = 2802 plf ; LL Roof = 396 plf }

B(2#8, 2#8), T(2#8),
□ #3 @ 3" *

→ ZB-19 (12" x 22")



P (Roof RB-5) { D = 20.76 + 1.01 = 21.77 ; L_r = 3.78 @ = 7' }

q { DL slab = 150 psf x 19.5' = 2925 plf ; LL slab = 60 psf x 4.5' + 40 psf x 15' = 870 plf }

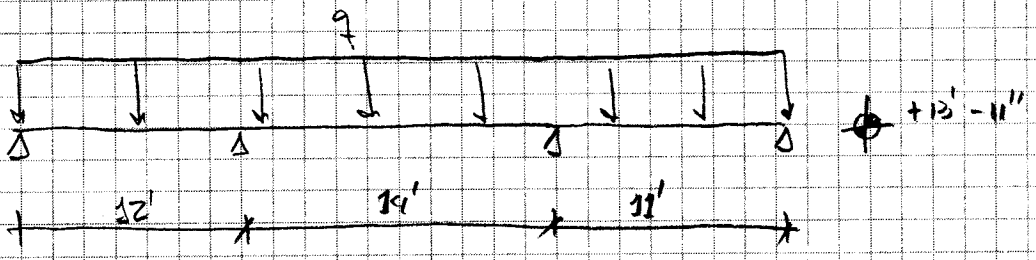
q_1 { DL Roof = 2821 plf ; DL Wall + TB = 701 plf } / 3522 plf ; { LL Roof = 540 plf }

DL TC = 150 pcf x 0.67' x 1' x 10'
= 1005 lbs

B(4#7, 2#7), T(4#7),
□ #3 @ 6" *

* See calculation report

→ ZB - 8 (8" x 24")

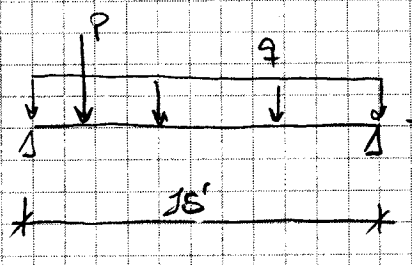


$$q \begin{cases} DL_{slab} + SDL = 150 \text{ psf} \\ LL_{slab} = 100 \text{ psf} \end{cases}$$

T. Width = 15.5'

B (2 #6), T (2 #6), I (1 #5), □ #3 - 8 @ 6" E.S. & Bal 10" *

→ ZB - 20 (12" x 22")



$$q \begin{cases} DL_{roof+slab+SDL} = 2923 \text{ pf} + 2821 \text{ pf} \\ DL_{FB+Wall} = 703 \text{ pf} \\ LL_{Roof} = 540 \text{ pf} \\ LL_{slab} = 870 \text{ pf} \end{cases}$$

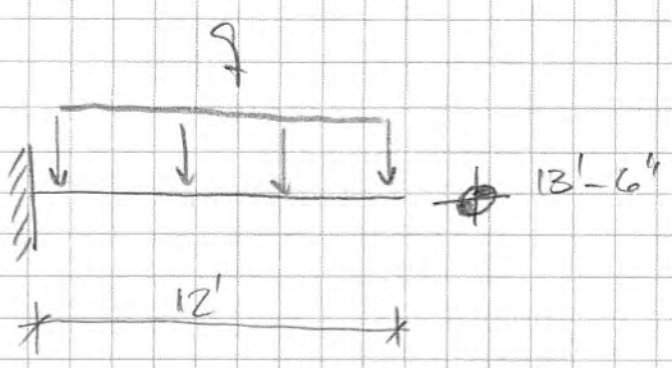
6443 pf

$$P_{(Roof\ BBS)} \begin{cases} D = 21.77 \\ L_r = 3.78 \\ @ = 1.67' \end{cases}$$

B (3#8, 3#8), T (3#8), □ #3 - 32 @ 2" E.S. & Bal 8" *

* See calculation report

→ 2B-22 (8" x 20") Cont. 2B-18

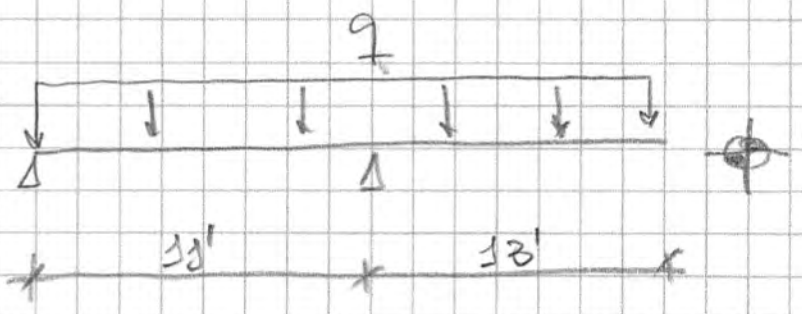


$$q \begin{cases} DL_{slab} + SDL = 150\,pcf \times 9.83' + 25\,psf \\ \qquad\qquad = 150\,psf \\ LL_{slab} = 60\,psf \end{cases}$$

Trib. Width = 9'

B(2#8), T(2#8, 2#8)
□ #3 @ 6" *

→ 2B-23 (12" x 20")



$$q \begin{cases} DL_{slab+SDL} = 150\ psf \\ LL_{slab} = 100\ psf \end{cases}$$

Trib. Width = 7'

B(3#8), T(3#8, 2#8), □ #3 @ 8" *

* See calculation report

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

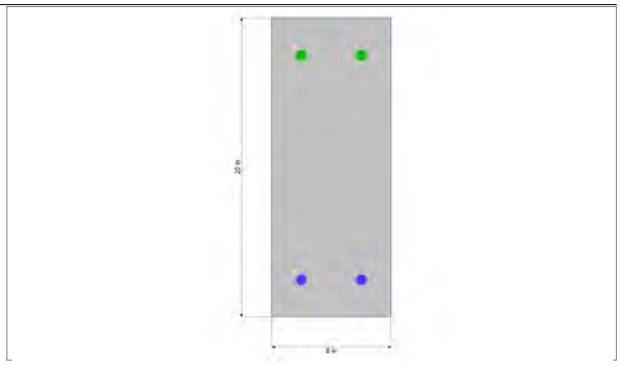
DESCRIPTION: 2B-1 (8"x20")

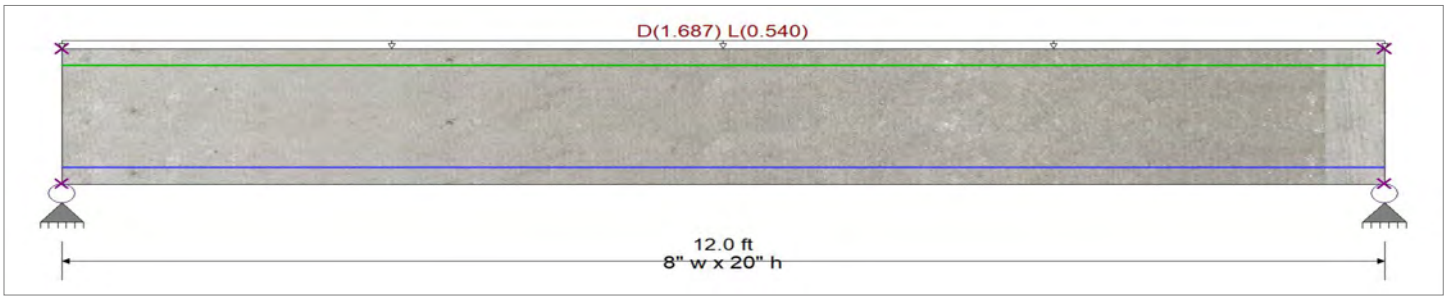
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ *7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2





Cross Section & Reinforcing Details

Rectangular Section, Width = 8.0 in, Height = 20.0 in
Span #1 Reinforcing....
2-#6 at 2.50 in from Bottom, from 0.0 to 12.0 ft in this span
2-#6 at 2.50 in from Top, from 0.0 to 12.0 ft in this span

Beam self weight calculated and added to loads
Loads on all spans...
D = 1.687, L = 0.540

Uniform Load on ALL spans : D = 1.687, L = 0.540 k/ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.831**	: 1
Section used for this span	**Typical Section**	
Mu : Applied	55.471	k-ft
Mn * Phi : Allowable	66.746	k-ft
Location of maximum on span	6.011	ft
Span # where maximum occurs	Span # 1	

Maximum Deflection

Max Downward Transient Deflection	0.012 in	Ratio =	12290	>=360.0	Span: 1 : L Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 1 : L Only
Max Downward Total Deflection	0.140 in	Ratio =	1031	>=180.0	Span: 1 : +D+L
Max Upward Total Deflection	0.000 in	Ratio =	0	<180.0	Span: 1 : +D+L

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
Overall MAXimum	14.329	14.329
Overall MINimum	3.240	3.240
D Only	11.089	11.089
+D+L	14.329	14.329
+D+0.750L	13.519	13.519
+0.60D	6.653	6.653

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-1 (8"x20")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
L Only	3.240	3.240

Shear Stirrup Requirements

Between 0.00 to 3.65 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 8.000 in
Between 3.67 to 8.33 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 8.35 to 11.98 ft, PhiVc < Vu, Req'd Vs = 2.734, use #3 stirrups spaced at 8.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	12.000	55.47	66.75	0.83
+1.40D					
Span # 1	1	12.000	46.57	66.75	0.70
+1.20D+1.60L					
Span # 1	1	12.000	55.47	66.75	0.83
+1.20D+L					
Span # 1	1	12.000	49.64	66.75	0.74
+1.20D					
Span # 1	1	12.000	39.92	66.75	0.60
+0.90D					
Span # 1	1	12.000	29.94	66.75	0.45

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+L	1	0.1396	6.000		0.0000	0.000

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

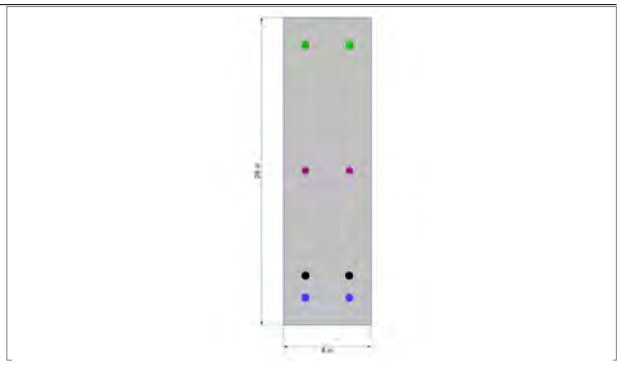
DESCRIPTION: 2B-2 (8"x28")

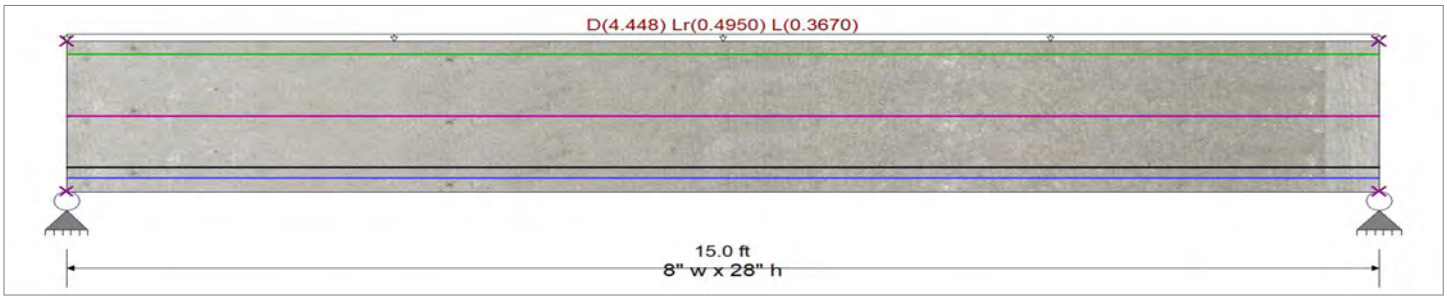
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c^1/2 * 7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β 1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2





Cross Section & Reinforcing Details

Rectangular Section, Width = 8.0 in, Height = 28.0 in
Span #1 Reinforcing....
2-#6 at 2.50 in from Bottom, from 0.0 to 15.0 ft in this span
2-#5 at 14.0 in from Bottom, from 0.0 to 15.0 ft in this span
2-#6 at 2.50 in from Top, from 0.0 to 15.0 ft in this span
2-#6 at 4.50 in from Bottom, from 0.0 to 15.0 ft in this s|

Beam self weight calculated and added to loads
Loads on all spans...
D = 4.448, Lr = 0.4950, L = 0.3670
Uniform Load on ALL spans : D = 4.448, Lr = 0.4950, L = 0.3670 k/ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.892** : 1	
Section used for this span	**Typical Section**	
Mu : Applied	190.329	k-ft
Mn * Phi : Allowable	213.362	k-ft
Location of maximum on span	7.514	ft
Span # where maximum occurs	Span # 1	

Maximum Deflection

Max Downward Transient Deflection	0.010 in	Ratio =	18836	>=360.0	Span: 1 : L Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 1 : Lr Only
Max Downward Total Deflection	0.280 in	Ratio =	643	>=180.0	Span: 1 : +D+0.750Lr+0.750L
Max Upward Total Deflection	0.000 in	Ratio =	0	<180.0	Span: 1 : +D+0.750Lr+0.750L

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
Overall MAXimum	39.900	39.900
Overall MINimum	2.752	2.752
D Only	35.052	35.052
+D+L	37.804	37.804
+D+Lr	38.764	38.764

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-2 (8"x28")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
+D+0.750Lr+0.750L	39.900	39.900
+D+0.750L	37.116	37.116
+0.60D	21.031	21.031
Lr Only	3.712	3.712
L Only	2.752	2.752

Shear Stirrup Requirements

Between 0.00 to 4.26 ft, PhiVc < Vu, Req'd Vs = 0.01314, use #3 stirrups spaced at 6.000 in
Between 4.29 to 5.93 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 12.000 in
Between 5.96 to 9.04 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 9.07 to 10.71 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 12.000 in
Between 10.74 to 14.97 ft, PhiVc < Vu, Req'd Vs = 25.736, use #3 stirrups spaced at 6.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	15.000	190.33	213.36	0.89
+1.40D					
Span # 1	1	15.000	184.02	213.36	0.86
+1.20D+0.50Lr+1.60L					
Span # 1	1	15.000	181.21	213.36	0.85
+1.20D+1.60L					
Span # 1	1	15.000	174.25	213.36	0.82
+1.20D+1.60Lr+L					
Span # 1	1	15.000	190.33	213.36	0.89
+1.20D+1.60Lr					
Span # 1	1	15.000	180.01	213.36	0.84
+1.20D+L					
Span # 1	1	15.000	168.05	213.36	0.79
+1.20D					
Span # 1	1	15.000	157.73	213.36	0.74
+1.20D+0.50Lr+L					
Span # 1	1	15.000	175.01	213.36	0.82
+0.90D					
Span # 1	1	15.000	118.30	213.36	0.55

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+0.750Lr+0.750L	1	0.2799	7.500		0.0000	0.000

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

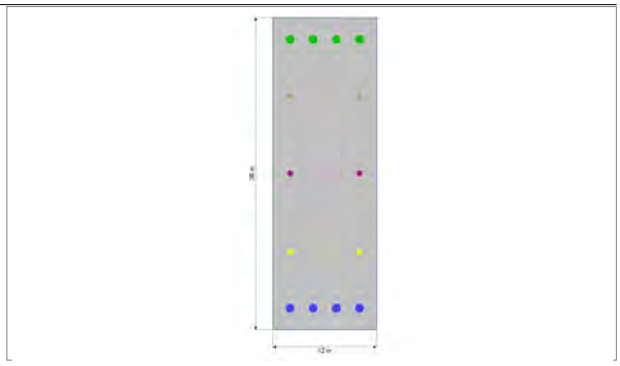
DESCRIPTION: 2B-3 (12"x36")

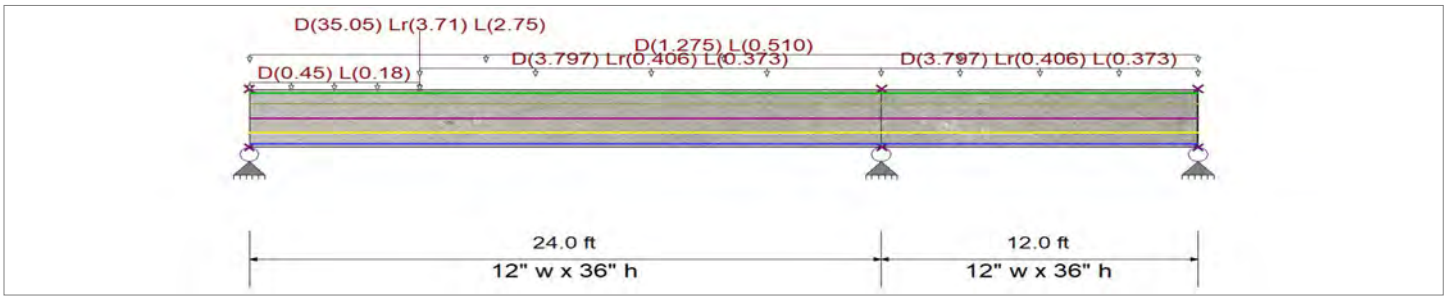
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	= 5.0 ksi	ϕ Phi Values	Flexure :	0.90	
fr = f'c$^{1/2}$ *7.50	= 530.33 psi		Shear :	0.750	
ψ Density	= 145.0 pcf	β_1	=	0.80	
λ LtWt Factor	= 1.0				
Elastic Modulus	= 4,030.51 ksi	Fy - Stirrups		40.0 ksi	
fy - Main Rebar	= 60.0 ksi	E - Stirrups	=	29,000.0 ksi	
E - Main Rebar	= 29,000.0 ksi	Stirrup Bar Size #		3	
		Number of Resisting Legs Per Stirrup	=	3.0	





Cross Section & Reinforcing Details

Rectangular Section, Width = 12.0 in, Height = 36.0 in
Span #1 Reinforcing....
4-#8 at 2.50 in from Bottom, from 0.0 to 24.0 ft in this span
2-#5 at 18.0 in from Bottom, from 0.0 to 24.0 ft in this span
2-#5 at 9.0 in from Top, from 0.0 to 24.0 ft in this span
4-#8 at 2.50 in from Top, from 0.0 to 24.0 ft in this span
2-#5 at 9.0 in from Bottom, from 0.0 to 24.0 ft in this spa
Span #2 Reinforcing....
4-#8 at 2.50 in from Bottom, from 0.0 to 12.0 ft in this span
2-#5 at 18.0 in from Bottom, from 0.0 to 12.0 ft in this span
2-#5 at 9.0 in from Top, from 0.0 to 12.0 ft in this span
4-#8 at 2.50 in from Top, from 0.0 to 12.0 ft in this span
2-#5 at 9.0 in from Bottom, from 0.0 to 12.0 ft in this spa

Beam self weight calculated and added to loads
Loads on all spans...
D = 1.275, L = 0.510
Uniform Load on ALL spans : D = 1.275, L = 0.510 k/ft

Load for Span Number 1
Point Load : D = 35.050, Lr = 3.710, L = 2.750 k @ 6.50 ft
Uniform Load : D = 0.450, L = 0.180 k/ft, Extent = 0.0 -->> 6.50 ft, Tributary Width = 1.0 ft
Uniform Load : D = 3.797, Lr = 0.4060, L = 0.3730 k/ft, Extent = 6.50 -->> 24.0 ft, Tributary Width = 1.0 ft

Load for Span Number 2
Uniform Load : D = 3.797, Lr = 0.4060, L = 0.3730 k/ft, Tributary Width = 1.0 ft

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-3 (12"x36")

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.880**	: 1
Section used for this span	**Typical Section**	
Mu : Applied	-510.39	k-ft
Mn * Phi : Allowable	580.0	k-ft
Location of maximum on span	0.000	ft
Span # where maximum occurs	Span # 2	

Maximum Deflection

Max Downward Transient Deflection	0.022 in	Ratio =	12922	>=360.0	Span: 2 : L Only
Max Upward Transient Deflection	-0.002 in	Ratio =	61665	>=360.0	Span: 2 : L Only
Max Downward Total Deflection	0.411 in	Ratio =	700	>=180.0	Span: 2 : +D+L
Max Upward Total Deflection	-0.037 in	Ratio =	3940	>=180.0	Span: 2 : +D+0.750Lr+0.750L

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2	Support 3
Overall MAXimum	68.150	175.863	4.332
Overall MINimum	4.186	11.283	0.218
D Only	57.952	150.311	3.283
+D+L	67.345	173.153	4.332
+D+Lr	62.154	161.549	3.530
+D+0.750Lr+0.750L	68.150	175.863	4.261
+D+0.750L	64.994	167.451	4.065
+0.60D	34.536	90.892	1.499
Lr Only	4.186	11.283	0.218
L Only	9.354	22.956	0.973

Shear Stirrup Requirements

Between 0.00 to 6.47 ft, PhiVc < Vu, Req'd Vs = 21.411, use #3 stirrups spaced at 8.000 in
Between 6.56 to 10.78 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 10.87 to 13.52 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 16.000 in
Between 13.62 to 29.24 ft, PhiVc < Vu, Req'd Vs = 0.3488, use #3 stirrups spaced at 4.000 in
Between 29.29 to 32.18 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 16.000 in
Between 32.23 to 35.95 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	24.000	503.20	580.00	0.87
Span # 2	2	12.000	-510.39	580.00	0.88
+1.40D					
Span # 1	1	24.000	479.20	580.00	0.83
Span # 2	2	12.000	-483.06	580.00	0.83
+1.20D+0.50Lr+1.60L					
Span # 1	1	24.000	502.20	580.00	0.87
Span # 2	2	12.000	-510.39	580.00	0.88
+1.20D+1.60L					
Span # 1	1	24.000	488.50	580.00	0.84
Span # 2	2	12.000	-497.08	580.00	0.86
+1.20D+1.60Lr+L					
Span # 1	1	24.000	503.20	580.00	0.87
Span # 2	2	12.000	-508.54	580.00	0.88
+1.20D+1.60Lr					
Span # 1	1	24.000	454.65	580.00	0.78
Span # 2	2	12.000	-456.64	580.00	0.79
+1.20D+L					
Span # 1	1	24.000	459.32	580.00	0.79
Span # 2	2	12.000	-465.95	580.00	0.80
+1.20D					
Span # 1	1	24.000	410.75	580.00	0.71
Span # 2	2	12.000	-414.05	580.00	0.71
+1.20D+0.50Lr+L					
Span # 1	1	24.000	473.03	580.00	0.82
Span # 2	2	12.000	-479.26	580.00	0.83
+0.90D					

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-3 (12"x36")

Load Combination Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
Span # 1	1	24.000	308.06	580.00	0.53
Span # 2	2	12.000	-310.54	580.00	0.54

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+0.750Lr+0.750L	1	0.4110	10.629	+D+0.750Lr+0.750L	-0.0037	24.171
+D+L	2	0.0000	12.000	+D+0.750Lr+0.750L	-0.0365	3.600

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

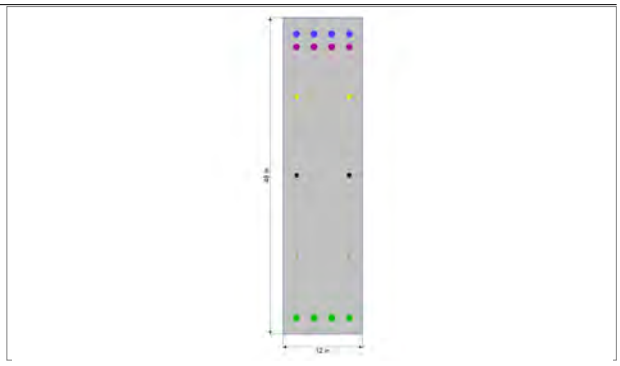
DESCRIPTION: 2B-4 (12"x48")

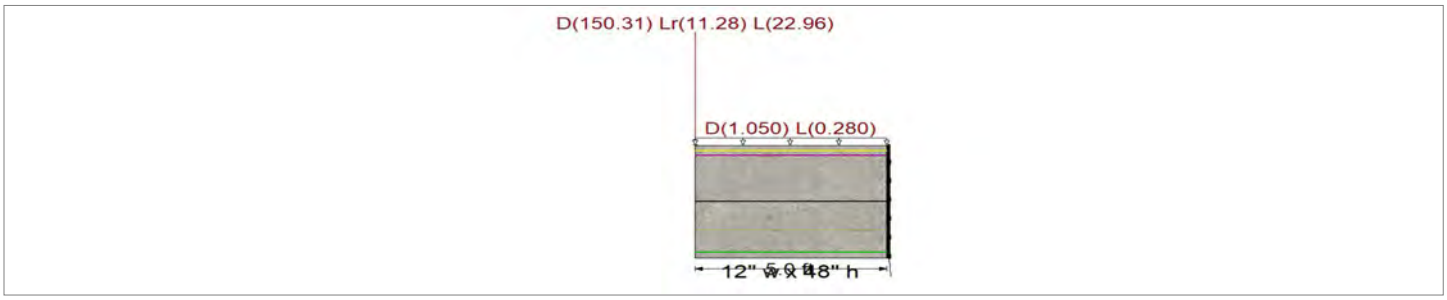
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ *7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	4.0





Cross Section & Reinforcing Details

Rectangular Section, Width = 12.0 in, Height = 48.0 in
Span #1 Reinforcing....

4-#8 at 2.50 in from Top, from 0.0 to 5.0 ft in this span
4-#8 at 4.50 in from Top, from 0.0 to 5.0 ft in this span
2-#5 at 12.0 in from Top, from 0.0 to 5.0 ft in this span
4-#8 at 2.50 in from Bottom, from 0.0 to 5.0 ft in this spa
2-#5 at 24.0 in from Bottom, from 0.0 to 5.0 ft in this spa
2-#5 at 12.0 in from Bottom, from 0.0 to 5.0 ft in this spa

Beam self weight calculated and added to loads
Loads on all spans...
D = 1.050, L = 0.280

Uniform Load on ALL spans : D = 1.050, L = 0.280 k/ft

Point Load : D = 150.310, Lr = 11.280, L = 22.960 k @ 0.0 ft, (2B-3 (2))

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.843**	: 1
Section used for this span	**Typical Section**	
Mu : Applied	-1,143.60	k-ft
Mn * Phi : Allowable	1,356.62	k-ft
Location of maximum on span	5.000	ft
Span # where maximum occurs	Span # 1	

Maximum Deflection

Max Downward Transient Deflection	0.004 in	Ratio =	31632	>=360.0	Span: 1 : Lr Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 1 : L Only
Max Downward Total Deflection	0.053 in	Ratio =	2246	>=180.0	Span: 1 : +D+0.750Lr+0.750L
Max Upward Total Deflection	0.000 in	Ratio =	0	<180.0	Span: 1 : +D+0.750Lr+0.750L

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
Overall MAXimum		185.190
Overall MINimum		11.280
D Only		158.460

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-4 (12"x48")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
+D+L		182.820
+D+Lr		169.740
+D+0.750Lr+0.750L		185.190
+D+0.750L		176.730
+0.60D		95.076
Lr Only		11.280
L Only		24.360

Shear Stirrup Requirements

Between 0.00 to 0.00 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 0.01 to 4.99 ft, Vs>(4bdf'c^.5) ACI 9.7.6.2.2, Req'd Vs = 168.718, use #3 stirrups spaced at 3.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	5.000	-1,143.60	1,356.62	0.84
+1.40D					
Span # 1	1	5.000	-1,080.52	1,356.62	0.80
+1.20D+0.50Lr+1.60L					
Span # 1	1	5.000	-1,143.60	1,356.62	0.84
+1.20D+1.60L					
Span # 1	1	5.000	-1,115.41	1,356.62	0.82
+1.20D+1.60Lr+L					
Span # 1	1	5.000	-1,134.67	1,356.62	0.84
+1.20D+1.60Lr					
Span # 1	1	5.000	-1,016.38	1,356.62	0.75
+1.20D+L					
Span # 1	1	5.000	-1,044.44	1,356.62	0.77
+1.20D					
Span # 1	1	5.000	-926.16	1,356.62	0.68
+1.20D+0.50Lr+L					
Span # 1	1	5.000	-1,072.64	1,356.62	0.79
+0.90D					
Span # 1	1	5.000	-694.62	1,356.62	0.51

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+0.750Lr+0.750L	1	0.0534	0.000		0.0000	0.000

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

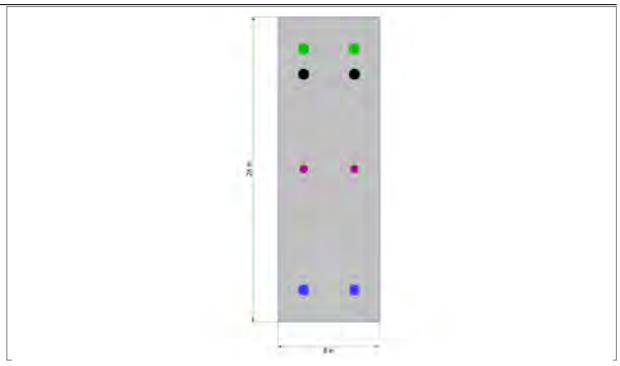
DESCRIPTION: 2B-5 (8"x24")

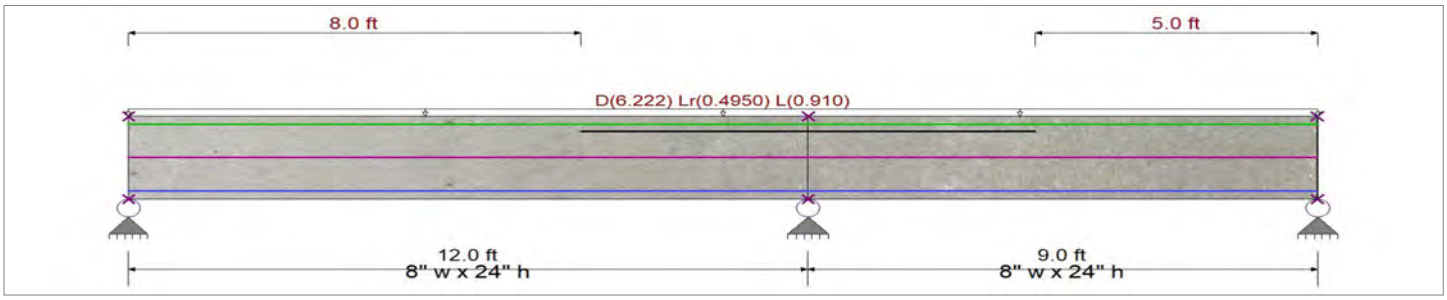
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ *7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2





Cross Section & Reinforcing Details

Rectangular Section, Width = 8.0 in, Height = 24.0 in
Span #1 Reinforcing....
2-#7 at 2.50 in from Bottom, from 0.0 to 12.0 ft in this span
2-#5 at 12.0 in from Bottom, from 0.0 to 12.0 ft in this span
2-#7 at 2.50 in from Top, from 0.0 to 12.0 ft in this span
2-#7 at 4.50 in from Top, from 8.0 to 0.0 ft in this span
Span #2 Reinforcing....
2-#7 at 2.50 in from Bottom, from 0.0 to 9.0 ft in this span
2-#5 at 12.0 in from Bottom, from 0.0 to 9.0 ft in this span
2-#7 at 2.50 in from Top, from 0.0 to 9.0 ft in this span
2-#7 at 4.50 in from Top, from 0.0 to 4.0 ft in this span

Beam self weight calculated and added to loads
Loads on all spans...
D = 6.222, Lr = 0.4950, L = 0.910
Uniform Load on ALL spans : D = 6.222, Lr = 0.4950, L = 0.910 k/ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.784** : 1	
Section used for this span	**Typical Section**	
Mu : Applied	107.464	k-ft
Mn * Phi : Allowable	137.067	k-ft
Location of maximum on span	4.800	ft
Span # where maximum occurs	Span # 1	

Maximum Deflection

Max Downward Transient Deflection	0.006 in	Ratio =	24183	>=360.0	Span: 2 : L Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 2 : L Only
Max Downward Total Deflection	0.127 in	Ratio =	1135	>=180.0	Span: 2 : +0.60D
Max Upward Total Deflection	-0.008 in	Ratio =	13498	>=180.0	Span: 2 : +D+0.750Lr+0.750L

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2	Support 3
Overall MAXimum	34.752	101.903	20.195
Overall MINimum	2.367	6.605	1.423

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-5 (8"x24")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2	Support 3
D Only	29.959	87.272	17.491
+D+L	34.093	99.920	19.819
+D+Lr	32.207	94.154	18.756
+D+0.750Lr+0.750L	34.752	101.903	20.195
+D+0.750L	33.058	96.762	19.234
+0.60D	18.435	51.291	11.108
Lr Only	2.367	6.605	1.423
L Only	4.351	12.143	2.616

Shear Stirrup Requirements

Between 0.00 to 2.79 ft, PhiVc < Vu, Req'd Vs = 0.08612, use #3 stirrups spaced at 5.000 in
Between 2.84 to 3.82 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 3.87 to 5.73 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 5.78 to 6.76 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 6.81 to 15.89 ft, PhiVc < Vu, Req'd Vs = 0.2331, use #3 stirrups spaced at 3.000 in
Between 15.93 to 17.11 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 17.14 to 19.13 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 19.16 to 20.96 ft, PhiVc < Vu, Req'd Vs = 6.287, use #3 stirrups spaced at 10.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	12.000	107.46	137.07	0.78
Span # 2	2	9.000	-137.50	226.27	0.61
+1.40D					
Span # 1	1	12.000	102.66	137.07	0.75
Span # 2	2	9.000	-131.35	226.27	0.58
+1.20D+0.50Lr+1.60L					
Span # 1	1	12.000	107.46	137.07	0.78
Span # 2	2	9.000	-137.50	226.27	0.61
+1.20D+1.60L					
Span # 1	1	12.000	104.63	137.07	0.76
Span # 2	2	9.000	-133.88	226.27	0.59
+1.20D+1.60Lr+L					
Span # 1	1	12.000	107.45	137.07	0.78
Span # 2	2	9.000	-137.48	226.27	0.61
+1.20D+1.60Lr					
Span # 1	1	12.000	97.05	137.07	0.71
Span # 2	2	9.000	-124.17	226.27	0.55
+1.20D+L					
Span # 1	1	12.000	98.39	137.07	0.72
Span # 2	2	9.000	-125.90	226.27	0.56
+1.20D					
Span # 1	1	12.000	87.99	137.07	0.64
Span # 2	2	9.000	-112.59	226.27	0.50
+1.20D+0.50Lr+L					
Span # 1	1	12.000	101.22	137.07	0.74
Span # 2	2	9.000	-129.52	226.27	0.57
+0.90D					
Span # 1	1	12.000	65.99	137.07	0.48
Span # 2	2	9.000	-84.44	226.27	0.37

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+0.750Lr+0.750L	1	0.1269	5.314	+D+0.750Lr+0.750L	-0.0014	12.129
+0.60D	2	0.0027	6.043	+D+0.750Lr+0.750L	-0.0080	1.414

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

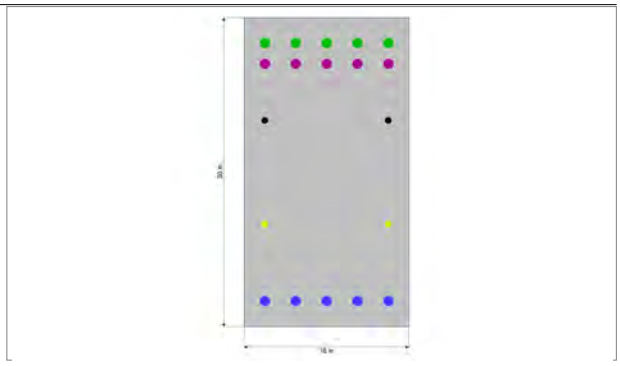
DESCRIPTION: 2B-6 (16"x30")

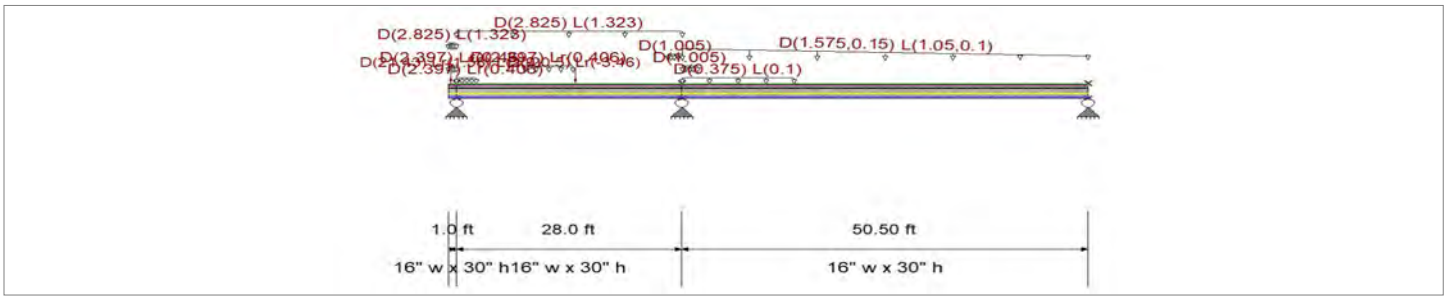
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ *7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	5.0





Cross Section & Reinforcing Details

Rectangular Section, Width = 16.0 in, Height = 30.0 in

Span #1 Reinforcing....
- 5-#8 at 2.50 in from Bottom, from 0.0 to 1.0 ft in this span
- 5-#8 at 4.50 in from Top, from 0.0 to 1.0 ft in this span
- 2-#5 at 10.0 in from Bottom, from 0.0 to 1.0 ft in this span
- 5-#8 at 2.50 in from Top, from 0.0 to 1.0 ft in this span
- 2-#5 at 10.0 in from Top, from 0.0 to 1.0 ft in this span

Span #2 Reinforcing....
- 5-#8 at 2.50 in from Bottom, from 0.0 to 28.0 ft in this span
- 5-#8 at 4.50 in from Top, from 0.0 to 28.0 ft in this span
- 2-#5 at 10.0 in from Bottom, from 0.0 to 28.0 ft in this span
- 5-#8 at 2.50 in from Top, from 0.0 to 28.0 ft in this span
- 2-#5 at 10.0 in from Top, from 0.0 to 28.0 ft in this span

Span #3 Reinforcing....
- 5-#8 at 2.50 in from Bottom, from 0.0 to 50.50 ft in this span
- 5-#8 at 4.50 in from Top, from 0.0 to 50.50 ft in this span
- 2-#5 at 10.0 in from Bottom, from 0.0 to 50.50 ft in this span
- 5-#8 at 2.50 in from Top, from 0.0 to 50.50 ft in this spa
- 2-#5 at 10.0 in from Top, from 0.0 to 50.50 ft in this spa

Beam self weight calculated and added to loads

Load for Span Number 1
- Point Load : D = 21.430, Lr = 1.580, L = 2.90 k @ 0.330 ft, (Reacc. 2B-5)
- Uniform Load : D = 2.397, Lr = 0.460 k/ft, Tributary Width = 1.0 ft
- Uniform Load : D = 2.825, L = 1.323 k/ft, Tributary Width = 1.0 ft

Load for Span Number 2
- Point Load : D = -20.50, Lr = -3.460 k @ 14.750 ft, (Reacc. RB-3)
- Uniform Load : D = 2.397, Lr = 0.4060 k/ft, Extent = 0.0 -->> 2.50 ft, Tributary Width = 1.0 ft
- Uniform Load : D = 2.397, Lr = 0.4060 k/ft, Extent = 8.50 -->> 14.50 ft, Tributary Width = 1.0 ft
- Uniform Load : D = 1.005 k/ft, Extent = 26.250 -->> 28.0 ft, Tributary Width = 1.0 ft
- Uniform Load : D = 2.825, L = 1.323 k/ft, Tributary Width = 1.0 ft

Load for Span Number 3
- Uniform Load : D = 0.3750, L = 0.10 k/ft, Extent = 0.0 -->> 14.0 ft, Tributary Width = 1.0 ft
- Uniform Load : D = 1.005 k/ft, Extent = 0.0 -->> 2.0 ft, Tributary Width = 1.0 ft

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-6 (16"x30")

Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Varying Uniform Load : D= 1.575->0.150, L= 1.050->0.10 k/ft, Extent = 0.0 -->> 50.50 ft, Trib Width = 1.0 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.876**	: 1
Section used for this span	**Typical Section**	
Mu : Applied	467.579	k-ft
Mn * Phi : Allowable	533.78	k-ft
Location of maximum on span	28.280	ft
Span # where maximum occurs	Span # 3	

Maximum Deflection

Max Downward Transient Deflection	0.284 in	Ratio =	2137	>=360.0	Span: 3 : Lr Only
Max Upward Transient Deflection	-0.005 in	Ratio =	4944	>=360.0	Span: 3 : L Only
Max Downward Total Deflection	1.901 in	Ratio =	318	>=180.0	Span: 3 : +D+L
Max Upward Total Deflection	-0.019 in	Ratio =	1280	>=180.0	Span: 3 : +D+L

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2	Support 3	Support 4
Overall MAXimum		78.346	163.332	26.653
Overall MINimum		2.943	-0.961	0.049
D Only		62.809	113.831	19.986
+D+L		78.346	163.332	26.653
+D+Lr		65.728	112.907	20.022
+D+0.750Lr+0.750L		76.631	150.294	25.002
+D+0.750L		74.438	150.995	24.973
+0.60D		38.414	67.167	12.396
Lr Only		2.943	-0.961	0.049
L Only		16.923	47.346	7.436

Shear Stirrup Requirements

Between 0.00 to 0.33 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 0.33 to 6.41 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 13.000 in
Between 6.60 to 12.57 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 12.76 to 21.35 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 13.000 in
Between 21.53 to 37.75 ft, PhiVc < Vu, Req'd Vs = 0.5284, use #3 stirrups spaced at 7.000 in
Between 38.09 to 48.19 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 13.000 in
Between 48.53 to 68.39 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 68.73 to 79.16 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 13.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	1.000	-25.20	917.77	0.03
Span # 2	2	28.000	-744.73	917.77	0.81
Span # 3	3	50.500	467.58	533.78	0.88
+1.40D					
Span # 1	1	1.000	-23.84	917.77	0.03
Span # 2	2	28.000	-550.98	917.77	0.60
Span # 3	3	50.500	354.40	533.78	0.66
+1.20D+0.50Lr+1.60L					
Span # 1	1	1.000	-25.20	917.77	0.03
Span # 2	2	28.000	-743.58	917.77	0.81
Span # 3	3	50.500	467.58	533.78	0.88
+1.20D+1.60L					
Span # 1	1	1.000	-24.56	917.77	0.03
Span # 2	2	28.000	-744.73	917.77	0.81
Span # 3	3	50.500	467.03	533.78	0.87
+1.20D+1.60Lr+L					
Span # 1	1	1.000	-25.05	917.77	0.03
Span # 2	2	28.000	-638.86	917.77	0.70
Span # 3	3	50.500	407.55	533.78	0.76
+1.20D+1.60Lr					
Span # 1	1	1.000	-22.48	917.77	0.02

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-6 (16"x30")

Load Combination Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
Span # 2	2	28.000	-468.57	917.77	0.51
Span # 3	3	50.500	305.49	533.78	0.57
+1.20D+L					
Span # 1	1	1.000	-23.01	917.77	0.03
Span # 2	2	28.000	-642.56	917.77	0.70
Span # 3	3	50.500	405.80	533.78	0.76
+1.20D					
Span # 1	1	1.000	-20.44	917.77	0.02
Span # 2	2	28.000	-472.27	917.77	0.51
Span # 3	3	50.500	303.77	533.78	0.57
+1.20D+0.50Lr+L					
Span # 1	1	1.000	-23.65	917.77	0.03
Span # 2	2	28.000	-641.40	917.77	0.70
Span # 3	3	50.500	406.34	533.78	0.76
+0.90D					
Span # 1	1	1.000	-15.33	917.77	0.02
Span # 2	2	28.000	-354.20	917.77	0.39
Span # 3	3	50.500	227.83	533.78	0.43

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+L	1	0.0105	1.560	+D+L	-0.0187	0.000
+D+L	2	0.1125	8.400	+D+L	-0.0715	24.080
+D+L	3	1.9014	27.270		0.0000	24.080

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

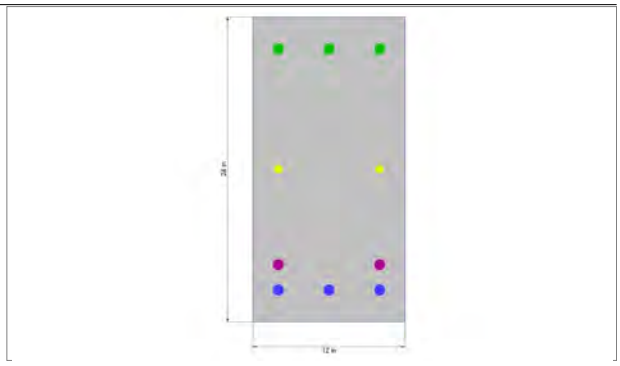
DESCRIPTION: 2B-7 (12"x24")

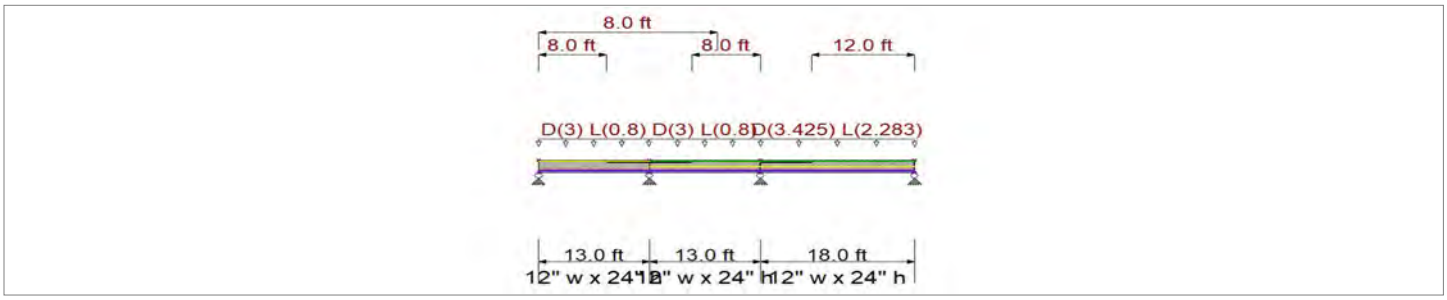
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ * 7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	3.0





Cross Section & Reinforcing Details

Rectangular Section, Width = 12.0 in, Height = 24.0 in

Span #1 Reinforcing....
- 3-#7 at 2.50 in from Bottom, from 0.0 to 13.0 ft in this span
- 2-#7 at 4.50 in from Bottom, from 0.0 to 13.0 ft in this span
- 2-#5 at 12.0 in from Top, from 0.0 to 13.0 ft in this span
- 3-#7 at 2.50 in from Top, from 0.0 to 13.0 ft in this span
- 2-#7 at 4.50 in from Top, from 8.0 to 0.0 ft in this span

Span #2 Reinforcing....
- 3-#7 at 2.50 in from Bottom, from 0.0 to 13.0 ft in this span
- 2-#7 at 4.50 in from Bottom, from 0.0 to 13.0 ft in this span
- 2-#5 at 12.0 in from Bottom, from 0.0 to 13.0 ft in this span
- 3-#7 at 2.50 in from Top, from 0.0 to 13.0 ft in this span
- 2-#7 at 4.50 in from Top, from 0.0 to 5.0 ft in this span
- 2-#7 at 4.50 in from Top, from 8.0 to 0.0 ft in this span

Span #3 Reinforcing....
- 3-#7 at 2.50 in from Bottom, from 0.0 to 18.0 ft in this span
- 2-#7 at 4.50 in from Bottom, from 0.0 to 18.0 ft in this span
- 2-#5 at 12.0 in from Bottom, from 0.0 to 18.0 ft in this span
- 3-#7 at 2.50 in from Top, from 0.0 to 18.0 ft in this span
- 2-#7 at 4.50 in from Top, from 0.0 to 6.0 ft in this span

Beam self weight calculated and added to loads

Load for Span Number 1
Uniform Load : D = 3.0, L = 0.80 k/ft, Tributary Width = 1.0 ft

Load for Span Number 2
Uniform Load : D = 3.0, L = 0.80 k/ft, Tributary Width = 1.0 ft

Load for Span Number 3
Uniform Load : D = 3.425, L = 2.283 k/ft, Tributary Width = 1.0 ft

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 | Arch-Deco Design & Construction Inc. | (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-7 (12"x24")

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio = **0.796** : 1

Section used for this span	**Typical Section**
Mu : Applied	225.319 k-ft
Mn * Phi : Allowable	283.240 k-ft
Location of maximum on span	10.560 ft
Span # where maximum occurs	Span # 3

Maximum Deflection

Max Downward Transient Deflection	0.074 in	Ratio =	2909	>=360.0	Span: 3 : L Only
Max Upward Transient Deflection	-0.013 in	Ratio =	11570	>=360.0	Span: 3 : L Only
Max Downward Total Deflection	0.334 in	Ratio =	646	>=180.0	Span: 3 : +D+L
Max Upward Total Deflection	-0.043 in	Ratio =	3625	>=180.0	Span: 3 : +D+L

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2	Support 3	Support 4
Overall MAXimum	23.311	46.176	100.617	44.201
Overall MINimum	5.166	5.420	34.408	16.388
D Only	18.344	40.055	66.929	27.082
+D+L	23.311	46.176	100.617	44.201
+D+0.750L	22.122	44.495	92.325	39.889
+0.60D	10.898	24.443	39.717	16.388
L Only	5.166	5.420	34.408	16.901

Shear Stirrup Requirements

Between 0.00 to 2.95 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 3.03 to 8.41 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 8.49 to 14.39 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 14.47 to 19.93 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 20.02 to 25.91 ft, PhiVc < Vu, Req'd Vs = 18.177, use #3 stirrups spaced at 10.000 in
Between 26.00 to 33.08 ft, PhiVc < Vu, Req'd Vs = 0.1803, use #3 stirrups spaced at 3.000 in
Between 33.20 to 34.88 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 35.00 to 38.12 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 38.24 to 39.92 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 40.04 to 43.88 ft, PhiVc < Vu, Req'd Vs = 27.673, use #3 stirrups spaced at 7.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	13.000	85.09	283.24	0.30
Span # 2	2	13.000	-221.65	284.28	0.78
Span # 3	3	18.000	225.32	283.24	0.80
+1.40D					
Span # 1	1	13.000	69.76	283.24	0.25
Span # 2	2	13.000	-147.42	284.28	0.52
Span # 3	3	18.000	-150.60	284.28	0.53
+1.20D+1.60L					
Span # 1	1	13.000	85.09	283.24	0.30
Span # 2	2	13.000	-221.65	284.28	0.78
Span # 3	3	18.000	225.32	283.24	0.80
+1.20D+L					
Span # 1	1	13.000	75.58	283.24	0.27
Span # 2	2	13.000	-185.92	284.28	0.65
Span # 3	3	18.000	-189.49	284.28	0.67
+1.20D					
Span # 1	1	13.000	59.79	283.24	0.21
Span # 2	2	13.000	-126.36	284.28	0.44
Span # 3	3	18.000	-129.09	284.28	0.45
+0.90D					
Span # 1	1	13.000	44.84	283.24	0.16
Span # 2	2	13.000	-94.77	284.28	0.33
Span # 3	3	18.000	-96.82	284.28	0.34

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+L	1	0.0447	5.980	+D+L	-0.0019	13.260

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-7 (12"x24")

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+L	2	0.0121	13.360	+D+L	-0.0430	9.100
+D+L	3	0.3343	9.720		0.0000	9.100

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

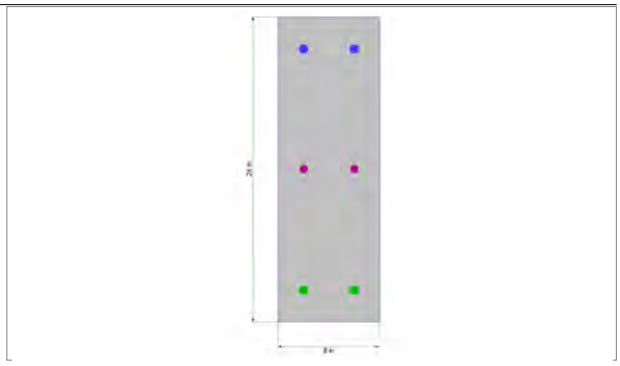
DESCRIPTION: 2B-8 (8"x24")

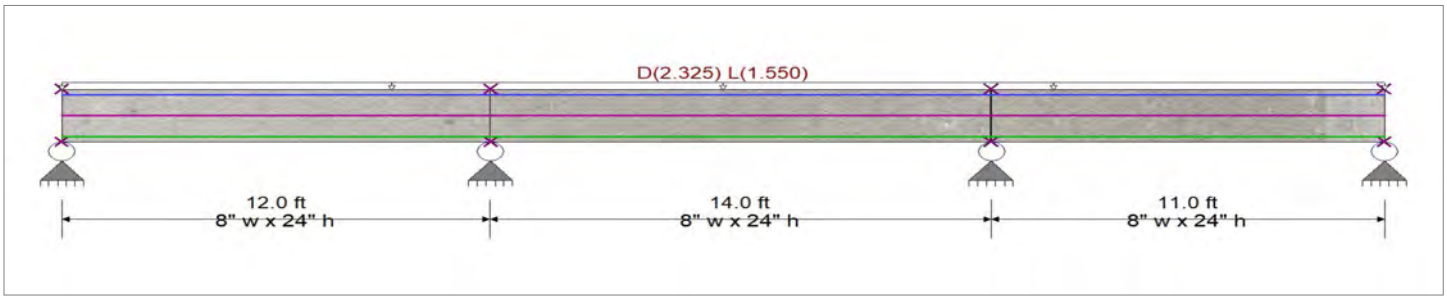
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ *7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2.0





Cross Section & Reinforcing Details

Rectangular Section, Width = 8.0 in, Height = 24.0 in

Span #1 Reinforcing....
- 2-#6 at 2.50 in from Top, from 0.0 to 12.0 ft in this span
- 2-#6 at 2.50 in from Bottom, from 0.0 to 12.0 ft in this s
- 2-#5 at 12.0 in from Top, from 0.0 to 12.0 ft in this span

Span #2 Reinforcing....
- 2-#6 at 2.50 in from Top, from 0.0 to 14.0 ft in this span
- 2-#6 at 2.50 in from Bottom, from 0.0 to 14.0 ft in this s
- 2-#5 at 12.0 in from Top, from 0.0 to 14.0 ft in this span

Span #3 Reinforcing....
- 2-#6 at 2.50 in from Top, from 0.0 to 11.0 ft in this span
- 2-#6 at 2.50 in from Bottom, from 0.0 to 11.0 ft in this s
- 2-#5 at 12.0 in from Top, from 0.0 to 11.0 ft in this span

Beam self weight calculated and added to loads
Loads on all spans...
D = 0.150, L = 0.10

Uniform Load on ALL spans : D = 0.150, L = 0.10 ksf, Tributary Width = 15.50 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.869**	: 1
Section used for this span	**Typical Section**	
Mu : Applied	-95.295	k-ft
Mn * Phi : Allowable	109.676	k-ft
Location of maximum on span	0.000	ft
Span # where maximum occurs	Span # 2	

Maximum Deflection

Max Downward Transient Deflection	0.008 in	Ratio =	16973	>=360.0	Span: 3 : L Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 3 : L Only
Max Downward Total Deflection	0.037 in	Ratio =	3898	>=180.0	Span: 3 : +D+L
Max Upward Total Deflection	0.000 in	Ratio =	0	<180.0	Span: 3 : +D+L

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 | Arch-Deco Design & Construction Inc. | (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-8 (8"x24")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2	Support 3	Support 4
Overall MAXimum	19.571	58.100	55.286	17.571
Overall MINimum	6.885	22.018	20.826	6.178
D Only	11.651	36.354	34.898	10.276
+D+L	19.571	58.100	55.286	17.571
+D+0.750L	17.677	52.443	50.400	15.670
+0.60D	6.885	22.018	20.826	6.178
L Only	7.063	22.586	21.364	6.337

Shear Stirrup Requirements

Between 0.00 to 2.88 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 2.96 to 6.16 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 6.24 to 8.16 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 8.24 to 15.45 ft, PhiVc < Vu, Req'd Vs = 0.07540, use #3 stirrups spaced at 6.000 in
Between 15.55 to 17.41 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 17.51 to 20.77 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 20.87 to 22.73 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 22.83 to 29.23 ft, PhiVc < Vu, Req'd Vs = 0.1297, use #3 stirrups spaced at 7.000 in
Between 29.30 to 31.21 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 31.28 to 34.58 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 34.65 to 36.93 ft, PhiVc < Vu, Req'd Vs = 2.352, use #3 stirrups spaced at 10.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	12.000	-92.04	109.68	0.84
Span # 2	2	14.000	-95.30	109.68	0.87
Span # 3	3	11.000	-85.42	109.68	0.78
+1.40D					
Span # 1	1	12.000	-58.98	109.68	0.54
Span # 2	2	14.000	-61.06	109.68	0.56
Span # 3	3	11.000	-54.74	109.68	0.50
+1.20D+1.60L					
Span # 1	1	12.000	-92.04	109.68	0.84
Span # 2	2	14.000	-95.30	109.68	0.87
Span # 3	3	11.000	-85.42	109.68	0.78
+1.20D+L					
Span # 1	1	12.000	-76.48	109.68	0.70
Span # 2	2	14.000	-79.19	109.68	0.72
Span # 3	3	11.000	-70.98	109.68	0.65
+1.20D					
Span # 1	1	12.000	-50.55	109.68	0.46
Span # 2	2	14.000	-52.34	109.68	0.48
Span # 3	3	11.000	-46.92	109.68	0.43
+0.90D					
Span # 1	1	12.000	-37.91	109.68	0.35
Span # 2	2	14.000	-39.26	109.68	0.36
Span # 3	3	11.000	-35.19	109.68	0.32

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+L	1	0.0369	5.040		0.0000	0.000
+D+L	2	0.0300	7.000		0.0000	0.000
+D+L	3	0.0174	6.380		0.0000	0.000

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

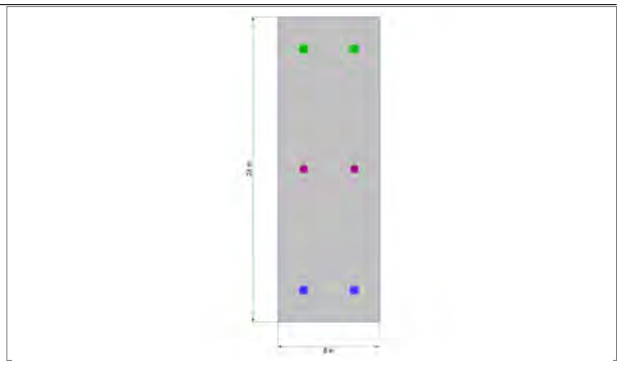
DESCRIPTION: 2B-9 (8"x24")

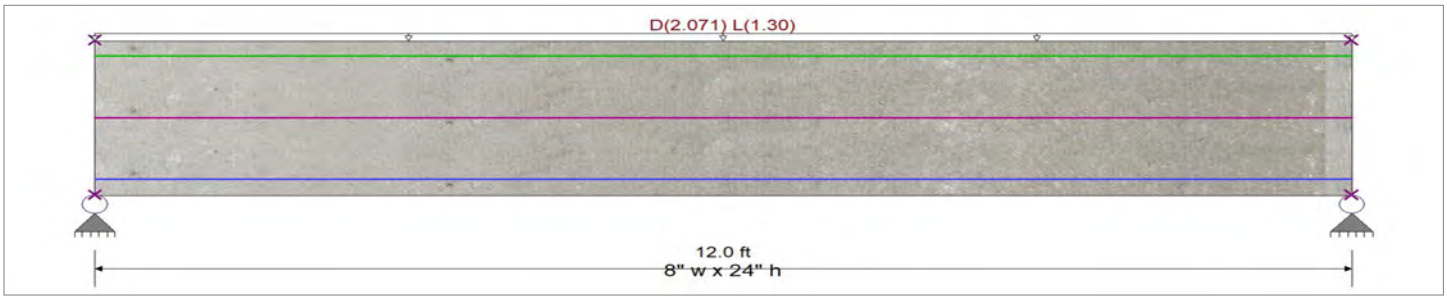
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ * 7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2





Cross Section & Reinforcing Details

Rectangular Section, Width = 8.0 in, Height = 24.0 in
Span #1 Reinforcing....
2-#6 at 2.50 in from Bottom, from 0.0 to 12.0 ft in this span
2-#5 at 12.0 in from Bottom, from 0.0 to 12.0 ft in this span
2-#6 at 2.50 in from Top, from 0.0 to 12.0 ft in this span

Beam self weight calculated and added to loads
Loads on all spans...
D = 2.071, L = 1.30

Uniform Load on ALL spans : D = 2.071, L = 1.30 k/ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.787**	: 1
Section used for this span	**Typical Section**	
Mu : Applied	86.349	k-ft
Mn * Phi : Allowable	109.676	k-ft
Location of maximum on span	5.989	ft
Span # where maximum occurs	Span # 1	

Maximum Deflection

Max Downward Transient Deflection	0.016 in	Ratio =	8821	>=360.0	Span: 1 : L Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 1 : L Only
Max Downward Total Deflection	0.125 in	Ratio =	1149	>=180.0	Span: 1 : +D+L
Max Upward Total Deflection	0.000 in	Ratio =	0	<180.0	Span: 1 : +D+L

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
Overall MAXimum	21.386	21.386
Overall MINimum	7.800	7.800
D Only	13.586	13.586
+D+L	21.386	21.386
+D+0.750L	19.436	19.436

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-9 (8"x24")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
+0.60D	8.152	8.152
L Only	7.800	7.800

Shear Stirrup Requirements

Between 0.00 to 4.13 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 4.15 to 7.85 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 7.87 to 11.98 ft, PhiVc < Vu, Req'd Vs = 8.640, use #3 stirrups spaced at 10.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	12.000	86.35	109.68	0.79
+1.40D					
Span # 1	1	12.000	57.06	109.68	0.52
+1.20D+1.60L					
Span # 1	1	12.000	86.35	109.68	0.79
+1.20D+L					
Span # 1	1	12.000	72.31	109.68	0.66
+1.20D					
Span # 1	1	12.000	48.91	109.68	0.45
+0.90D					
Span # 1	1	12.000	36.68	109.68	0.33

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+L	1	0.1253	6.000		0.0000	0.000

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

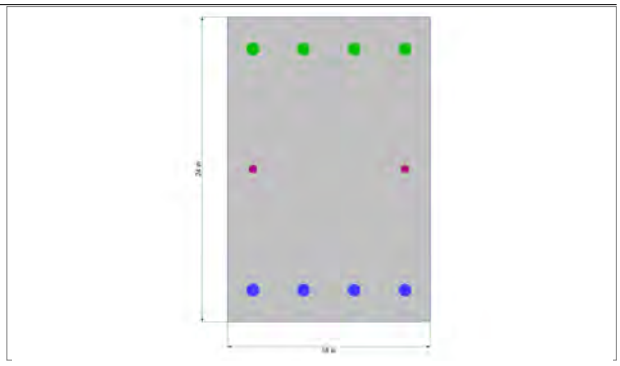
DESCRIPTION: 2B-10 (16"x24")

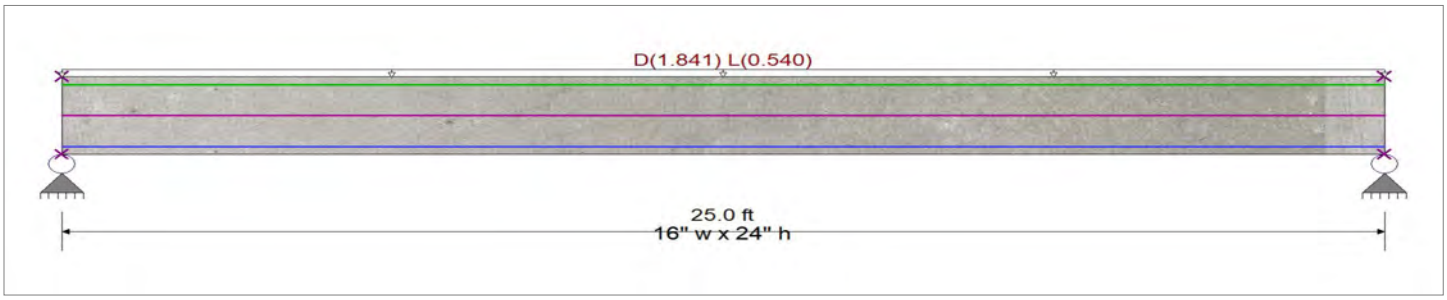
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ *7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2





Cross Section & Reinforcing Details

Rectangular Section, Width = 16.0 in, Height = 24.0 in
Span #1 Reinforcing....
4-#8 at 2.50 in from Bottom, from 0.0 to 25.0 ft in this span
2-#5 at 12.0 in from Bottom, from 0.0 to 25.0 ft in this span
4-#8 at 2.50 in from Top, from 0.0 to 25.0 ft in this span

Beam self weight calculated and added to loads
Loads on all spans...
D = 1.841, L = 0.540

Uniform Load on ALL spans : D = 1.841, L = 0.540 k/ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.884**	: 1
Section used for this span	**Typical Section**	
Mu : Applied	276.343	k-ft
Mn * Phi : Allowable	312.619	k-ft
Location of maximum on span	12.523	ft
Span # where maximum occurs	Span # 1	

Maximum Deflection

Max Downward Transient Deflection	0.064 in	Ratio =	4697	>=360.0	Span: 1 : L Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 1 : L Only
Max Downward Total Deflection	0.823 in	Ratio =	364	>=180.0	Span: 1 : +D+L
Max Upward Total Deflection	0.000 in	Ratio =	0	<180.0	Span: 1 : +D+L

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
Overall MAXimum	34.596	34.596
Overall MINimum	6.750	6.750
D Only	27.846	27.846
+D+L	34.596	34.596
+D+0.750L	32.908	32.908

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-10 (16"x24")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
+0.60D	16.708	16.707
L Only	6.750	6.750

Shear Stirrup Requirements

Between 0.00 to 7.42 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 11.5.6.3, use #3 stirrups spaced at 10.000 in
Between 7.47 to 17.53 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 17.58 to 24.95 ft, PhiVc < Vu, Req'd Vs = 2.465, use #3 stirrups spaced at 10.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	25.000	276.34	312.62	0.88
+1.40D					
Span # 1	1	25.000	243.65	312.62	0.78
+1.20D+1.60L					
Span # 1	1	25.000	276.34	312.62	0.88
+1.20D+L					
Span # 1	1	25.000	251.03	312.62	0.80
+1.20D					
Span # 1	1	25.000	208.84	312.62	0.67
+0.90D					
Span # 1	1	25.000	156.63	312.62	0.50

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+L	1	0.8229	12.500		0.0000	0.000

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

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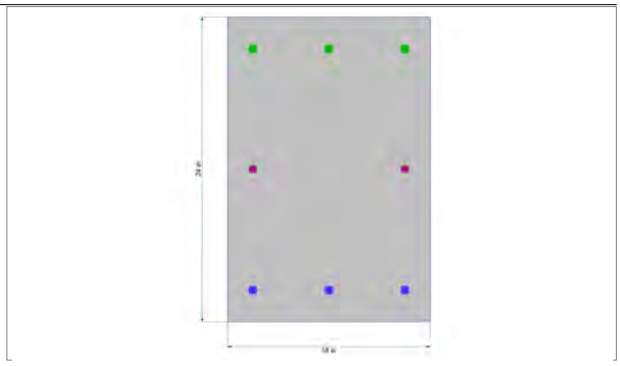
DESCRIPTION: 2B-11 (16"x24")

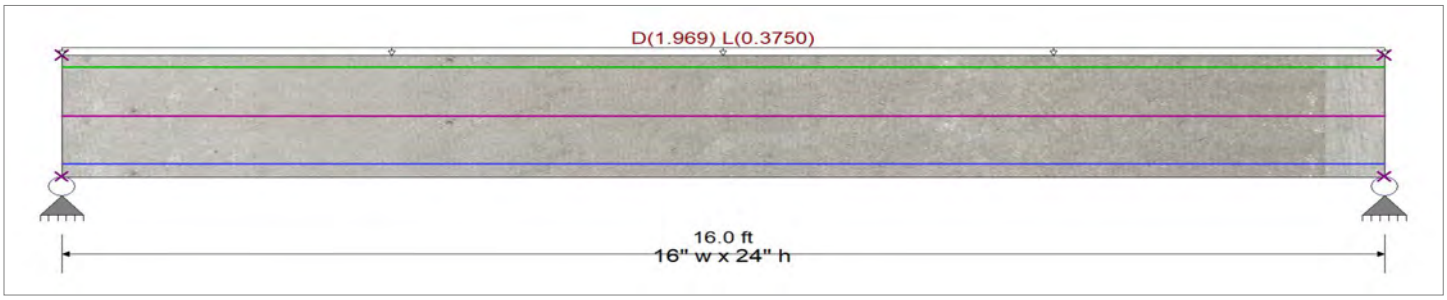
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	= 5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c^1/2 * 7.50	= 530.33 psi		Shear :	0.750
ψ Density	= 145.0 pcf	β 1	=	0.80
λ LtWt Factor	= 1.0			
Elastic Modulus	= 4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	= 60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	= 29,000.0 ksi	Stirrup Bar Size #		3
		Number of Resisting Legs Per Stirrup	=	2





Cross Section & Reinforcing Details

Rectangular Section, Width = 16.0 in, Height = 24.0 in
Span #1 Reinforcing....
3-#6 at 2.50 in from Bottom, from 0.0 to 16.0 ft in this span
2-#5 at 12.0 in from Bottom, from 0.0 to 16.0 ft in this span
3-#6 at 2.50 in from Top, from 0.0 to 16.0 ft in this span

Beam self weight calculated and added to loads
Loads on all spans...
D = 1.969, L = 0.3750
Uniform Load on ALL spans : D = 1.969, L = 0.3750 k/ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio = **0.711** : 1
Section used for this span **Typical Section**
Mu : Applied 109.657 k-ft
Mn * Phi : Allowable 154.223 k-ft
Location of maximum on span 7.985 ft
Span # where maximum occurs Span # 1

Maximum Deflection

Max Downward Transient Deflection	0.007 in	Ratio =	25803	>=360.0	Span: 1 : L Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 1 : L Only
Max Downward Total Deflection	0.088 in	Ratio =	2180	>=180.0	Span: 1 : +D+L
Max Upward Total Deflection	0.000 in	Ratio =	0	<180.0	Span: 1 : +D+L

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
Overall MAXimum	21.845	21.845
Overall MINimum	3.000	3.000
D Only	18.845	18.845
+D+L	21.845	21.845
+D+0.750L	21.095	21.095

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-11 (16"x24")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
+0.60D	11.307	11.307
L Only	3.000	3.000

Shear Stirrup Requirements

Between 0.00 to 2.65 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 11.5.6.3, use #3 stirrups spaced at 10.000 in
Between 2.68 to 13.32 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 13.35 to 15.97 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 11.5.6.3, use #3 stirrups spaced at 10.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	16.000	109.66	154.22	0.71
+1.40D					
Span # 1	1	16.000	105.53	154.22	0.68
+1.20D+1.60L					
Span # 1	1	16.000	109.66	154.22	0.71
+1.20D+L					
Span # 1	1	16.000	102.46	154.22	0.66
+1.20D					
Span # 1	1	16.000	90.46	154.22	0.59
+0.90D					
Span # 1	1	16.000	67.84	154.22	0.44

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+L	1	0.0880	8.000		0.0000	0.000

Project Title:
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Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

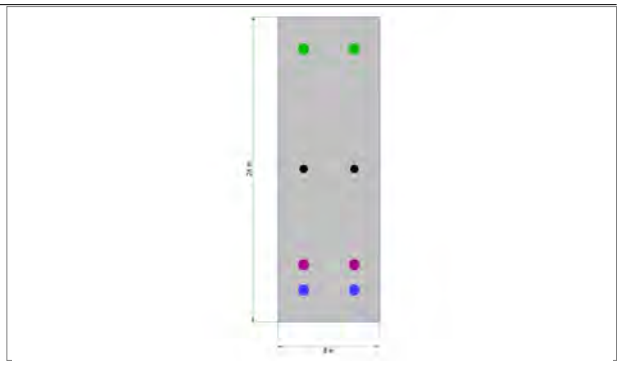
DESCRIPTION: 2B-12 (8"x24")

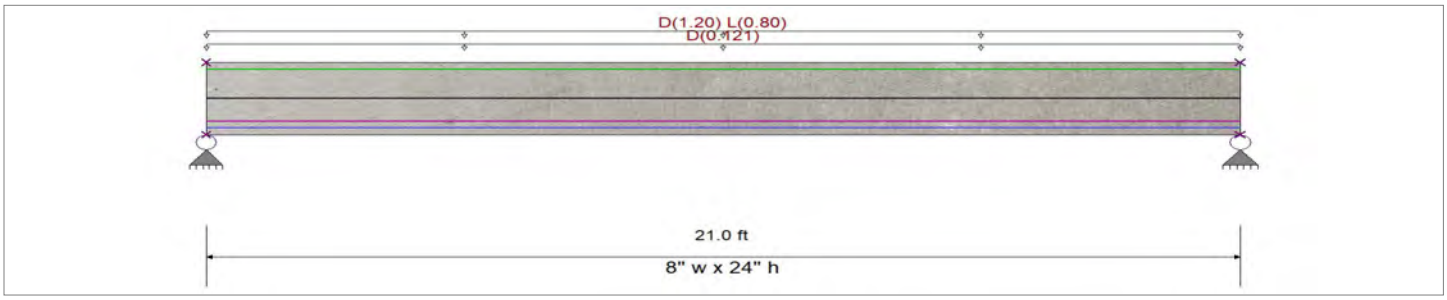
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ * 7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2





Cross Section & Reinforcing Details

Rectangular Section, Width = 8.0 in, Height = 24.0 in
Span #1 Reinforcing....
2-#7 at 2.50 in from Bottom, from 0.0 to 21.0 ft in this span
2-#7 at 4.50 in from Bottom, from 0.0 to 21.0 ft in this span
2-#7 at 2.50 in from Top, from 0.0 to 21.0 ft in this span
2-#5 at 12.0 in from Top, from 0.0 to 21.0 ft in this span

Beam self weight calculated and added to loads
Loads on all spans...
D = 0.150, L = 0.10

Uniform Load on ALL spans : D = 0.150, L = 0.10 ksf, Tributary Width = 8.0 ft

Load for Span Number 1
Uniform Load : D = 0.1210 k/ft, Tributary Width = 1.0 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.755** : 1				
Section used for this span	**Typical Section**				
Mu : Applied	170.733 k-ft				
Mn * Phi : Allowable	226.271 k-ft				
Location of maximum on span	10.519 ft				
Span # where maximum occurs	Span # 1				

Maximum Deflection

Max Downward Transient Deflection	0.125 in	Ratio =	2013	>=360.0	Span: 1 : L Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 1 : L Only
Max Downward Total Deflection	0.568 in	Ratio =	443	>=180.0	Span: 1 : +D+L
Max Upward Total Deflection	0.000 in	Ratio =	0	<180.0	Span: 1 : +D+L

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
Overall MAXimum	24.301	24.300
Overall MINimum	8.400	8.400
D Only	15.901	15.901

Project Title:
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Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-12 (8"x24")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
+D+L	24.301	24.300
+D+0.750L	22.201	22.201
+0.60D	9.540	9.540
L Only	8.400	8.400

Shear Stirrup Requirements

Between 0.00 to 7.57 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 7.61 to 13.39 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 13.43 to 20.96 ft, PhiVc < Vu, Req'd Vs = 9.527, use #3 stirrups spaced at 10.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	21.000	170.73	226.27	0.75
+1.40D					
Span # 1	1	21.000	116.87	226.27	0.52
+1.20D+1.60L					
Span # 1	1	21.000	170.73	226.27	0.75
+1.20D+L					
Span # 1	1	21.000	144.27	226.27	0.64
+1.20D					
Span # 1	1	21.000	100.17	226.27	0.44
+0.90D					
Span # 1	1	21.000	75.13	226.27	0.33

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+L	1	0.5685	10.500		0.0000	0.000

Project Title:
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Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

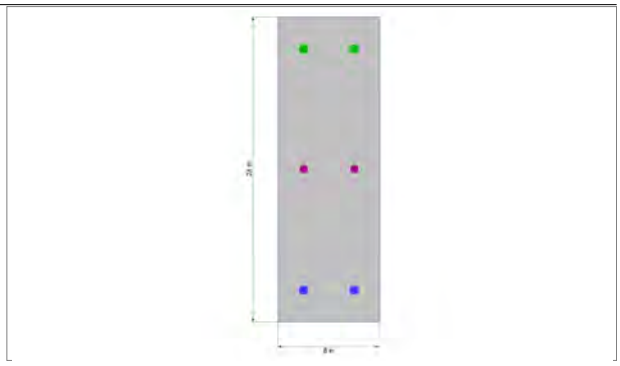
DESCRIPTION: 2B-13 (8"x24")

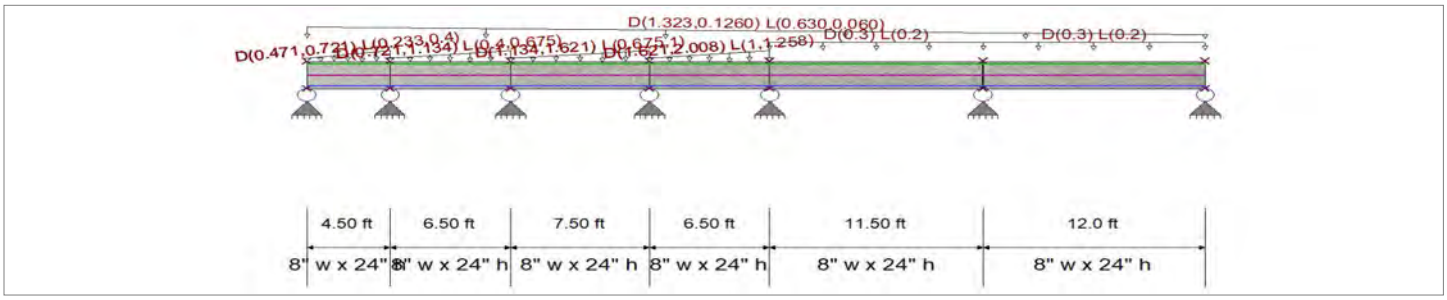
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ * 7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2





Cross Section & Reinforcing Details

Rectangular Section, Width = 8.0 in, Height = 24.0 in

Span #1 Reinforcing....
- 2-#6 at 2.50 in from Bottom, from 0.0 to 4.50 ft in this span
- 2-#6 at 2.50 in from Top, from 0.0 to 4.50 ft in this span
- 2-#5 at 12.0 in from Bottom, from 0.0 to 4.50 ft in this span

Span #2 Reinforcing....
- 2-#6 at 2.50 in from Bottom, from 0.0 to 6.50 ft in this span
- 2-#6 at 2.50 in from Top, from 0.0 to 6.50 ft in this span
- 2-#5 at 12.0 in from Bottom, from 0.0 to 6.50 ft in this span

Span #3 Reinforcing....
- 2-#6 at 2.50 in from Bottom, from 0.0 to 7.50 ft in this span
- 2-#6 at 2.50 in from Top, from 0.0 to 7.50 ft in this span
- 2-#5 at 12.0 in from Bottom, from 0.0 to 7.50 ft in this span

Span #4 Reinforcing....
- 2-#6 at 2.50 in from Bottom, from 0.0 to 6.50 ft in this span
- 2-#6 at 2.50 in from Top, from 0.0 to 6.50 ft in this span
- 2-#5 at 12.0 in from Bottom, from 0.0 to 6.50 ft in this span

Span #5 Reinforcing....
- 2-#6 at 2.50 in from Bottom, from 0.0 to 11.50 ft in this span
- 2-#6 at 2.50 in from Top, from 0.0 to 11.50 ft in this spa
- 2-#5 at 12.0 in from Bottom, from 0.0 to 11.50 ft in this span

Span #6 Reinforcing....
- 2-#6 at 2.50 in from Bottom, from 0.0 to 12.0 ft in this span
- 2-#6 at 2.50 in from Top, from 0.0 to 12.0 ft in this span
- 2-#5 at 12.0 in from Bottom, from 0.0 to 12.0 ft in this span

Beam self weight calculated and added to loads
Loads on all spans...
Varying Uniform Load : D= 1.323->0.1260, Lf= 0.630->0.060 k/ft, Extent = 0.0 -->> 48.50 ft

Load for Span Number 1
Varying Uniform Load : D= 0.4710->0.7210, L= 0.2330->0.40 k/ft, Extent = 0.0 -->> 4.50 ft, Trib Width = 1.0 ft

Load for Span Number 2
Varying Uniform Load : D= 0.7210->1.134, L= 0.40->0.6750 k/ft, Extent = 0.0 -->> 6.50 ft, Trib Width = 1.0 ft

Load for Span Number 3

Project Title:
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Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-13 (8"x24")

Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Varying Uniform Load : D= 1.134->1.621, L= 0.6750->1.0 k/ft, Extent = 0.0 -->> 7.50 ft, Trib Width = 1.0 ft

Load for Span Number 4
Varying Uniform Load : D= 1.621->2.008, L= 1.0->1.258 k/ft, Extent = 0.0 -->> 6.50 ft, Trib Width = 1.0 ft

Load for Span Number 5
Uniform Load : D = 0.30, L = 0.20 k/ft, Tributary Width = 1.0 ft

Load for Span Number 6
Uniform Load : D = 0.30, L = 0.20 k/ft, Tributary Width = 1.0 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.225**	: 1
Section used for this span	**Typical Section**	
Mu : Applied	-24.665	k-ft
Mn * Phi : Allowable	109.676	k-ft
Location of maximum on span	0.000	ft
Span # where maximum occurs	Span # 6	

Maximum Deflection

Max Downward Transient Deflection	0.002 in	Ratio =	79321	>=360.0	Span: 6 : L Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 6 : L Only
Max Downward Total Deflection	0.006 in	Ratio =	23924	>=180.0	Span: 6 : +D+L
Max Upward Total Deflection	0.000 in	Ratio =	0	<180.0	Span: 6 : +D+L

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2	Support 3	Support 4	Support 5	Support 6	Support 7
Overall MAXimum	4.663	18.201	26.012	28.846	22.818	17.238	4.593
Overall MINimum	1.418	5.727	8.597	10.045	7.599	5.210	1.374
D Only	3.245	12.474	17.415	18.801	15.219	12.028	3.219
+D+L	4.663	18.201	26.012	28.846	22.818	17.238	4.593
+D+0.750L	4.309	16.769	23.863	26.334	20.918	15.936	4.250
+0.60D	1.947	7.485	10.449	11.281	9.131	7.217	1.931
L Only	1.418	5.727	8.597	10.045	7.599	5.210	1.374

Shear Stirrup Requirements

Between 0.00 to 4.09 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 4.19 to 5.09 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 5.24 to 9.67 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 9.82 to 12.70 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 12.88 to 16.63 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 16.80 to 19.98 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 20.13 to 23.52 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 23.67 to 25.52 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 25.78 to 35.72 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 35.98 to 37.05 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 37.32 to 48.23 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	4.500	-10.41	109.68	0.09
Span # 2	2	6.500	-18.87	109.68	0.17
Span # 3	3	7.500	-21.21	109.68	0.19
Span # 4	4	6.500	-22.96	109.68	0.21
Span # 5	5	11.500	-21.74	109.68	0.20
Span # 6	6	12.000	-24.67	109.68	0.22
+1.40D					
Span # 1	1	4.500	-7.54	109.68	0.07
Span # 2	2	6.500	-13.28	109.68	0.12
Span # 3	3	7.500	-14.93	109.68	0.14
Span # 4	4	6.500	-15.57	109.68	0.14
Span # 5	5	11.500	-16.13	109.68	0.15

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-13 (8"x24")

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
Span # 6	6	12.000	-18.30	109.68	0.17
+1.20D+1.60L					
Span # 1	1	4.500	-10.41	109.68	0.09
Span # 2	2	6.500	-18.87	109.68	0.17
Span # 3	3	7.500	-21.21	109.68	0.19
Span # 4	4	6.500	-22.96	109.68	0.21
Span # 5	5	11.500	-21.74	109.68	0.20
Span # 6	6	12.000	-24.67	109.68	0.22
+1.20D+L					
Span # 1	1	4.500	-8.93	109.68	0.08
Span # 2	2	6.500	-16.06	109.68	0.15
Span # 3	3	7.500	-18.05	109.68	0.16
Span # 4	4	6.500	-19.35	109.68	0.18
Span # 5	5	11.500	-18.77	109.68	0.17
Span # 6	6	12.000	-21.30	109.68	0.19
+1.20D					
Span # 1	1	4.500	-6.46	109.68	0.06
Span # 2	2	6.500	-11.38	109.68	0.10
Span # 3	3	7.500	-12.80	109.68	0.12
Span # 4	4	6.500	-13.35	109.68	0.12
Span # 5	5	11.500	-13.83	109.68	0.13
Span # 6	6	12.000	-15.68	109.68	0.14
+0.90D					
Span # 1	1	4.500	-4.84	109.68	0.04
Span # 2	2	6.500	-8.54	109.68	0.08
Span # 3	3	7.500	-9.60	109.68	0.09
Span # 4	4	6.500	-10.01	109.68	0.09
Span # 5	5	11.500	-10.37	109.68	0.09
Span # 6	6	12.000	-11.76	109.68	0.11

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+L	1	0.0002	1.841	+D+L	-0.0000	4.295
+D+L	2	0.0006	2.659	+D+L	-0.0000	6.205
+D+L	3	0.0019	3.750	D Only	-0.0000	7.795
+D+L	4	0.0006	3.250		0.0000	7.795
+D+L	5	0.0030	5.750		0.0000	7.795
+D+L	6	0.0060	7.091		0.0000	7.795

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

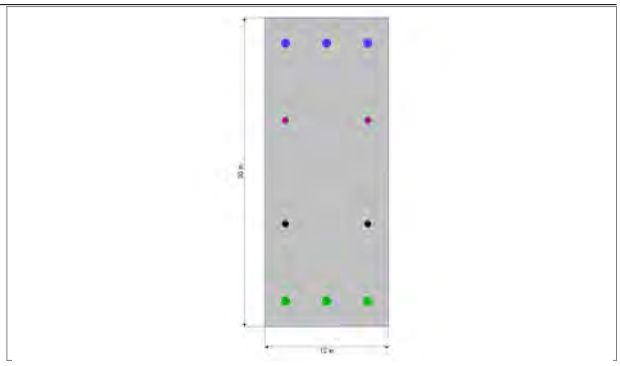
DESCRIPTION: 2B-14 (12"x30")

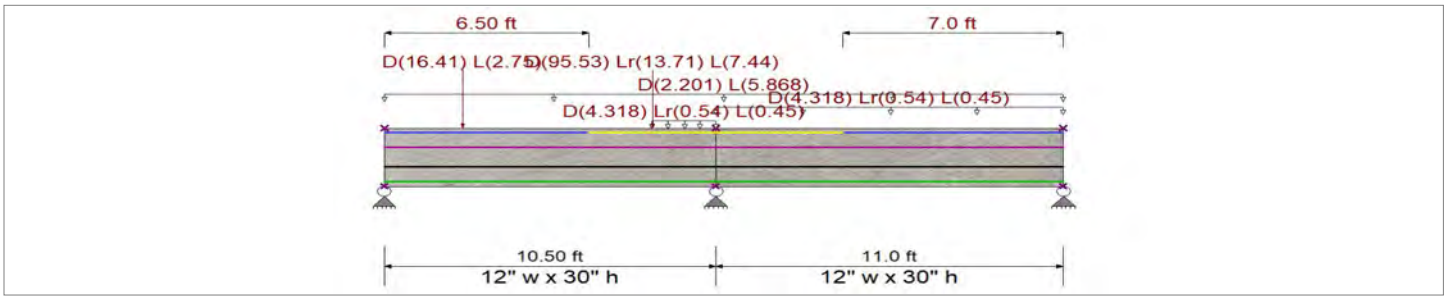
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ * 7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	3.0





Cross Section & Reinforcing Details

Rectangular Section, Width = 12.0 in, Height = 30.0 in
Span #1 Reinforcing....
3-#7 at 2.50 in from Top, from 0.0 to 10.50 ft in this span
2-#5 at 10.0 in from Top, from 0.0 to 10.50 ft in this span
2-#7 at 4.50 in from Top, from 6.50 to 0.0 ft in this span
3-#7 at 2.50 in from Bottom, from 0.0 to 10.50 ft in this s
2-#5 at 10.0 in from Bottom, from 0.0 to 10.50 ft in this s
Span #2 Reinforcing....
3-#7 at 2.50 in from Top, from 0.0 to 11.0 ft in this span
2-#5 at 10.0 in from Top, from 0.0 to 11.0 ft in this span
2-#7 at 4.50 in from Top, from 0.0 to 4.0 ft in this span
3-#7 at 2.50 in from Bottom, from 0.0 to 11.0 ft in this sp
2-#5 at 10.0 in from Bottom, from 0.0 to 11.0 ft in this sp

Beam self weight calculated and added to loads
Loads on all spans...
D = 0.150, L = 0.40

Uniform Load on ALL spans : D = 0.150, L = 0.40 ksf, Tributary Width = 14.670 ft

Load for Span Number 1
Uniform Load : D = 4.318, Lr = 0.540, L = 0.450 k/ft, Extent = 8.50 -->> 10.50 ft, Tributary Width = 1.0 ft

Point Load : D = 95.530, Lr = 13.710, L = 7.440 k @ 8.50 ft

Point Load : D = 16.410, L = 2.750 k @ 2.50 ft

Load for Span Number 2
Uniform Load : D = 4.318, Lr = 0.540, L = 0.450 k/ft, Tributary Width = 1.0 ft

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-14 (12"x30")

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.844**	: 1
Section used for this span	**Typical Section**	
Mu : Applied	-342.340	k-ft
Mn * Phi : Allowable	405.722	k-ft
Location of maximum on span	0.000	ft
Span # where maximum occurs	Span # 2	

Maximum Deflection

Max Downward Transient Deflection	0.008 in	Ratio =	16201	>=360.0	Span: 2 : Lr Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 2 : L Only
Max Downward Total Deflection	0.050 in	Ratio =	2530	>=180.0	Span: 2 : +D+L
Max Upward Total Deflection	-0.003 in	Ratio =	42402	>=180.0	Span: 2 : +D+0.750Lr+0.750L

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2	Support 3
Overall MAXimum	57.499	256.176	51.706
Overall MINimum	1.341	17.730	1.659
D Only	32.756	164.242	26.180
+D+L	57.499	256.176	51.706
+D+Lr	34.067	182.031	27.810
+D+0.750Lr+0.750L	52.429	246.275	46.673
+D+0.750L	51.680	232.475	45.674
+0.60D	18.397	101.002	14.508
Lr Only	1.341	17.730	1.659
L Only	25.489	90.475	26.238

Shear Stirrup Requirements

Between 0.00 to 2.49 ft, PhiVc < Vu, Req'd Vs = 10.397, use #3 stirrups spaced at 6.000 in
Between 2.53 to 2.91 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 13.000 in
Between 2.96 to 5.61 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 5.66 to 8.49 ft, PhiVc < Vu, Req'd Vs = 14.969, use #3 stirrups spaced at 13.000 in
Between 8.53 to 10.46 ft, Vu > PhiVc, Req'd Vs = Vs > Vs:max per 22.5.1.2, use #3 stirrups spaced at 0.000 in
Between 10.50 to 11.76 ft, Vs>(4bdf'c^.5) ACI 9.7.6.2.2, Req'd Vs = 70.153, use #3 stirrups spaced at 2.000 in
Between 11.80 to 15.66 ft, PhiVc < Vu, Req'd Vs = 0.2240, use #3 stirrups spaced at 3.000 in
Between 15.71 to 16.70 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 13.000 in
Between 16.74 to 18.58 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 18.63 to 19.66 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 13.000 in
Between 19.70 to 21.46 ft, PhiVc < Vu, Req'd Vs = 33.587, use #3 stirrups spaced at 8.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	10.500	-332.80	405.72	0.82
Span # 2	2	11.000	-342.34	405.72	0.84
+1.40D					
Span # 1	1	10.500	-205.43	405.72	0.51
Span # 2	2	11.000	-212.44	405.72	0.52
+1.20D+0.50Lr+1.60L					
Span # 1	1	10.500	-332.80	405.72	0.82
Span # 2	2	11.000	-342.34	405.72	0.84
+1.20D+1.60L					
Span # 1	1	10.500	-325.88	405.72	0.80
Span # 2	2	11.000	-335.13	405.72	0.83
+1.20D+1.60Lr+L					
Span # 1	1	10.500	-291.86	405.72	0.72
Span # 2	2	11.000	-300.81	405.72	0.74
+1.20D+1.60Lr					
Span # 1	1	10.500	-198.23	405.72	0.49
Span # 2	2	11.000	-205.16	405.72	0.51
+1.20D+L					
Span # 1	1	10.500	-269.71	405.72	0.66
Span # 2	2	11.000	-277.74	405.72	0.68
+1.20D					
Span # 1	1	10.500	-176.08	405.72	0.43

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-14 (12"x30")

Load Combination Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
Span # 2	2	11.000	-182.09	405.72	0.45
+1.20D+0.50Lr+L					
Span # 1	1	10.500	-276.63	405.72	0.68
Span # 2	2	11.000	-284.95	405.72	0.70
+0.90D					
Span # 1	1	10.500	-132.06	405.72	0.33
Span # 2	2	11.000	-136.57	405.72	0.34

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+L	1	0.0498	4.950	+D+0.750Lr+0.750L	-0.0009	10.657
+D+L	2	0.0157	6.757	+D+0.750Lr+0.750L	-0.0031	1.100

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 | Arch-Deco Design & Construction Inc. | (c) ENERCALC INC 1983-2021

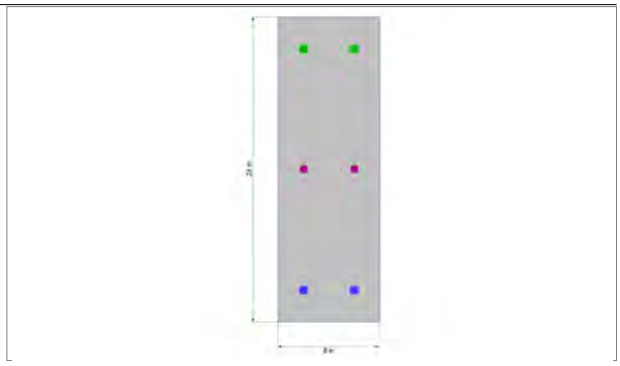
DESCRIPTION: 2B-15 (8"x24")

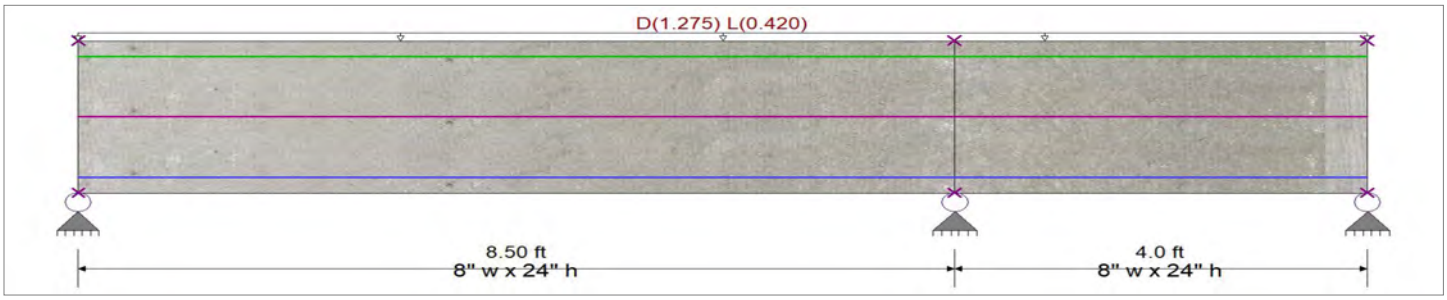
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ * 7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2.0





Cross Section & Reinforcing Details

Rectangular Section, Width = 8.0 in, Height = 24.0 in

Span #1 Reinforcing....
- 2-#6 at 2.50 in from Bottom, from 0.0 to 8.50 ft in this span
- 2-#5 at 12.0 in from Bottom, from 0.0 to 8.50 ft in this span
- 2-#6 at 2.50 in from Top, from 0.0 to 8.50 ft in this span

Span #2 Reinforcing....
- 2-#6 at 2.50 in from Bottom, from 0.0 to 4.0 ft in this span
- 2-#5 at 12.0 in from Bottom, from 0.0 to 4.0 ft in this span
- 2-#6 at 2.50 in from Top, from 0.0 to 4.0 ft in this span

Beam self weight calculated and added to loads
Loads on all spans...
D = 1.275, L = 0.420

Uniform Load on ALL spans : D = 1.275, L = 0.420 k/ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.150** : 1	
Section used for this span	**Typical Section**	
Mu : Applied	-16.506	k-ft
Mn * Phi : Allowable	109.676	k-ft
Location of maximum on span	0.000	ft
Span # where maximum occurs	Span # 2	

Maximum Deflection

Max Downward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 2 : L Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 2 : L Only
Max Downward Total Deflection	0.003 in	Ratio =	30723	>=180.0	Span: 2 : +D+L
Max Upward Total Deflection	0.000 in	Ratio =	0	<180.0	Span: 2 : +D+L

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2	Support 3
Overall MAXimum	6.519	16.510	0.575
Overall MINimum	1.450	3.672	0.128

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-15 (8"x24")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2	Support 3
D Only	5.069	12.838	0.447
+D+L	6.519	16.510	0.575
+D+0.750L	6.156	15.592	0.543
+0.60D	3.041	7.703	0.268
L Only	1.450	3.672	0.128

Shear Stirrup Requirements

Between 0.00 to 7.56 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 7.60 to 8.47 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 8.50 to 12.48 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	8.500	-16.08	109.68	0.15
Span # 2	2	4.000	-16.51	109.68	0.15
+1.40D					
Span # 1	1	8.500	-13.58	109.68	0.12
Span # 2	2	4.000	-13.94	109.68	0.13
+1.20D+1.60L					
Span # 1	1	8.500	-16.08	109.68	0.15
Span # 2	2	4.000	-16.51	109.68	0.15
+1.20D+L					
Span # 1	1	8.500	-14.42	109.68	0.13
Span # 2	2	4.000	-14.80	109.68	0.13
+1.20D					
Span # 1	1	8.500	-11.64	109.68	0.11
Span # 2	2	4.000	-11.95	109.68	0.11
+0.90D					
Span # 1	1	8.500	-8.73	109.68	0.08
Span # 2	2	4.000	-8.96	109.68	0.08

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+L	1	0.0033	3.764	+D+L	-0.0000	8.557
+D+L	2	0.0000	4.000	+D+L	-0.0003	1.429

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

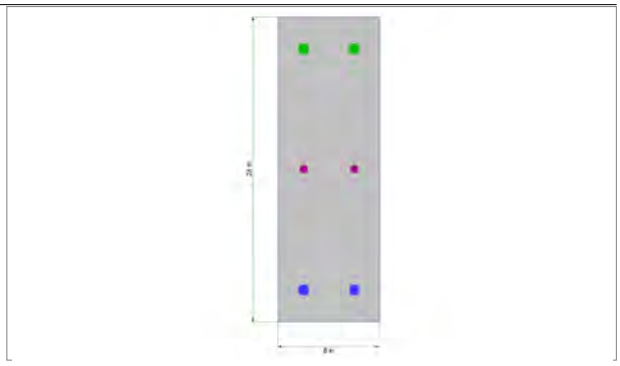
DESCRIPTION: 2B-16 (8"x24")

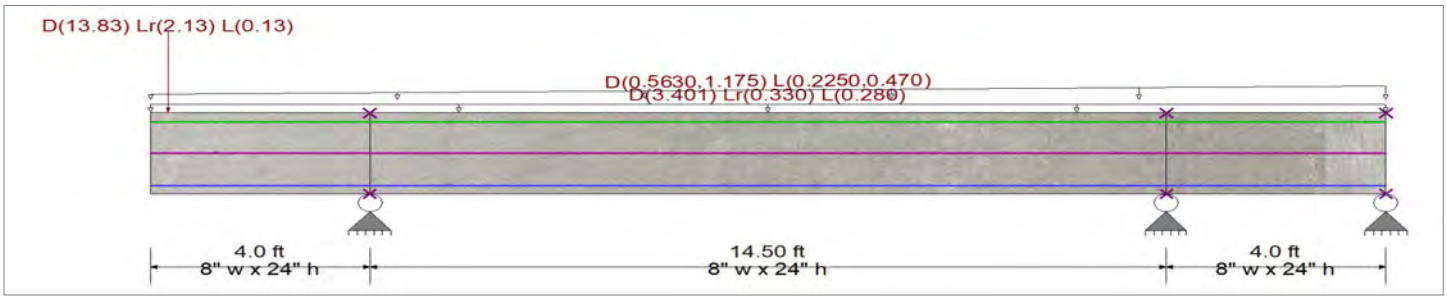
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ *7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2.0





Cross Section & Reinforcing Details

Rectangular Section, Width = 8.0 in, Height = 24.0 in

Span #1 Reinforcing....
- 2-#7 at 2.50 in from Bottom, from 0.0 to 4.0 ft in this span
- 2-#5 at 12.0 in from Bottom, from 0.0 to 4.0 ft in this span
- 2-#7 at 2.50 in from Top, from 0.0 to 4.0 ft in this span

Span #2 Reinforcing....
- 2-#7 at 2.50 in from Bottom, from 0.0 to 14.50 ft in this span
- 2-#5 at 12.0 in from Bottom, from 0.0 to 14.50 ft in this span
- 2-#7 at 2.50 in from Top, from 0.0 to 14.50 ft in this spa

Span #3 Reinforcing....
- 2-#7 at 2.50 in from Bottom, from 0.0 to 4.0 ft in this span
- 2-#5 at 12.0 in from Bottom, from 0.0 to 4.0 ft in this span
- 2-#7 at 2.50 in from Top, from 0.0 to 4.0 ft in this span

Beam self weight calculated and added to loads
Loads on all spans...
D = 3.401, Lr = 0.330, L = 0.280

Uniform Load on ALL spans : D = 3.401, Lr = 0.330, L = 0.280 k/ft

Varying Uniform Load : D= 0.5630->1.175, Lf= 0.2250->0.470 k/ft, Extent = 0.0 -->> 22.50 ft

Load for Span Number 1
Point Load : D = 13.830, Lr = 2.130, L = 0.130 k @ 0.330 ft

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-16 (8"x24")

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.894**	: 1
Section used for this span	**Typical Section**	
Mu : Applied	-122.530	k-ft
Mn * Phi : Allowable	137.067	k-ft
Location of maximum on span	0.000	ft
Span # where maximum occurs	Span # 2	

Maximum Deflection

Max Downward Transient Deflection	0.005 in	Ratio =	18364	>=360.0	Span: 3 : L Only
Max Upward Transient Deflection	-0.004 in	Ratio =	21464	>=360.0	
Max Downward Total Deflection	0.072 in	Ratio =	1324	>=180.0	
Max Upward Total Deflection	-0.004 in	Ratio =	12321	>=180.0	Span: 3 : +D+L

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2	Support 3	Support 4
Overall MAXimum		73.814	64.152	-6.186
Overall MINimum		6.120	3.238	-0.051
D Only		64.503	54.315	-4.564
+D+L		70.565	64.125	-6.186
+D+Lr		70.859	57.607	-4.657
+D+0.750Lr+0.750L		73.814	64.152	-5.858
+D+0.750L		69.060	61.625	-5.743
+0.60D		38.505	33.502	-3.454
Lr Only		6.368	3.238	-0.051
L Only		6.120	9.541	-1.412

Shear Stirrup Requirements

Between 0.00 to 0.32 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 0.35 to 8.35 ft, PhiVc < Vu, Req'd Vs = 0.6138, use #3 stirrups spaced at 4.000 in
Between 8.45 to 10.19 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 10.28 to 12.89 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 12.99 to 14.63 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 14.73 to 20.87 ft, PhiVc < Vu, Req'd Vs = 0.008419, use #3 stirrups spaced at 5.000 in
Between 20.90 to 22.34 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 22.37 to 22.47 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	4.000	-121.34	137.07	0.89
Span # 2	2	14.500	-122.53	137.07	0.89
Span # 3	3	4.000	-93.32	137.07	0.68
+1.40D					
Span # 1	1	4.000	-116.88	137.07	0.85
Span # 2	2	14.500	-118.03	137.07	0.86
Span # 3	3	4.000	-85.69	137.07	0.63
+1.20D+0.50Lr+1.60L					
Span # 1	1	4.000	-112.69	137.07	0.82
Span # 2	2	14.500	-113.81	137.07	0.83
Span # 3	3	4.000	-93.32	137.07	0.68
+1.20D+1.60L					
Span # 1	1	4.000	-107.50	137.07	0.78
Span # 2	2	14.500	-108.58	137.07	0.79
Span # 3	3	4.000	-91.90	137.07	0.67
+1.20D+1.60Lr+L					
Span # 1	1	4.000	-121.34	137.07	0.89
Span # 2	2	14.500	-122.53	137.07	0.89
Span # 3	3	4.000	-89.53	137.07	0.65
+1.20D+1.60Lr					
Span # 1	1	4.000	-116.77	137.07	0.85
Span # 2	2	14.500	-117.90	137.07	0.86
Span # 3	3	4.000	-78.00	137.07	0.57
+1.20D+L					
Span # 1	1	4.000	-104.76	137.07	0.76

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-16 (8"x24")

Load Combination Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
Span # 2	2	14.500	-105.80	137.07	0.77
Span # 3	3	4.000	-84.98	137.07	0.62
+1.20D					
Span # 1	1	4.000	-100.19	137.07	0.73
Span # 2	2	14.500	-101.17	137.07	0.74
Span # 3	3	4.000	-73.45	137.07	0.54
+1.20D+0.50Lr+L					
Span # 1	1	4.000	-109.94	137.07	0.80
Span # 2	2	14.500	-111.03	137.07	0.81
Span # 3	3	4.000	-86.40	137.07	0.63
+0.90D					
Span # 1	1	4.000	-75.14	137.07	0.55
Span # 2	2	14.500	-75.87	137.07	0.55
Span # 3	3	4.000	-55.09	137.07	0.40

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+Lr	1	0.0724	0.000		0.0000	0.000
+D+L	2	0.0749	7.250	+D+L	-0.0005	14.580
	3	0.0000	7.250	+D+L	-0.0039	1.360

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 | Arch-Deco Design & Construction Inc. | (c) ENERCALC INC 1983-2021

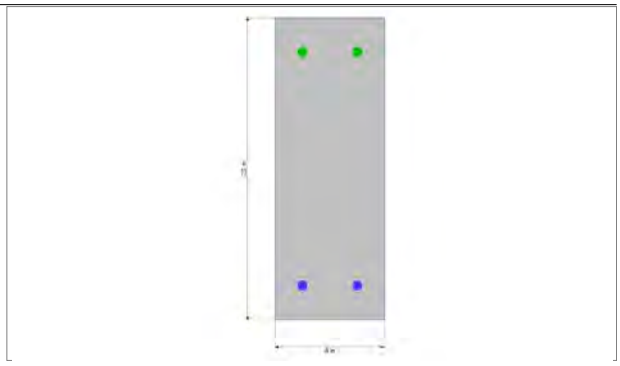
DESCRIPTION: 2B-17 (8"x22")

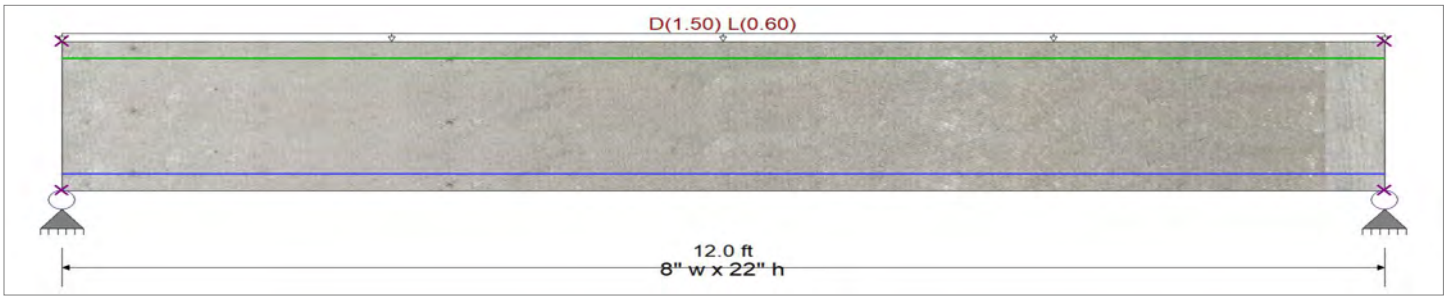
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ * 7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2.0





Cross Section & Reinforcing Details

Rectangular Section, Width = 8.0 in, Height = 22.0 in
Span #1 Reinforcing....
2-#6 at 2.50 in from Bottom, from 0.0 to 12.0 ft in this span
2-#6 at 2.50 in from Top, from 0.0 to 12.0 ft in this span

Beam self weight calculated and added to loads
Loads on all spans...
D = 0.150, L = 0.060

Uniform Load on ALL spans : D = 0.150, L = 0.060 ksf, Tributary Width = 10.0 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.717**	: 1
Section used for this span	**Typical Section**	
Mu : Applied	53.508	k-ft
Mn * Phi : Allowable	74.666	k-ft
Location of maximum on span	6.011	ft
Span # where maximum occurs	Span # 1	

Maximum Deflection

Max Downward Transient Deflection	0.010 in	Ratio =	14722	>=360.0	Span: 1 : L Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 1 : L Only
Max Downward Total Deflection	0.072 in	Ratio =	2001	>=180.0	Span: 1 : +D+L
Max Upward Total Deflection	0.000 in	Ratio =	0	<180.0	Span: 1 : +D+L

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
Overall MAXimum	13.663	13.663
Overall MINimum	3.600	3.600
D Only	10.063	10.063
+D+L	13.663	13.663
+D+0.750L	12.763	12.763
+0.60D	6.038	6.038

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-17 (8"x22")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
L Only	3.600	3.600

Shear Stirrup Requirements

Between 0.00 to 3.26 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 9.000 in
Between 3.28 to 8.72 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 8.74 to 11.98 ft, PhiVc < Vu, Req'd Vs = 0.4670, use #3 stirrups spaced at 9.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	12.000	53.51	74.67	0.72
+1.40D					
Span # 1	1	12.000	42.27	74.67	0.57
+1.20D+1.60L					
Span # 1	1	12.000	53.51	74.67	0.72
+1.20D+L					
Span # 1	1	12.000	47.03	74.67	0.63
+1.20D					
Span # 1	1	12.000	36.23	74.67	0.49
+0.90D					
Span # 1	1	12.000	27.17	74.67	0.36

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+L	1	0.0719	6.000		0.0000	0.000

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

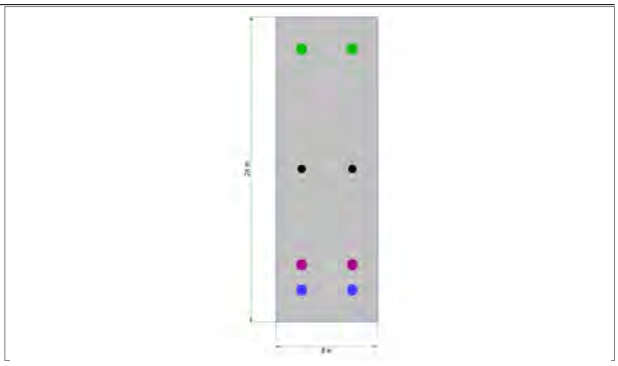
DESCRIPTION: 2B-18 (8"x24")

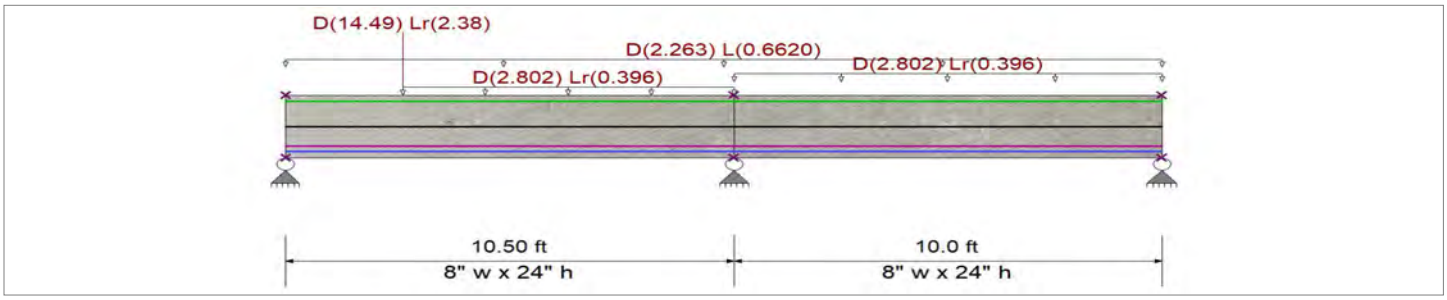
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = $f'c^{1/2}$ * 7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2.0





Cross Section & Reinforcing Details

Rectangular Section, Width = 8.0 in, Height = 24.0 in
Span #1 Reinforcing....
2-#7 at 2.50 in from Bottom, from 0.0 to 10.50 ft in this span
2-#7 at 4.50 in from Bottom, from 0.0 to 10.50 ft in this span
2-#7 at 2.50 in from Top, from 0.0 to 10.50 ft in this spa
2-#5 at 12.0 in from Top, from 0.0 to 10.50 ft in this spa
Span #2 Reinforcing....
2-#7 at 2.50 in from Bottom, from 0.0 to 10.0 ft in this span
2-#7 at 4.50 in from Bottom, from 0.0 to 10.0 ft in this span
2-#7 at 2.50 in from Top, from 0.0 to 10.0 ft in this span
2-#5 at 12.0 in from Top, from 0.0 to 10.0 ft in this span

Beam self weight calculated and added to loads
Loads on all spans...
D = 2.263, L = 0.6620
Uniform Load on ALL spans : D = 2.263, L = 0.6620 k/ft

Load for Span Number 1
Point Load : D = 14.490, Lr = 2.380 k @ 2.750 ft
Uniform Load : D = 2.802, Lr = 0.3960 k/ft, Extent = 2.750 -->> 10.50 ft, Tributary Width = 1.0 ft

Load for Span Number 2
Uniform Load : D = 2.802, Lr = 0.3960 k/ft, Tributary Width = 1.0 ft

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-18 (8"x24")

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio = **0.786** : 1

Section used for this span	**Typical Section**
Mu : Applied	-110.228 k-ft
Mn * Phi : Allowable	140.215 k-ft
Location of maximum on span	0.000 ft
Span # where maximum occurs	Span # 2

Maximum Deflection

Max Downward Transient Deflection	0.002 in	Ratio =	52634	>=360.0	Span: 2 : Lr Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 2 : L Only
Max Downward Total Deflection	0.054 in	Ratio =	2327	>=180.0	Span: 2 : +D+0.750Lr+0.750L
Max Upward Total Deflection	0.000 in	Ratio =	0	<180.0	Span: 2 : +D+Lr

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2	Support 3
Overall MAXimum	29.383	79.519	22.914
Overall MINimum	2.280	5.789	1.340
D Only	25.467	69.266	19.847
+D+L	28.306	77.356	22.489
+D+Lr	27.868	74.806	21.315
+D+0.750Lr+0.750L	29.383	79.519	22.914
+D+0.750L	27.606	75.315	21.838
+0.60D	14.880	42.381	11.487
Lr Only	2.280	5.789	1.340
L Only	2.646	8.486	2.439

Shear Stirrup Requirements

Between 0.00 to 2.74 ft, PhiVc < Vu, Req'd Vs = 5.194, use #3 stirrups spaced at 10.000 in
Between 2.79 to 4.54 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 4.59 to 6.09 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 6.13 to 13.89 ft, PhiVc < Vu, Req'd Vs = 0.2912, use #3 stirrups spaced at 4.000 in
Between 13.93 to 15.64 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 15.68 to 18.21 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 18.26 to 20.46 ft, PhiVc < Vu, Req'd Vs = 4.012, use #3 stirrups spaced at 10.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	10.500	-107.90	140.22	0.77
Span # 2	2	10.000	-110.23	140.22	0.79
+1.40D					
Span # 1	1	10.500	-104.24	140.22	0.74
Span # 2	2	10.000	-106.49	140.22	0.76
+1.20D+0.50Lr+1.60L					
Span # 1	1	10.500	-106.12	140.22	0.76
Span # 2	2	10.000	-108.41	140.22	0.77
+1.20D+1.60L					
Span # 1	1	10.500	-102.99	140.22	0.73
Span # 2	2	10.000	-105.21	140.22	0.75
+1.20D+1.60Lr+L					
Span # 1	1	10.500	-107.90	140.22	0.77
Span # 2	2	10.000	-110.23	140.22	0.79
+1.20D+1.60Lr					
Span # 1	1	10.500	-99.37	140.22	0.71
Span # 2	2	10.000	-101.52	140.22	0.72
+1.20D+L					
Span # 1	1	10.500	-97.87	140.22	0.70
Span # 2	2	10.000	-99.99	140.22	0.71
+1.20D					
Span # 1	1	10.500	-89.35	140.22	0.64
Span # 2	2	10.000	-91.28	140.22	0.65
+1.20D+0.50Lr+L					
Span # 1	1	10.500	-101.01	140.22	0.72
Span # 2	2	10.000	-103.19	140.22	0.74

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-18 (8"x24")

Load Combination Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
+0.90D					
Span # 1	1	10.500	-67.01	140.22	0.48
Span # 2	2	10.000	-68.46	140.22	0.49

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+0.750Lr+0.750L	1	0.0541	4.350	+D+0.750Lr+0.750L	-0.0004	10.643
+D+0.750Lr+0.750L	2	0.0163	5.857	+D+Lr	-0.0009	0.429

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 | Arch-Deco Design & Construction Inc. | (c) ENERCALC INC 1983-2021

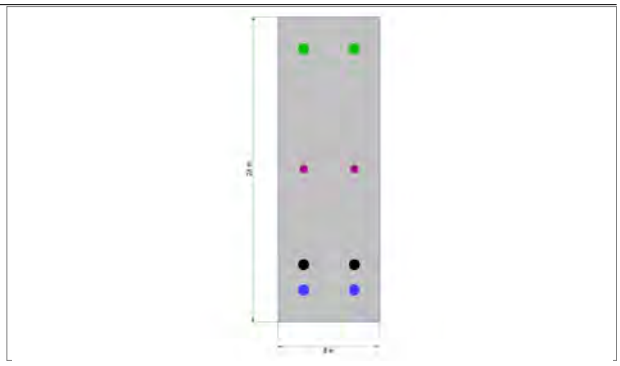
DESCRIPTION: 2B-19 (8"x24")

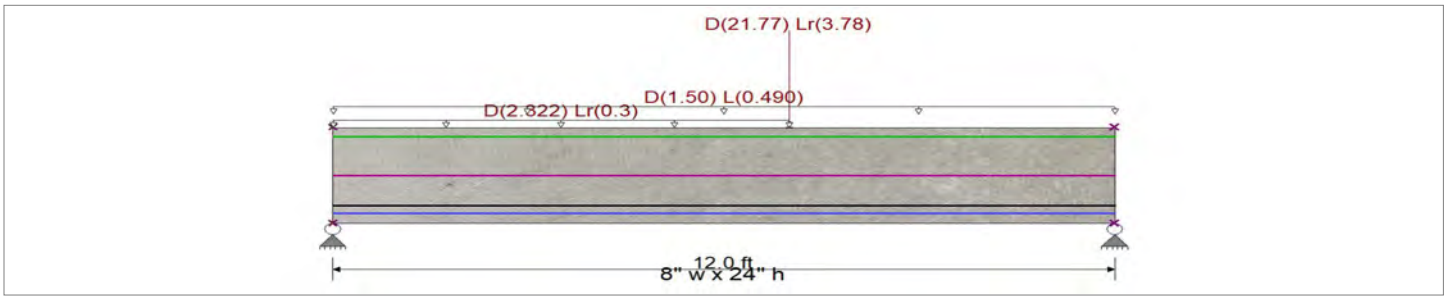
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ * 7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2.0





Cross Section & Reinforcing Details

Rectangular Section, Width = 8.0 in, Height = 24.0 in
Span #1 Reinforcing....
2-#7 at 2.50 in from Bottom, from 0.0 to 12.0 ft in this span
2-#5 at 12.0 in from Bottom, from 0.0 to 12.0 ft in this span
2-#7 at 2.50 in from Top, from 0.0 to 12.0 ft in this span
2-#7 at 4.50 in from Bottom, from 0.0 to 12.0 ft in this sp

Beam self weight calculated and added to loads
Loads on all spans...
D = 1.50, L = 0.490
Uniform Load on ALL spans : D = 1.50, L = 0.490 k/ft
Point Load : D = 21.770, Lr = 3.780 k @ 7.0 ft, (Reacc. RB-5)
Uniform Load : D = 2.322, Lr = 0.30 k/ft, Extent = 0.0 -->> 7.0 ft, Tributary Width = 1.0 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.757** : 1	
Section used for this span	**Typical Section**	
Mu : Applied	171.291	k-ft
Mn * Phi : Allowable	226.271	k-ft
Location of maximum on span	6.995	ft
Span # where maximum occurs	Span # 1	

Maximum Deflection

Max Downward Transient Deflection	0.008 in	Ratio =	17017	>=360.0	Span: 1 : Lr Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 1 : Lr Only
Max Downward Total Deflection	0.178 in	Ratio =	810	>=180.0	Span: 1 : +D+0.750Lr+0.750L
Max Upward Total Deflection	0.000 in	Ratio =	0	<180.0	Span: 1 : +D+0.750Lr+0.750L

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
Overall MAXimum	35.246	31.918
Overall MINimum	2.940	2.817

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-19 (8"x24")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
D Only	30.744	27.600
+D+L	33.684	30.540
+D+Lr	33.807	30.417
+D+0.750Lr+0.750L	35.246	31.918
+D+0.750L	32.949	29.805
+0.60D	18.446	16.560
Lr Only	3.062	2.817
L Only	2.940	2.940

Shear Stirrup Requirements

Between 0.00 to 4.50 ft, PhiVc < Vu, Req'd Vs = 0.01118, use #3 stirrups spaced at 6.000 in
Between 4.52 to 6.19 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 10.000 in
Between 6.21 to 6.99 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 7.02 to 11.98 ft, PhiVc < Vu, Req'd Vs = 17.574, use #3 stirrups spaced at 8.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	12.000	171.29	226.27	0.76
+1.40D					
Span # 1	1	12.000	163.55	226.27	0.72
+1.20D+0.50Lr+1.60L					
Span # 1	1	12.000	160.95	226.27	0.71
+1.20D+1.60L					
Span # 1	1	12.000	153.91	226.27	0.68
+1.20D+1.60Lr+L					
Span # 1	1	12.000	171.29	226.27	0.76
+1.20D+1.60Lr					
Span # 1	1	12.000	162.71	226.27	0.72
+1.20D+L					
Span # 1	1	12.000	148.76	226.27	0.66
+1.20D					
Span # 1	1	12.000	140.18	226.27	0.62
+1.20D+0.50Lr+L					
Span # 1	1	12.000	155.80	226.27	0.69
+0.90D					
Span # 1	1	12.000	105.14	226.27	0.46

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+0.750Lr+0.750L	1	0.1777	6.000		0.0000	0.000

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 | Arch-Deco Design & Construction Inc. | (c) ENERCALC INC 1983-2021

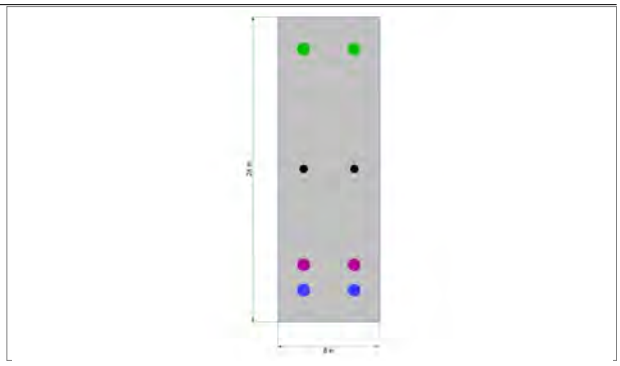
DESCRIPTION: 2B-20 (8"x24")

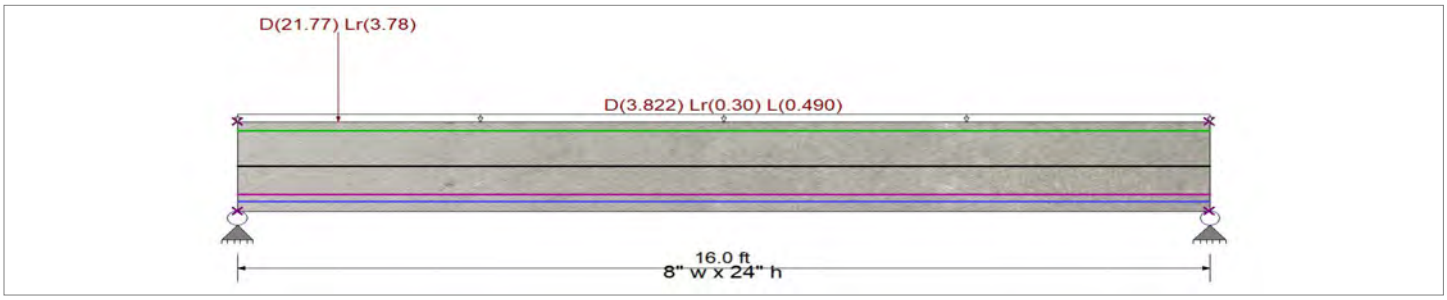
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ * 7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		4
			Number of Resisting Legs Per Stirrup	=	2.0





Cross Section & Reinforcing Details

Rectangular Section, Width = 8.0 in, Height = 24.0 in
Span #1 Reinforcing....
2-#8 at 2.50 in from Bottom, from 0.0 to 16.0 ft in this span
2-#8 at 4.50 in from Bottom, from 0.0 to 16.0 ft in this span
2-#8 at 2.50 in from Top, from 0.0 to 16.0 ft in this span
2-#5 at 12.0 in from Top, from 0.0 to 16.0 ft in this span

Beam self weight calculated and added to loads
Loads on all spans...
D = 3.822, Lr = 0.30, L = 0.490
Uniform Load on ALL spans : D = 3.822, Lr = 0.30, L = 0.490 k/ft
Point Load : D = 21.770, Lr = 3.780 k @ 1.670 ft, (Reacc. RB-5)

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.748** : 1				
Section used for this span	**Typical Section**				
Mu : Applied	213.066 k-ft				
Mn * Phi : Allowable	285.017 k-ft				
Location of maximum on span	7.432 ft				
Span # where maximum occurs	Span # 1				

Maximum Deflection

Max Downward Transient Deflection	0.019 in	Ratio =	9873	>=360.0	Span: 1 : Lr Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 1 : L Only
Max Downward Total Deflection	0.368 in	Ratio =	521	>=180.0	Span: 1 : +D+0.750Lr+0.750L
Max Upward Total Deflection	0.000 in	Ratio =	0	<180.0	Span: 1 : +D+0.750Lr+0.750L

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
Overall MAXimum	58.900	39.431
Overall MINimum	3.920	2.795
D Only	51.620	34.395
+D+L	55.540	38.315

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-20 (8"x24")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
+D+Lr	57.406	37.189
+D+0.750Lr+0.750L	58.900	39.431
+D+0.750L	54.560	37.335
+0.60D	30.972	20.637
Lr Only	5.785	2.795
L Only	3.920	3.920

Shear Stirrup Requirements

Between 0.00 to 1.66 ft, Vs>(4bdf'c^.5) ACI 9.7.6.2.2, Req'd Vs = 41.087, use #4 stirrups spaced at 5.000 in
Between 1.69 to 5.92 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #4 stirrups spaced at 10.000 in
Between 5.95 to 8.95 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #4 stirrups spaced at 0.000 in
Between 8.98 to 15.97 ft, PhiVc < Vu, Req'd Vs = 25.246, use #4 stirrups spaced at 10.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	16.000	213.07	285.02	0.75
+1.40D					
Span # 1	1	16.000	206.24	285.02	0.72
+1.20D+0.50Lr+1.60L					
Span # 1	1	16.000	208.21	285.02	0.73
+1.20D+1.60L					
Span # 1	1	16.000	201.75	285.02	0.71
+1.20D+1.60Lr+L					
Span # 1	1	16.000	213.07	285.02	0.75
+1.20D+1.60Lr					
Span # 1	1	16.000	197.48	285.02	0.69
+1.20D+L					
Span # 1	1	16.000	192.38	285.02	0.67
+1.20D					
Span # 1	1	16.000	176.77	285.02	0.62
+1.20D+0.50Lr+L					
Span # 1	1	16.000	198.84	285.02	0.70
+0.90D					
Span # 1	1	16.000	132.58	285.02	0.47

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+0.750Lr+0.750L	1	0.3682	7.738		0.0000	0.000

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-21 (8"x26")

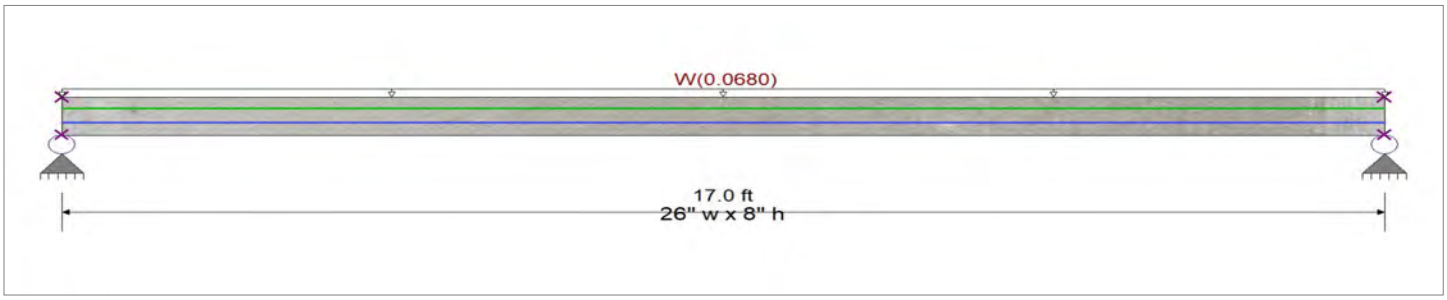
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ * 7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2.0





Cross Section & Reinforcing Details

Rectangular Section, Width = 26.0 in, Height = 8.0 in
Span #1 Reinforcing....
3-#5 at 2.50 in from Bottom, from 0.0 to 17.0 ft in this span
3-#5 at 2.50 in from Top, from 0.0 to 17.0 ft in this span

Beam self weight calculated and added to loads
Loads on all spans...
W = 0.0680
Uniform Load on ALL spans : W = 0.0680 k/ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.394** : 1	
Section used for this span	**Typical Section**	
Mu : Applied	11.536	k-ft
Mn * Phi : Allowable	29.253	k-ft
Location of maximum on span	8.515	ft
Span # where maximum occurs	Span # 1	

Maximum Deflection

Max Downward Transient Deflection	0.029 in	Ratio =	7140	>=360.0	Span: 1 : W Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 1 : W Only
Max Downward Total Deflection	0.105 in	Ratio =	1940	>=180.0	Span: 1 : +D+0.60W
Max Upward Total Deflection	0.000 in	Ratio =	0	<180.0	Span: 1 : +D+0.60W

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
Overall MAXimum	2.127	2.127
Overall MINimum	0.578	0.578
D Only	1.780	1.780
+D+L	1.780	1.780
+D+0.750L	1.780	1.780
+D+0.60W	2.127	2.127

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-21 (8"x26")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
+D+0.750L+0.450W	2.040	2.040
+0.60D+0.60W	1.415	1.415
+0.60D	1.068	1.068
W Only	0.578	0.578

Shear Stirrup Requirements

Entire Beam Span Length : Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	17.000	11.54	29.25	0.39
+1.40D					
Span # 1	1	17.000	10.59	29.25	0.36
+1.20D+1.60L					
Span # 1	1	17.000	9.08	29.25	0.31
+1.20D+L					
Span # 1	1	17.000	9.08	29.25	0.31
+1.20D+0.50W					
Span # 1	1	17.000	10.31	29.25	0.35
+1.20D+L+W					
Span # 1	1	17.000	11.54	29.25	0.39
+0.90D+W					
Span # 1	1	17.000	9.27	29.25	0.32
+0.90D					
Span # 1	1	17.000	6.81	29.25	0.23

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+0.60W	1	0.1051	8.500		0.0000	0.000

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

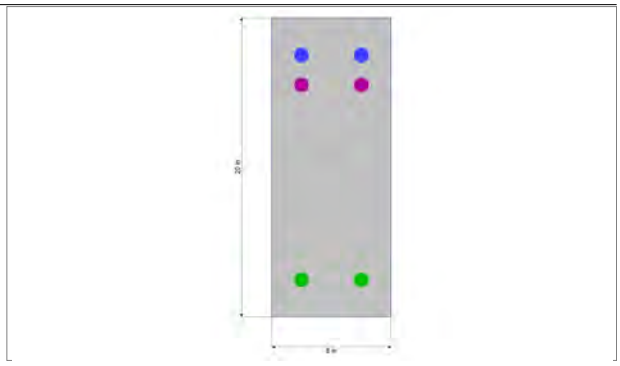
DESCRIPTION: 2B-22 (8"x20")

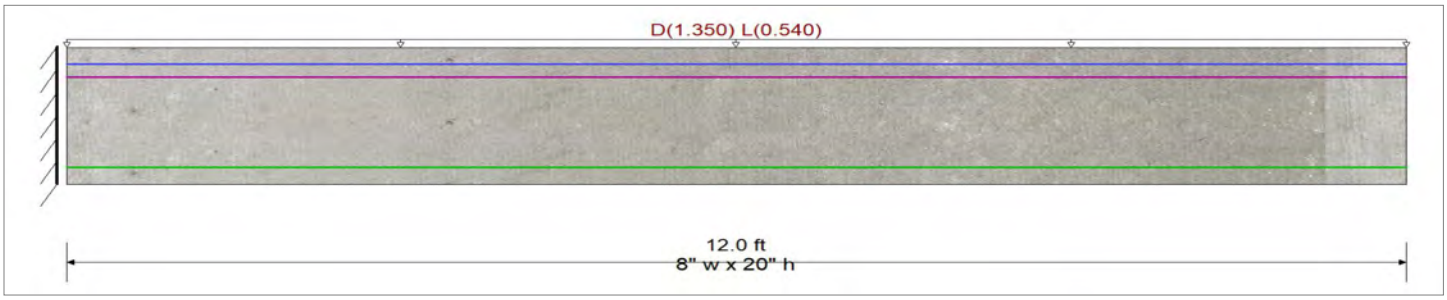
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ * 7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2.0





Cross Section & Reinforcing Details

Rectangular Section, Width = 8.0 in, Height = 20.0 in
Span #1 Reinforcing....
2-#8 at 2.50 in from Top, from 0.0 to 12.0 ft in this span
2-#8 at 4.50 in from Top, from 0.0 to 12.0 ft in this span
2-#8 at 2.50 in from Bottom, from 0.0 to 12.0 ft in this sp

Beam self weight calculated and added to loads
Loads on all spans...
D = 0.150, L = 0.060

Uniform Load on ALL spans : D = 0.150, L = 0.060 ksf, Tributary Width = 9.0 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.941**	: 1
Section used for this span	**Typical Section**	
Mu : Applied	-192.768	k-ft
Mn * Phi : Allowable	204.90	k-ft
Location of maximum on span	0.000	ft
Span # where maximum occurs	Span # 1	

Maximum Deflection

Max Downward Transient Deflection	0.138 in	Ratio =	2084	>=360.0	Span: 1 : L Only
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360.0	Span: 1 : L Only
Max Downward Total Deflection	0.699 in	Ratio =	410	>=180.0	Span: 1 : +D+L
Max Upward Total Deflection	0.000 in	Ratio =	0	<180.0	Span: 1 : +D+L

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
Overall MAXimum	24.613	
Overall MINimum	6.480	
D Only	18.133	
+D+L	24.613	
+D+0.750L	22.993	

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-22 (8"x20")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2
+0.60D	10.880	
L Only	6.480	

Shear Stirrup Requirements

Between 0.00 to 5.70 ft, PhiVc < Vu, Req'd Vs = 0.002054, use #3 stirrups spaced at 6.000 in
Between 5.73 to 8.44 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 8.000 in
Between 8.46 to 11.98 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	12.000	-192.77	204.90	0.94
+1.40D					
Span # 1	1	12.000	-152.32	204.90	0.74
+1.20D+1.60L					
Span # 1	1	12.000	-192.77	204.90	0.94
+1.20D+L					
Span # 1	1	12.000	-169.44	204.90	0.83
+1.20D					
Span # 1	1	12.000	-130.56	204.90	0.64
+0.90D					
Span # 1	1	12.000	-97.92	204.90	0.48

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+L	1	0.6992	12.000		0.0000	0.000

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 | Arch-Deco Design & Construction Inc. | (c) ENERCALC INC 1983-2021

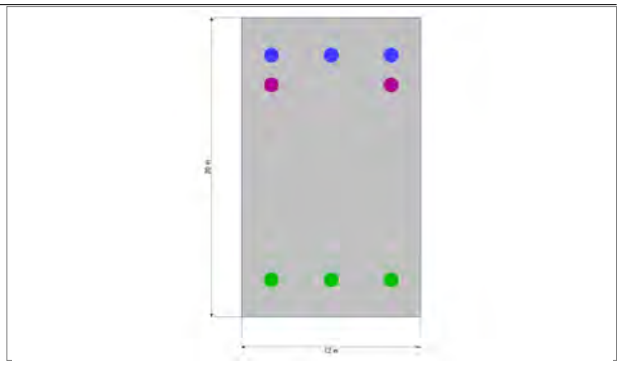
DESCRIPTION: 2B-23 (12"x20")

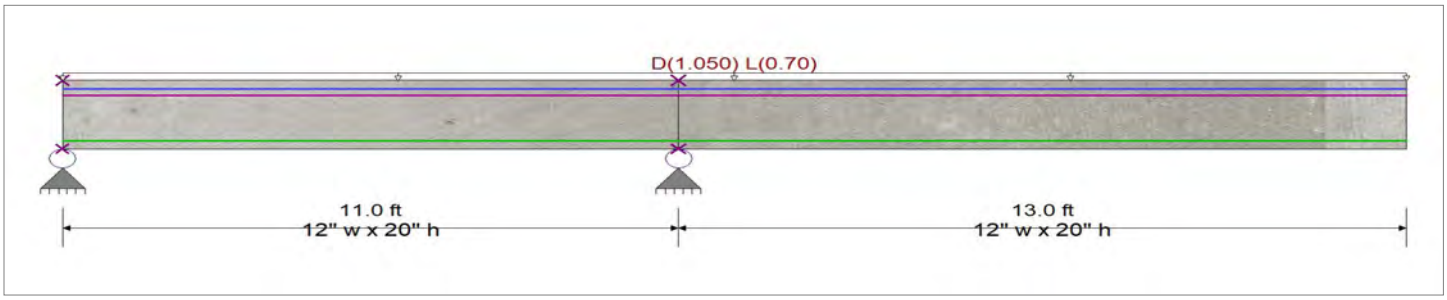
CODE REFERENCES

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

f'c	=	5.0 ksi	ϕ Phi Values	Flexure :	0.90
fr = f'c$^{1/2}$ * 7.50	=	530.33 psi		Shear :	0.750
ψ Density	=	145.0 pcf	β_1	=	0.80
λ LtWt Factor	=	1.0			
Elastic Modulus	=	4,030.51 ksi	Fy - Stirrups		40.0 ksi
fy - Main Rebar	=	60.0 ksi	E - Stirrups	=	29,000.0 ksi
E - Main Rebar	=	29,000.0 ksi	Stirrup Bar Size #		3
			Number of Resisting Legs Per Stirrup	=	2.0





Cross Section & Reinforcing Details

Rectangular Section, Width = 12.0 in, Height = 20.0 in
Span #1 Reinforcing....
3-#8 at 2.50 in from Top, from 0.0 to 11.0 ft in this span
3-#8 at 2.50 in from Bottom, from 0.0 to 11.0 ft in this sp
2-#8 at 4.50 in from Top, from 0.0 to 11.0 ft in this span
Span #2 Reinforcing....
3-#8 at 2.50 in from Top, from 0.0 to 13.0 ft in this span
3-#8 at 2.50 in from Bottom, from 0.0 to 13.0 ft in this sp
2-#8 at 4.50 in from Top, from 0.0 to 13.0 ft in this span

Beam self weight calculated and added to loads
Loads on all spans...
D = 0.150, L = 0.10

Uniform Load on ALL spans : D = 0.150, L = 0.10 ksf, Tributary Width = 7.0 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.857**	: 1
Section used for this span	**Typical Section**	
Mu : Applied	-225.615	k-ft
Mn * Phi : Allowable	263.361	k-ft
Location of maximum on span	0.000	ft
Span # where maximum occurs	Span # 2	

Maximum Deflection

Max Downward Transient Deflection	0.334 in	Ratio =	934	>=360.0	Span: 2 : L Only
Max Upward Transient Deflection	-0.020 in	Ratio =	6730	>=360.0	Span: 2 : L Only
Max Downward Total Deflection	1.300 in	Ratio =	240	>=180.0	Span: 2 : +D+L
Max Upward Total Deflection	-0.085 in	Ratio =	1546	>=180.0	Span: 2 : +D+L

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2	Support 3
Overall MAXimum	-4.345	52.145	
Overall MINimum	-1.527	18.327	

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Beam

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 2B-23 (12"x20")

Vertical Reactions

Support notation : Far left is #1

Load Combination	Support 1	Support 2	Support 3
D Only	-2.818	33.818	
+D+L	-4.345	52.145	
+D+0.750L	-3.964	47.564	
+0.60D	-1.691	20.291	
L Only	-1.527	18.327	

Shear Stirrup Requirements

Between 0.00 to 2.78 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in
Between 2.83 to 19.17 ft, PhiVc/2 < Vu <= PhiVc, Req'd Vs = Min 9.6.3.1, use #3 stirrups spaced at 8.000 in
Between 19.22 to 23.95 ft, Vu < PhiVc/2, Req'd Vs = Not Reqd 9.6.3.1, use #3 stirrups spaced at 0.000 in

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment	Span #	Location (ft) along Beam	Bending Stress Results (k-ft) Mu : Max	Phi*Mnx	Stress Ratio
MAXimum BENDING Envelope					
Span # 1	1	11.000	-224.04	263.36	0.85
Span # 2	2	13.000	-225.62	263.36	0.86
+1.40D					
Span # 1	1	11.000	-151.74	263.36	0.58
Span # 2	2	13.000	-152.80	263.36	0.58
+1.20D+1.60L					
Span # 1	1	11.000	-224.04	263.36	0.85
Span # 2	2	13.000	-225.62	263.36	0.86
+1.20D+L					
Span # 1	1	11.000	-188.80	263.36	0.72
Span # 2	2	13.000	-190.13	263.36	0.72
+1.20D					
Span # 1	1	11.000	-130.06	263.36	0.49
Span # 2	2	13.000	-130.98	263.36	0.50
+0.90D					
Span # 1	1	11.000	-97.54	263.36	0.37
Span # 2	2	13.000	-98.23	263.36	0.37

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl (in)	Location in Span (ft	Load Combination	Max. "+" Defl (in)	Location in Span (ft
+D+L	1	0.0095	11.186	+D+L	-0.0854	7.071
+D+L	2	1.2997	13.000		0.0000	7.071

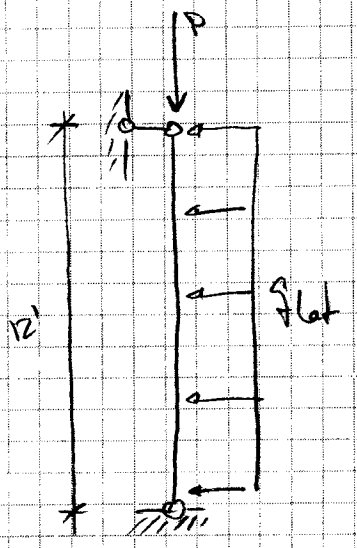
COLUMNS DESIGN

(ACI 318-14/ASCI)

Ph: (305) 710-2987 Email: 3aengdrafting@gmail.com

Steel Columns Design:

→ HSS-1 (worst)

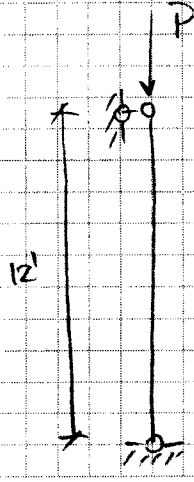


		D	L_r	L
P	Reacc 1 ZB-13			
	Reacc 2 ZB-14	170.57	17.73	90.64

q_{lat} (LRL, Zone 4, 20 sft) $= 43.44 \text{ psf} / 0.6 \times 6' = 434 \text{ plf}$

HSS 8x8x½ *

→ HSS-2 (worst)

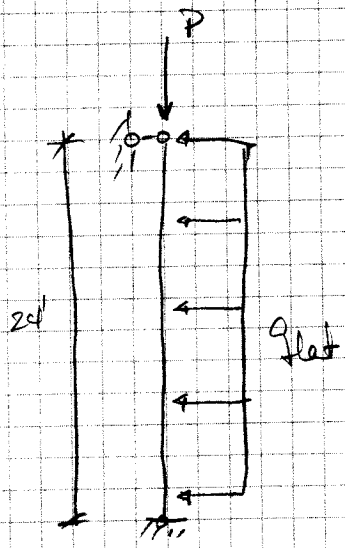


		D	L_r	L
P	Reacc 2 RB-1			
	Reacc 1 RB-7	183.83	19.98	11.88
	Reacc 2 ZB-7			

HSS 6x6x⅝ *

* See calculation report

→ HSS-3 (Worst)

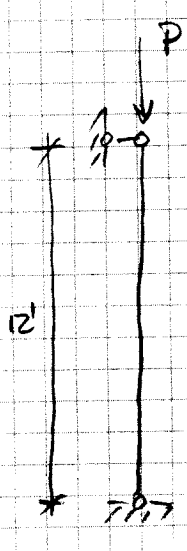


P (Reacc RB-4) { D: 28.80; Lr: 8.30

qlat = (C&C, Zone 4, 20 sft) 43.44 psf / 0.6 × 5.5' = 398 plf

HSS 8 × 4 × 5/8 *

→ HSS-4



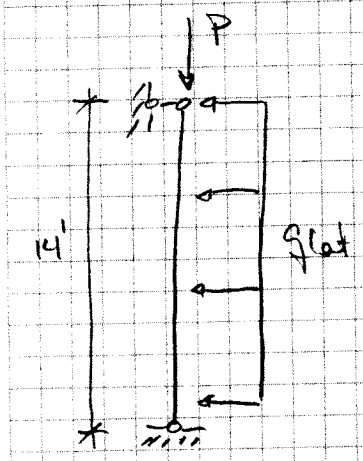
P { Reacc 3 RB-1; Reacc 2 RB-3; Reacc 2 2B-6; Reacc 3 2B-7

	D	Lr	L
	302.14	19.71	84.22

HSS 10 × 10 × 5/8 *

*See calculation report

→ HSS-S



P (Reacc. 2 RB-9)	D	L_r
	28.99	5.87

glat = 38.09 psf / 0.6 x 14' = 889 plf
(C&C, Zone 4 20 sft)

HSS 6x4x3/8 *

* See calculation report

Project Title:
Engineer:
Project ID:
Project Descr:

Steel Column

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: HSS-1 (worst)

Code References

Calculations per AISC 360-16, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : ASCE 7-16

General Information

Steel Section Name : **HSS8x8x1/2**

Analysis Method :	Allowable Strength
Steel Stress Grade	
Fy : Steel Yield	46.0 ksi
E : Elastic Bending Modulus	29,000.0 ksi

Overall Column Height	12.0 ft
Top & Bottom Fixity	Top & Bottom Pinned

Brace condition for deflection (buckling) along columns :
X-X (width) axis :
Unbraced Length for buckling ABOUT Y-Y Axis = 12.0 ft, K = 1.0
Y-Y (depth) axis :
Unbraced Length for buckling ABOUT X-X Axis = 12.0 ft, K = 1.0

Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Column self weight included : 586.20 lbs * Dead Load Factor
AXIAL LOADS . . .
Axial Load at 12.0 ft, D = 170.570, LR = 17.730, L = 90.640 k
BENDING LOADS . . .
Lat. Uniform Load creating Mx-x, W = 0.4340 k/ft

DESIGN SUMMARY

Bending & Shear Check Results

PASS Max. Axial+Bending Stress Ratio = **0.8257** : 1

Load Combination	+D+0.750Lr+0.750L+0.450W	
Location of max.above base	5.960	ft
At maximum location values are . . .		
Pa : Axial	252.434	k
Pn / Omega : Allowable	319.761	k
Ma-x : Applied	3.515	k-ft
Mn-x / Omega : Allowable	86.078	k-ft
Ma-y : Applied	0.0	k-ft
Mn-y / Omega : Allowable	86.078	k-ft

PASS Maximum Shear Stress Ratio **0.01539** : 1

Load Combination	+D+0.60W	
Location of max.above base	12.0	ft
At maximum location values are . . .		
Va : Applied	1.562	k
Vn / Omega : Allowable	101.519	k

Maximum Load Reactions . .

Top along X-X	0.0 k
Bottom along X-X	0.0 k
Top along Y-Y	2.604 k
Bottom along Y-Y	2.604 k

Maximum Load Deflections . . .

Along Y-Y 0.05646 in at 6.040 ft above base
for load combination : W Only

Along X-X 0.0 in at 0.0 ft above base
for load combination :

Load Combination Results

Load Combination	Maximum Axial + Bending Stress Ratios: Stress Ratio	Status	Location	Cbx	Cby	KxLx/Ry	KyLy/Rx	Maximum Shear Ratios: Stress Ratio	Status	Location
D Only	0.535	PASS	0.00 ft	1.14	1.00	47.37	47.37	0.000	PASS	0.00 ft
+D+L	0.819	PASS	0.00 ft	1.14	1.00	47.37	47.37	0.000	PASS	0.00 ft
+D+Lr	0.591	PASS	0.00 ft	1.14	1.00	47.37	47.37	0.000	PASS	0.00 ft
+D+0.750Lr+0.750L	0.789	PASS	0.00 ft	1.14	1.00	47.37	47.37	0.000	PASS	0.00 ft
+D+0.750L	0.748	PASS	0.00 ft	1.14	1.00	47.37	47.37	0.000	PASS	0.00 ft
+D+0.60W	0.584	PASS	5.96 ft	1.14	1.00	47.37	47.37	0.015	PASS	12.00 ft
+D+0.750Lr+0.750L+0.450	0.826	PASS	5.96 ft	1.14	1.00	47.37	47.37	0.012	PASS	0.00 ft
+D+0.750L+0.450W	0.784	PASS	5.96 ft	1.14	1.00	47.37	47.37	0.012	PASS	0.00 ft
+0.60D+0.60W	0.370	PASS	5.96 ft	1.14	1.00	47.37	47.37	0.015	PASS	12.00 ft
+0.60D	0.321	PASS	0.00 ft	1.14	1.00	47.37	47.37	0.000	PASS	12.00 ft

Maximum Reactions

Note: Only non-zero reactions are listed.

Load Combination	Axial Reaction @ Base	X-X Axis Reaction k @ Base	@ Top	Y-Y Axis Reaction @ Base	@ Top	Mx - End Moments k-ft @ Base	@ Top	My - End Moments @ Base	@ Top
D Only	171.156								
+D+L	261.796								
+D+Lr	188.886								

Project Title:
Engineer:
Project ID:
Project Descr:

Steel Column

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: HSS-1 (worst)

Maximum Reactions

Note: Only non-zero reactions are listed.

Load Combination	Axial Reaction @ Base	X-X Axis Reaction @ Base	@ Top	k	Y-Y Axis Reaction @ Base	@ Top	Mx - End Moments k-ft @ Base	@ Top	My - End Moments @ Base	@ Top
+D+0.750Lr+0.750L	252.434									
+D+0.750L	239.136									
+D+0.60W	171.156				1.562	1.562				
+D+0.750Lr+0.750L+0.450W	252.434				1.172	1.172				
+D+0.750L+0.450W	239.136				1.172	1.172				
+0.60D+0.60W	102.694				1.562	1.562				
+0.60D	102.694									
Lr Only	17.730									
L Only	90.640									
W Only					2.604	2.604				

Extreme Reactions

Item	Extreme Value	Axial Reaction @ Base	X-X Axis Reaction @ Base	@ Top	k	Y-Y Axis Reaction @ Base	@ Top	Mx - End Moments k-ft @ Base	@ Top	My - End Moments @ Base	@ Top
Axial @ Base	Maximum	261.796									
"	Minimum					2.604	2.604				
Reaction, X-X Axis Base	Maximum	171.156									
"	Minimum	171.156									
Reaction, Y-Y Axis Base	Maximum					2.604	2.604				
"	Minimum	171.156									
Reaction, X-X Axis Top	Maximum	171.156									
"	Minimum	171.156									
Reaction, Y-Y Axis Top	Maximum	171.156									
"	Minimum	171.156									
Moment, X-X Axis Base	Maximum	171.156									
"	Minimum	171.156									
Moment, Y-Y Axis Base	Maximum	171.156									
"	Minimum	171.156									
Moment, X-X Axis Top	Maximum	171.156									
"	Minimum	171.156									
Moment, Y-Y Axis Top	Maximum	171.156									
"	Minimum	171.156									

Maximum Deflections for Load Combinations

Load Combination	Max. X-X Deflection	Distance	Max. Y-Y Deflection	Distance
D Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+L	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+Lr	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+0.750Lr+0.750L	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+0.750L	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+0.60W	0.0000 in	0.000 ft	0.034 in	6.040 ft
+D+0.750Lr+0.750L+0.450W	0.0000 in	0.000 ft	0.025 in	6.040 ft
+D+0.750L+0.450W	0.0000 in	0.000 ft	0.025 in	6.040 ft
+0.60D+0.60W	0.0000 in	0.000 ft	0.034 in	6.040 ft
+0.60D	0.0000 in	0.000 ft	0.000 in	0.000 ft
Lr Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
L Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
W Only	0.0000 in	0.000 ft	0.056 in	6.040 ft

Steel Section Properties : HSS8x8x1/2

Steel Section Properties : HSS8x8x1/2

Project Title:
Engineer:
Project ID:
Project Descr:

Steel Column

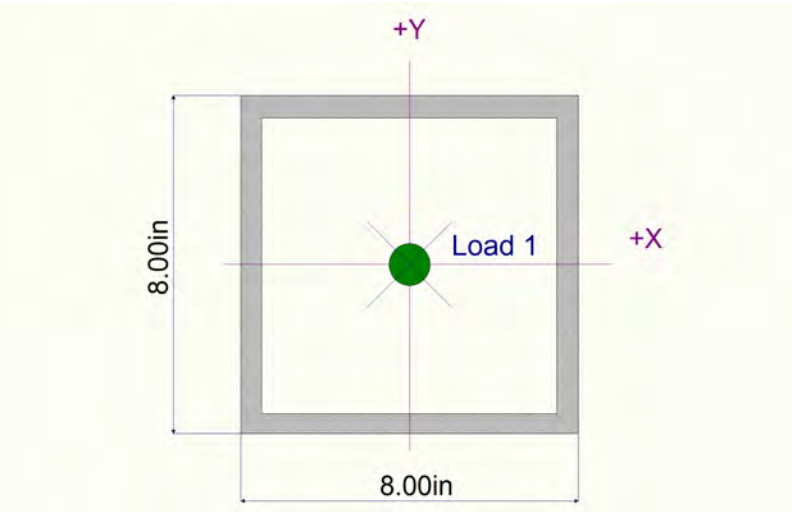
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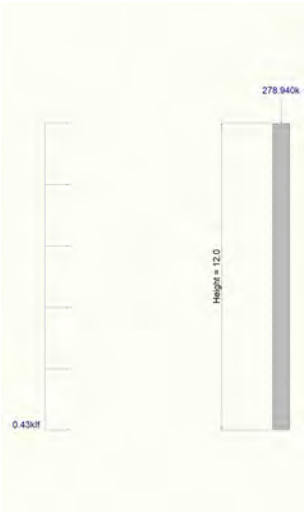
LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

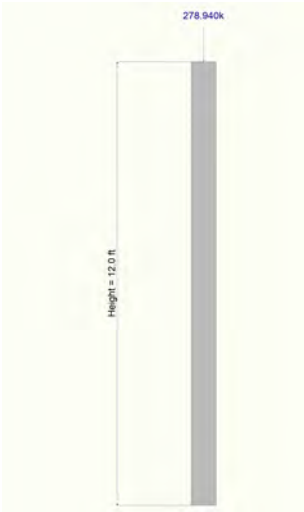
DESCRIPTION: HSS-1 (worst)

Depth	=	8.000	in	I xx	=	125.00	in^4	J	=	204.000 in^4
Design Thick	=	0.465	in	S xx	=	31.20	in^3			
Width	=	8.000	in	R xx	=	3.040	in			
Wall Thick	=	0.500	in	Zx	=	37.500	in^3			
Area	=	13.500	in^2	I yy	=	125.000	in^4	C	=	52.400 in^3
Weight	=	48.850	plf	S yy	=	31.200	in^3			
				R yy	=	3.040	in			
Ycg	=	0.000	in							

Sketches







Project Title:
Engineer:
Project ID:
Project Descr:

Steel Column

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: HSS-2 (worst)

Code References

Calculations per AISC 360-16, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : ASCE 7-16

General Information

Steel Section Name : **HSS6x6x5/8**

Analysis Method :	Allowable Strength
Steel Stress Grade	
Fy : Steel Yield	46.0 ksi
E : Elastic Bending Modulus	29,000.0 ksi

Overall Column Height	12.0 ft
Top & Bottom Fixity	Top & Bottom Pinned

Brace condition for deflection (buckling) along columns :
X-X (width) axis :
Unbraced Length for buckling ABOUT Y-Y Axis = 12.0 ft, K = 1.0
Y-Y (depth) axis :
Unbraced Length for buckling ABOUT X-X Axis = 12.0 ft, K = 1.0

Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Column self weight included : 505.17 lbs * Dead Load Factor
AXIAL LOADS . . .
Axial Load at 12.0 ft, D = 187.830, LR = 19.480, L = 11.880 k

DESIGN SUMMARY

Bending & Shear Check Results

PASS Max. Axial+Bending Stress Ratio = **0.8840** : 1

Load Combination	+D+0.750Lr+0.750L	
Location of max.above base	0.0	ft
At maximum location values are . . .		
Pa : Axial	211.855	k
Pn / Omega : Allowabl	239.652	k
Ma-x : Applied	0.0	k-ft
Mn-x / Omega : Allowable	53.253	k-ft
Ma-y : Applied	0.0	k-ft
Mn-y / Omega : Allowable	53.253	k-ft

PASS Maximum Shear Stress Rati **0.0** : 1

Load Combination	0.0	
Location of max.above base	0.0	ft
At maximum location values are . . .		
Va : Applied	0.0	k
Vn / Omega : Allowable	0.0	k

Maximum Load Reactions . .

Top along X-X	0.0 k
Bottom along X-X	0.0 k
Top along Y-Y	0.0 k
Bottom along Y-Y	0.0 k

Maximum Load Deflections . . .

Along Y-Y 0.0 in at 0.0 ft above base
for load combination :

Along X-X 0.0 in at 0.0 ft above base
for load combination :

Load Combination Results

Load Combination	Maximum Axial + Bending Stress Ratios: Stress Ratio	Status	Location	Cbx	Cby	KxLx/Ry	KyLy/Rx	Maximum Shear Ratios: Stress Ratio	Status	Location
D Only	0.786	PASS	0.00 ft	1.00	1.00	66.36	66.36	0.000	PASS	0.00 ft
+D+L	0.835	PASS	0.00 ft	1.00	1.00	66.36	66.36	0.000	PASS	0.00 ft
+D+Lr	0.867	PASS	0.00 ft	1.00	1.00	66.36	66.36	0.000	PASS	0.00 ft
+D+0.750Lr+0.750L	0.884	PASS	0.00 ft	1.00	1.00	66.36	66.36	0.000	PASS	0.00 ft
+D+0.750L	0.823	PASS	0.00 ft	1.00	1.00	66.36	66.36	0.000	PASS	0.00 ft
+0.60D	0.472	PASS	0.00 ft	1.00	1.00	66.36	66.36	0.000	PASS	0.00 ft

Maximum Reactions

Note: Only non-zero reactions are listed.

Load Combination	Axial Reaction @ Base	X-X Axis Reaction @ Base	@ Top	k	Y-Y Axis Reaction @ Base	@ Top	Mx - End Moments @ Base	k-ft @ Top	My - End Moments @ Base	@ Top
D Only	188.335									
+D+L	200.215									
+D+Lr	207.815									
+D+0.750Lr+0.750L	211.855									
+D+0.750L	197.245									
+0.60D	113.001									
Lr Only	19.480									
L Only	11.880									

Project Title:
Engineer:
Project ID:
Project Descr:

Steel Column

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: HSS-2 (worst)

Extreme Reactions

Item	Extreme Value	Axial Reaction @ Base	X-X Axis Reaction @ Base	@ Top	k	Y-Y Axis Reaction @ Base	@ Top	Mx - End Moments @ Base	@ Top	k-ft	My - End Moments @ Base	@ Top
Axial @ Base	Maximum	211.855										
"	Minimum	11.880										
Reaction, X-X Axis Base	Maximum	188.335										
"	Minimum	188.335										
Reaction, Y-Y Axis Base	Maximum	188.335										
"	Minimum	188.335										
Reaction, X-X Axis Top	Maximum	188.335										
"	Minimum	188.335										
Reaction, Y-Y Axis Top	Maximum	188.335										
"	Minimum	188.335										
Moment, X-X Axis Base	Maximum	188.335										
"	Minimum	188.335										
Moment, Y-Y Axis Base	Maximum	188.335										
"	Minimum	188.335										
Moment, X-X Axis Top	Maximum	188.335										
"	Minimum	188.335										
Moment, Y-Y Axis Top	Maximum	188.335										
"	Minimum	188.335										

Maximum Deflections for Load Combinations

Load Combination	Max. X-X Deflection	Distance	Max. Y-Y Deflection	Distance
D Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+L	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+Lr	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+0.750Lr+0.750L	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+0.750L	0.0000 in	0.000 ft	0.000 in	0.000 ft
+0.60D	0.0000 in	0.000 ft	0.000 in	0.000 ft
Lr Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
L Only	0.0000 in	0.000 ft	0.000 in	0.000 ft

Steel Section Properties : HSS6x6x5/8

Depth	=	6.000 in	I xx	=	55.20 in^4	J	=	94.900 in^4
Design Thick	=	0.581 in	S xx	=	18.40 in^3			
Width	=	6.000 in	R xx	=	2.170 in			
Wall Thick	=	0.624 in	Zx	=	23.200 in^3			
Area	=	11.700 in^2	I yy	=	55.200 in^4	C	=	33.400 in^3
Weight	=	42.097 plf	S yy	=	18.400 in^3			
			R yy	=	2.170 in			
Ycg	=	0.000 in						

Project Title:
Engineer:
Project ID:
Project Descr:

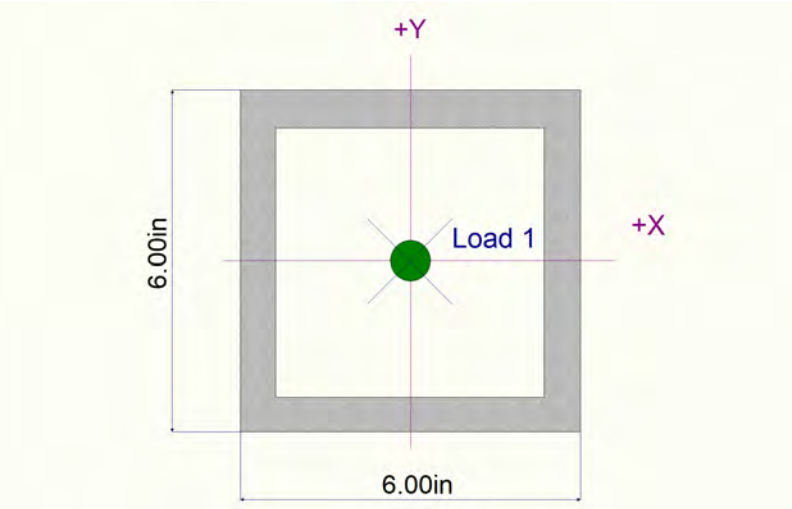
Steel Column

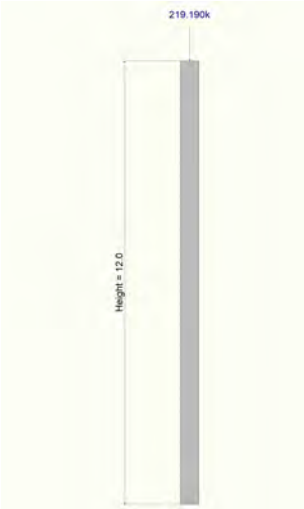
Project File: NR134.ec6

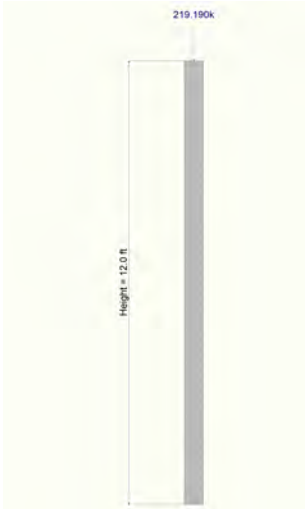
LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: HSS-2 (worst)

Sketches







Project Title:
Engineer:
Project ID:
Project Descr:

Steel Column

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: HSS-3 (worst)

Code References

Calculations per AISC 360-16, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : ASCE 7-16

General Information

Steel Section Name : **HSS8x4x5/8**

Analysis Method :	Allowable Strength
Steel Stress Grade	
Fy : Steel Yield	46.0 ksi
E : Elastic Bending Modulus	29,000.0 ksi

Overall Column Height	24.0 ft
Top & Bottom Fixity	Top & Bottom Pinned

Brace condition for deflection (buckling) along columns :
X-X (width) axis :
Unbraced Length for buckling ABOUT Y-Y Axis = 24.0 ft, K = 1.0
Y-Y (depth) axis :
Unbraced Length for buckling ABOUT X-X Axis = 24.0 ft, K = 1.0

Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Column self weight included : 1,010.33 lbs * Dead Load Factor
AXIAL LOADS . . .
Axial Load at 24.0 ft, D = 28.50, LR = 5.30 k
BENDING LOADS . . .
Lat. Uniform Load creating Mx-x, W = 0.3980 k/ft

DESIGN SUMMARY

Bending & Shear Check Results

PASS Max. Axial+Bending Stress Ratio = **0.8749** : 1

Load Combination	+D+0.750Lr+0.450W	
Location of max.above base	12.081	ft
At maximum location values are . . .		
Pa : Axial	33.485	k
Pn / Omega : Allowable	48.343	k
Ma-x : Applied	12.895	k-ft
Mn-x / Omega : Allowable	62.894	k-ft
Ma-y : Applied	0.0	k-ft
Mn-y / Omega : Allowable	38.104	k-ft

PASS Maximum Shear Stress Ratio = **0.02385** : 1

Load Combination	+D+0.60W	
Location of max.above base	0.0	ft
At maximum location values are . . .		
Va : Applied	2.866	k
Vn / Omega : Allowable	120.161	k

Maximum Load Reactions . .

Top along X-X	0.0 k
Bottom along X-X	0.0 k
Top along Y-Y	4.776 k
Bottom along Y-Y	4.776 k

Maximum Load Deflections . . .

Along Y-Y 1.263 in at 12.081 ft above base
for load combination : W Only

Along X-X 0.0 in at 0.0 ft above base
for load combination :

Load Combination Results

Load Combination	Maximum Axial + Bending Stress Ratios: Stress Ratio	Status	Location	Cbx	Cby	KxLx/Ry	KyLy/Rx	Maximum Shear Ratios: Stress Ratio	Status	Location
D Only	0.610	PASS	0.00 ft	1.14	1.00	109.09	190.73	0.000	PASS	0.00 ft
+D+Lr	0.720	PASS	0.00 ft	1.14	1.00	109.09	190.73	0.000	PASS	0.00 ft
+D+0.750Lr	0.693	PASS	0.00 ft	1.14	1.00	109.09	190.73	0.000	PASS	0.00 ft
+D+0.60W	0.853	PASS	12.08 ft	1.14	1.00	109.09	190.73	0.024	PASS	0.00 ft
+D+0.750Lr+0.450W	0.875	PASS	12.08 ft	1.14	1.00	109.09	190.73	0.018	PASS	0.00 ft
+D+0.450W	0.793	PASS	12.08 ft	1.14	1.00	109.09	190.73	0.018	PASS	0.00 ft
+0.60D+0.60W	0.609	PASS	11.92 ft	1.14	1.00	109.09	190.73	0.024	PASS	0.00 ft
+0.60D	0.366	PASS	0.00 ft	1.14	1.00	109.09	190.73	0.000	PASS	0.00 ft

Maximum Reactions

Note: Only non-zero reactions are listed.

Load Combination	Axial Reaction @ Base	X-X Axis Reaction k @ Base	@ Top	Y-Y Axis Reaction @ Base	@ Top	Mx - End Moments k-ft @ Base	@ Top	My - End Moments @ Base	@ Top
D Only	29.510								
+D+Lr	34.810								
+D+0.750Lr	33.485								
+D+0.60W	29.510			2.866	2.866				
+D+0.750Lr+0.450W	33.485			2.149	2.149				

Project Title:
Engineer:
Project ID:
Project Descr:

Steel Column

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: HSS-3 (worst)

Maximum Reactions

Note: Only non-zero reactions are listed.

Load Combination	Axial Reaction @ Base	X-X Axis Reaction @ Base	X-X Axis Reaction @ Top	k	Y-Y Axis Reaction @ Base	Y-Y Axis Reaction @ Top	Mx - End Moments k-ft @ Base	Mx - End Moments k-ft @ Top	My - End Moments @ Base	My - End Moments @ Top
+D+0.450W	29.510				2.149	2.149				
+0.60D+0.60W	17.706				2.866	2.866				
+0.60D	17.706									
Lr Only	5.300									
W Only					4.776	4.776				

Extreme Reactions

Item	Extreme Value	Axial Reaction @ Base	X-X Axis Reaction @ Base	X-X Axis Reaction @ Top	k	Y-Y Axis Reaction @ Base	Y-Y Axis Reaction @ Top	Mx - End Moments k-ft @ Base	Mx - End Moments k-ft @ Top	My - End Moments @ Base	My - End Moments @ Top
Axial @ Base	Maximum	34.810									
"	Minimum					4.776	4.776				
Reaction, X-X Axis Base	Maximum	29.510									
"	Minimum	29.510									
Reaction, Y-Y Axis Base	Maximum					4.776	4.776				
"	Minimum	29.510									
Reaction, X-X Axis Top	Maximum	29.510									
"	Minimum	29.510									
Reaction, Y-Y Axis Top	Maximum	29.510									
"	Minimum	29.510									
Moment, X-X Axis Base	Maximum	29.510									
"	Minimum	29.510									
Moment, Y-Y Axis Base	Maximum	29.510									
"	Minimum	29.510									
Moment, X-X Axis Top	Maximum	29.510									
"	Minimum	29.510									
Moment, Y-Y Axis Top	Maximum	29.510									
"	Minimum	29.510									

Maximum Deflections for Load Combinations

Load Combination	Max. X-X Deflection	Distance	Max. Y-Y Deflection	Distance
D Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+Lr	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+0.750Lr	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+0.60W	0.0000 in	0.000 ft	0.758 in	12.081 ft
+D+0.750Lr+0.450W	0.0000 in	0.000 ft	0.568 in	12.081 ft
+D+0.450W	0.0000 in	0.000 ft	0.568 in	12.081 ft
+0.60D+0.60W	0.0000 in	0.000 ft	0.758 in	12.081 ft
+0.60D	0.0000 in	0.000 ft	0.000 in	0.000 ft
Lr Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
W Only	0.0000 in	0.000 ft	1.263 in	12.081 ft

Steel Section Properties : HSS8x4x5/8

Depth	=	8.000 in	I xx	=	82.00 in^4	J	=	70.300 in^4
Design Thick	=	0.581 in	S xx	=	20.50 in^3	Cw	=	28.70 in^6
Width	=	4.000 in	R xx	=	2.640 in			
Wall Thick	=	0.624 in	Zx	=	27.400 in^3			
Area	=	11.700 in^2	I yy	=	26.600 in^4	C	=	28.700 in^3
Weight	=	42.097 plf	S yy	=	13.300 in^3			
			R yy	=	1.510 in			
			Zy	=	16.600 in^3			
Ycg	=	0.000 in						

Project Title:
Engineer:
Project ID:
Project Descr:

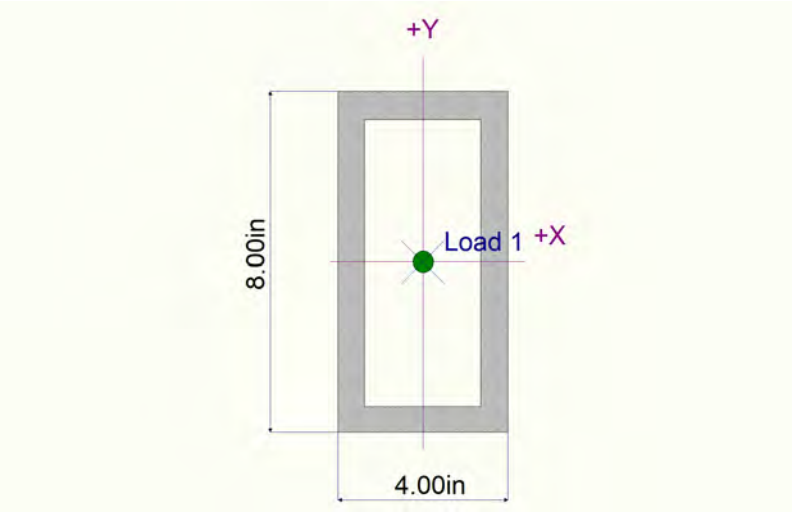
Steel Column

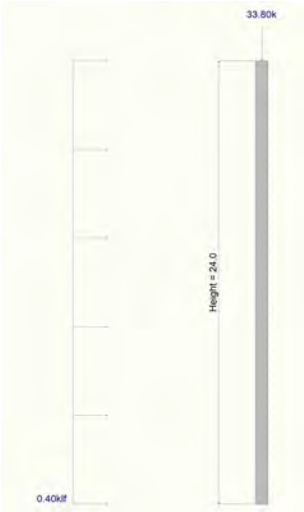
Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: HSS-3 (worst)

Sketches







Project Title:
Engineer:
Project ID:
Project Descr:

Steel Base Plate

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: PL12x8 (HSS-3)

Code Reference

Calculations per AISC Design Guide # 1, IBC 2018, CBC 2019, ASCE 7-16, AISC 360-16
Load Combination Set : ASCE 7-16

General Information

Material Properties

AISC Design Method	Load Resistance Factor Design		Φ c : LRFD Resistance Factor	0.65
Steel Plate Fy	=	36 ksi		
Concrete Support f'c	=	3 ksi		
Assumed Bearing Area	Full Bearing		Nominal Bearing Fp per J8	2.550 ksi

Column & Plate

Column Properties

Steel Section	HSS8x4x5/8			
Depth	8 in	Area	11.7	in^2
Width	4 in	Ixx		in^4
Flange Thickness	0.581 in	Iyy		in^4
Web Thickness	in			

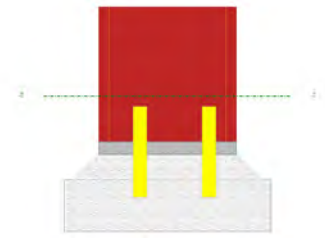
Plate Dimensions

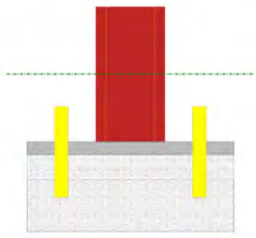
N : Length	8.0 in
B : Width	12.0 in
Thickness	0.750 in

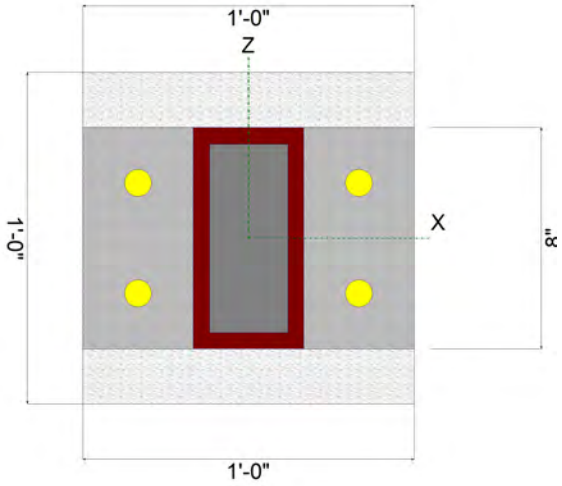
Support Dimensions

Width along "X"	12.0 in
Length along "Z"	12.0 in

Column assumed welded to base plate



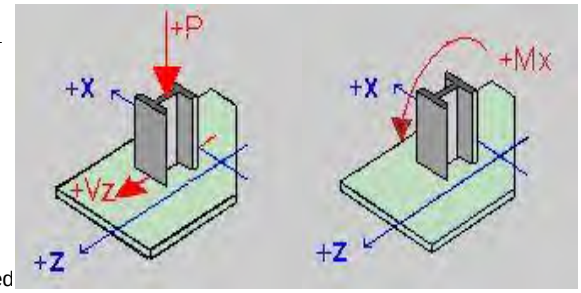




Applied Loads

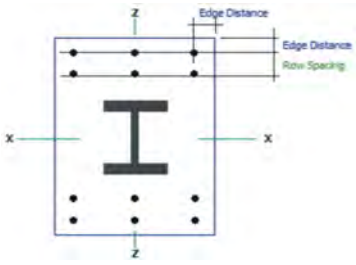
	P-Y	V-Z	M-X
D : Dead Load	28.50 k	k	k-ft
L : Live	k	k	k-ft
Lr : Roof Live	5.30 k	k	k-ft
S : Snow	k	k	k-ft
W : Wind	k	k	k-ft
E : Earthquake	k	k	k-ft
H : Lateral Earth	k	k	k-ft

" P " = Gravity load, "+" sign is downward "+" Moments create higher soil pressure at +Z ed
"+" Shears push plate towards +Z edge



Anchor Bolts

Anchor Bolt or Rod Description	5/8"	
Max of Tension or Pullout Capacity.....		k
Shear Capacity.....		k
Edge distance : bolt to plate.....	2.0	in
Number of Bolts in each Row.....	2	
Number of Bolt Rows.....	1	



Project Title:
Engineer:
Project ID:
Project Descr:

Steel Base Plate

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: PL12x8 (HSS-3)

GOVERNING DESIGN LOAD CASE SUMMARY

Plate Design Summary

Design Method	**Load Resistance Factor Design**
Governing Load Combinati	**+1.20D+1.60Lr**
Governing Load Case Typ	**Axial Load Only**
Governing STRESS RATIO	**0.5468**
Design Plate Size	**8" x 1'-0" x 0 -3/4"**

Pu : Axial	0.000	k
Mu : Moment	0.000	k-ft

Mu : Max. Moment	3.737	k-in
fb : Max. Bending Stress	17.715	ksi
Fb : Allowable : Fy * Phi	32.400	ksi
Bending Stress Rati	0.547	
	Bending Stress OK	
fu : Max. Plate Bearing Stress	0.445	ksi
Fp : Allowable :	1.658	ksi
Bearing Stress Ratio	0.268	
	Bearing Stress OK	

Load Comb. : +1.40D

Axial Load Only, No Moment

Loading

Pu : Axial	39.900	k
Design Plate Height	8.000	in
Design Plate Width	12.000	in
Will be different from entry if partial bearing used.		
A1 : Plate Area	96.000	in^2
A2: Support Area	96.000	in^2
sqrt(A2/A1)	1.000	

Distance for Moment Calculation

" m "	0.200	in
" n "	4.100	in
X	0.000	in^2
Lambda	0.000	
n'	0.000	in
n' * Lambda	0.000	in
L = max(m, n, n")	4.100	in

Bearing Stresses

Fp : Allowable	1.658	ksi
fu : Max. Bearing Pressure	0.416	ksi
Stress Ratio	**0.251**	

Plate Bending Stresses

Mmax = Fu * L^2 / 2	3.493	k-in
fb : Actual	16.561	ksi
Fb : Allowable	32.400	ksi
Stress Ratio	**0.511**	

Load Comb. : +1.20D+0.50Lr

Axial Load Only, No Moment

Loading

Pu : Axial	36.850	k
Design Plate Height	8.000	in
Design Plate Width	12.000	in
Will be different from entry if partial bearing used.		
A1 : Plate Area	96.000	in^2
A2: Support Area	96.000	in^2
sqrt(A2/A1)	1.000	

Distance for Moment Calculation

" m "	0.200	in
" n "	4.100	in
X	0.000	in^2
Lambda	0.000	
n'	0.000	in
n' * Lambda	0.000	in
L = max(m, n, n")	4.100	in

Bearing Stresses

Fp : Allowable	1.658	ksi
fu : Max. Bearing Pressure	0.384	ksi
Stress Ratio	**0.232**	

Plate Bending Stresses

Mmax = Fu * L^2 / 2	3.226	k-in
fb : Actual	15.295	ksi
Fb : Allowable	32.400	ksi
Stress Ratio	**0.472**	

Project Title:
Engineer:
Project ID:
Project Descr:

Steel Base Plate

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: PL12x8 (HSS-3)

Load Comb. : +1.20D

Axial Load Only, No Moment

Loading

Pu : Axial	34.200	k
Design Plate Height	8.000	in
Design Plate Width	12.000	in
Will be different from entry if partial bearing used.		
A1 : Plate Area	96.000	in^2
A2: Support Area	96.000	in^2
sqrt(A2/A1)	1.000	

Distance for Moment Calculation

" m "	0.200	in
" n "	4.100	in
X	0.000	in^2
Lambda	0.000	
n'	0.000	in
n' * Lambda	0.000	in
L = max(m, n, n")	4.100	in

Bearing Stresses

Fp : Allowable	1.658 ksi
fu : Max. Bearing Pressure	0.356 ksi
Stress Ratio	**0.215**

Plate Bending Stresses

Mmax = Fu * L^2 / 2	2.994 k-in
fb : Actual	14.195 ksi
Fb : Allowable	32.400 ksi
Stress Ratio	**0.438**

Load Comb. : +1.20D+1.60Lr

Axial Load Only, No Moment

Loading

Pu : Axial	42.680	k
Design Plate Height	8.000	in
Design Plate Width	12.000	in
Will be different from entry if partial bearing used.		
A1 : Plate Area	96.000	in^2
A2: Support Area	96.000	in^2
sqrt(A2/A1)	1.000	

Distance for Moment Calculation

" m "	0.200	in
" n "	4.100	in
X	0.000	in^2
Lambda	0.000	
n'	0.000	in
n' * Lambda	0.000	in
L = max(m, n, n")	4.100	in

Bearing Stresses

Fp : Allowable	1.658 ksi
fu : Max. Bearing Pressure	0.445 ksi
Stress Ratio	**0.268**

Plate Bending Stresses

Mmax = Fu * L^2 / 2	3.737 k-in
fb : Actual	17.715 ksi
Fb : Allowable	32.400 ksi
Stress Ratio	**0.547**

Load Comb. : +0.90D

Axial Load Only, No Moment

Loading

Pu : Axial	25.650	k
Design Plate Height	8.000	in
Design Plate Width	12.000	in
Will be different from entry if partial bearing used.		
A1 : Plate Area	96.000	in^2
A2: Support Area	96.000	in^2
sqrt(A2/A1)	1.000	

Distance for Moment Calculation

" m "	0.200	in
" n "	4.100	in
X	0.000	in^2
Lambda	0.000	
n'	0.000	in
n' * Lambda	0.000	in
L = max(m, n, n")	4.100	in

Bearing Stresses

Fp : Allowable	1.658 ksi
fu : Max. Bearing Pressure	0.267 ksi
Stress Ratio	**0.161**

Plate Bending Stresses

Mmax = Fu * L^2 / 2	2.246 k-in
fb : Actual	10.646 ksi
Fb : Allowable	32.400 ksi
Stress Ratio	**0.329**

Project Title:
Engineer:
Project ID:
Project Descr:

Steel Column

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: HSS-4 (worst)

Code References

Calculations per AISC 360-16, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : ASCE 7-16

General Information

Steel Section Name : **HSS10x10x5/8**

Analysis Method :	Allowable Strength
Steel Stress Grade	
Fy : Steel Yield	46.0 ksi
E : Elastic Bending Modulus	29,000.0 ksi

Overall Column Height	12.0 ft
Top & Bottom Fixity	Top & Bottom Pinned

Brace condition for deflection (buckling) along columns :
X-X (width) axis :
Unbraced Length for buckling ABOUT Y-Y Axis = 12.0 ft, K = 1.0
Y-Y (depth) axis :
Unbraced Length for buckling ABOUT X-X Axis = 12.0 ft, K = 1.0

Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Column self weight included : 913.50 lbs * Dead Load Factor
AXIAL LOADS . . .
Axial Load at 12.0 ft, D = 302.140, LR = 19.740, L = 84.220 k

DESIGN SUMMARY

Bending & Shear Check Results

PASS Max. Axial+Bending Stress Ratio = **0.7374** : 1

Load Combination	+D+L	
Location of max.above base	0.0	ft
At maximum location values are . . .		
Pa : Axial	387.274	k
Pn / Omega : Allowable	525.18	k
Ma-x : Applied	0.0	k-ft
Mn-x / Omega : Allowable	168.024	k-ft
Ma-y : Applied	0.0	k-ft
Mn-y / Omega : Allowable	168.024	k-ft

PASS Maximum Shear Stress Ratio **0.0** : 1

Load Combination	0.0	
Location of max.above base	0.0	ft
At maximum location values are . . .		
Va : Applied	0.0	k
Vn / Omega : Allowable	0.0	k

Maximum Load Reactions . .

Top along X-X	0.0 k
Bottom along X-X	0.0 k
Top along Y-Y	0.0 k
Bottom along Y-Y	0.0 k

Maximum Load Deflections . . .

Along Y-Y 0.0 in at 0.0 ft above base
for load combination :

Along X-X 0.0 in at 0.0 ft above base
for load combination :

Load Combination Results

Load Combination	Maximum Axial + Bending Stress Ratios: Stress Ratio	Status	Location	Cbx	Cby	KxLx/Ry	KyLy/Rx	Maximum Shear Ratios: Stress Ratio	Status	Location
D Only	0.577	PASS	0.00 ft	1.00	1.00	37.89	37.89	0.000	PASS	0.00 ft
+D+L	0.737	PASS	0.00 ft	1.00	1.00	37.89	37.89	0.000	PASS	0.00 ft
+D+Lr	0.615	PASS	0.00 ft	1.00	1.00	37.89	37.89	0.000	PASS	0.00 ft
+D+0.750Lr+0.750L	0.726	PASS	0.00 ft	1.00	1.00	37.89	37.89	0.000	PASS	0.00 ft
+D+0.750L	0.697	PASS	0.00 ft	1.00	1.00	37.89	37.89	0.000	PASS	0.00 ft
+0.60D	0.346	PASS	0.00 ft	1.00	1.00	37.89	37.89	0.000	PASS	0.00 ft

Maximum Reactions

Note: Only non-zero reactions are listed.

Load Combination	Axial Reaction @ Base	X-X Axis Reaction k @ Base	@ Top	Y-Y Axis Reaction @ Base	@ Top	Mx - End Moments k-ft @ Base	@ Top	My - End Moments @ Base	@ Top
D Only	303.054								
+D+L	387.274								
+D+Lr	322.794								
+D+0.750Lr+0.750L	381.024								
+D+0.750L	366.219								
+0.60D	181.832								
Lr Only	19.740								
L Only	84.220								

Project Title:
Engineer:
Project ID:
Project Descr:

Steel Column

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: HSS-4 (worst)

Extreme Reactions

Item	Extreme Value	Axial Reaction @ Base	X-X Axis Reaction @ Base	@ Top	k	Y-Y Axis Reaction @ Base	@ Top	Mx - End Moments @ Base	@ Top	k-ft	My - End Moments @ Base	@ Top
Axial @ Base	Maximum	387.274										
"	Minimum	19.740										
Reaction, X-X Axis Base	Maximum	303.054										
"	Minimum	303.054										
Reaction, Y-Y Axis Base	Maximum	303.054										
"	Minimum	303.054										
Reaction, X-X Axis Top	Maximum	303.054										
"	Minimum	303.054										
Reaction, Y-Y Axis Top	Maximum	303.054										
"	Minimum	303.054										
Moment, X-X Axis Base	Maximum	303.054										
"	Minimum	303.054										
Moment, Y-Y Axis Base	Maximum	303.054										
"	Minimum	303.054										
Moment, X-X Axis Top	Maximum	303.054										
"	Minimum	303.054										
Moment, Y-Y Axis Top	Maximum	303.054										
"	Minimum	303.054										

Maximum Deflections for Load Combinations

Load Combination	Max. X-X Deflection	Distance	Max. Y-Y Deflection	Distance
D Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+L	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+Lr	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+0.750Lr+0.750L	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+0.750L	0.0000 in	0.000 ft	0.000 in	0.000 ft
+0.60D	0.0000 in	0.000 ft	0.000 in	0.000 ft
Lr Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
L Only	0.0000 in	0.000 ft	0.000 in	0.000 ft

Steel Section Properties : HSS10x10x5/8

Depth	=	10.000 in	I xx	=	304.00 in^4	J	=	498.000 in^4	
Design Thick	=	0.581 in	S xx	=	60.80 in^3				
Width	=	10.000 in	R xx	=	3.800 in				
Wall Thick	=	0.624 in	Zx	=	73.200 in^3				
Area	=	21.000 in^2	I yy	=	304.000 in^4	C	=	102.000 in^3	
Weight	=	76.125 plf	S yy	=	60.800 in^3				
			R yy	=	3.800 in				
Ycg	=	0.000 in							

Project Title:
Engineer:
Project ID:
Project Descr:

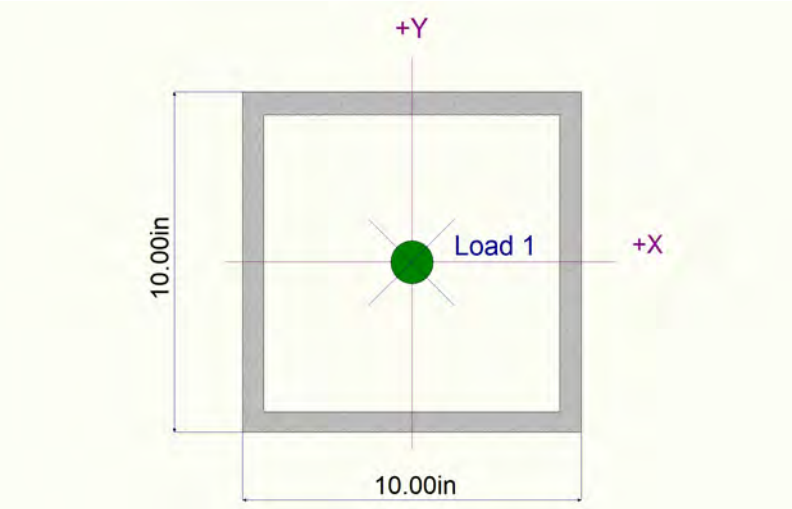
Steel Column

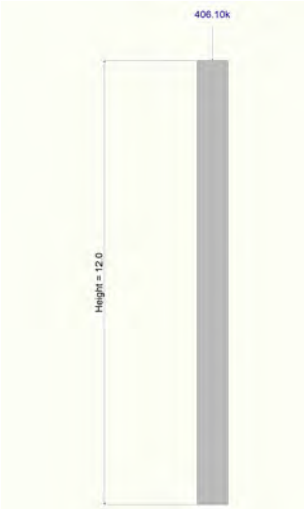
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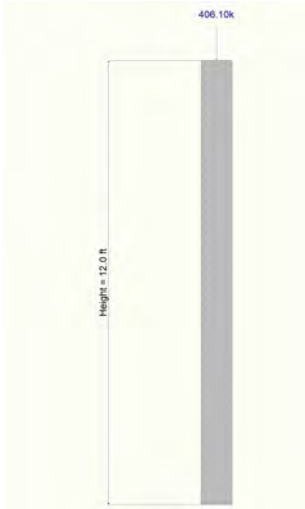
LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: HSS-4 (worst)

Sketches







Project Title:
Engineer:
Project ID:
Project Descr:

Steel Base Plate

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: PL16x16x1 (HSS-4)

Code Reference

Calculations per AISC Design Guide # 1, IBC 2018, CBC 2019, ASCE 7-16, AISC 360-16
Load Combination Set : ASCE 7-16

General Information

Material Properties

AISC Design Method	Load Resistance Factor Design		Φ c : LRFD Resistance Factor	0.65
Steel Plate Fy	=	36 ksi		
Concrete Support f'c	=	3 ksi		
Assumed Bearing Area	Full Bearing		Nominal Bearing Fp per J8	3.188 ksi

Column & Plate

Column Properties

Steel Section	HSS10x10x5/8		
Depth	10 in	Area	21 in^2
Width	10 in	Ixx	in^4
Flange Thickness	0.581 in	Iyy	in^4
Web Thickness	in		

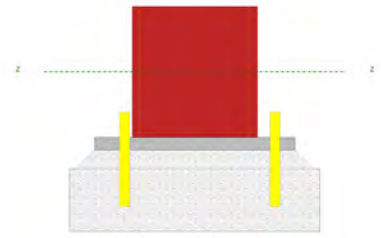
Plate Dimensions

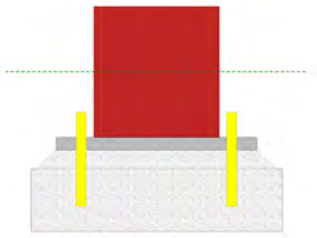
N : Length	16.0 in
B : Width	16.0 in
Thickness	1.0 in

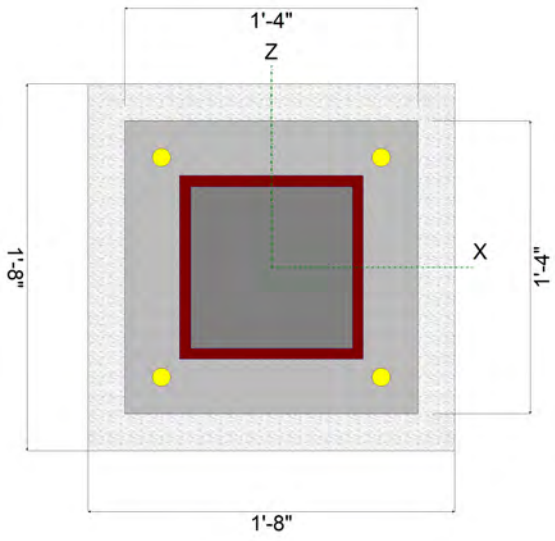
Support Dimensions

Width along "X"	20.0 in
Length along "Z'	20.0 in

Column assumed welded to base plate



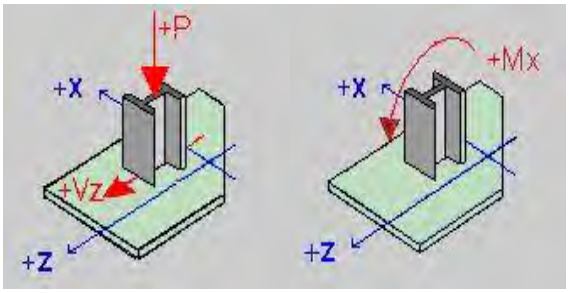




Applied Loads

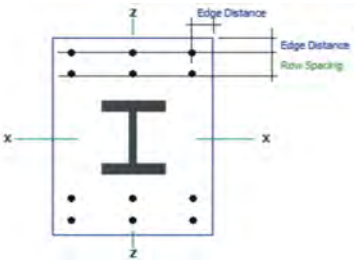
	P-Y	V-Z	M-X
D : Dead Load	302.140 k	k	k-ft
L : Live	84.220 k	k	k-ft
Lr : Roof Live	19.740 k	k	k-ft
S : Snow	k	k	k-ft
W : Wind	k	k	k-ft
E : Earthquake	k	k	k-ft
H : Lateral Earth	k	k	k-ft

" P " = Gravity load, "+" sign is downward "+" Moments create higher soil pressure at +Z ed
"+" Shears push plate towards +Z edge



Anchor Bolts

Anchor Bolt or Rod Description	3/4"	
Max of Tension or Pullout Capacity.....		k
Shear Capacity.....		k
Edge distance : bolt to plate.....	2.0	in
Number of Bolts in each Row.....	2	
Number of Bolt Rows.....	1	



Project Title:
Engineer:
Project ID:
Project Descr:

Steel Base Plate

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: PL16x16x1 (HSS-4)

GOVERNING DESIGN LOAD CASE SUMMARY

Plate Design Summary

Design Method	**Load Resistance Factor Design**
Governing Load Combinat	**+1.20D+0.50Lr+1.60L**
Governing Load Case Typ	**Axial Load Only**
Governing STRESS RATI(	**0.9562**
Design Plate Size	**1'-4" x 1'-4" x 1"**
Pu : Axial	0.000 k
Mu : Moment	0.000 k-ft

Mu : Max. Moment	10.463	k-in
fb : Max. Bending Stress	27.902	ksi
Fb : Allowable : Fy * Phi	32.400	ksi
Bending Stress Rati	0.861	
	Bending Stress OK	
fu : Max. Plate Bearing Stress	1.981	ksi
Fp : Allowable :	2.072	ksi
Bearing Stress Ratic	0.956	
	Bearing Stress OK	

Load Comb. : +1.40D

Axial Load Only, No Moment

Loading

Pu : Axial	422.996	k
Design Plate Height	16.000	in
Design Plate Width	16.000	in
Will be different from entry if partial bearing used.		
A1 : Plate Area	256.000	in^2
A2: Support Area	400.000	in^2
sqrt(A2/A1)	1.250	

Distance for Moment Calculation

" m "	3.250	in
" n "	3.250	in
X	0.000	in^2
Lambda	0.000	
n'	0.000	in
n' * Lambda	0.000	in
L = max(m, n, n")	3.250	in

Bearing Stresses

Fp : Allowable	2.072	ksi
fu : Max. Bearing Pressure	1.652	ksi
Stress Ratio	**0.798**	

Plate Bending Stresses

Mmax = Fu * L^2 / 2	8.726	k-in
fb : Actual	23.270	ksi
Fb : Allowable	32.400	ksi
Stress Ratio	**0.718**	

Load Comb. : +1.20D+0.50Lr+1.60L

Axial Load Only, No Moment

Loading

Pu : Axial	507.190	k
Design Plate Height	16.000	in
Design Plate Width	16.000	in
Will be different from entry if partial bearing used.		
A1 : Plate Area	256.000	in^2
A2: Support Area	400.000	in^2
sqrt(A2/A1)	1.250	

Distance for Moment Calculation

" m "	3.250	in
" n "	3.250	in
X	0.000	in^2
Lambda	0.000	
n'	0.000	in
n' * Lambda	0.000	in
L = max(m, n, n")	3.250	in

Bearing Stresses

Fp : Allowable	2.072	ksi
fu : Max. Bearing Pressure	1.981	ksi
Stress Ratio	**0.956**	

Plate Bending Stresses

Mmax = Fu * L^2 / 2	10.463	k-in
fb : Actual	27.902	ksi
Fb : Allowable	32.400	ksi
Stress Ratio	**0.861**	

Project Title:
Engineer:
Project ID:
Project Descr:

Steel Base Plate

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: PL16x16x1 (HSS-4)

Load Comb. : +1.20D+1.60L

Axial Load Only, No Moment

Loading		
Pu : Axial	497.320	k
Design Plate Height	16.000	in
Design Plate Width	16.000	in
Will be different from entry if partial bearing used.		
A1 : Plate Area	256.000	in^2
A2: Support Area	400.000	in^2
sqrt(A2/A1)	1.250	

Distance for Moment Calculation		
" m "	3.250	in
" n "	3.250	in
X	0.000	in^2
Lambda	0.000	
n'	0.000	in
n' * Lambda	0.000	in
L = max(m, n, n")	3.250	in

Bearing Stresses	
Fp : Allowable	2.072 ksi
fu : Max. Bearing Pressure	1.943 ksi
Stress Ratio	**0.938**

Plate Bending Stresses	
Mmax = Fu * L^2 / 2	10.260 k-in
fb : Actual	27.359 ksi
Fb : Allowable	32.400 ksi
Stress Ratio	**0.844**

Load Comb. : +1.20D+1.60Lr+L

Axial Load Only, No Moment

Loading		
Pu : Axial	478.372	k
Design Plate Height	16.000	in
Design Plate Width	16.000	in
Will be different from entry if partial bearing used.		
A1 : Plate Area	256.000	in^2
A2: Support Area	400.000	in^2
sqrt(A2/A1)	1.250	

Distance for Moment Calculation		
" m "	3.250	in
" n "	3.250	in
X	0.000	in^2
Lambda	0.000	
n'	0.000	in
n' * Lambda	0.000	in
L = max(m, n, n")	3.250	in

Bearing Stresses	
Fp : Allowable	2.072 ksi
fu : Max. Bearing Pressure	1.869 ksi
Stress Ratio	**0.902**

Plate Bending Stresses	
Mmax = Fu * L^2 / 2	9.869 k-in
fb : Actual	26.317 ksi
Fb : Allowable	32.400 ksi
Stress Ratio	**0.812**

Load Comb. : +1.20D+1.60Lr

Axial Load Only, No Moment

Loading		
Pu : Axial	394.152	k
Design Plate Height	16.000	in
Design Plate Width	16.000	in
Will be different from entry if partial bearing used.		
A1 : Plate Area	256.000	in^2
A2: Support Area	400.000	in^2
sqrt(A2/A1)	1.250	

Distance for Moment Calculation		
" m "	3.250	in
" n "	3.250	in
X	0.000	in^2
Lambda	0.000	
n'	0.000	in
n' * Lambda	0.000	in
L = max(m, n, n")	3.250	in

Bearing Stresses	
Fp : Allowable	2.072 ksi
fu : Max. Bearing Pressure	1.540 ksi
Stress Ratio	**0.743**

Plate Bending Stresses	
Mmax = Fu * L^2 / 2	8.131 k-in
fb : Actual	21.683 ksi
Fb : Allowable	32.400 ksi
Stress Ratio	**0.669**

Project Title:
Engineer:
Project ID:
Project Descr:

Steel Base Plate

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: PL16x16x1 (HSS-4)

Load Comb. : +1.20D+L

Axial Load Only, No Moment

Loading		
Pu : Axial	446.788	k
Design Plate Height	16.000	in
Design Plate Width	16.000	in
Will be different from entry if partial bearing used.		
A1 : Plate Area	256.000	in^2
A2: Support Area	400.000	in^2
sqrt(A2/A1)	1.250	

Distance for Moment Calculation		
" m "	3.250	in
" n "	3.250	in
X	0.000	in^2
Lambda	0.000	
n'	0.000	in
n' * Lambda	0.000	in
L = max(m, n, n")	3.250	in

Bearing Stresses		
Fp : Allowable	2.072	ksi
fu : Max. Bearing Pressure	1.745	ksi
Stress Ratio	**0.842**	

Plate Bending Stresses		
Mmax = Fu * L^2 / 2	9.217	k-in
fb : Actual	24.579	ksi
Fb : Allowable	32.400	ksi
Stress Ratio	**0.759**	

Load Comb. : +1.20D

Axial Load Only, No Moment

Loading		
Pu : Axial	362.568	k
Design Plate Height	16.000	in
Design Plate Width	16.000	in
Will be different from entry if partial bearing used.		
A1 : Plate Area	256.000	in^2
A2: Support Area	400.000	in^2
sqrt(A2/A1)	1.250	

Distance for Moment Calculation		
" m "	3.250	in
" n "	3.250	in
X	0.000	in^2
Lambda	0.000	
n'	0.000	in
n' * Lambda	0.000	in
L = max(m, n, n")	3.250	in

Bearing Stresses		
Fp : Allowable	2.072	ksi
fu : Max. Bearing Pressure	1.416	ksi
Stress Ratio	**0.684**	

Plate Bending Stresses		
Mmax = Fu * L^2 / 2	7.480	k-in
fb : Actual	19.946	ksi
Fb : Allowable	32.400	ksi
Stress Ratio	**0.616**	

Load Comb. : +1.20D+0.50Lr+L

Axial Load Only, No Moment

Loading		
Pu : Axial	456.658	k
Design Plate Height	16.000	in
Design Plate Width	16.000	in
Will be different from entry if partial bearing used.		
A1 : Plate Area	256.000	in^2
A2: Support Area	400.000	in^2
sqrt(A2/A1)	1.250	

Distance for Moment Calculation		
" m "	3.250	in
" n "	3.250	in
X	0.000	in^2
Lambda	0.000	
n'	0.000	in
n' * Lambda	0.000	in
L = max(m, n, n")	3.250	in

Bearing Stresses		
Fp : Allowable	2.072	ksi
fu : Max. Bearing Pressure	1.784	ksi
Stress Ratio	**0.861**	

Plate Bending Stresses		
Mmax = Fu * L^2 / 2	9.421	k-in
fb : Actual	25.122	ksi
Fb : Allowable	32.400	ksi
Stress Ratio	**0.775**	

Project Title:
Engineer:
Project ID:
Project Descr:

Steel Base Plate

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: PL16x16x1 (HSS-4)

Load Comb. : +0.90D

Axial Load Only, No Moment

Loading

Pu : Axial	271.926	k
Design Plate Height	16.000	in
Design Plate Width	16.000	in

Will be different from entry if partial bearing used.

A1 : Plate Area	256.000	in^2
A2: Support Area	400.000	in^2
sqrt(A2/A1)	1.250	

Distance for Moment Calculation

" m "	3.250	in
" n "	3.250	in
X	0.000	in^2
Lambda	0.000	
n'	0.000	in
n' * Lambda	0.000	in
L = max(m, n, n")	3.250	in

Bearing Stresses

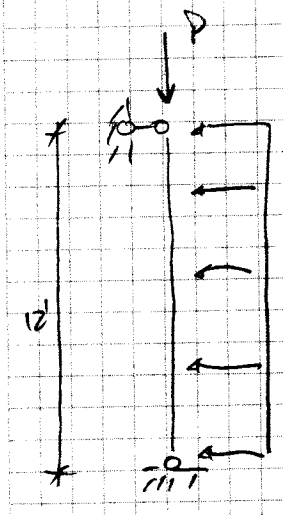
Fp : Allowable	2.072	ksi
fu : Max. Bearing Pressure	1.062	ksi
Stress Ratio	**0.513**	

Plate Bending Stresses

Mmax = Fu * L^2 / 2	5.610	k-in
fb : Actual	14.959	ksi
Fb : Allowable	32.400	ksi
Stress Ratio	**0.462**	

Concrete Columns Design:

→ TC-1 (8" x 12")



P (Reacc 3 2D-S)	D	Lr	L
	74.9	5.92	10.89

q_{lat} (CxC, Zone 4 20 psf) = 413.44 psf / 0.6 × 6' = 434 pf

→ the other columns have been design by $A_{s,min} = 1\% A_g$

TC-2 (8" x 16") = 1.28 in² → 6 #5

TC-3 (8" x 24") = 1.92 in² → 8 #5

TC-4 (8" x 28") = 2.24 in² → 8 #5

TC-5 (8" x 34") = 2.72 in² → 10 #5

TC-6 (8" x 42") = 3.36 in² → 8 #6

TC-7 (8" x 44") = 3.52 in² → 8 #6

TC-8 (See detail) = 1.28 in² → 6 #5

TC-9 (See detail) = 1.60 in² → 6 #5

TC-10 (See detail) = 3.17 in² → 12 #5

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Column

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: TC-1 (8"x12")

Code References

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : ASCE 7-16

General Information

f'c : Concrete 28 day streng	=	5.0 ksi
E =	=	4,030.51 ksi
Density	=	150.0 pcf
β	=	0.80
fy - Main Rebar	=	60.0 ksi
E - Main Rebar	=	29,000.0 ksi
Allow. Reinforcing Limits		*ASTM A615 Bars Used*
Min. Reinf.	=	1.0 %
Max. Reinf.	=	8.0 %

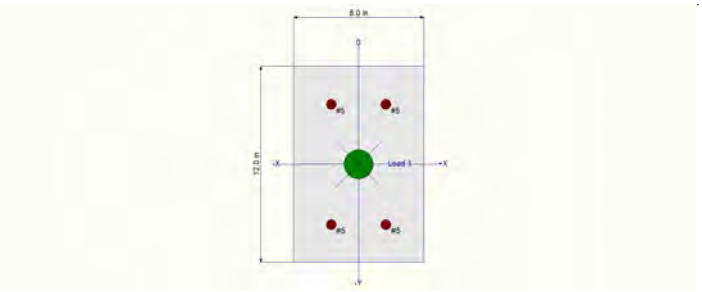
Overall Column Height = 12.0 ft
End Fixity Top & Bottom Pinned

Brace condition for deflection (buckling) along colum
X-X (width) axis :
Unbraced Length for buckling ABOUT Y-Y Axis = 12.0 ft, K = 1.0
Y-Y (depth) axis :
Unbraced Length for buckling ABOUT X-X Axis = 12.0 ft, K = 1.0

Column Cross Section

Column Dimensions : 12.0in high x 8.0in Wide, Column Edge to Rebar Edge Cover = 2.0in

Column Reinforcing : 4 - #5 bars @ corners,



Applied Loads

Entered loads are factored per load combinations specified by user.

Column self weight included : 1,200.0 lbs * Dead Load Factor
AXIAL LOADS . . .
Axial Load at 12.0 ft above base, D = 74.90, LR = 5.920, L = 10.890 k
BENDING LOADS . . .
Lat. Uniform Load creating Mx-x, W = 0.4340 k/ft

DESIGN SUMMARY

Load Combination +1.20D+0.50Lr+1.60L
Location of max.above base 11.919 ft

Maximum Stress Ratio **0.468 : 1**
Ratio = (Pu^2+Mu^2)^.5 / (PhiPn^2+PhiMn^2)^.5

Pu =	111.704 k	φ * Pn =	239.127 k
Mu-x =	12.511 k-ft	φ * Mn-x =	26.493 k-ft
Mu-y =	0.0 k-ft	φ * Mn-y =	0.0 k-ft
Mu Angle =	0.0 deg		
Mu at Angle =	12.511 k-ft	φMn at Angle =	26.715 k-ft

Pn & Mn values located at Pu-Mu vector intersection with capacity curve

Column Capacities . .

Pnmax : Nominal Max. Compressive Axial Capaci	477.130 k
Pnmin : Nominal Min. Tension Axial Capacity	k
φ Pn, max : Usable Compressive Axial Capacity	248.108 k
φ Pn, min : Usable Tension Axial Capacity	k

Maximum SERVICE Load Reactions .

Top along Y-Y	0.0 k	Bottom along Y-Y	0.0 k
Top along X-X	2.604 k	Bottom along X-X	2.604 k

Maximum SERVICE Load Deflections . .

Along Y-Y 0.04408 in at 6.040 ft above base
for load combination : W Only
Along X-X 0.0 in at 0.0 ft above base
for load combination :

General Section Informatio φ = 0.650 β = 0.80 θ = 0.80
ρ : % Reinforcing 1.292 % Rebar % Ok
Reinforcing Area 1.240 in^2
Concrete Area 96.0 in^2

Governing Load Combination Results

Governing Factored Load Combination	Moment X-X	Moment Y-Y	Dist. from base ft	Axial Load Pu k	φ * Pn	Bending Analysis k-ft δx	δx * Mux	δy	δy * Muy	Alpha (deg)	δ Mu	φ Mn	Utilization Ratio
+1.40D		M2,min	11.92	106.54	240.36			1.000	7.46	90.000	7.46	16.68	**0.445**

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Column

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: TC-1 (8"x12")

Governing Load Combination Results

Governing Factored Load Combination	Moment X-X	Moment Y-Y	Dist. from base ft	Axial Load k Pu	φ * Pn	Bending Analysis k-ft δx	δx * Mux	δy	δy * Muy	Alpha (deg)	δ Mu	φ Mn	Utilization Ratio
+1.40D	M2,min		11.92	106.54	239.13	1.400	11.93			0.000	11.93	26.71	**0.446**
+1.20D+0.50Lr+1.60L		M2,min	11.92	111.70	240.36			1.000	7.82	90.000	7.82	16.68	**0.466**
+1.20D+0.50Lr+1.60L	M2,min		11.92	111.70	239.13	1.400	12.51			0.000	12.51	26.71	**0.468**
+1.20D+1.60L		M2,min	11.92	108.74	240.36			1.000	7.61	90.000	7.61	16.68	**0.454**
+1.20D+1.60L	M2,min		11.92	108.74	239.13	1.400	12.18			0.000	12.18	26.71	**0.455**
+1.20D+1.60Lr+L		M2,min	11.92	111.68	240.36			1.000	7.82	90.000	7.82	16.68	**0.466**
+1.20D+1.60Lr+L	M2,min		11.92	111.68	239.13	1.400	12.51			0.000	12.51	26.71	**0.468**
+1.20D+1.60Lr+0.50W			11.92	100.79	239.13	1.400	11.29			0.000	11.29	26.71	**0.422**
+1.20D+L		M2,min	11.92	102.21	240.36			1.000	7.15	90.000	7.15	16.68	**0.427**
+1.20D+L	M2,min		11.92	102.21	241.76	1.348	11.02			0.000	11.02	25.93	**0.424**
+1.20D+0.50W			11.92	91.32	239.13	1.400	10.23			0.000	10.23	26.71	**0.383**
+1.20D+0.50Lr+L+W			11.92	105.17	239.13	1.400	11.78			0.000	11.78	26.71	**0.441**
+1.20D+L+W			11.92	102.21	239.13	1.400	11.45			0.000	11.45	26.71	**0.428**
+0.90D+W			11.92	68.49	211.60	1.400	10.94			0.000	10.94	33.75	**0.324**
+0.90D		M2,min	11.92	68.49	240.36			1.000	4.79	90.000	4.79	16.68	**0.286**
+0.90D	M2,min		11.92	68.49	248.11	1.000	5.48			0.000	5.48	19.85	**0.276**

Maximum Reactions

Note: Only non-zero reactions are listed.

Load Combination	X-X Axis Reaction @ Base	@ Top	k	Y-Y Axis Reaction @ Base	@ Top	Axial Reaction @ Base	My - End Moments @ Base	k-ft @ Top	Mx - End Moments @ Base	@ Top
D Only						76.100				
+D+L						86.990				
+D+Lr						82.020				
+D+0.750Lr+0.750L						88.708				
+D+0.750L						84.268				
+D+0.60W				1.562	1.562	76.100				
+D+0.750Lr+0.750L+0.450W				1.172	1.172	88.708				
+D+0.750L+0.450W				1.172	1.172	84.268				
+0.60D+0.60W				1.562	1.562	45.660				
+0.60D						45.660				
Lr Only						5.920				
L Only						10.890				
W Only				2.604	2.604					

Maximum Moment Reactions

Note: Only non-zero reactions are listed.

Load Combination	Moment About X-X Axis @ Base	@ Top		Moment About Y-Y Axis @ Base	@ Top	
D Only			k-ft			k-ft
+D+L			k-ft			k-ft
+D+Lr			k-ft			k-ft
+D+0.750Lr+0.750L			k-ft			k-ft
+D+0.750L			k-ft			k-ft
+D+0.60W			k-ft			k-ft
+D+0.750Lr+0.750L+0.450W			k-ft			k-ft
+D+0.750L+0.450W			k-ft			k-ft
+0.60D+0.60W			k-ft			k-ft
+0.60D			k-ft			k-ft
Lr Only			k-ft			k-ft
L Only			k-ft			k-ft
W Only			k-ft			k-ft

Maximum Deflections for Load Combinations

Load Combination	Max. X-X Deflection		Distance		Max. Y-Y Deflection		Distance	
D Only	0.0000	in	0.000	ft	0.000	in	0.000	ft
+D+L	0.0000	in	0.000	ft	0.000	in	0.000	ft
+D+Lr	0.0000	in	0.000	ft	0.000	in	0.000	ft
+D+0.750Lr+0.750L	0.0000	in	0.000	ft	0.000	in	0.000	ft
+D+0.750L	0.0000	in	0.000	ft	0.000	in	0.000	ft

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Column

Project File: NR134.ec6

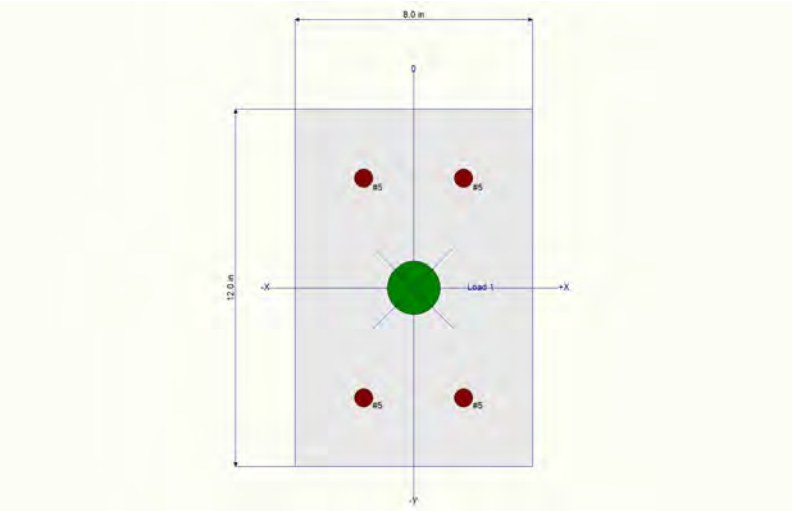
LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: TC-1 (8"x12")

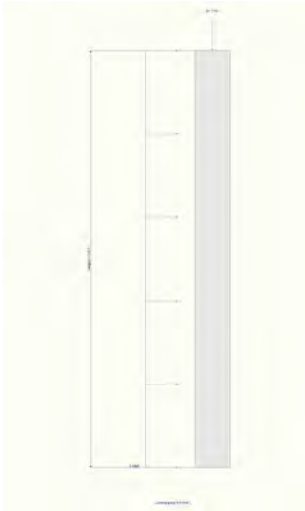
Maximum Deflections for Load Combinations

Load Combination	Max. X-X Deflection		Distance		Max. Y-Y Deflection		Distance	
+D+0.60W	0.0000	in	0.000	ft	0.026	in	6.040	ft
+D+0.750Lr+0.750L+0.450W	0.0000	in	0.000	ft	0.020	in	6.040	ft
+D+0.750L+0.450W	0.0000	in	0.000	ft	0.020	in	6.040	ft
+0.60D+0.60W	0.0000	in	0.000	ft	0.026	in	6.040	ft
+0.60D	0.0000	in	0.000	ft	0.000	in	0.000	ft
Lr Only	0.0000	in	0.000	ft	0.000	in	0.000	ft
L Only	0.0000	in	0.000	ft	0.000	in	0.000	ft
W Only	0.0000	in	0.000	ft	0.044	in	6.040	ft

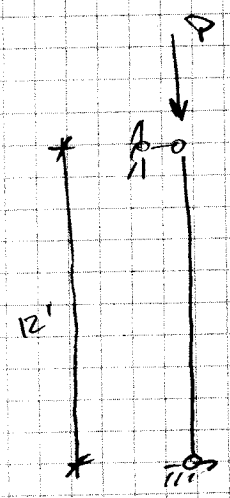
Sketches







→ TC-3 (8"x24")



P (Reacc ZB-4)

D	L	L
158.96	11.28	24.36

→ C-1 (ϕ16") = 2.01 m² → 8 #5.

* See calculation report

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Column

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: TC-3 (8"x24")

Code References

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : ASCE 7-16

General Information

f'c : Concrete 28 day streng	=	5.0 ksi
E =	=	4,030.51 ksi
Density	=	150.0 pcf
β	=	0.80
fy - Main Rebar	=	60.0 ksi
E - Main Rebar	=	29,000.0 ksi
Allow. Reinforcing Limits		*ASTM A615 Bars Used*
Min. Reinf.	=	1.0 %
Max. Reinf.	=	8.0 %

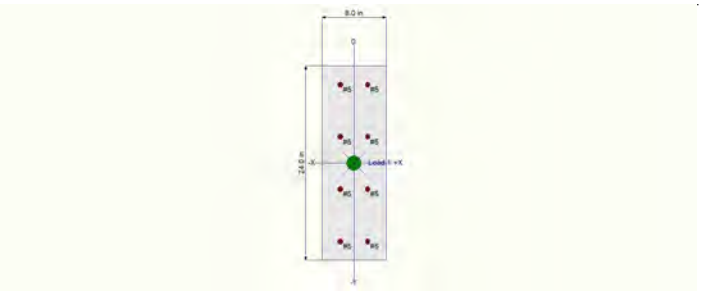
Overall Column Height = 12.0 ft
End Fixity Top & Bottom Pinned

Brace condition for deflection (buckling) along colum
X-X (width) axis :
Fully braced against buckling ABOUT Y-Y Axis
Y-Y (depth) axis :
Unbraced Length for buckling ABOUT X-X Axis = 12.0 ft, K = 1.0

Column Cross Section

Column Dimensions : 24.0in high x 8.0in Wide, Column Edge to Rebar Edge Cover = 2.0in

Column Reinforcing : 4 - #5 bars @ corners,, 2 - #5 bars left & right between corner bars



Applied Loads

Entered loads are factored per load combinations specified by user.

Column self weight included : 2,400.0 lbs * Dead Load Factor
AXIAL LOADS . . .
2B-4: Axial Load at 12.0 ft above base, D = 158.460, LR = 11.280, L = 24.360 k

DESIGN SUMMARY

Load Combination +1.20D+0.50Lr+1.60L
Location of max.above base 11.919 ft

Maximum Stress Ratio **0.479 : 1**
Ratio = (Pu^2+Mu^2)^.5 / (PhiPn^2+PhiMn^2)^.5

Pu =	237.648 k	φ * Pn =	496.215 k
Mu-x =	0.0 k-ft	φ * Mn-x =	0.0 k-ft
Mu-y =	0.0 k-ft	φ * Mn-y =	0.0 k-ft
Mu Angle =	0.0 deg		
Mu at Angle =	0.0 k-ft	φMn at Angle =	0.0 k-ft

Pn & Mn values located at Pu-Mu vector intersection with capacity curve

Column Capacities . .

Pnmax : Nominal Max. Compressive Axial Capacit	954.26 k
Pnmin : Nominal Min. Tension Axial Capacity	k
φ Pn, max : Usable Compressive Axial Capacity	496.215 k
φ Pn, min : Usable Tension Axial Capacity	k

Maximum SERVICE Load Reactions .

Top along Y-Y	0.0 k	Bottom along Y-Y	0.0 k
Top along X-X	0.0 k	Bottom along X-X	0.0 k

Maximum SERVICE Load Deflections . .

Along Y-Y 0.0 in at 0.0 ft above base
for load combination :

Along X-X 0.0 in at 0.0 ft above base
for load combination :

General Section Informatio φ = 0.650 β = 0.80 θ = 0.80

ρ : % Reinforcing	1.292 %	Rebar % Ok
Reinforcing Area	2.480 in^2	
Concrete Area	192.0 in^2	

Governing Load Combination Results

Governing Factored Load Combination	Moment X-X	Moment Y-Y	Dist. from base ft	Axial Load k Pu	φ * Pn	δx	δx * Mux	δy	δy * Muy	Bending Analysis k-ft Alpha (deg)	δ Mu	φ Mn	Utilization Ratio
+1.40D			11.92	225.20	496.22					0.000			**0.454**
+1.20D+0.50Lr+1.60L			11.92	237.65	496.22					0.000			**0.479**
+1.20D+1.60L			11.92	232.01	496.22					0.000			**0.468**

Project Title:
Engineer:
Project ID:
Project Descr:

Concrete Column

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: TC-3 (8"x24")

Governing Load Combination Results

Governing Factored Load Combination	Moment X-X	Moment Y-Y	Dist. from base ft	Axial Load k Pu	φ * Pn	Bending Analysis k-ft δx	δx * Mux	δy	δy * Muy	Alpha (deg)	δ Mu	φ Mn	Utilization Ratio
+1.20D+1.60Lr+L			11.92	235.44	496.22					0.000			**0.474**
+1.20D+1.60Lr			11.92	211.08	496.22					0.000			**0.425**
+1.20D+L			11.92	217.39	496.22					0.000			**0.438**
+1.20D			11.92	193.03	496.22					0.000			**0.389**
+1.20D+0.50Lr+L			11.92	223.03	496.22					0.000			**0.449**
+0.90D			11.92	144.77	496.22					0.000			**0.292**

Maximum Reactions

Note: Only non-zero reactions are listed.

Load Combination	X-X Axis Reaction @ Base	@ Top	k Y-Y Axis Reaction @ Base	@ Top	Axial Reaction @ Base	My - End Moments k-ft @ Base	@ Top	Mx - End Moments @ Base	@ Top
D Only					160.860				
+D+L					185.220				
+D+Lr					172.140				
+D+0.750Lr+0.750L					187.590				
+D+0.750L					179.130				
+0.60D					96.516				
Lr Only					11.280				
L Only					24.360				

Maximum Moment Reactions

Note: Only non-zero reactions are listed.

Load Combination	Moment About X-X Axis @ Base	@ Top	Moment About Y-Y Axis @ Base	@ Top
D Only	k-ft		k-ft	
+D+L	k-ft		k-ft	
+D+Lr	k-ft		k-ft	
+D+0.750Lr+0.750L	k-ft		k-ft	
+D+0.750L	k-ft		k-ft	
+0.60D	k-ft		k-ft	
Lr Only	k-ft		k-ft	
L Only	k-ft		k-ft	

Maximum Deflections for Load Combinations

Load Combination	Max. X-X Deflection	Distance	Max. Y-Y Deflection	Distance
D Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+L	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+Lr	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+0.750Lr+0.750L	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+0.750L	0.0000 in	0.000 ft	0.000 in	0.000 ft
+0.60D	0.0000 in	0.000 ft	0.000 in	0.000 ft
Lr Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
L Only	0.0000 in	0.000 ft	0.000 in	0.000 ft

Project Title:
Engineer:
Project ID:
Project Descr:

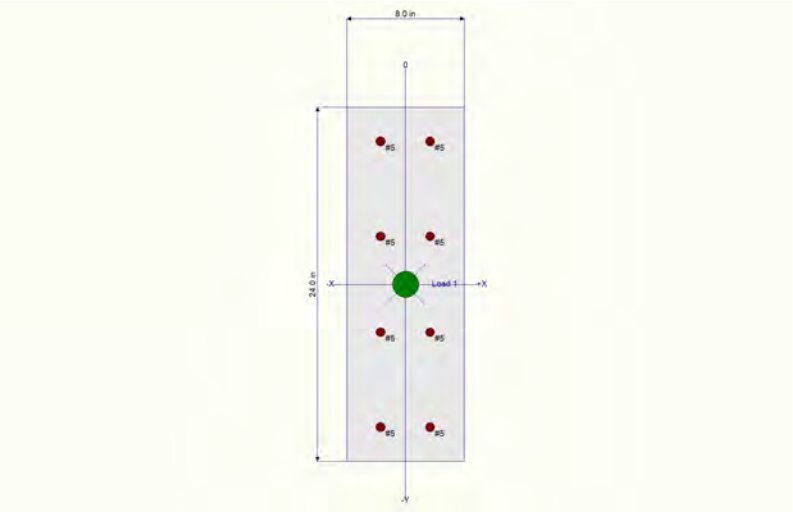
Concrete Column

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: TC-3 (8"x24")

Sketches







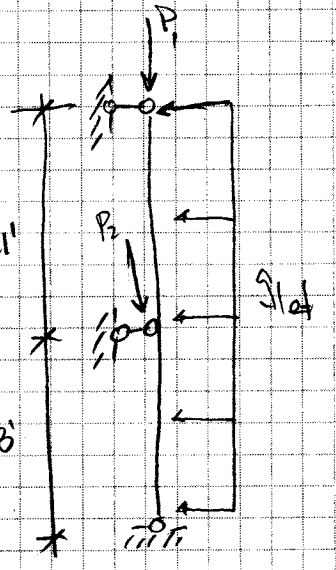
MASONRY WALL DESIGN

(TMS 402-16/ TMS 402-16)

Ph: (305) 710-2987 Email: 3aengdrafting@gmail.com

Masonry Wall Design:

→ 8" Masonry Wall (Shear Bearing).

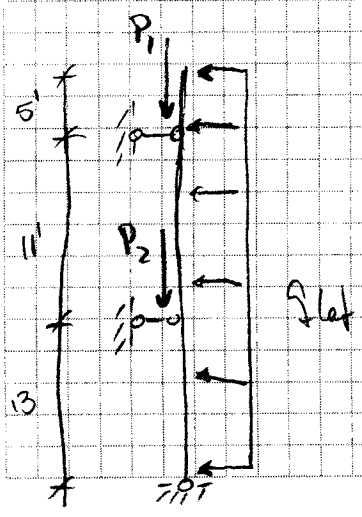


$$P_1 \begin{cases} DL_{Roof} = 2821 \text{ pf} \\ LL_{Roof} = 540 \text{ pf} \end{cases}$$

$$P_2 \begin{cases} DL_{slab} = 2925 \text{ pf} \\ LL_{slab} = 870 \text{ pf} \end{cases}$$

$$q_{lat} = 43.66 \text{ psf} / 0.6 = 73 \text{ psf}$$

→ 8" Masonry Wall (Wind Lateral, Slender)



$$P_1 \begin{cases} DL_{Roof+SDL} = 150 \text{ psf} \times 4' = 600 \text{ pf} \\ LL_{Roof} = 30 \text{ psf} \times 4' = 120 \text{ pf} \end{cases}$$

$$P_2 \begin{cases} DL_{WB} = 150 \text{ psf} \times 0.67' \times 3' = 301 \text{ pf} \end{cases}$$

$$q_{lat} = 73 \text{ psf}$$

* See calculation report

Project Title:
Engineer:
Project ID:
Project Descr:

Masonry Slender Wall

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 8" masonry wall (more bearing)

Code References

Calculations per TMS 402-16, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : ASCE 7-16

General Information

Calculations per TMS 402-16, IBC 2018, CBC 2019, ASCE 7-16

Construction Type : Grouted Hollow Concrete Masonry

F'm	=	1.50 ksi	Nom. Wall Thickness	8 in	Temp Diff across thickness	=	deg F
Fy - Yield	=	60.0 ksi	Actual Thickness	7.625 in	Min Allow Out-of-plane Defl Ra	=	0.0
Fr - Rupture	=	61.0 psi	Rebar "d" distance	3.8125 in			
Em = f'm *	=	900.0	Lower Level Rebar . . .		Minimum Vertical Steel %	=	0.0020
Max % of ρ bal.	=	0.006848	Bar Size #	5			
Grout Density	=	140 pcf	Bar Spacing	32 in			
Block Weight		Normal Weight	Upper Level Rebar . . .				
Wall Weight	=	51.0 psf	Bar Size #	5			
			Bar Spacing	32 in			

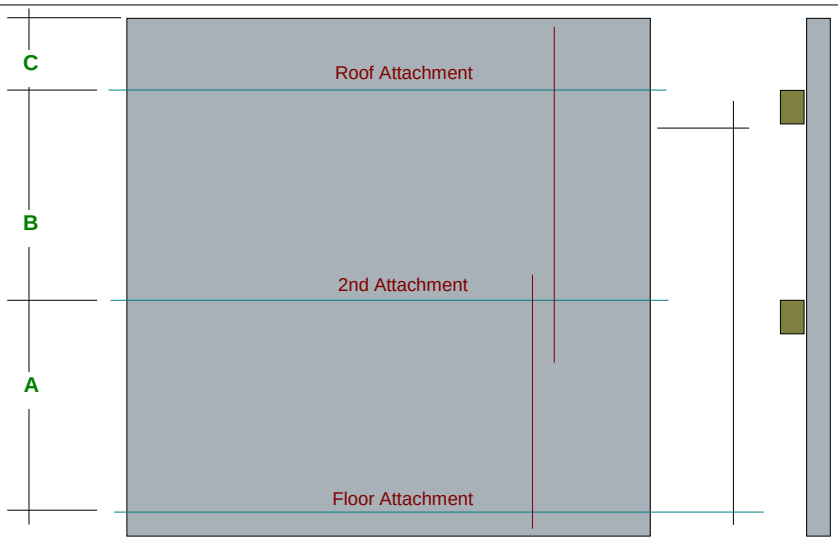
Wall is grouted at rebar cells only

Two-Story Wall Dimensions

A	1st Story Height	=	13.0 ft
B	2nd Story Height	=	11.0 ft
C	Parapet height	=	ft

Wall Support Condition: Top & Bottom Pinned

Initial Lateral Disp. @ Top Support: in



Vertical Loads

Vertical Uniform Loads . . . (Applied per foot of Strip Width)		DL : Dead	Lr : Roof Live	Lf : Floor Live	S : Snow	W : Wind
Ledger Load	Eccentricity 6.750 in					k/ft
Concentric Load		2.821	0.540			k/ft

Mid-Height Vertical Uniform Loads . . . (Applied per foot of Strip Width)		DL : Dead	Lr : Roof Live	Lf : Floor Live	S : Snow	W : Wind
Ledger Load	Eccentricity 6.0 in					k/ft
Concentric Load		2.925		0.87		k/ft

Lateral Loads

Wind Loads :
Full area WIND load 73.0 psf

Seismic Loads :
Wall Weight Seismic Load Input Method : Direct entry of Lateral Wall Weight
Seismic Wall Lateral Load psf

Fp 1.0 = 0.0 psf

Project Title:
Engineer:
Project ID:
Project Descr:

Masonry Slender Wall

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 8" masonry wall (more bearing)

DESIGN SUMMARY

Results reported for "Strip Width" of 12.0 in

	Governing Load Combination . . .	Actual Values . . .		Allowable Values . . .	
PASS	Moment Capacity Check	Maximum Bending Stress Rat	**0.5105**		
	+0.90D+W	Max Mu	-1.252 k-ft	Phi * Mn	2.453 k-ft
PASS	Service Deflection Check	Actual Defl. Ratio L/	334	Allowable Defl. Ratio	150.0
	W Only	Max. Deflection	0.4672 in		
PASS	Axial Load Check	Max Pu / Ag	73.631 psi	Max. Allow. Defl.	0.880 in
	+1.20D+0.50Lr+L+W	Location	13.183 ft	0.2 * f'm	300.0 psi
PASS	Reinforcing Limit Check				
		Actual As/bd	0.002541	Max Allow As/bd	0.006849

Maximum Reactions *for Load Combination...*

Top Horizontal	W Only	0.2892 k
Base Horizontal	W Only	0.3795 k
Mid-Ht Horizontal	W Only	1.083 k
Vertical Reaction	+D+0.750Lr+0.750L+0.450	8.028 k

Design Maximum Combinations - Moments

Results reported for "Strip Width" = 12 in.

Load Combination	Axial Load Pu k	0.2*f'm*b*t k	Moment Values Mcr k-ft	Mu k-ft	Phi	Phi Mn k-ft	As in^2	As Ratio	0.6 * rho bal	Bar 'd'
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000	0.0000	0.00
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000	0.0000	0.00
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000	0.0000	0.00
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000	0.0000	0.00
+1.20D+1.60Lr+0.50W at 13.00 to 13.	4.922	17.640	0.46	0.59	0.90	2.90	0.116	0.0025	0.0058	0.00
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000	0.0000	0.00
+1.20D+0.50W at 13.00 to 13.37	4.058	17.640	0.46	0.58	0.90	2.69	0.116	0.0025	0.0062	0.00
+1.20D+0.50Lr+L+W at 13.00 to 13.3	4.330	17.640	0.46	1.29	0.90	2.76	0.116	0.0025	0.0057	0.00
+1.20D+L+W at 13.00 to 13.37	4.060	17.640	0.46	1.28	0.90	2.69	0.116	0.0025	0.0058	0.00
+0.90D+W at 13.00 to 13.37	3.045	17.640	0.46	1.25	0.90	2.45	0.116	0.0025	0.0068	0.00
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000	0.0000	0.00
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000	0.0000	0.00

Design Maximum Combinations - Deflections

Results reported for "Strip Width" = 12 in.

Load Combination	Axial Load Pu k	Moment Values Mcr k-ft	Mactual k-ft	Stiffness I gross in^4	I cracked in^4	I effective in^4	Deflections Deflection in	Defl. Ratio
	0.000	0.00	0.00	0.00	0.00	0.000	0.000	0.0
	0.000	0.00	0.00	0.00	0.00	0.000	0.000	0.0
	0.000	0.00	0.00	0.00	0.00	0.000	0.000	0.0
	0.000	0.00	0.00	0.00	0.00	0.000	0.000	0.0
	0.000	0.00	0.00	0.00	0.00	0.000	0.000	0.0
+D+0.60W at 5.20 to 5.63	6.705	0.46	0.64	342.40	39.48	44.659	0.106	1,475.6
+D+0.750Lr+0.750L+0.450W at 19.23 to	3.469	0.46	0.26	342.40	41.42	342.400	0.054	2,442.7
+D+0.750L+0.450W at 19.23 to 19.60	3.064	0.46	0.26	342.40	40.60	342.400	0.056	2,372.9
+0.60D+0.60W at 5.20 to 5.63	4.023	0.46	0.63	342.40	33.64	38.700	0.110	1,412.2
	0.000	0.00	0.00	0.00	0.00	0.000	0.000	0.0
	0.000	0.00	0.00	0.00	0.00	0.000	0.000	0.0
	0.000	0.00	0.00	0.00	0.00	0.000	0.000	0.0
	0.000	0.00	0.00	0.00	0.00	0.000	0.000	0.0
	0.000	0.00	0.00	0.00	0.00	0.000	0.000	0.0
W Only at 5.63 to 6.07	0.000	0.46	0.97	342.40	23.68	24.342	0.467	333.9

Project Title:
Engineer:
Project ID:
Project Descr:

Masonry Slender Wall

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 8" masonry wall (more bearing)

0.000	0.00	0.00	0.00	0.00	0.000	0.000	0.0

Reactions - Vertical & Horizontal

Load Combination	Base Horizontal	Mid Horizontal	Top Horizontal	Vertical @ Wall Base
D Only	0.0 k	0.000 k	0.00 k	6.970 k

Reactions - Vertical & Horizontal

Load Combination	Base Horizontal	Mid Horizontal	Top Horizontal	Vertical @ Wall Base
+D+L	0.0 k	0.000 k	0.00 k	7.840 k
+D+Lr	0.0 k	0.000 k	0.00 k	7.510 k
+D+0.750Lr+0.750L	0.0 k	0.000 k	0.00 k	8.027 k
+D+0.750L	0.0 k	0.000 k	0.00 k	7.622 k
+D+0.60W	0.2 k	0.653 k	0.17 k	6.970 k
+D+0.750Lr+0.750L+0.450W	0.2 k	0.492 k	0.13 k	8.028 k
+D+0.750L+0.450W	0.2 k	0.491 k	0.13 k	7.623 k
+0.60D+0.60W	0.2 k	0.650 k	0.17 k	4.182 k
+D+0.70E	0.0 k	0.000 k	0.00 k	6.970 k
+D+0.750L+0.5250E	0.0 k	0.000 k	0.00 k	7.622 k
+0.60D+0.70E	0.0 k	0.000 k	0.00 k	4.182 k
Lr Only	0.0 k	0.000 k	0.00 k	0.540 k
L Only	0.0 k	0.000 k	0.00 k	0.870 k
W Only	0.4 k	1.083 k	0.29 k	0.000 k
E Only	0.0 k	0.000 k	0.00 k	0.000 k

Project Title:
Engineer:
Project ID:
Project Descr:

Masonry Slender Wall

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 8" masonry wall (wind lateral, slender)

Code References

Calculations per TMS 402-16, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : ASCE 7-16

General Information

Calculations per TMS 402-16, IBC 2018, CBC 2019, ASCE 7-16

Construction Type : Grouted Hollow Concrete Masonry

F'm	=	1.50 ksi	Nom. Wall Thickness	8 in	Temp Diff across thickness	=	deg F
Fy - Yield	=	60.0 ksi	Actual Thickness	7.625 in	Min Allow Out-of-plane Defl Ra	=	0.0
Fr - Rupture	=	61.0 psi	Rebar "d" distance	3.8125 in			
Em = f'm *	=	900.0	Lower Level Rebar . . .		Minimum Vertical Steel %	=	0.0020
Max % of ρ bal.	=	0.008418	Bar Size #	5			
Grout Density	=	140 pcf	Bar Spacing	32 in			
Block Weight		Normal Weight	Upper Level Rebar . . .				
Wall Weight	=	51.0 psf	Bar Size #	5			
			Bar Spacing	32 in			

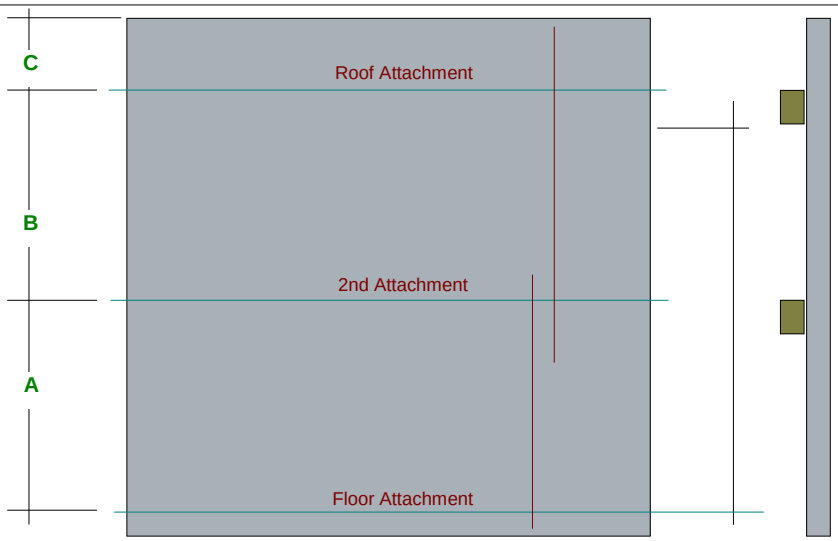
Wall is grouted at rebar cells only

Two-Story Wall Dimensions

A	1st Story Height	=	13.0 ft
B	2nd Story Height	=	11.0 ft
C	Parapet height	=	5.0 ft

Wall Support Condition Top & Bottom Pinned

Initial Lateral Disp. @ Top Support in



Vertical Loads

Vertical Uniform Loads . . .	(Applied per foot of Strip Width)	DL : Dead	Lr : Roof Live	Lf : Floor Live	S : Snow	W : Wind
Ledger Load	Eccentricity 6.750 in					k/ft
Concentric Load		0.6	0.120			k/ft

Mid-Height Vertical Uniform Loads . . .	(Applied per foot of Strip Width)	DL : Dead	Lr : Roof Live	Lf : Floor Live	S : Snow	W : Wind
Ledger Load	Eccentricity 6.0 in					k/ft
Concentric Load		0.1010				k/ft

Lateral Loads

Wind Loads :
Full area WIND load 73.0 psf

Seismic Loads :
Wall Weight Seismic Load Input Method : Direct entry of Lateral Wall Weight
Seismic Wall Lateral Load psf

Fp 1.0 = 0.0 psf

Project Title:
Engineer:
Project ID:
Project Descr:

Masonry Slender Wall

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: 8" masonry wall (wind lateral, slender)

DESIGN SUMMARY

Results reported for "Strip Width" of 12.0 in

Governing Load Combination . . .		Actual Values . . .		Allowable Values . . .	
PASS Moment Capacity Check +0.90D+W	Maximum Bending Stress Rat	**0.5163**			
	Max Mu	-1.043 k-ft	Phi * Mn	2.020 k-ft	
PASS Service Deflection Check W Only	Actual Defl. Ratio L/	283	Allowable Defl. Ratio	150.0	
	Max. Deflection	0.5513 in			
PASS Axial Load Check +1.20D+0.50Lr+W	Max Pu / Ag	40.098 psi	Max. Allow. Defl.	0.880 in	
	Location	5.417 ft	0.2 * f'm	300.0 psi	
PASS Reinforcing Limit Check	Actual As/bd	0.002541	Max Allow As/bd	0.008419	

Maximum Reactions *for Load Combination...*

Top Horizontal	W Only	0.7514 k
Base Horizontal	W Only	0.3915 k
Mid-Ht Horizontal	W Only	0.9741 k
Vertical Reaction	+D+Lr	2.30 k

Design Maximum Combinations - Moments

Results reported for "Strip Width" = 12 in.

Load Combination	Axial Load Pu k	0.2*f'm*b*t k	Moment Values Mcr k-ft	Mu k-ft	Phi	Phi Mn k-ft	As in^2	As Ratio	0.6 * rho bal	Bar 'd'
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000	0.0000	0.00
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000	0.0000	0.00
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000	0.0000	0.00
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000	0.0000	0.00
+1.20D+1.60Lr+0.50W at 5.20 to 5.63	2.490	17.640	0.46	0.56	0.90	2.32	0.116	0.0025	0.0080	0.00
+1.20D+0.50W at 5.20 to 5.63	2.298	17.640	0.46	0.56	0.90	2.27	0.116	0.0025	0.0081	0.00
+1.20D+0.50Lr+W at 5.20 to 5.63	2.358	17.640	0.46	1.11	0.90	2.29	0.116	0.0025	0.0081	0.00
+1.20D+W at 5.20 to 5.63	2.298	17.640	0.46	1.11	0.90	2.27	0.116	0.0025	0.0081	0.00
+0.90D+W at 5.20 to 5.63	1.723	17.640	0.46	1.10	0.90	2.13	0.116	0.0025	0.0083	0.00
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000	0.0000	0.00
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000	0.0000	0.00

Design Maximum Combinations - Deflections

Results reported for "Strip Width" = 12 in.

Load Combination	Axial Load Pu k	Moment Values Mcr k-ft	Mactual k-ft	Stiffness I gross in^4	I cracked in^4	I effective in^4	Deflections Deflection in	Defl. Ratio
	0.000	0.00	0.00	0.00	0.00	0.000	0.000	0.0
	0.000	0.00	0.00	0.00	0.00	0.000	0.000	0.0
	0.000	0.00	0.00	0.00	0.00	0.000	0.000	0.0
+D+0.60W at 5.63 to 6.07	1.893	0.46	0.66	342.40	28.57	31.990	0.162	960.3
+D+0.750Lr+0.450W at 5.63 to 6.07	1.983	0.46	0.52	342.40	28.80	41.650	0.058	2,677.7
+D+0.450W at 5.63 to 6.07	1.893	0.46	0.52	342.40	28.57	41.377	0.058	2,672.5
+0.60D+0.60W at 5.63 to 6.07	1.136	0.46	0.65	342.40	26.67	30.030	0.167	935.3
	0.000	0.00	0.00	0.00	0.00	0.000	0.000	0.0
	0.000	0.00	0.00	0.00	0.00	0.000	0.000	0.0
	0.000	0.00	0.00	0.00	0.00	0.000	0.000	0.0
	0.000	0.00	0.00	0.00	0.00	0.000	0.000	0.0
W Only at 5.63 to 6.07	0.000	0.46	1.04	342.40	23.68	24.199	0.551	283.0
	0.000	0.00	0.00	0.00	0.00	0.000	0.000	0.0

Reactions - Vertical & Horizontal

Load Combination	Base Horizontal	Mid Horizontal	Top Horizontal	Vertical @ Wall Base
D Only	0.0 k	0.000 k	0.00 k	2.180 k

DESCRIPTION: 8" masonry wall (wind lateral, slender)

+D+Lr	0.0 k	0.000 k	0.00 k	2.300 k
+D+0.750Lr	0.0 k	0.000 k	0.00 k	2.270 k
+D+0.60W	0.2 k	0.581 k	0.45 k	2.180 k

Reactions - Vertical & Horizontal

Load Combination	Base Horizontal	Mid Horizontal	Top Horizontal	Vertical @ Wall Base
+D+0.750Lr+0.450W	0.2 k	0.423 k	0.35 k	2.270 k
+D+0.450W	0.2 k	0.423 k	0.35 k	2.180 k
+0.60D+0.60W	0.2 k	0.579 k	0.45 k	1.308 k
+D+0.70E	0.0 k	0.000 k	0.00 k	2.180 k
+D+0.5250E	0.0 k	0.000 k	0.00 k	2.180 k
+0.60D+0.70E	0.0 k	0.000 k	0.00 k	1.308 k
Lr Only	0.0 k	0.000 k	0.00 k	0.120 k
W Only	0.4 k	0.974 k	0.75 k	0.000 k
E Only	0.0 k	0.000 k	0.00 k	0.000 k

FOOTINGS DESIGN

(ACI 318-14)

Ph: (305) 710-2987 Email: 3aengdrafting@gmail.com

Footing Design

→ KLF - 48 (klarsf)

P
- $DL_{Roof+slab} = 150\ psf \times (19' + 18') = 5250$
- $DL_{TB+wall} = 101\ plf \times 2 + 600\ psf \times 25' = 1702\ plf$ / 6952 plf
- $LL_{Roof} = 30\ psf \times 17' = 510\ plf$
- $LL_{slab} = 40\ psf \times 18' = 720\ plf$
- $kLL = 41.05\ psf / 0.6 \times 17' = 1163\ plf$

O.B. Loads: $DL_{slab\ on\ grade} = 150\ pcf \times 0.33' = 50\ psf$
$DL_{soil} = 110\ pcf \times 2' = 220\ psf$ / 270 psf
$LL_{slab} = 40\ psf$

→ KLF - 30 (klarsf)

P
- $DL_{slab} = 150\ psf \times 15.5' = 2325\ plf$
- $DL_{TB+wall} = 101\ plf + 600\ psf \times 16' = 10061\ plf$ / 3386 plf
- $LL_{slab} = 100\ psf \times 15.5' = 1550\ plf$
- $kLL = 41.05\ psf / 0.6 \times 15.5' = 1060\ plf$

Project Title:
Engineer:
Project ID:
Project Descr:

Wall Footing

Project File: NR134.ec6

DESCRIPTION: WF-30 (worst)

Code References

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : ASCE 7-16

General Information

Material Properties

f'c : Concrete 28 day strength	=	3.0 ksi
fy : Rebar Yield	=	60.0 ksi
Ec : Concrete Elastic Modulus	=	3,122.0 ksi
Concrete Density	=	145.0 pcf
φ Values Flexure	=	0.90
Shear	=	0.750

Analysis Settings

Min Steel % Bending Reinf.	=	
Min Allow % Temp Reinf.	=	0.00180
Min. Overturning Safety Factor	=	1.0 : 1
Min. Sliding Safety Factor	=	1.0 : 1
AutoCalc Footing Weight as DL :		Yes

Soil Design Values

Allowable Soil Bearing	=	2.50 ksf
Increase Bearing By Footing Weight	=	No
Soil Passive Resistance (for Sliding)	=	250.0 pcf
Soil/Concrete Friction Coeff.	=	0.30

Increases based on footing Depth

Reference Depth below Surface	=	ft
Allow. Pressure Increase per foot of depth	=	ksf
when base footing is below	=	ft

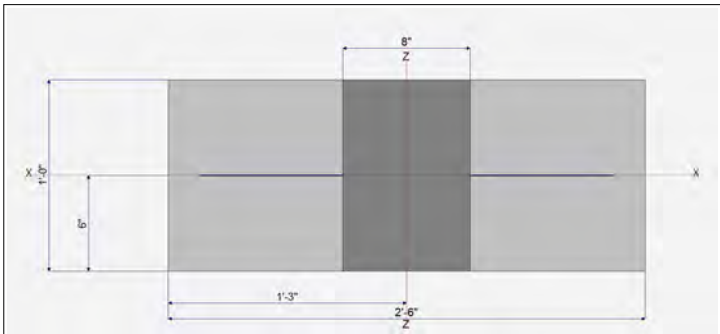
Increases based on footing Width

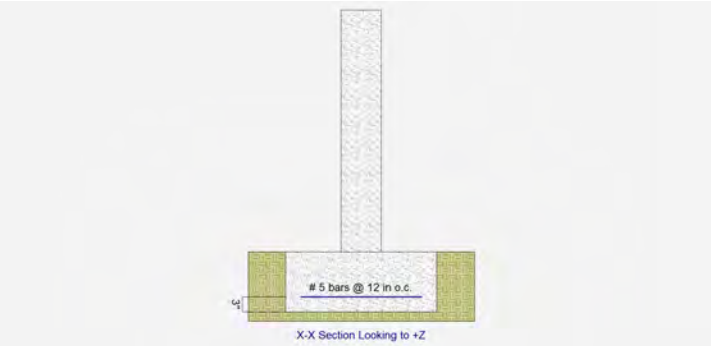
Allow. Pressure Increase per foot of width	=	ksf
when footing is wider than	=	ft
Adjusted Allowable Bearing Pressure	=	2.50 ksf

Dimensions

Footing Width	=	2.50 ft	Footing Thickness	=	12.0 in	Bars along X-X Axis	
Wall Thickness	=	8.0 in	Rebar Centerline to Edge of Concrete...			Bar spacing	= 12.00
Wall center offset from center of footing	=	0 in	at Bottom of footing	=	3.0 in	Reinforcing Bar Size	= # 5

Reinforcing





Applied Loads

		D	Lr	L	S	W	E	H	
P : Column Load	=	3.386	0.0	1.550		-1.060			k
OB : Overburden	=	0.270		0.040					ksf
V-x	=								k
M-zz	=								k-ft

Vx applied = in above top of footing

Project Title:
Engineer:
Project ID:
Project Descr:

Wall Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: WF-30 (worst)

DESIGN SUMMARY

Design OK

	Factor of Safety	Item	Applied	Capacity	Governing Load Combination
PASS	n/a	Overturning - Z-Z	0.0 k-ft	0.0 k-ft	No Overturning
PASS	n/a	Sliding - X-X	0.0 k	0.0 k	No Sliding
PASS	4.003	Uplift	-0.6360 k	2.546 k	+0.60D+0.60W

	Utilization Ratio	Item	Applied	Capacity	Governing Load Combination
PASS	0.9387	Soil Bearing	2.347 ksf	2.50 ksf	+D+L
PASS	0.09749	Z Flexure (+X)	1.183 k-ft	12.131 k-ft	+1.20D+1.60L
PASS	0.04448	Z Flexure (-X)	0.5396 k-ft	12.131 k-ft	+0.90D
PASS	0.05289	1-way Shear (+X)	4.345 psi	82.158 psi	+1.20D+1.60L
PASS	0.05289	1-way Shear (-X)	4.345 psi	82.158 psi	+1.20D+1.60L

Detailed Results

Soil Bearing

Rotation Axis & Load Combination...	Gross Allowable	Xecc	Actual Soil Bearing Stress -X	Actual Soil Bearing Stress +X	Actual / Allowable Ratio
, D Only	2.50 ksf	0.0 in	1.697 ksf	1.697 ksf	0.679
, +D+L	2.50 ksf	0.0 in	2.347 ksf	2.347 ksf	0.939
, +D+0.750L	2.50 ksf	0.0 in	2.184 ksf	2.184 ksf	0.874
, +D+0.60W	2.50 ksf	0.0 in	1.443 ksf	1.443 ksf	0.577
, +D+0.750L+0.450W	2.50 ksf	0.0 in	1.994 ksf	1.994 ksf	0.797
, +0.60D+0.60W	2.50 ksf	0.0 in	0.7640 ksf	0.7640 ksf	0.306
, +0.60D	2.50 ksf	0.0 in	1.018 ksf	1.018 ksf	0.407

Overturning Stability

Units : k-ft

Rotation Axis & Load Combination...	Overturning Moment	Resisting Moment	Stability Ratio	Status
Footing Has NO Overturning				

Sliding Stability

Force Application Axis Load Combination...	Sliding Force	Resisting Force	Sliding SafetyRatio	Status
Footing Has NO Sliding				

Footing Flexure

Flexure Axis & Load Combination	Mu k-ft	Which Side ?	Tension @ Bot. or Top ?	As Req'd in^2	Gvrn. As in^2	Actual As in^2	Phi*Mn k-ft	Status
, +1.40D	0.8393	-X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.40D	0.8393	+X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+1.60L	1.183	-X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+1.60L	1.183	+X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+L	1.009	-X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+L	1.009	+X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+0.50W	0.6304	-X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+0.50W	0.6304	+X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+L+W	0.8309	-X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+L+W	0.8309	+X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +0.90D+W	0.3615	-X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +0.90D+W	0.3615	+X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +0.90D	0.5396	-X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +0.90D	0.5396	+X	Bottom	0.2592	Min Temp %	0.31	12.131	OK

One Way Shear

Units : k

Load Combination...	Vu @ -X	Vu @ +X	Vu:Max	Phi Vn	Vu / Phi*Vn	Status
+1.40D	3.084 psi	3.084 psi	3.084 psi	82.158 psi	0.03754	OK
+1.20D+1.60L	4.345 psi	4.345 psi	4.345 psi	82.158 psi	0.05289	OK
+1.20D+L	3.707 psi	3.707 psi	3.707 psi	82.158 psi	0.04512	OK
+1.20D+0.50W	2.316 psi	2.316 psi	2.316 psi	82.158 psi	0.02819	OK
+1.20D+L+W	3.053 psi	3.053 psi	3.053 psi	82.158 psi	0.03716	OK
+0.90D+W	1.328 psi	1.328 psi	1.328 psi	82.158 psi	0.01617	OK
+0.90D	1.983 psi	1.983 psi	1.983 psi	82.158 psi	0.02413	OK

Project Title:
Engineer:
Project ID:
Project Descr:

Wall Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: WF-48 (worst)

Code References

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : ASCE 7-16

General Information

Material Properties

f'c : Concrete 28 day strength	=	3.0 ksi
fy : Rebar Yield	=	60.0 ksi
Ec : Concrete Elastic Modulus	=	3,122.0 ksi
Concrete Density	=	145.0 pcf
φ Values Flexure	=	0.90
Shear	=	0.750

Analysis Settings

Min Steel % Bending Reinf.	=	
Min Allow % Temp Reinf.	=	0.00180
Min. Overturning Safety Factor	=	1.0 : 1
Min. Sliding Safety Factor	=	1.0 : 1
AutoCalc Footing Weight as DL :		Yes

Soil Design Values

Allowable Soil Bearing	=	2.50 ksf
Increase Bearing By Footing Weight	=	No
Soil Passive Resistance (for Sliding)	=	250.0 pcf
Soil/Concrete Friction Coeff.	=	0.30

Increases based on footing Depth

Reference Depth below Surface	=	ft
Allow. Pressure Increase per foot of depth	=	ksf
when base footing is below	=	ft

Increases based on footing Width

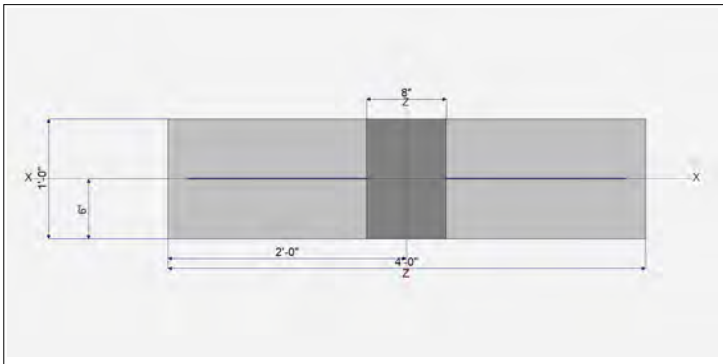
Allow. Pressure Increase per foot of width	=	ksf
when footing is wider than	=	ft

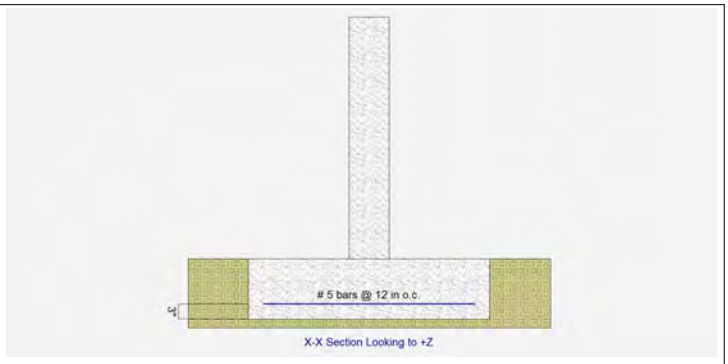
Adjusted Allowable Bearing Pressure = 2.50 ksf

Dimensions

Footing Width	=	4.0 ft	Footing Thickness	=	12.0 in	Bars along X-X Axis	
Wall Thickness	=	8.0 in	Rebar Centerline to Edge of Concrete...			Bar spacing	= 12.00
Wall center offset from center of footing	=	0 in	at Bottom of footing	=	3.0 in	Reinforcing Bar Size	= # 5

Reinforcing





Applied Loads

		D	Lr	L	S	W	E	H	
P : Column Load	=	6.952	0.510	0.720		-1.163			k
OB : Overburden	=	0.270		0.040					ksf
V-x	=								k
M-zz	=								k-ft

Vx applied = in above top of footing

Project Title:
Engineer:
Project ID:
Project Descr:

Wall Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: WF-48 (worst)

DESIGN SUMMARY

Design OK

	Factor of Safety	Item	Applied	Capacity	Governing Load Combination
PASS	n/a	Overturning - Z-Z	0.0 k-ft	0.0 k-ft	No Overturning
PASS	n/a	Sliding - X-X	0.0 k	0.0 k	No Sliding
PASS	7.250	Uplift	-0.6978 k	5.059 k	+0.60D+0.60W

	Utilization Ratio	Item	Applied	Capacity	Governing Load Combination
PASS	0.9455	Soil Bearing	2.364 ksf	2.50 ksf	+D+0.750Lr+0.750L
PASS	0.3062	Z Flexure (+X)	3.715 k-ft	12.131 k-ft	+1.20D+0.50Lr+1.60L
PASS	0.1894	Z Flexure (-X)	2.297 k-ft	12.131 k-ft	+0.90D
PASS	0.2733	1-way Shear (+X)	22.454 psi	82.158 psi	+1.20D+0.50Lr+1.60L
PASS	0.2733	1-way Shear (-X)	22.454 psi	82.158 psi	+1.20D+0.50Lr+1.60L

Detailed Results

Soil Bearing

Rotation Axis & Load Combination...	Gross Allowable	Xecc	Actual Soil Bearing Stress -X	Actual Soil Bearing Stress +X	Actual / Allowable Ratio
, D Only	2.50 ksf	0.0 in	2.108 ksf	2.108 ksf	0.843
, +D+L	2.50 ksf	0.0 in	2.321 ksf	2.321 ksf	0.929
, +D+Lr	2.50 ksf	0.0 in	2.236 ksf	2.236 ksf	0.894
, +D+0.750Lr+0.750L	2.50 ksf	0.0 in	2.364 ksf	2.364 ksf	0.946
, +D+0.750L	2.50 ksf	0.0 in	2.268 ksf	2.268 ksf	0.907
, +D+0.60W	2.50 ksf	0.0 in	1.934 ksf	1.934 ksf	0.773
, +D+0.750Lr+0.750L+0.450W	2.50 ksf	0.0 in	2.233 ksf	2.233 ksf	0.893
, +D+0.750L+0.450W	2.50 ksf	0.0 in	2.137 ksf	2.137 ksf	0.855
, +0.60D+0.60W	2.50 ksf	0.0 in	1.090 ksf	1.090 ksf	0.436
, +0.60D	2.50 ksf	0.0 in	1.265 ksf	1.265 ksf	0.506

Overturning Stability

Units : k-ft

Rotation Axis & Load Combination...	Overturning Moment	Resisting Moment	Stability Ratio	Status
Footing Has NO Overturning				

Sliding Stability

Force Application Axis Load Combination...	Sliding Force	Resisting Force	Sliding SafetyRatio	Status
Footing Has NO Sliding				

Footing Flexure

Flexure Axis & Load Combination	Mu k-ft	Which Side ?	Tension @ Bot. or Top ?	As Req'd in^2	Gvrn. As in^2	Actual As in^2	Phi*Mn k-ft	Status
, +1.40D	3.574	-X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.40D	3.574	+X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+0.50Lr+1.60L	3.715	-X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+0.50Lr+1.60L	3.715	+X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+1.60L	3.626	-X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+1.60L	3.626	+X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+1.60Lr+L	3.698	-X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+1.60Lr+L	3.698	+X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+1.60Lr+0.50W	3.145	-X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+1.60Lr+0.50W	3.145	+X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+L	3.415	-X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+L	3.415	+X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+0.50W	2.861	-X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+0.50W	2.861	+X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+0.50Lr+L+W	3.1	-X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+0.50Lr+L+W	3.1	+X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+L+W	3.011	-X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +1.20D+L+W	3.011	+X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +0.90D+W	1.894	-X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +0.90D+W	1.894	+X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +0.90D	2.297	-X	Bottom	0.2592	Min Temp %	0.31	12.131	OK
, +0.90D	2.297	+X	Bottom	0.2592	Min Temp %	0.31	12.131	OK

Project Title:
Engineer:
Project ID:
Project Descr:

Wall Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: WF-48 (worst)

One Way Shear

Units : k

Load Combination...	Vu @ -X	Vu @ +X	Vu:Max	Phi Vn	Vu / Phi*Vn	Status
+1.40D	21.602 psi	21.602 psi	21.602 psi	82.158 psi	0.2629	OK
+1.20D+0.50Lr+1.60L	22.454 psi	22.454 psi	22.454 psi	82.158 psi	0.2733	OK
+1.20D+1.60L	21.919 psi	21.919 psi	21.919 psi	82.158 psi	0.2668	OK
+1.20D+1.60Lr+L	22.355 psi	22.355 psi	22.355 psi	82.158 psi	0.2721	OK
+1.20D+1.60Lr+0.50W	19.008 psi	19.008 psi	19.008 psi	82.158 psi	0.2314	OK
+1.20D+L	20.643 psi	20.643 psi	20.643 psi	82.158 psi	0.2513	OK
+1.20D+0.50W	17.296 psi	17.296 psi	17.296 psi	82.158 psi	0.2105	OK
+1.20D+0.50Lr+L+W	18.737 psi	18.737 psi	18.737 psi	82.158 psi	0.2281	OK
+1.20D+L+W	18.202 psi	18.202 psi	18.202 psi	82.158 psi	0.2215	OK
+0.90D+W	11.446 psi	11.446 psi	11.446 psi	82.158 psi	0.1393	OK
+0.90D	13.887 psi	13.887 psi	13.887 psi	82.158 psi	0.169	OK

→ F-70 (Worst) / 16"

P(Reace 3 2B-5)

D	Lr	L
74.9	5.92	10.89
+1.41		
76.31		

D_{LTC} = 150 pcf × 1' × 0.67' × 14'
= 1407 lbs.

→ F-126 (Worst) / 24"

P(Reace 2B-13(7) + 2B-14(2))

D	Lr	L
170.57	17.73	90.64

→ F-106 (Worst) / 18"

P(Reace 2B-4)

D	Lr	L
158.46	11.28	24.36
+2.79		
161.25		

D_{LTC} = 150 pcf × 1.33' × 1' × 14'
= 2793 lbs.

* See calculation report

→ F-60 (worst)

P(Reacc 2, 2B-16)

D	Lr	L
54.32	3.24	9.54

→ F-50 (worst).

P(Reacc 2, 2B-19)

D	Lr	L
39.03 + 1.41 = 40.44	3.31	5.22

→ F-40 (worst)

P(Reacc 1, 2B-13)

D	L
18.70	10.04

* See calculation report

→ F - 240160 (Worst)

	D	Lv	L
P_1 (Reacc RB-1(2) + RB-7(1) + 2B-7(2))	187.83	19.48	11,88
P_2 (Reacc RB-1(3) + RB-3(2) + 2B-6(2) + 2B-7(3))	302.14	19.74	84.22

* See calculation report

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 | Arch-Deco Design & Construction Inc. | (c) ENERCALC INC 1983-2021

DESCRIPTION: F-40 (worst)

Code References

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : ASCE 7-16

General Information

Material Properties

f'c : Concrete 28 day strength	=	3.0	ksi
fy : Rebar Yield	=	60.0	ksi
Ec : Concrete Elastic Modulus	=	3,122.0	ksi
Concrete Density	=	145.0	pcf
φ Values Flexure	=	0.90	
Shear	=	0.750	

Analysis Settings

Min Steel % Bending Reinf.	=	
Min Allow % Temp Reinf.	=	0.00180
Min. Overturning Safety Factor	=	1.0 : 1
Min. Sliding Safety Factor	=	1.0 : 1
Add Ftg Wt for Soil Pressure	:	Yes
Use ftg wt for stability, moments & shears	:	Yes
Add Pedestal Wt for Soil Pressure	:	No
Use Pedestal wt for stability, mom & shear	:	No

Soil Design Values

Allowable Soil Bearing	=	2.50	ksf
Soil Density	=	110.0	pcf
Increase Bearing By Footing Weight	=	No	
Soil Passive Resistance (for Sliding)	=	250.0	pcf
Soil/Concrete Friction Coeff.	=	0.30	

Increases based on footing Depth

Footing base depth below soil surface	=	3.0	ft
Allow press. increase per foot of depth	=		ksf
when footing base is below	=		ft

Increases based on footing plan dimension

Allowable pressure increase per foot of depth	=		ksf
when max. length or width is greater than	=		ft

Dimensions

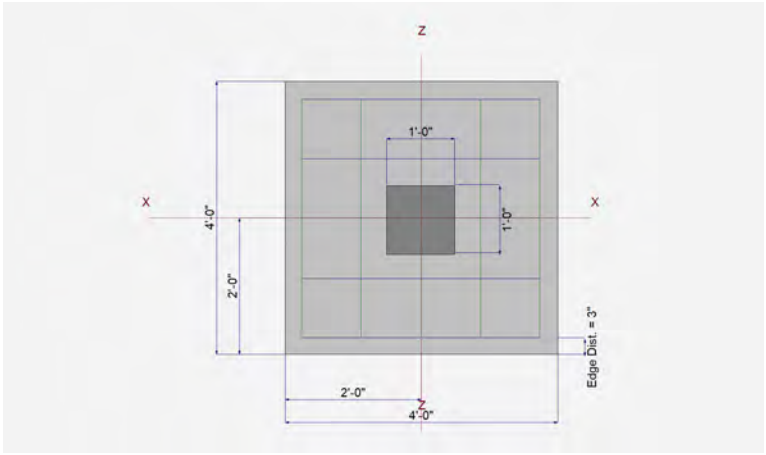
Width parallel to X-X Axis	=	4.0 ft
Length parallel to Z-Z Axis	=	4.0 ft
Footing Thickness	=	12.0 in

Pedestal dimensions...

px : parallel to X-X Axis	=	12.0 in
pz : parallel to Z-Z Axis	=	12.0 in
Height	=	24.0 in

Rebar Centerline to Edge of Concrete...

at Bottom of footing	=	3.0 in

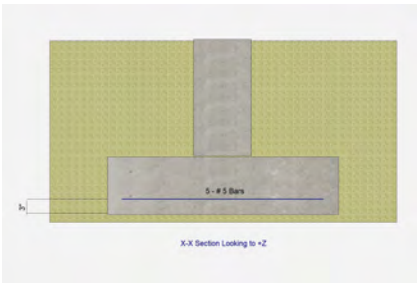


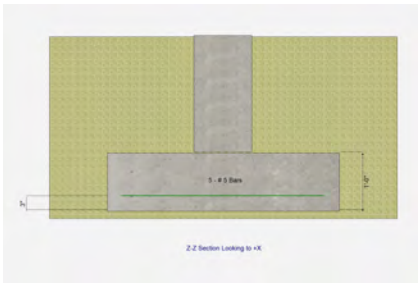
Reinforcing

Bars parallel to X-X Axis			
Number of Bars	=		5.0
Reinforcing Bar Size	=	#	5
Bars parallel to Z-Z Axis			
Number of Bars	=		5.0
Reinforcing Bar Size	=	#	5

Bandwidth Distribution Check (ACI 15.4.4.2)

Direction Requiring Closer Separation	
	n/a
# Bars required within zone	n/a
# Bars required on each side of zone	n/a





Applied Loads

		D	Lr	L	S	W	E	H	
P : Column Load	=	18.70		10.040					k
OB : Overburden	=	0.050		0.040					ksf
M-xx	=								k-ft
M-zz	=								k-ft
V-x	=								k
V-z	=								k

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: F-40 (worst)

DESIGN SUMMARY

Design OK

	Min. Ratio	Item	Applied	Capacity	Governing Load Combination
PASS	0.8928	Soil Bearing	2.232 ksf	2.50 ksf	+D+L about Z-Z axis
PASS	n/a	Overturning - X-X	0.0 k-ft	0.0 k-ft	No Overturning
PASS	n/a	Overturning - Z-Z	0.0 k-ft	0.0 k-ft	No Overturning
PASS	n/a	Sliding - X-X	0.0 k	0.0 k	No Sliding
PASS	n/a	Sliding - Z-Z	0.0 k	0.0 k	No Sliding
PASS	n/a	Uplift	0.0 k	0.0 k	No Uplift
PASS	0.1783	Z Flexure (+X)	2.680 k-ft/ft	15.031 k-ft/ft	+1.20D+1.60L
PASS	0.1783	Z Flexure (-X)	2.680 k-ft/ft	15.031 k-ft/ft	+1.20D+1.60L
PASS	0.1783	X Flexure (+Z)	2.680 k-ft/ft	15.031 k-ft/ft	+1.20D+1.60L
PASS	0.1783	X Flexure (-Z)	2.680 k-ft/ft	15.031 k-ft/ft	+1.20D+1.60L
PASS	0.2040	1-way Shear (+X)	16.764 psi	82.158 psi	+1.20D+1.60L
PASS	0.2040	1-way Shear (-X)	16.764 psi	82.158 psi	+1.20D+1.60L
PASS	0.2040	1-way Shear (+Z)	16.764 psi	82.158 psi	+1.20D+1.60L
PASS	0.2040	1-way Shear (-Z)	16.764 psi	82.158 psi	+1.20D+1.60L
PASS	0.2474	2-way Punching	40.657 psi	164.317 psi	+1.20D+1.60L

Detailed Results

Soil Bearing

Rotation Axis & Load Combination...	Gross Allowable	Xecc (in)	Zecc (in)	Actual Soil Bearing Stress @ Location Bottom, -Z	Top, +Z	Left, -X	Right, +X	Actual / Allow Ratio
X-X, D Only	2.50	n/a	0.0	1.567	1.567	n/a	n/a	0.627
X-X, +D+L	2.50	n/a	0.0	2.232	2.232	n/a	n/a	0.893
X-X, +D+0.750L	2.50	n/a	0.0	2.066	2.066	n/a	n/a	0.826
X-X, +0.60D	2.50	n/a	0.0	0.9401	0.9401	n/a	n/a	0.376
Z-Z, D Only	2.50	0.0	n/a	n/a	n/a	1.567	1.567	0.627
Z-Z, +D+L	2.50	0.0	n/a	n/a	n/a	2.232	2.232	0.893
Z-Z, +D+0.750L	2.50	0.0	n/a	n/a	n/a	2.066	2.066	0.826
Z-Z, +0.60D	2.50	0.0	n/a	n/a	n/a	0.9401	0.9401	0.376

Overturning Stability

Rotation Axis & Load Combination...	Overturning Moment	Resisting Moment	Stability Ratio	Status
Footing Has NO Overturning				

Sliding Stability

All units k

Force Application Axis Load Combination...	Sliding Force	Resisting Force	Stability Ratio	Status
Footing Has NO Sliding				

Footing Flexure

Flexure Axis & Load Combination	Mu k-ft	Side	Tension Surface	As Req'd in^2	Gvrn. As in^2	Actual As in^2	Phi*Mn k-ft	Status
X-X, +1.40D	1.814	+Z	Bottom	0.2592	AsMin	0.3875	15.031	OK
X-X, +1.40D	1.814	-Z	Bottom	0.2592	AsMin	0.3875	15.031	OK
X-X, +1.20D+1.60L	2.680	+Z	Bottom	0.2592	AsMin	0.3875	15.031	OK
X-X, +1.20D+1.60L	2.680	-Z	Bottom	0.2592	AsMin	0.3875	15.031	OK
X-X, +1.20D+L	2.258	+Z	Bottom	0.2592	AsMin	0.3875	15.031	OK
X-X, +1.20D+L	2.258	-Z	Bottom	0.2592	AsMin	0.3875	15.031	OK
X-X, +1.20D	1.555	+Z	Bottom	0.2592	AsMin	0.3875	15.031	OK
X-X, +1.20D	1.555	-Z	Bottom	0.2592	AsMin	0.3875	15.031	OK
X-X, +0.90D	1.166	+Z	Bottom	0.2592	AsMin	0.3875	15.031	OK
X-X, +0.90D	1.166	-Z	Bottom	0.2592	AsMin	0.3875	15.031	OK
Z-Z, +1.40D	1.814	-X	Bottom	0.2592	AsMin	0.3875	15.031	OK
Z-Z, +1.40D	1.814	+X	Bottom	0.2592	AsMin	0.3875	15.031	OK
Z-Z, +1.20D+1.60L	2.680	-X	Bottom	0.2592	AsMin	0.3875	15.031	OK
Z-Z, +1.20D+1.60L	2.680	+X	Bottom	0.2592	AsMin	0.3875	15.031	OK
Z-Z, +1.20D+L	2.258	-X	Bottom	0.2592	AsMin	0.3875	15.031	OK
Z-Z, +1.20D+L	2.258	+X	Bottom	0.2592	AsMin	0.3875	15.031	OK
Z-Z, +1.20D	1.555	-X	Bottom	0.2592	AsMin	0.3875	15.031	OK

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: F-40 (worst)

Footing Flexure

Flexure Axis & Load Combination	Mu k-ft	Side	Tension Surface	As Req'd in^2	Gvrn. As in^2	Actual As in^2	Phi*Mn k-ft	Status
Z-Z, +1.20D	1.555	+X	Bottom	0.2592	AsMin	0.3875	15.031	OK
Z-Z, +0.90D	1.166	-X	Bottom	0.2592	AsMin	0.3875	15.031	OK
Z-Z, +0.90D	1.166	+X	Bottom	0.2592	AsMin	0.3875	15.031	OK

One Way Shear

Load Combination...	Vu @ -X	Vu @ +X	Vu @ -Z	Vu @ +Z	Vu:Max	Phi Vn	Vu / Phi*Vn	Status
+1.40D	11.35 psi	11.35 psi	11.35 psi	11.35 psi	11.35 psi	82.16 psi	0.14	OK
+1.20D+1.60L	16.76 psi	16.76 psi	16.76 psi	16.76 psi	16.76 psi	82.16 psi	0.20	OK
+1.20D+L	14.13 psi	14.13 psi	14.13 psi	14.13 psi	14.13 psi	82.16 psi	0.17	OK
+1.20D	9.73 psi	9.73 psi	9.73 psi	9.73 psi	9.73 psi	82.16 psi	0.12	OK
+0.90D	7.30 psi	7.30 psi	7.30 psi	7.30 psi	7.30 psi	82.16 psi	0.09	OK

Two-Way "Punching" Shear

All units k

Load Combination...	Vu	Phi*Vn	Vu / Phi*Vn	Status
+1.40D	27.52 psi	164.32 psi	0.1675	OK
+1.20D+1.60L	40.66 psi	164.32 psi	0.2474	OK
+1.20D+L	34.26 psi	164.32 psi	0.2085	OK
+1.20D	23.59 psi	164.32 psi	0.1436	OK
+0.90D	17.69 psi	164.32 psi	0.1077	OK

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 | Arch-Deco Design & Construction Inc. | (c) ENERCALC INC 1983-2021

DESCRIPTION: F-50 (worst)

Code References

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : ASCE 7-16

General Information

Material Properties

f'c : Concrete 28 day strength	=	3.0	ksi
fy : Rebar Yield	=	60.0	ksi
Ec : Concrete Elastic Modulus	=	3,122.0	ksi
Concrete Density	=	145.0	pcf
φ Values Flexure	=	0.90	
Shear	=	0.750	

Analysis Settings

Min Steel % Bending Reinf.	=	
Min Allow % Temp Reinf.	=	0.00180
Min. Overturning Safety Factor	=	1.0 : 1
Min. Sliding Safety Factor	=	1.0 : 1
Add Ftg Wt for Soil Pressure	:	Yes
Use ftg wt for stability, moments & shears	:	Yes
Add Pedestal Wt for Soil Pressure	:	No
Use Pedestal wt for stability, mom & shear	:	No

Soil Design Values

Allowable Soil Bearing	=	2.50	ksf
Soil Density	=	110.0	pcf
Increase Bearing By Footing Weight	=	No	
Soil Passive Resistance (for Sliding)	=	250.0	pcf
Soil/Concrete Friction Coeff.	=	0.30	

Increases based on footing Depth

Footing base depth below soil surface	=	3.0	ft
Allow press. increase per foot of depth	=		ksf
when footing base is below	=		ft

Increases based on footing plan dimension

Allowable pressure increase per foot of depth	=		ksf
when max. length or width is greater than	=		ft

Dimensions

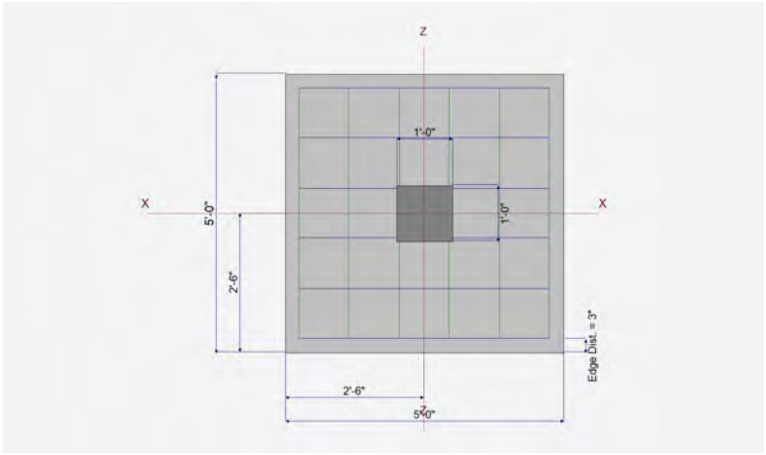
Width parallel to X-X Axis	=	5.0 ft
Length parallel to Z-Z Axis	=	5.0 ft
Footing Thickness	=	12.0 in

Pedestal dimensions...

px : parallel to X-X Axis	=	12.0 in
pz : parallel to Z-Z Axis	=	12.0 in
Height	=	24.0 in

Rebar Centerline to Edge of Concrete...

at Bottom of footing	=	3.0 in

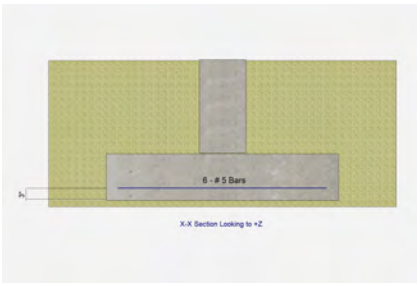


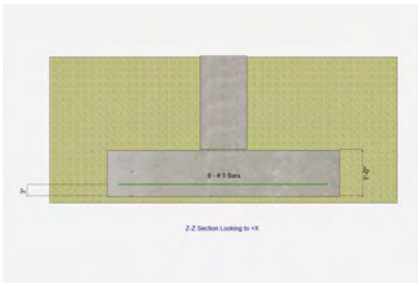
Reinforcing

Bars parallel to X-X Axis			
Number of Bars	=		6.0
Reinforcing Bar Size	=	#	5
Bars parallel to Z-Z Axis			
Number of Bars	=		6.0
Reinforcing Bar Size	=	#	5

Bandwidth Distribution Check (ACI 15.4.4.2)

Direction Requiring Closer Separation	
	n/a
# Bars required within zone	n/a
# Bars required on each side of zone	n/a





Applied Loads

		D	Lr	L	S	W	E	H	
P : Column Load	=	40.440	3.310	5.220					k
OB : Overburden	=	0.050		0.040					ksf
M-xx	=								k-ft
M-zz	=								k-ft
V-x	=								k
V-z	=								k

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: F-50 (worst)

DESIGN SUMMARY

Design OK

	Min. Ratio	Item	Applied	Capacity	Governing Load Combination
PASS	0.9228	Soil Bearing	2.307 ksf	2.50 ksf	+D+0.750Lr+0.750L about Z-Z axis
PASS	n/a	Overturning - X-X	0.0 k-ft	0.0 k-ft	No Overturning
PASS	n/a	Overturning - Z-Z	0.0 k-ft	0.0 k-ft	No Overturning
PASS	n/a	Sliding - X-X	0.0 k	0.0 k	No Sliding
PASS	n/a	Sliding - Z-Z	0.0 k	0.0 k	No Sliding
PASS	n/a	Uplift	0.0 k	0.0 k	No Uplift
PASS	0.3247	Z Flexure (+X)	4.694 k-ft/ft	14.455 k-ft/ft	+1.20D+1.60Lr+L
PASS	0.3247	Z Flexure (-X)	4.694 k-ft/ft	14.455 k-ft/ft	+1.20D+1.60Lr+L
PASS	0.3247	X Flexure (+Z)	4.694 k-ft/ft	14.455 k-ft/ft	+1.20D+1.60Lr+L
PASS	0.3247	X Flexure (-Z)	4.694 k-ft/ft	14.455 k-ft/ft	+1.20D+1.60Lr+L
PASS	0.3307	1-way Shear (+X)	27.167 psi	82.158 psi	+1.20D+1.60Lr+L
PASS	0.3307	1-way Shear (-X)	27.167 psi	82.158 psi	+1.20D+1.60Lr+L
PASS	0.3307	1-way Shear (+Z)	27.167 psi	82.158 psi	+1.20D+1.60Lr+L
PASS	0.3307	1-way Shear (-Z)	27.167 psi	82.158 psi	+1.20D+1.60Lr+L
PASS	0.4145	2-way Punching	68.111 psi	164.317 psi	+1.20D+1.60Lr+L

Detailed Results

Soil Bearing

Rotation Axis & Load Combination...	Gross Allowable	Xecc (in)	Zecc (in)	Actual Soil Bearing Stress @ Location Bottom, -Z	Top, +Z	Left, -X	Right, +X	Actual / Allow Ratio
X-X, D Only	2.50	n/a	0.0	2.022	2.022	n/a	n/a	0.809
X-X, +D+L	2.50	n/a	0.0	2.269	2.269	n/a	n/a	0.908
X-X, +D+Lr	2.50	n/a	0.0	2.154	2.154	n/a	n/a	0.862
X-X, +D+0.750Lr+0.750L	2.50	n/a	0.0	2.307	2.307	n/a	n/a	0.923
X-X, +D+0.750L	2.50	n/a	0.0	2.207	2.207	n/a	n/a	0.883
X-X, +0.60D	2.50	n/a	0.0	1.213	1.213	n/a	n/a	0.485
Z-Z, D Only	2.50	0.0	n/a	n/a	n/a	2.022	2.022	0.809
Z-Z, +D+L	2.50	0.0	n/a	n/a	n/a	2.269	2.269	0.908
Z-Z, +D+Lr	2.50	0.0	n/a	n/a	n/a	2.154	2.154	0.862
Z-Z, +D+0.750Lr+0.750L	2.50	0.0	n/a	n/a	n/a	2.307	2.307	0.923
Z-Z, +D+0.750L	2.50	0.0	n/a	n/a	n/a	2.207	2.207	0.883
Z-Z, +0.60D	2.50	0.0	n/a	n/a	n/a	1.213	1.213	0.485

Overturning Stability

Rotation Axis & Load Combination...	Overturning Moment	Resisting Moment	Stability Ratio	Status
Footing Has NO Overturning				

All units k

Sliding Stability

Force Application Axis Load Combination...	Sliding Force	Resisting Force	Stability Ratio	Status
Footing Has NO Sliding				

Footing Flexure

Flexure Axis & Load Combination	Mu k-ft	Side	Tension Surface	As Req'd in^2	Gvrn. As in^2	Actual As in^2	Phi*Mn k-ft	Status
X-X, +1.40D	4.499	+Z	Bottom	0.2592	AsMin	0.3720	14.455	OK
X-X, +1.40D	4.499	-Z	Bottom	0.2592	AsMin	0.3720	14.455	OK
X-X, +1.20D+0.50Lr+1.60L	4.652	+Z	Bottom	0.2592	AsMin	0.3720	14.455	OK
X-X, +1.20D+0.50Lr+1.60L	4.652	-Z	Bottom	0.2592	AsMin	0.3720	14.455	OK
X-X, +1.20D+1.60L	4.519	+Z	Bottom	0.2592	AsMin	0.3720	14.455	OK
X-X, +1.20D+1.60L	4.519	-Z	Bottom	0.2592	AsMin	0.3720	14.455	OK
X-X, +1.20D+1.60Lr+L	4.694	+Z	Bottom	0.2592	AsMin	0.3720	14.455	OK
X-X, +1.20D+1.60Lr+L	4.694	-Z	Bottom	0.2592	AsMin	0.3720	14.455	OK
X-X, +1.20D+1.60Lr	4.280	+Z	Bottom	0.2592	AsMin	0.3720	14.455	OK
X-X, +1.20D+1.60Lr	4.280	-Z	Bottom	0.2592	AsMin	0.3720	14.455	OK
X-X, +1.20D+L	4.271	+Z	Bottom	0.2592	AsMin	0.3720	14.455	OK
X-X, +1.20D+L	4.271	-Z	Bottom	0.2592	AsMin	0.3720	14.455	OK
X-X, +1.20D	3.856	+Z	Bottom	0.2592	AsMin	0.3720	14.455	OK

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: F-50 (worst)

Footing Flexure

Flexure Axis & Load Combination	Mu k-ft	Side	Tension Surface	As Req'd in^2	Gvrn. As in^2	Actual As in^2	Phi*Mn k-ft	Status
X-X, +1.20D	3.856	-Z	Bottom	0.2592	AsMin	0.3720	14.455	OK
X-X, +1.20D+0.50Lr+L	4.403	+Z	Bottom	0.2592	AsMin	0.3720	14.455	OK
X-X, +1.20D+0.50Lr+L	4.403	-Z	Bottom	0.2592	AsMin	0.3720	14.455	OK
X-X, +0.90D	2.892	+Z	Bottom	0.2592	AsMin	0.3720	14.455	OK
X-X, +0.90D	2.892	-Z	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +1.40D	4.499	-X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +1.40D	4.499	+X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +1.20D+0.50Lr+1.60L	4.652	-X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +1.20D+0.50Lr+1.60L	4.652	+X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +1.20D+1.60L	4.519	-X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +1.20D+1.60L	4.519	+X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +1.20D+1.60Lr+L	4.694	-X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +1.20D+1.60Lr+L	4.694	+X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +1.20D+1.60Lr	4.280	-X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +1.20D+1.60Lr	4.280	+X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +1.20D+L	4.271	-X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +1.20D+L	4.271	+X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +1.20D	3.856	-X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +1.20D	3.856	+X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +1.20D+0.50Lr+L	4.403	-X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +1.20D+0.50Lr+L	4.403	+X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +0.90D	2.892	-X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +0.90D	2.892	+X	Bottom	0.2592	AsMin	0.3720	14.455	OK

One Way Shear

Load Combination...	Vu @ -X	Vu @ +X	Vu @ -Z	Vu @ +Z	Vu:Max	Phi Vn	Vu / Phi*Vn	Status
+1.40D	26.04 psi	26.04 psi	26.04 psi	26.04 psi	26.04 psi	82.16 psi	0.32	OK
+1.20D+0.50Lr+1.60L	26.92 psi	26.92 psi	26.92 psi	26.92 psi	26.92 psi	82.16 psi	0.33	OK
+1.20D+1.60L	26.15 psi	26.15 psi	26.15 psi	26.15 psi	26.15 psi	82.16 psi	0.32	OK
+1.20D+1.60Lr+L	27.17 psi	27.17 psi	27.17 psi	27.17 psi	27.17 psi	82.16 psi	0.33	OK
+1.20D+1.60Lr	24.77 psi	24.77 psi	24.77 psi	24.77 psi	24.77 psi	82.16 psi	0.30	OK
+1.20D+L	24.72 psi	24.72 psi	24.72 psi	24.72 psi	24.72 psi	82.16 psi	0.30	OK
+1.20D	22.32 psi	22.32 psi	22.32 psi	22.32 psi	22.32 psi	82.16 psi	0.27	OK
+1.20D+0.50Lr+L	25.48 psi	25.48 psi	25.48 psi	25.48 psi	25.48 psi	82.16 psi	0.31	OK
+0.90D	16.74 psi	16.74 psi	16.74 psi	16.74 psi	16.74 psi	82.16 psi	0.20	OK

Two-Way "Punching" Shear

All units k

Load Combination...	Vu	Phi*Vn	Vu / Phi*Vn	Status
+1.40D	65.28 psi	164.32psi	0.3973	OK
+1.20D+0.50Lr+1.60L	67.49 psi	164.32psi	0.4107	OK
+1.20D+1.60L	65.57 psi	164.32psi	0.3991	OK
+1.20D+1.60Lr+L	68.11 psi	164.32psi	0.4145	OK
+1.20D+1.60Lr	62.10 psi	164.32psi	0.3779	OK
+1.20D+L	61.96 psi	164.32psi	0.3771	OK
+1.20D	55.95 psi	164.32psi	0.3405	OK
+1.20D+0.50Lr+L	63.89 psi	164.32psi	0.3888	OK
+0.90D	41.96 psi	164.32psi	0.2554	OK

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 | Arch-Deco Design & Construction Inc. | (c) ENERCALC INC 1983-2021

DESCRIPTION: F-60 (worst)

Code References

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : ASCE 7-16

General Information

Material Properties

f'c : Concrete 28 day strength	=	3.0	ksi
fy : Rebar Yield	=	60.0	ksi
Ec : Concrete Elastic Modulus	=	3,122.0	ksi
Concrete Density	=	145.0	pcf
φ Values Flexure	=	0.90	
Shear	=	0.750	

Analysis Settings

Min Steel % Bending Reinf.	=	
Min Allow % Temp Reinf.	=	0.00180
Min. Overturning Safety Factor	=	1.0 : 1
Min. Sliding Safety Factor	=	1.0 : 1
Add Ftg Wt for Soil Pressure	:	Yes
Use ftg wt for stability, moments & shears	:	Yes
Add Pedestal Wt for Soil Pressure	:	No
Use Pedestal wt for stability, mom & shear	:	No

Soil Design Values

Allowable Soil Bearing	=	2.50	ksf
Soil Density	=	110.0	pcf
Increase Bearing By Footing Weight	=	No	
Soil Passive Resistance (for Sliding)	=	250.0	pcf
Soil/Concrete Friction Coeff.	=	0.30	

Increases based on footing Depth

Footing base depth below soil surface	=	3.0	ft
Allow press. increase per foot of depth	=		ksf
when footing base is below	=		ft

Increases based on footing plan dimension

Allowable pressure increase per foot of depth	=		ksf
when max. length or width is greater than	=		ft

Dimensions

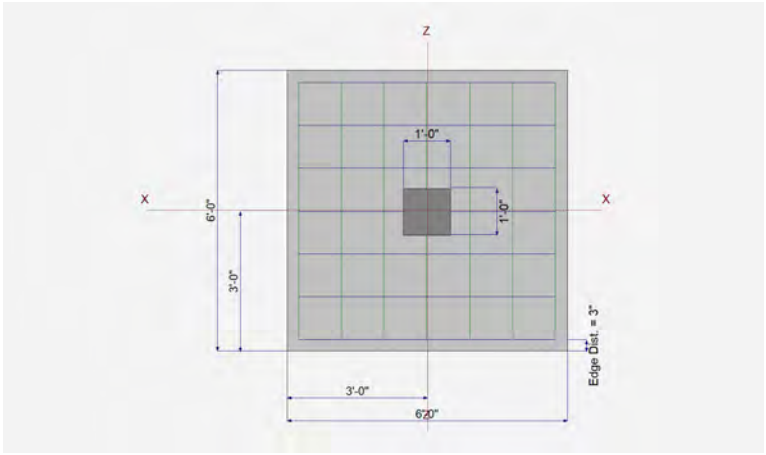
Width parallel to X-X Axis	=	6.0 ft
Length parallel to Z-Z Axis	=	6.0 ft
Footing Thickness	=	12.0 in

Pedestal dimensions...

px : parallel to X-X Axis	=	12.0 in
pz : parallel to Z-Z Axis	=	12.0 in
Height	=	24.0 in

Rebar Centerline to Edge of Concrete...

at Bottom of footing	=	3.0 in



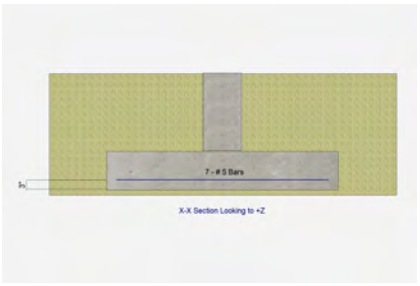
Reinforcing

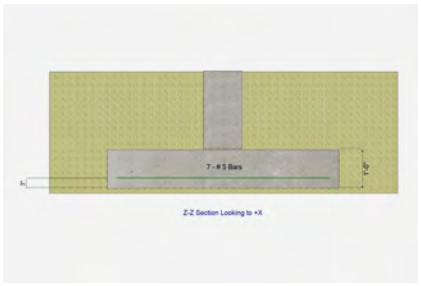
Bars parallel to X-X Axis			
Number of Bars	=		7.0
Reinforcing Bar Size	=	#	5
Bars parallel to Z-Z Axis			
Number of Bars	=		7.0
Reinforcing Bar Size	=	#	5

Bandwidth Distribution Check (ACI 15.4.4.2)

Direction Requiring Closer Separation: n/a

# Bars required within zone	n/a
# Bars required on each side of zone	n/a





Applied Loads

		D	Lr	L	S	W	E	H	
P : Column Load	=	54.320	3.240	9.540					k
OB : Overburden	=	0.050		0.040					ksf
M-xx	=								k-ft
M-zz	=								k-ft
V-x	=								k
V-z	=								k

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: F-60 (worst)

DESIGN SUMMARY

Design OK

	Min. Ratio	Item	Applied	Capacity	Governing Load Combination
PASS	0.8880	Soil Bearing	2.220 ksf	2.50 ksf	+D+L about Z-Z axis
PASS	n/a	Overturning - X-X	0.0 k-ft	0.0 k-ft	No Overturning
PASS	n/a	Overturning - Z-Z	0.0 k-ft	0.0 k-ft	No Overturning
PASS	n/a	Sliding - X-X	0.0 k	0.0 k	No Sliding
PASS	n/a	Sliding - Z-Z	0.0 k	0.0 k	No Sliding
PASS	n/a	Uplift	0.0 k	0.0 k	No Uplift
PASS	0.5039	Z Flexure (+X)	7.090 k-ft/ft	14.070 k-ft/ft	+1.20D+0.50Lr+1.60L
PASS	0.5039	Z Flexure (-X)	7.090 k-ft/ft	14.070 k-ft/ft	+1.20D+0.50Lr+1.60L
PASS	0.5039	X Flexure (+Z)	7.090 k-ft/ft	14.070 k-ft/ft	+1.20D+0.50Lr+1.60L
PASS	0.5039	X Flexure (-Z)	7.090 k-ft/ft	14.070 k-ft/ft	+1.20D+0.50Lr+1.60L
PASS	0.4449	1-way Shear (+X)	36.554 psi	82.158 psi	+1.20D+0.50Lr+1.60L
PASS	0.4449	1-way Shear (-X)	36.554 psi	82.158 psi	+1.20D+0.50Lr+1.60L
PASS	0.4449	1-way Shear (+Z)	36.554 psi	82.158 psi	+1.20D+0.50Lr+1.60L
PASS	0.4449	1-way Shear (-Z)	36.554 psi	82.158 psi	+1.20D+0.50Lr+1.60L
PASS	0.5983	2-way Punching	98.319 psi	164.317 psi	+1.20D+0.50Lr+1.60L

Detailed Results

Soil Bearing

Rotation Axis & Load Combination...	Gross Allowable	Xecc (in)	Zecc (in)	Actual Soil Bearing Stress @ Location Bottom, -Z	Top, +Z	Left, -X	Right, +X	Actual / Allow Ratio
X-X, D Only	2.50	n/a	0.0	1.916	1.916	n/a	n/a	0.766
X-X, +D+L	2.50	n/a	0.0	2.220	2.220	n/a	n/a	0.888
X-X, +D+Lr	2.50	n/a	0.0	2.006	2.006	n/a	n/a	0.802
X-X, +D+0.750Lr+0.750L	2.50	n/a	0.0	2.212	2.212	n/a	n/a	0.885
X-X, +D+0.750L	2.50	n/a	0.0	2.144	2.144	n/a	n/a	0.858
X-X, +0.60D	2.50	n/a	0.0	1.150	1.150	n/a	n/a	0.460
Z-Z, D Only	2.50	0.0	n/a	n/a	n/a	1.916	1.916	0.766
Z-Z, +D+L	2.50	0.0	n/a	n/a	n/a	2.220	2.220	0.888
Z-Z, +D+Lr	2.50	0.0	n/a	n/a	n/a	2.006	2.006	0.802
Z-Z, +D+0.750Lr+0.750L	2.50	0.0	n/a	n/a	n/a	2.212	2.212	0.885
Z-Z, +D+0.750L	2.50	0.0	n/a	n/a	n/a	2.144	2.144	0.858
Z-Z, +0.60D	2.50	0.0	n/a	n/a	n/a	1.150	1.150	0.460

Overturning Stability

Rotation Axis & Load Combination...	Overturning Moment	Resisting Moment	Stability Ratio	Status
Footing Has NO Overturning				

All units k

Sliding Stability

Force Application Axis Load Combination...	Sliding Force	Resisting Force	Stability Ratio	Status
Footing Has NO Sliding				

Footing Flexure

Flexure Axis & Load Combination	Mu k-ft	Side	Tension Surface	As Req'd in^2	Gvrn. As in^2	Actual As in^2	Phi*Mn k-ft	Status
X-X, +1.40D	6.568	+Z	Bottom	0.2592	AsMin	0.3617	14.070	OK
X-X, +1.40D	6.568	-Z	Bottom	0.2592	AsMin	0.3617	14.070	OK
X-X, +1.20D+0.50Lr+1.60L	7.090	+Z	Bottom	0.2592	AsMin	0.3617	14.070	OK
X-X, +1.20D+0.50Lr+1.60L	7.090	-Z	Bottom	0.2592	AsMin	0.3617	14.070	OK
X-X, +1.20D+1.60L	6.949	+Z	Bottom	0.2592	AsMin	0.3617	14.070	OK
X-X, +1.20D+1.60L	6.949	-Z	Bottom	0.2592	AsMin	0.3617	14.070	OK
X-X, +1.20D+1.60Lr+L	6.904	+Z	Bottom	0.2592	AsMin	0.3617	14.070	OK
X-X, +1.20D+1.60Lr+L	6.904	-Z	Bottom	0.2592	AsMin	0.3617	14.070	OK
X-X, +1.20D+1.60Lr	6.080	+Z	Bottom	0.2592	AsMin	0.3617	14.070	OK
X-X, +1.20D+1.60Lr	6.080	-Z	Bottom	0.2592	AsMin	0.3617	14.070	OK
X-X, +1.20D+L	6.454	+Z	Bottom	0.2592	AsMin	0.3617	14.070	OK
X-X, +1.20D+L	6.454	-Z	Bottom	0.2592	AsMin	0.3617	14.070	OK
X-X, +1.20D	5.630	+Z	Bottom	0.2592	AsMin	0.3617	14.070	OK

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 | Arch-Deco Design & Construction Inc. | (c) ENERCALC INC 1983-2021

DESCRIPTION: F-60 (worst)

Footing Flexure

Flexure Axis & Load Combination	Mu k-ft	Side	Tension Surface	As Req'd in^2	Gvrn. As in^2	Actual As in^2	Phi*Mn k-ft	Status
X-X, +1.20D	5.630	-Z	Bottom	0.2592	AsMin	0.3617	14.070	OK
X-X, +1.20D+0.50Lr+L	6.595	+Z	Bottom	0.2592	AsMin	0.3617	14.070	OK
X-X, +1.20D+0.50Lr+L	6.595	-Z	Bottom	0.2592	AsMin	0.3617	14.070	OK
X-X, +0.90D	4.222	+Z	Bottom	0.2592	AsMin	0.3617	14.070	OK
X-X, +0.90D	4.222	-Z	Bottom	0.2592	AsMin	0.3617	14.070	OK
Z-Z, +1.40D	6.568	-X	Bottom	0.2592	AsMin	0.3617	14.070	OK
Z-Z, +1.40D	6.568	+X	Bottom	0.2592	AsMin	0.3617	14.070	OK
Z-Z, +1.20D+0.50Lr+1.60L	7.090	-X	Bottom	0.2592	AsMin	0.3617	14.070	OK
Z-Z, +1.20D+0.50Lr+1.60L	7.090	+X	Bottom	0.2592	AsMin	0.3617	14.070	OK
Z-Z, +1.20D+1.60L	6.949	-X	Bottom	0.2592	AsMin	0.3617	14.070	OK
Z-Z, +1.20D+1.60L	6.949	+X	Bottom	0.2592	AsMin	0.3617	14.070	OK
Z-Z, +1.20D+1.60Lr+L	6.904	-X	Bottom	0.2592	AsMin	0.3617	14.070	OK
Z-Z, +1.20D+1.60Lr+L	6.904	+X	Bottom	0.2592	AsMin	0.3617	14.070	OK
Z-Z, +1.20D+1.60Lr	6.080	-X	Bottom	0.2592	AsMin	0.3617	14.070	OK
Z-Z, +1.20D+1.60Lr	6.080	+X	Bottom	0.2592	AsMin	0.3617	14.070	OK
Z-Z, +1.20D+L	6.454	-X	Bottom	0.2592	AsMin	0.3617	14.070	OK
Z-Z, +1.20D+L	6.454	+X	Bottom	0.2592	AsMin	0.3617	14.070	OK
Z-Z, +1.20D	5.630	-X	Bottom	0.2592	AsMin	0.3617	14.070	OK
Z-Z, +1.20D	5.630	+X	Bottom	0.2592	AsMin	0.3617	14.070	OK
Z-Z, +1.20D+0.50Lr+L	6.595	-X	Bottom	0.2592	AsMin	0.3617	14.070	OK
Z-Z, +1.20D+0.50Lr+L	6.595	+X	Bottom	0.2592	AsMin	0.3617	14.070	OK
Z-Z, +0.90D	4.222	-X	Bottom	0.2592	AsMin	0.3617	14.070	OK
Z-Z, +0.90D	4.222	+X	Bottom	0.2592	AsMin	0.3617	14.070	OK

One Way Shear

Load Combination...	Vu @ -X	Vu @ +X	Vu @ -Z	Vu @ +Z	Vu:Max	Phi Vn	Vu / Phi*Vn	Status
+1.40D	33.87 psi	33.87 psi	33.87 psi	33.87 psi	33.87 psi	82.16 psi	0.41	OK
+1.20D+0.50Lr+1.60L	36.55 psi	36.55 psi	36.55 psi	36.55 psi	36.55 psi	82.16 psi	0.44	OK
+1.20D+1.60L	35.83 psi	35.83 psi	35.83 psi	35.83 psi	35.83 psi	82.16 psi	0.44	OK
+1.20D+1.60Lr+L	35.60 psi	35.60 psi	35.60 psi	35.60 psi	35.60 psi	82.16 psi	0.43	OK
+1.20D+1.60Lr	31.35 psi	31.35 psi	31.35 psi	31.35 psi	31.35 psi	82.16 psi	0.38	OK
+1.20D+L	33.28 psi	33.28 psi	33.28 psi	33.28 psi	33.28 psi	82.16 psi	0.41	OK
+1.20D	29.03 psi	29.03 psi	29.03 psi	29.03 psi	29.03 psi	82.16 psi	0.35	OK
+1.20D+0.50Lr+L	34.00 psi	34.00 psi	34.00 psi	34.00 psi	34.00 psi	82.16 psi	0.41	OK
+0.90D	21.77 psi	21.77 psi	21.77 psi	21.77 psi	21.77 psi	82.16 psi	0.27	OK

Two-Way "Punching" Shear

All units k

Load Combination...	Vu	Phi*Vn	Vu / Phi*Vn	Status
+1.40D	91.08 psi	164.32psi	0.5543	OK
+1.20D+0.50Lr+1.60L	98.32 psi	164.32psi	0.5983	OK
+1.20D+1.60L	96.37 psi	164.32psi	0.5865	OK
+1.20D+1.60Lr+L	95.75 psi	164.32psi	0.5827	OK
+1.20D+1.60Lr	84.31 psi	164.32psi	0.5131	OK
+1.20D+L	89.51 psi	164.32psi	0.5447	OK
+1.20D	78.07 psi	164.32psi	0.4751	OK
+1.20D+0.50Lr+L	91.46 psi	164.32psi	0.5566	OK
+0.90D	58.55 psi	164.32psi	0.3563	OK

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 | Arch-Deco Design & Construction Inc. | (c) ENERCALC INC 1983-2021

DESCRIPTION: F-70 (worst)

Code References

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : ASCE 7-16

General Information

Material Properties

f'c : Concrete 28 day strength	=	3.0	ksi
fy : Rebar Yield	=	60.0	ksi
Ec : Concrete Elastic Modulus	=	3,122.0	ksi
Concrete Density	=	145.0	pcf
φ Values Flexure	=	0.90	
Shear	=	0.750	

Analysis Settings

Min Steel % Bending Reinf.	=	
Min Allow % Temp Reinf.	=	0.00180
Min. Overturning Safety Factor	=	1.0 : 1
Min. Sliding Safety Factor	=	1.0 : 1
Add Ftg Wt for Soil Pressure	:	Yes
Use ftg wt for stability, moments & shears	:	Yes
Add Pedestal Wt for Soil Pressure	:	No
Use Pedestal wt for stability, mom & shear	:	No

Soil Design Values

Allowable Soil Bearing	=	2.50	ksf
Soil Density	=	110.0	pcf
Increase Bearing By Footing Weight	=	No	
Soil Passive Resistance (for Sliding)	=	250.0	pcf
Soil/Concrete Friction Coeff.	=	0.30	

Increases based on footing Depth

Footing base depth below soil surface	=	3.0	ft
Allow press. increase per foot of depth	=		ksf
when footing base is below	=		ft

Increases based on footing plan dimension

Allowable pressure increase per foot of depth	=		ksf
when max. length or width is greater than	=		ft

Dimensions

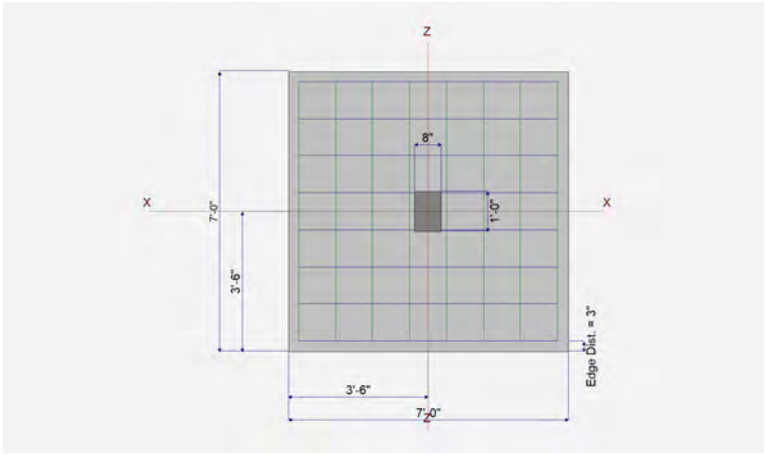
Width parallel to X-X Axis	=	7.0 ft
Length parallel to Z-Z Axis	=	7.0 ft
Footing Thickness	=	16.0 in

Pedestal dimensions...

px : parallel to X-X Axis	=	8.0 in
pz : parallel to Z-Z Axis	=	12.0 in
Height	=	24.0 in

Rebar Centerline to Edge of Concrete...

at Bottom of footing	=	3.0 in

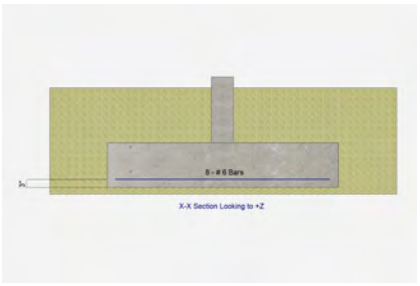


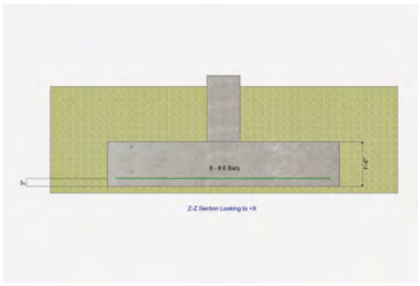
Reinforcing

Bars parallel to X-X Axis			
Number of Bars	=		8.0
Reinforcing Bar Size	=	#	6
Bars parallel to Z-Z Axis			
Number of Bars	=		8.0
Reinforcing Bar Size	=	#	6

Bandwidth Distribution Check (ACI 15.4.4.2)

Direction Requiring Closer Separation	n/a
# Bars required within zone	n/a
# Bars required on each side of zone	n/a





Applied Loads

		D	Lr	L	S	W	E	H	
P : Column Load	=	76.310	5.920	10.890					k
OB : Overburden	=	0.050		0.040					ksf
M-xx	=								k-ft
M-zz	=								k-ft
V-x	=								k
V-z	=								k

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: F-70 (worst)

DESIGN SUMMARY

Design OK

	Min. Ratio	Item	Applied	Capacity	Governing Load Combination
PASS	0.9072	Soil Bearing	2.268 ksf	2.50 ksf	+D+0.750Lr+0.750L about Z-Z axis
PASS	n/a	Overturning - X-X	0.0 k-ft	0.0 k-ft	No Overturning
PASS	n/a	Overturning - Z-Z	0.0 k-ft	0.0 k-ft	No Overturning
PASS	n/a	Sliding - X-X	0.0 k	0.0 k	No Sliding
PASS	n/a	Sliding - Z-Z	0.0 k	0.0 k	No Sliding
PASS	n/a	Uplift	0.0 k	0.0 k	No Uplift
PASS	0.4039	Z Flexure (+X)	11.432 k-ft/ft	28.302 k-ft/ft	+1.20D+0.50Lr+1.60L
PASS	0.4039	Z Flexure (-X)	11.432 k-ft/ft	28.302 k-ft/ft	+1.20D+0.50Lr+1.60L
PASS	0.3625	X Flexure (+Z)	10.261 k-ft/ft	28.302 k-ft/ft	+1.20D+0.50Lr+1.60L
PASS	0.3625	X Flexure (-Z)	10.261 k-ft/ft	28.302 k-ft/ft	+1.20D+0.50Lr+1.60L
PASS	0.3736	1-way Shear (+X)	30.694 psi	82.158 psi	+1.20D+0.50Lr+1.60L
PASS	0.3736	1-way Shear (-X)	30.694 psi	82.158 psi	+1.20D+0.50Lr+1.60L
PASS	0.3362	1-way Shear (+Z)	27.625 psi	82.158 psi	+1.20D+0.50Lr+1.60L
PASS	0.3362	1-way Shear (-Z)	27.625 psi	82.158 psi	+1.20D+0.50Lr+1.60L
PASS	0.5276	2-way Punching	86.691 psi	164.317 psi	+1.20D+0.50Lr+1.60L

Detailed Results

Soil Bearing

Rotation Axis & Load Combination...	Gross Allowable	Xecc (in)	Zecc (in)	Actual Soil Bearing Stress @ Location Bottom, -Z	Top, +Z	Left, -X	Right, +X	Actual / Allow Ratio
X-X, D Only	2.50	n/a	0.0	1.981	1.981	n/a	n/a	0.792
X-X, +D+L	2.50	n/a	0.0	2.243	2.243	n/a	n/a	0.897
X-X, +D+Lr	2.50	n/a	0.0	2.102	2.102	n/a	n/a	0.841
X-X, +D+0.750Lr+0.750L	2.50	n/a	0.0	2.268	2.268	n/a	n/a	0.907
X-X, +D+0.750L	2.50	n/a	0.0	2.177	2.177	n/a	n/a	0.871
X-X, +0.60D	2.50	n/a	0.0	1.189	1.189	n/a	n/a	0.476
Z-Z, D Only	2.50	0.0	n/a	n/a	n/a	1.981	1.981	0.792
Z-Z, +D+L	2.50	0.0	n/a	n/a	n/a	2.243	2.243	0.897
Z-Z, +D+Lr	2.50	0.0	n/a	n/a	n/a	2.102	2.102	0.841
Z-Z, +D+0.750Lr+0.750L	2.50	0.0	n/a	n/a	n/a	2.268	2.268	0.907
Z-Z, +D+0.750L	2.50	0.0	n/a	n/a	n/a	2.177	2.177	0.871
Z-Z, +0.60D	2.50	0.0	n/a	n/a	n/a	1.189	1.189	0.476

Overturning Stability

Rotation Axis & Load Combination...	Overturning Moment	Resisting Moment	Stability Ratio	Status
Footing Has NO Overturning				

All units k

Sliding Stability

Force Application Axis Load Combination...	Sliding Force	Resisting Force	Stability Ratio	Status
Footing Has NO Sliding				

Footing Flexure

Flexure Axis & Load Combination	Mu k-ft	Side	Tension Surface	As Req'd in^2	Gvrn. As in^2	Actual As in^2	Phi*Mn k-ft	Status
X-X, +1.40D	9.791	+Z	Bottom	0.3456	AsMin	0.5029	28.302	OK
X-X, +1.40D	9.791	-Z	Bottom	0.3456	AsMin	0.5029	28.302	OK
X-X, +1.20D+0.50Lr+1.60L	10.261	+Z	Bottom	0.3456	AsMin	0.5029	28.302	OK
X-X, +1.20D+0.50Lr+1.60L	10.261	-Z	Bottom	0.3456	AsMin	0.5029	28.302	OK
X-X, +1.20D+1.60L	9.989	+Z	Bottom	0.3456	AsMin	0.5029	28.302	OK
X-X, +1.20D+1.60L	9.989	-Z	Bottom	0.3456	AsMin	0.5029	28.302	OK
X-X, +1.20D+1.60Lr+L	10.260	+Z	Bottom	0.3456	AsMin	0.5029	28.302	OK
X-X, +1.20D+1.60Lr+L	10.260	-Z	Bottom	0.3456	AsMin	0.5029	28.302	OK
X-X, +1.20D+1.60Lr	9.262	+Z	Bottom	0.3456	AsMin	0.5029	28.302	OK
X-X, +1.20D+1.60Lr	9.262	-Z	Bottom	0.3456	AsMin	0.5029	28.302	OK
X-X, +1.20D+L	9.390	+Z	Bottom	0.3456	AsMin	0.5029	28.302	OK
X-X, +1.20D+L	9.390	-Z	Bottom	0.3456	AsMin	0.5029	28.302	OK
X-X, +1.20D	8.392	+Z	Bottom	0.3456	AsMin	0.5029	28.302	OK

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 | Arch-Deco Design & Construction Inc. | (c) ENERCALC INC 1983-2021

DESCRIPTION: F-70 (worst)

Footing Flexure

Flexure Axis & Load Combination	Mu k-ft	Side	Tension Surface	As Req'd in^2	Gvrn. As in^2	Actual As in^2	Phi*Mn k-ft	Status
X-X, +1.20D	8.392	-Z	Bottom	0.3456	AsMin	0.5029	28.302	OK
X-X, +1.20D+0.50Lr+L	9.662	+Z	Bottom	0.3456	AsMin	0.5029	28.302	OK
X-X, +1.20D+0.50Lr+L	9.662	-Z	Bottom	0.3456	AsMin	0.5029	28.302	OK
X-X, +0.90D	6.294	+Z	Bottom	0.3456	AsMin	0.5029	28.302	OK
X-X, +0.90D	6.294	-Z	Bottom	0.3456	AsMin	0.5029	28.302	OK
Z-Z, +1.40D	10.909	-X	Bottom	0.3456	AsMin	0.5029	28.302	OK
Z-Z, +1.40D	10.909	+X	Bottom	0.3456	AsMin	0.5029	28.302	OK
Z-Z, +1.20D+0.50Lr+1.60L	11.432	-X	Bottom	0.3456	AsMin	0.5029	28.302	OK
Z-Z, +1.20D+0.50Lr+1.60L	11.432	+X	Bottom	0.3456	AsMin	0.5029	28.302	OK
Z-Z, +1.20D+1.60L	11.129	-X	Bottom	0.3456	AsMin	0.5029	28.302	OK
Z-Z, +1.20D+1.60L	11.129	+X	Bottom	0.3456	AsMin	0.5029	28.302	OK
Z-Z, +1.20D+1.60Lr+L	11.431	-X	Bottom	0.3456	AsMin	0.5029	28.302	OK
Z-Z, +1.20D+1.60Lr+L	11.431	+X	Bottom	0.3456	AsMin	0.5029	28.302	OK
Z-Z, +1.20D+1.60Lr	10.320	-X	Bottom	0.3456	AsMin	0.5029	28.302	OK
Z-Z, +1.20D+1.60Lr	10.320	+X	Bottom	0.3456	AsMin	0.5029	28.302	OK
Z-Z, +1.20D+L	10.462	-X	Bottom	0.3456	AsMin	0.5029	28.302	OK
Z-Z, +1.20D+L	10.462	+X	Bottom	0.3456	AsMin	0.5029	28.302	OK
Z-Z, +1.20D	9.351	-X	Bottom	0.3456	AsMin	0.5029	28.302	OK
Z-Z, +1.20D	9.351	+X	Bottom	0.3456	AsMin	0.5029	28.302	OK
Z-Z, +1.20D+0.50Lr+L	10.765	-X	Bottom	0.3456	AsMin	0.5029	28.302	OK
Z-Z, +1.20D+0.50Lr+L	10.765	+X	Bottom	0.3456	AsMin	0.5029	28.302	OK
Z-Z, +0.90D	7.013	-X	Bottom	0.3456	AsMin	0.5029	28.302	OK
Z-Z, +0.90D	7.013	+X	Bottom	0.3456	AsMin	0.5029	28.302	OK

One Way Shear

Load Combination...	Vu @ -X	Vu @ +X	Vu @ -Z	Vu @ +Z	Vu:Max	Phi Vn	Vu / Phi*Vn	Status
+1.40D	29.29 psi	29.29 psi	26.36 psi	26.36 psi	29.29 psi	82.16 psi	0.36	OK
+1.20D+0.50Lr+1.60L	30.69 psi	30.69 psi	27.63 psi	27.63 psi	30.69 psi	82.16 psi	0.37	OK
+1.20D+1.60L	29.88 psi	29.88 psi	26.89 psi	26.89 psi	29.88 psi	82.16 psi	0.36	OK
+1.20D+1.60Lr+L	30.69 psi	30.69 psi	27.62 psi	27.62 psi	30.69 psi	82.16 psi	0.37	OK
+1.20D+1.60Lr	27.71 psi	27.71 psi	24.94 psi	24.94 psi	27.71 psi	82.16 psi	0.34	OK
+1.20D+L	28.09 psi	28.09 psi	25.28 psi	25.28 psi	28.09 psi	82.16 psi	0.34	OK
+1.20D	25.11 psi	25.11 psi	22.60 psi	22.60 psi	25.11 psi	82.16 psi	0.31	OK
+1.20D+0.50Lr+L	28.90 psi	28.90 psi	26.01 psi	26.01 psi	28.90 psi	82.16 psi	0.35	OK
+0.90D	18.83 psi	18.83 psi	16.95 psi	16.95 psi	18.83 psi	82.16 psi	0.23	OK

Two-Way "Punching" Shear

All units k

Load Combination...	Vu	Phi*Vn	Vu / Phi*Vn	Status
+1.40D	82.73 psi	164.32psi	0.5035	OK
+1.20D+0.50Lr+1.60L	86.69 psi	164.32psi	0.5276	OK
+1.20D+1.60L	84.39 psi	164.32psi	0.5136	OK
+1.20D+1.60Lr+L	86.69 psi	164.32psi	0.5276	OK
+1.20D+1.60Lr	78.26 psi	164.32psi	0.4763	OK
+1.20D+L	79.34 psi	164.32psi	0.4828	OK
+1.20D	70.91 psi	164.32psi	0.4315	OK
+1.20D+0.50Lr+L	81.63 psi	164.32psi	0.4968	OK
+0.90D	53.18 psi	164.32psi	0.3236	OK

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 | Arch-Deco Design & Construction Inc. | (c) ENERCALC INC 1983-2021

DESCRIPTION: F-106 (worst)

Code References

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : ASCE 7-16

General Information

Material Properties

f'c : Concrete 28 day strength	=	3.0	ksi
fy : Rebar Yield	=	60.0	ksi
Ec : Concrete Elastic Modulus	=	3,122.0	ksi
Concrete Density	=	145.0	pcf
φ Values Flexure	=	0.90	
Shear	=	0.750	

Analysis Settings

Min Steel % Bending Reinf.	=	
Min Allow % Temp Reinf.	=	0.00180
Min. Overturning Safety Factor	=	1.0 : 1
Min. Sliding Safety Factor	=	1.0 : 1
Add Ftg Wt for Soil Pressure	:	Yes
Use ftg wt for stability, moments & shears	:	Yes
Add Pedestal Wt for Soil Pressure	:	No
Use Pedestal wt for stability, mom & shear	:	No

Soil Design Values

Allowable Soil Bearing	=	2.50	ksf
Soil Density	=	110.0	pcf
Increase Bearing By Footing Weight	=	No	
Soil Passive Resistance (for Sliding)	=	250.0	pcf
Soil/Concrete Friction Coeff.	=	0.30	

Increases based on footing Depth

Footing base depth below soil surface	=	4.0	ft
Allow press. increase per foot of depth	=		ksf
when footing base is below	=		ft

Increases based on footing plan dimension

Allowable pressure increase per foot of depth	=		ksf
when max. length or width is greater than	=		ft

Dimensions

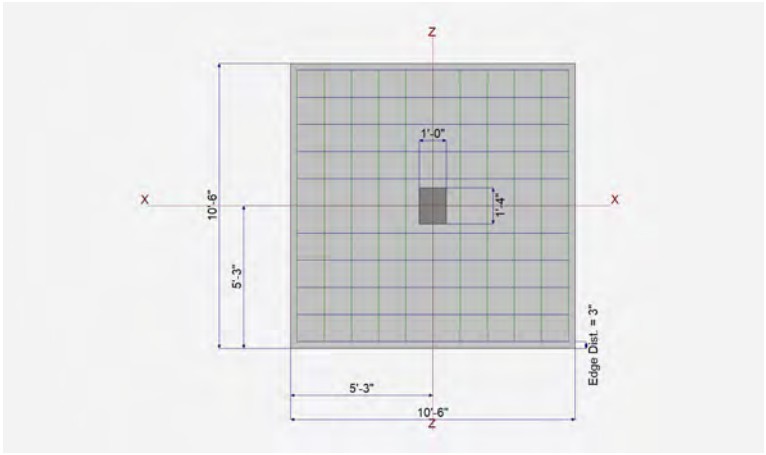
Width parallel to X-X Axis	=	10.50	ft
Length parallel to Z-Z Axis	=	10.50	ft
Footing Thickness	=	18.0	in

Pedestal dimensions...

px : parallel to X-X Axis	=	12.0	in
pz : parallel to Z-Z Axis	=	16.0	in
Height	=	24.0	in

Rebar Centerline to Edge of Concrete...

at Bottom of footing	=	3.0	in

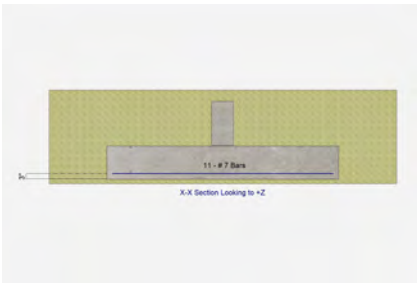


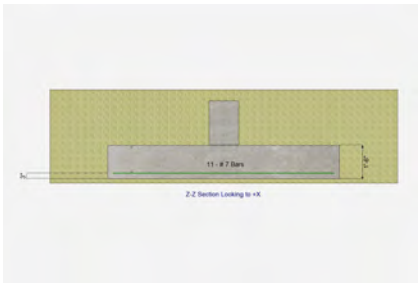
Reinforcing

Bars parallel to X-X Axis			
Number of Bars	=		11.0
Reinforcing Bar Size	=	#	7
Bars parallel to Z-Z Axis			
Number of Bars	=		11.0
Reinforcing Bar Size	=	#	7

Bandwidth Distribution Check (ACI 15.4.4.2)

Direction Requiring Closer Separation	n/a
# Bars required within zone	n/a
# Bars required on each side of zone	n/a





Applied Loads

		D	Lr	L	S	W	E	H	
P : Column Load	=	161.250	11.280	24.360					k
OB : Overburden	=	0.050		0.040					ksf
M-xx	=								k-ft
M-zz	=								k-ft
V-x	=								k
V-z	=								k

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 | Arch-Deco Design & Construction Inc. | (c) ENERCALC INC 1983-2021

DESCRIPTION: F-106 (worst)

DESIGN SUMMARY

Design OK

	Min. Ratio	Item	Applied	Capacity	Governing Load Combination
PASS	0.9092	Soil Bearing	2.273 ksf	2.50 ksf	+D+0.750Lr+0.750L about Z-Z axis
PASS	n/a	Overturning - X-X	0.0 k-ft	0.0 k-ft	No Overturning
PASS	n/a	Overturning - Z-Z	0.0 k-ft	0.0 k-ft	No Overturning
PASS	n/a	Sliding - X-X	0.0 k	0.0 k	No Sliding
PASS	n/a	Sliding - Z-Z	0.0 k	0.0 k	No Sliding
PASS	n/a	Uplift	0.0 k	0.0 k	No Uplift
PASS	0.5973	Z Flexure (+X)	24.302 k-ft/ft	40.685 k-ft/ft	+1.20D+0.50Lr+1.60L
PASS	0.5973	Z Flexure (-X)	24.302 k-ft/ft	40.685 k-ft/ft	+1.20D+0.50Lr+1.60L
PASS	0.5561	X Flexure (+Z)	22.626 k-ft/ft	40.685 k-ft/ft	+1.20D+0.50Lr+1.60L
PASS	0.5561	X Flexure (-Z)	22.626 k-ft/ft	40.685 k-ft/ft	+1.20D+0.50Lr+1.60L
PASS	0.5048	1-way Shear (+X)	41.470 psi	82.158 psi	+1.20D+0.50Lr+1.60L
PASS	0.5048	1-way Shear (-X)	41.470 psi	82.158 psi	+1.20D+0.50Lr+1.60L
PASS	0.4895	1-way Shear (+Z)	40.213 psi	82.158 psi	+1.20D+0.50Lr+1.60L
PASS	0.4895	1-way Shear (-Z)	40.213 psi	82.158 psi	+1.20D+0.50Lr+1.60L
PASS	0.7869	2-way Punching	129.293 psi	164.317 psi	+1.20D+0.50Lr+1.60L

Detailed Results

Soil Bearing

Rotation Axis & Load Combination...	Gross Allowable	Xecc (in)	Zecc (in)	Actual Soil Bearing Stress @ Location Bottom, -Z	Top, +Z	Left, -X	Right, +X	Actual / Allow Ratio
X-X, D Only	2.50	n/a	0.0	2.001	2.001	n/a	n/a	0.800
X-X, +D+L	2.50	n/a	0.0	2.262	2.262	n/a	n/a	0.905
X-X, +D+Lr	2.50	n/a	0.0	2.103	2.103	n/a	n/a	0.841
X-X, +D+0.750Lr+0.750L	2.50	n/a	0.0	2.273	2.273	n/a	n/a	0.909
X-X, +D+0.750L	2.50	n/a	0.0	2.197	2.197	n/a	n/a	0.879
X-X, +0.60D	2.50	n/a	0.0	1.201	1.201	n/a	n/a	0.480
Z-Z, D Only	2.50	0.0	n/a	n/a	n/a	2.001	2.001	0.800
Z-Z, +D+L	2.50	0.0	n/a	n/a	n/a	2.262	2.262	0.905
Z-Z, +D+Lr	2.50	0.0	n/a	n/a	n/a	2.103	2.103	0.841
Z-Z, +D+0.750Lr+0.750L	2.50	0.0	n/a	n/a	n/a	2.273	2.273	0.909
Z-Z, +D+0.750L	2.50	0.0	n/a	n/a	n/a	2.197	2.197	0.879
Z-Z, +0.60D	2.50	0.0	n/a	n/a	n/a	1.201	1.201	0.480

Overturning Stability

Rotation Axis & Load Combination...	Overturning Moment	Resisting Moment	Stability Ratio	Status
Footing Has NO Overturning				

All units k

Sliding Stability

Force Application Axis Load Combination...	Sliding Force	Resisting Force	Stability Ratio	Status
Footing Has NO Sliding				

Footing Flexure

Flexure Axis & Load Combination	Mu k-ft	Side	Tension Surface	As Req'd in^2	Gvrn. As in^2	Actual As in^2	Phi*Mn k-ft	Status
X-X, +1.40D	21.448	+Z	Bottom	0.3888	AsMin	0.6286	40.685	OK
X-X, +1.40D	21.448	-Z	Bottom	0.3888	AsMin	0.6286	40.685	OK
X-X, +1.20D+0.50Lr+1.60L	22.626	+Z	Bottom	0.3888	AsMin	0.6286	40.685	OK
X-X, +1.20D+0.50Lr+1.60L	22.626	-Z	Bottom	0.3888	AsMin	0.6286	40.685	OK
X-X, +1.20D+1.60L	22.089	+Z	Bottom	0.3888	AsMin	0.6286	40.685	OK
X-X, +1.20D+1.60L	22.089	-Z	Bottom	0.3888	AsMin	0.6286	40.685	OK
X-X, +1.20D+1.60Lr+L	22.419	+Z	Bottom	0.3888	AsMin	0.6286	40.685	OK
X-X, +1.20D+1.60Lr+L	22.419	-Z	Bottom	0.3888	AsMin	0.6286	40.685	OK
X-X, +1.20D+1.60Lr	20.103	+Z	Bottom	0.3888	AsMin	0.6286	40.685	OK
X-X, +1.20D+1.60Lr	20.103	-Z	Bottom	0.3888	AsMin	0.6286	40.685	OK
X-X, +1.20D+L	20.699	+Z	Bottom	0.3888	AsMin	0.6286	40.685	OK
X-X, +1.20D+L	20.699	-Z	Bottom	0.3888	AsMin	0.6286	40.685	OK
X-X, +1.20D	18.384	+Z	Bottom	0.3888	AsMin	0.6286	40.685	OK

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 | Arch-Deco Design & Construction Inc. | (c) ENERCALC INC 1983-2021

DESCRIPTION: F-106 (worst)

Footing Flexure

Flexure Axis & Load Combination	Mu k-ft	Side	Tension Surface	As Req'd in^2	Gvrn. As in^2	Actual As in^2	Phi*Mn k-ft	Status
X-X, +1.20D	18.384	-Z	Bottom	0.3888	AsMin	0.6286	40.685	OK
X-X, +1.20D+0.50Lr+L	21.237	+Z	Bottom	0.3888	AsMin	0.6286	40.685	OK
X-X, +1.20D+0.50Lr+L	21.237	-Z	Bottom	0.3888	AsMin	0.6286	40.685	OK
X-X, +0.90D	13.788	+Z	Bottom	0.3888	AsMin	0.6286	40.685	OK
X-X, +0.90D	13.788	-Z	Bottom	0.3888	AsMin	0.6286	40.685	OK
Z-Z, +1.40D	23.037	-X	Bottom	0.3888	AsMin	0.6286	40.685	OK
Z-Z, +1.40D	23.037	+X	Bottom	0.3888	AsMin	0.6286	40.685	OK
Z-Z, +1.20D+0.50Lr+1.60L	24.302	-X	Bottom	0.3888	AsMin	0.6286	40.685	OK
Z-Z, +1.20D+0.50Lr+1.60L	24.302	+X	Bottom	0.3888	AsMin	0.6286	40.685	OK
Z-Z, +1.20D+1.60L	23.725	-X	Bottom	0.3888	AsMin	0.6286	40.685	OK
Z-Z, +1.20D+1.60L	23.725	+X	Bottom	0.3888	AsMin	0.6286	40.685	OK
Z-Z, +1.20D+1.60Lr+L	24.080	-X	Bottom	0.3888	AsMin	0.6286	40.685	OK
Z-Z, +1.20D+1.60Lr+L	24.080	+X	Bottom	0.3888	AsMin	0.6286	40.685	OK
Z-Z, +1.20D+1.60Lr	21.593	-X	Bottom	0.3888	AsMin	0.6286	40.685	OK
Z-Z, +1.20D+1.60Lr	21.593	+X	Bottom	0.3888	AsMin	0.6286	40.685	OK
Z-Z, +1.20D+L	22.233	-X	Bottom	0.3888	AsMin	0.6286	40.685	OK
Z-Z, +1.20D+L	22.233	+X	Bottom	0.3888	AsMin	0.6286	40.685	OK
Z-Z, +1.20D	19.746	-X	Bottom	0.3888	AsMin	0.6286	40.685	OK
Z-Z, +1.20D	19.746	+X	Bottom	0.3888	AsMin	0.6286	40.685	OK
Z-Z, +1.20D+0.50Lr+L	22.810	-X	Bottom	0.3888	AsMin	0.6286	40.685	OK
Z-Z, +1.20D+0.50Lr+L	22.810	+X	Bottom	0.3888	AsMin	0.6286	40.685	OK
Z-Z, +0.90D	14.809	-X	Bottom	0.3888	AsMin	0.6286	40.685	OK
Z-Z, +0.90D	14.809	+X	Bottom	0.3888	AsMin	0.6286	40.685	OK

One Way Shear

Load Combination...	Vu @ -X	Vu @ +X	Vu @ -Z	Vu @ +Z	Vu:Max	Phi Vn	Vu / Phi*Vn	Status
+1.40D	39.31 psi	39.31 psi	38.12 psi	38.12 psi	39.31 psi	82.16 psi	0.48	OK
+1.20D+0.50Lr+1.60L	41.47 psi	41.47 psi	40.21 psi	40.21 psi	41.47 psi	82.16 psi	0.50	OK
+1.20D+1.60L	40.49 psi	40.49 psi	39.26 psi	39.26 psi	40.49 psi	82.16 psi	0.49	OK
+1.20D+1.60Lr+L	41.09 psi	41.09 psi	39.85 psi	39.85 psi	41.09 psi	82.16 psi	0.50	OK
+1.20D+1.60Lr	36.85 psi	36.85 psi	35.73 psi	35.73 psi	36.85 psi	82.16 psi	0.45	OK
+1.20D+L	37.94 psi	37.94 psi	36.79 psi	36.79 psi	37.94 psi	82.16 psi	0.46	OK
+1.20D	33.70 psi	33.70 psi	32.67 psi	32.67 psi	33.70 psi	82.16 psi	0.41	OK
+1.20D+0.50Lr+L	38.92 psi	38.92 psi	37.74 psi	37.74 psi	38.92 psi	82.16 psi	0.47	OK
+0.90D	25.27 psi	25.27 psi	24.51 psi	24.51 psi	25.27 psi	82.16 psi	0.31	OK

Two-Way "Punching" Shear

All units k

Load Combination...	Vu	Phi*Vn	Vu / Phi*Vn	Status
+1.40D	122.56 psi	164.32psi	0.7459	OK
+1.20D+0.50Lr+1.60L	129.29 psi	164.32psi	0.7869	OK
+1.20D+1.60L	126.22 psi	164.32psi	0.7682	OK
+1.20D+1.60Lr+L	128.11 psi	164.32psi	0.7796	OK
+1.20D+1.60Lr	114.88 psi	164.32psi	0.6991	OK
+1.20D+L	118.28 psi	164.32psi	0.7199	OK
+1.20D	105.05 psi	164.32psi	0.6393	OK
+1.20D+0.50Lr+L	121.35 psi	164.32psi	0.7385	OK
+0.90D	78.79 psi	164.32psi	0.4795	OK

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 | Arch-Deco Design & Construction Inc. | (c) ENERCALC INC 1983-2021

DESCRIPTION: F-126 (worst)

Code References

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : ASCE 7-16

General Information

Material Properties

f'c : Concrete 28 day strength	=	3.0	ksi
fy : Rebar Yield	=	60.0	ksi
Ec : Concrete Elastic Modulus	=	3,122.0	ksi
Concrete Density	=	145.0	pcf
φ Values Flexure	=	0.90	
Shear	=	0.750	

Analysis Settings

Min Steel % Bending Reinf.	=	
Min Allow % Temp Reinf.	=	0.00180
Min. Overturning Safety Factor	=	1.0 : 1
Min. Sliding Safety Factor	=	1.0 : 1
Add Ftg Wt for Soil Pressure	:	Yes
Use ftg wt for stability, moments & shears	:	Yes
Add Pedestal Wt for Soil Pressure	:	No
Use Pedestal wt for stability, mom & shear	:	No

Soil Design Values

Allowable Soil Bearing	=	2.50	ksf
Soil Density	=	110.0	pcf
Increase Bearing By Footing Weight	=	No	
Soil Passive Resistance (for Sliding)	=	250.0	pcf
Soil/Concrete Friction Coeff.	=	0.30	

Increases based on footing Depth

Footing base depth below soil surface	=	4.0	ft
Allow press. increase per foot of depth	=		ksf
when footing base is below	=		ft

Increases based on footing plan dimension

Allowable pressure increase per foot of depth	=		ksf
when max. length or width is greater than	=		ft

Dimensions

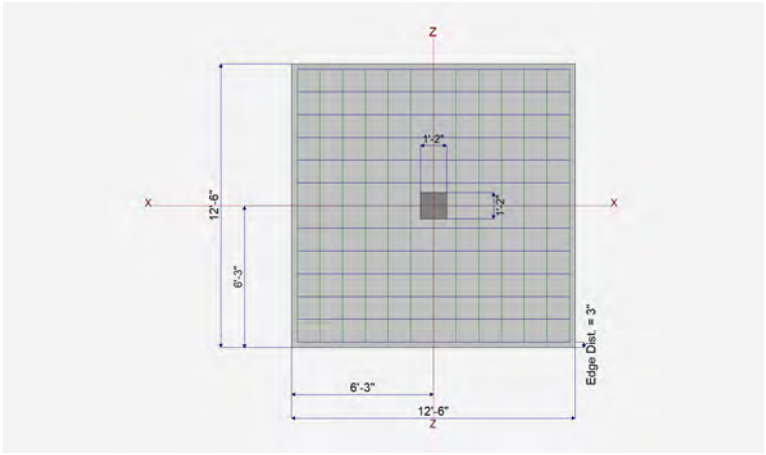
Width parallel to X-X Axis	=	12.50	ft
Length parallel to Z-Z Axis	=	12.50	ft
Footing Thickness	=	24.0	in

Pedestal dimensions...

px : parallel to X-X Axis	=	14.0	in
pz : parallel to Z-Z Axis	=	14.0	in
Height	=	24.0	in

Rebar Centerline to Edge of Concrete...

at Bottom of footing	=	3.0	in



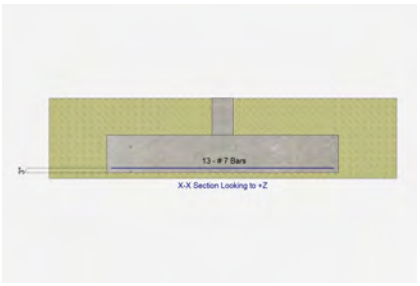
Reinforcing

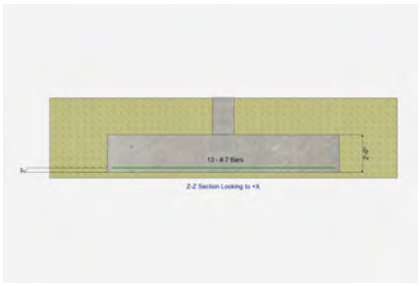
Bars parallel to X-X Axis			
Number of Bars	=		13.0
Reinforcing Bar Size	=	#	7
Bars parallel to Z-Z Axis			
Number of Bars	=		13.0
Reinforcing Bar Size	=	#	7

Bandwidth Distribution Check (ACI 15.4.4.2)

Direction Requiring Closer Separation — n/a

# Bars required within zone	n/a
# Bars required on each side of zone	n/a





Applied Loads

		D	Lr	L	S	W	E	H	
P : Column Load	=	170.570	17.730	90.640					k
OB : Overburden	=	0.050		0.040					ksf
M-xx	=								k-ft
M-zz	=								k-ft
V-x	=								k
V-z	=								k

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: F-126 (worst)

DESIGN SUMMARY

Design OK

	Min. Ratio	Item	Applied	Capacity	Governing Load Combination
PASS	0.9076	Soil Bearing	2.269 ksf	2.50 ksf	+D+L about Z-Z axis
PASS	n/a	Overturning - X-X	0.0 k-ft	0.0 k-ft	No Overturning
PASS	n/a	Overturning - Z-Z	0.0 k-ft	0.0 k-ft	No Overturning
PASS	n/a	Sliding - X-X	0.0 k	0.0 k	No Sliding
PASS	n/a	Sliding - Z-Z	0.0 k	0.0 k	No Sliding
PASS	n/a	Uplift	0.0 k	0.0 k	No Uplift
PASS	0.6426	Z Flexure (+X)	36.789 k-ft/ft	57.250 k-ft/ft	+1.20D+0.50Lr+1.60L
PASS	0.6426	Z Flexure (-X)	36.789 k-ft/ft	57.250 k-ft/ft	+1.20D+0.50Lr+1.60L
PASS	0.6426	X Flexure (+Z)	36.789 k-ft/ft	57.250 k-ft/ft	+1.20D+0.50Lr+1.60L
PASS	0.6426	X Flexure (-Z)	36.789 k-ft/ft	57.250 k-ft/ft	+1.20D+0.50Lr+1.60L
PASS	0.4289	1-way Shear (+X)	35.236 psi	82.158 psi	+1.20D+0.50Lr+1.60L
PASS	0.4289	1-way Shear (-X)	35.236 psi	82.158 psi	+1.20D+0.50Lr+1.60L
PASS	0.4289	1-way Shear (+Z)	35.236 psi	82.158 psi	+1.20D+0.50Lr+1.60L
PASS	0.4289	1-way Shear (-Z)	35.236 psi	82.158 psi	+1.20D+0.50Lr+1.60L
PASS	0.6985	2-way Punching	114.769 psi	164.317 psi	+1.20D+0.50Lr+1.60L

Detailed Results

Soil Bearing

Rotation Axis & Load Combination...	Gross Allowable	Xecc (in)	Zecc (in)	Actual Soil Bearing Stress @ Location Bottom, -Z	Top, +Z	Left, -X	Right, +X	Actual / Allow Ratio
X-X, D Only	2.50	n/a	0.0	1.649	1.649	n/a	n/a	0.660
X-X, +D+L	2.50	n/a	0.0	2.269	2.269	n/a	n/a	0.908
X-X, +D+Lr	2.50	n/a	0.0	1.763	1.763	n/a	n/a	0.705
X-X, +D+0.750Lr+0.750L	2.50	n/a	0.0	2.199	2.199	n/a	n/a	0.880
X-X, +D+0.750L	2.50	n/a	0.0	2.114	2.114	n/a	n/a	0.846
X-X, +0.60D	2.50	n/a	0.0	0.9896	0.9896	n/a	n/a	0.396
Z-Z, D Only	2.50	0.0	n/a	n/a	n/a	1.649	1.649	0.660
Z-Z, +D+L	2.50	0.0	n/a	n/a	n/a	2.269	2.269	0.908
Z-Z, +D+Lr	2.50	0.0	n/a	n/a	n/a	1.763	1.763	0.705
Z-Z, +D+0.750Lr+0.750L	2.50	0.0	n/a	n/a	n/a	2.199	2.199	0.880
Z-Z, +D+0.750L	2.50	0.0	n/a	n/a	n/a	2.114	2.114	0.846
Z-Z, +0.60D	2.50	0.0	n/a	n/a	n/a	0.9896	0.9896	0.396

Overturning Stability

Rotation Axis & Load Combination...	Overturning Moment	Resisting Moment	Stability Ratio	Status
Footing Has NO Overturning				

All units k

Sliding Stability

Force Application Axis Load Combination...	Sliding Force	Resisting Force	Stability Ratio	Status
Footing Has NO Sliding				

Footing Flexure

Flexure Axis & Load Combination	Mu k-ft	Side	Tension Surface	As Req'd in^2	Gvrn. As in^2	Actual As in^2	Phi*Mn k-ft	Status
X-X, +1.40D	24.484	+Z	Bottom	0.5184	AsMin	0.6240	57.250	OK
X-X, +1.40D	24.484	-Z	Bottom	0.5184	AsMin	0.6240	57.250	OK
X-X, +1.20D+0.50Lr+1.60L	36.789	+Z	Bottom	0.5184	AsMin	0.6240	57.250	OK
X-X, +1.20D+0.50Lr+1.60L	36.789	-Z	Bottom	0.5184	AsMin	0.6240	57.250	OK
X-X, +1.20D+1.60L	35.878	+Z	Bottom	0.5184	AsMin	0.6240	57.250	OK
X-X, +1.20D+1.60L	35.878	-Z	Bottom	0.5184	AsMin	0.6240	57.250	OK
X-X, +1.20D+1.60Lr+L	33.208	+Z	Bottom	0.5184	AsMin	0.6240	57.250	OK
X-X, +1.20D+1.60Lr+L	33.208	-Z	Bottom	0.5184	AsMin	0.6240	57.250	OK
X-X, +1.20D+1.60Lr	23.901	+Z	Bottom	0.5184	AsMin	0.6240	57.250	OK
X-X, +1.20D+1.60Lr	23.901	-Z	Bottom	0.5184	AsMin	0.6240	57.250	OK
X-X, +1.20D+L	30.294	+Z	Bottom	0.5184	AsMin	0.6240	57.250	OK
X-X, +1.20D+L	30.294	-Z	Bottom	0.5184	AsMin	0.6240	57.250	OK
X-X, +1.20D	20.986	+Z	Bottom	0.5184	AsMin	0.6240	57.250	OK

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: F-126 (worst)

Footing Flexure

Flexure Axis & Load Combination	Mu k-ft	Side	Tension Surface	As Req'd in^2	Gvrn. As in^2	Actual As in^2	Phi*Mn k-ft	Status
X-X, +1.20D	20.986	-Z	Bottom	0.5184	AsMin	0.6240	57.250	OK
X-X, +1.20D+0.50Lr+L	31.205	+Z	Bottom	0.5184	AsMin	0.6240	57.250	OK
X-X, +1.20D+0.50Lr+L	31.205	-Z	Bottom	0.5184	AsMin	0.6240	57.250	OK
X-X, +0.90D	15.739	+Z	Bottom	0.5184	AsMin	0.6240	57.250	OK
X-X, +0.90D	15.739	-Z	Bottom	0.5184	AsMin	0.6240	57.250	OK
Z-Z, +1.40D	24.484	-X	Bottom	0.5184	AsMin	0.6240	57.250	OK
Z-Z, +1.40D	24.484	+X	Bottom	0.5184	AsMin	0.6240	57.250	OK
Z-Z, +1.20D+0.50Lr+1.60L	36.789	-X	Bottom	0.5184	AsMin	0.6240	57.250	OK
Z-Z, +1.20D+0.50Lr+1.60L	36.789	+X	Bottom	0.5184	AsMin	0.6240	57.250	OK
Z-Z, +1.20D+1.60L	35.878	-X	Bottom	0.5184	AsMin	0.6240	57.250	OK
Z-Z, +1.20D+1.60L	35.878	+X	Bottom	0.5184	AsMin	0.6240	57.250	OK
Z-Z, +1.20D+1.60Lr+L	33.208	-X	Bottom	0.5184	AsMin	0.6240	57.250	OK
Z-Z, +1.20D+1.60Lr+L	33.208	+X	Bottom	0.5184	AsMin	0.6240	57.250	OK
Z-Z, +1.20D+1.60Lr	23.901	-X	Bottom	0.5184	AsMin	0.6240	57.250	OK
Z-Z, +1.20D+1.60Lr	23.901	+X	Bottom	0.5184	AsMin	0.6240	57.250	OK
Z-Z, +1.20D+L	30.294	-X	Bottom	0.5184	AsMin	0.6240	57.250	OK
Z-Z, +1.20D+L	30.294	+X	Bottom	0.5184	AsMin	0.6240	57.250	OK
Z-Z, +1.20D	20.986	-X	Bottom	0.5184	AsMin	0.6240	57.250	OK
Z-Z, +1.20D	20.986	+X	Bottom	0.5184	AsMin	0.6240	57.250	OK
Z-Z, +1.20D+0.50Lr+L	31.205	-X	Bottom	0.5184	AsMin	0.6240	57.250	OK
Z-Z, +1.20D+0.50Lr+L	31.205	+X	Bottom	0.5184	AsMin	0.6240	57.250	OK
Z-Z, +0.90D	15.739	-X	Bottom	0.5184	AsMin	0.6240	57.250	OK
Z-Z, +0.90D	15.739	+X	Bottom	0.5184	AsMin	0.6240	57.250	OK

One Way Shear

Load Combination...	Vu @ -X	Vu @ +X	Vu @ -Z	Vu @ +Z	Vu:Max	Phi Vn	Vu / Phi*Vn	Status
+1.40D	23.45 psi	23.45 psi	23.45 psi	23.45 psi	23.45 psi	82.16 psi	0.29	OK
+1.20D+0.50Lr+1.60L	35.24 psi	35.24 psi	35.24 psi	35.24 psi	35.24 psi	82.16 psi	0.43	OK
+1.20D+1.60L	34.36 psi	34.36 psi	34.36 psi	34.36 psi	34.36 psi	82.16 psi	0.42	OK
+1.20D+1.60Lr+L	31.81 psi	31.81 psi	31.81 psi	31.81 psi	31.81 psi	82.16 psi	0.39	OK
+1.20D+1.60Lr	22.89 psi	22.89 psi	22.89 psi	22.89 psi	22.89 psi	82.16 psi	0.28	OK
+1.20D+L	29.02 psi	29.02 psi	29.02 psi	29.02 psi	29.02 psi	82.16 psi	0.35	OK
+1.20D	20.10 psi	20.10 psi	20.10 psi	20.10 psi	20.10 psi	82.16 psi	0.24	OK
+1.20D+0.50Lr+L	29.89 psi	29.89 psi	29.89 psi	29.89 psi	29.89 psi	82.16 psi	0.36	OK
+0.90D	15.08 psi	15.08 psi	15.08 psi	15.08 psi	15.08 psi	82.16 psi	0.18	OK

Two-Way "Punching" Shear

All units k

Load Combination...	Vu	Phi*Vn	Vu / Phi*Vn	Status
+1.40D	76.38 psi	164.32psi	0.4648	OK
+1.20D+0.50Lr+1.60L	114.77 psi	164.32psi	0.6985	OK
+1.20D+1.60L	111.93 psi	164.32psi	0.6812	OK
+1.20D+1.60Lr+L	103.60 psi	164.32psi	0.6305	OK
+1.20D+1.60Lr	74.56 psi	164.32psi	0.4538	OK
+1.20D+L	94.51 psi	164.32psi	0.5751	OK
+1.20D	65.47 psi	164.32psi	0.3984	OK
+1.20D+0.50Lr+L	97.35 psi	164.32psi	0.5924	OK
+0.90D	49.10 psi	164.32psi	0.2988	OK

Project Title:
Engineer:
Project ID:
Project Descr:

Combined Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: F-240160 (worst)

Code References

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : ASCE 7-16

General Information

Material Properties

f'c : Concrete 28 day strength	3.0	ksi
fy : Rebar Yield	60.0	ksi
Ec : Concrete Elastic Modulus	3,122.0	ksi
Concrete Density	145.0	pcf
ϕ : Phi Values Flexure :	0.90	
Shear :	0.750	

Analysis/Design Settings

Calculate footing weight as dead load ?	Yes
Calculate Pedestal weight as dead load ?	No
Min Steel % Bending Reinf (based on 'd')	
Min Allow % Temp Reinf (based on thick)	0.00180
Min. Overturning Safety Factor	1.0: 1
Min. Sliding Safety Factor	1.0: 1

Soil Information

Allowable Soil Bearing	2.50	ksf
Increase Bearing By Footing Weight	No	
Soil Passive Sliding Resistance	250.0	pcf
(Uses entry for "Footing base depth below soil surface" for force)		
Coefficient of Soil/Concrete Friction	0.30	

Soil Bearing Increase

Footing base depth below soil surface	ft
Increases based on footing Depth	
Allowable pressure increase per foot	ksf
when base of footing is below	ft
Increases based on footing Width . . .	
Allowable pressure increase per foot	ksf
when maximum length or width is greater tha	ft
Maximum Allowed Bearing Pressure	10.0 ksf
(A value of zero implies no limit)	
Adjusted Allowable Soil Bearing	2.50 ksf
(Allowable Soil Bearing adjusted for footing weight and depth & width increases as specified by user.)	

Dimensions & Reinforcing

Distance Left of Column #1	=	5.0 ft
Between Columns	=	12.580 ft
Distance Right of Column #2	=	6.420 ft
Total Footing Length	=	24.0 ft
Footing Width	=	16.0 ft
Footing Thickness	=	18.0 in
Rebar Center to Concrete Edge @ Top	=	3.0 in
Rebar Center to Concrete Edge @ Bottom	=	3.0 in

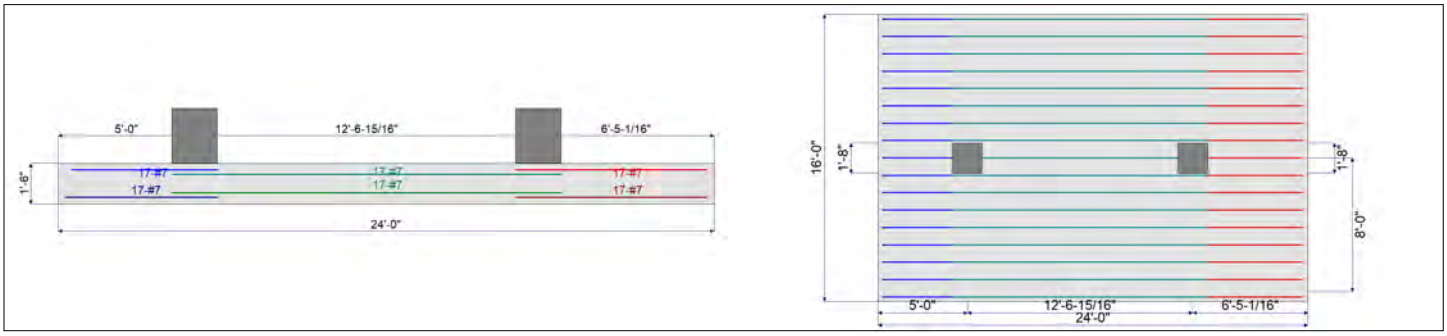
Pedestal dimensions...

		Col #1	Col #2	
Sq. Dim.	=	20.0	20.0	in
Height	=	24.0	24.0	in

	Count	Size #	As Provided	As Req'd
Bars left of Col #1				
Bottom Bars	17.0	7	10.20	6.221 in^2
Top Bars	17.0	7	10.20	0.0 in^2
Bars Btwn Cols				
Bottom Bars	17.0	7	10.20	9.880 in^2
Top Bars	17.0	7	10.20	6.221 in^2
Bars Right of Col #2				
Bottom Bars	17.0	7	10.20	9.840 in^2
Top Bars	17.0	7	10.20	0.0 in^2

Applied Loads

Applied @ Left Column		**D**	**Lr**	**L**	**S**	**W**	**E**	**H**	
Axial Load Downward	=	187.830	19.480	11.880					k
Moment (+CW)	=								k-ft
Shear (+X)	=								k
Applied @ Right Column									
Axial Load Downward	=	302.140	19.740	84.220					k
Moment (+CW)	=								k-ft
Shear (+X)	=								k
Overburden	=	0.050		0.040					



Project Title:
Engineer:
Project ID:
Project Descr:

Combined Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: F-240160 (worst)

DESIGN SUMMARY

Design OK

Factor of Safety	Item	Applied	Capacity	Governing Load Combination
PASS No OTM	Overturning	0.0 k-ft	0.0 k-ft	No OTM
PASS No Sliding	Sliding	0.0 k	177.724 k	No Sliding
PASS No Uplift	Uplift	0.0 k	0.0 k	No Uplift

Utilization Ratio	Item	Applied	Capacity	Governing Load Combination
PASS 0.9299	Soil Bearing	2.325 ksf	2.50 ksf	+D+L
PASS 0.3768	1-way Shear - Col #1	30.953 psi	82.158 psi	+1.20D+1.60Lr+L
PASS 0.7460	1-way Shear - Col #2	61.290 psi	82.158 psi	+1.20D+0.50Lr+1.60L
PASS 0.7353	2-way Punching - Col #1	120.822 psi	164.317 psi	+1.20D+1.60Lr+L
PASS 0.7230	2-way Punching - Col #2	118.795 psi	164.317 psi	+1.20D+1.60Lr+L
PASS No Bending	Flexure - Left of Col #1 - Top	0.0 k-ft	0.0 k-ft	N/A
PASS 0.3092	Flexure - Left of Col #1 - Bottom	204.011 k-ft	659.81 k-ft	+1.20D+1.60Lr+L
PASS 0.1273	Flexure - Between Cols - Top	-83.970 k-ft	659.81 k-ft	+1.20D+1.60Lr+L
PASS 0.9699	Flexure - Between Cols - Bottom	639.96 k-ft	659.81 k-ft	+1.20D+0.50Lr+1.60L
PASS No Bending	Flexure - Right of Col #2 - Top	0.0 k-ft	0.0 k-ft	N/A
PASS 0.9662	Flexure - Right of Col #2 - Bottom	637.52 k-ft	659.81 k-ft	+1.20D+0.50Lr+1.60L

Soil Bearing

Load Combination...	Total Bearing	Eccentricity from Ftg CL	Actual Soil Bearing Stress @ Left Edge	@ Right Edge	Allowable	Actual / Allow Ratio
D Only	592.41 k	0.627 ft	1.30 ksf	1.78 ksf	2.50 ksf	0.714
+D+L	703.65 k	1.078 ft	1.34 ksf	2.32 ksf	2.50 ksf	0.930
+D+Lr	631.63 k	0.546 ft	1.42 ksf	1.87 ksf	2.50 ksf	0.748
+D+0.750Lr+0.750L	705.26 k	0.910 ft	1.42 ksf	2.25 ksf	2.50 ksf	0.901
+D+0.750L	675.84 k	0.979 ft	1.33 ksf	2.19 ksf	2.50 ksf	0.876
+0.60D	355.45 k	0.627 ft	0.78 ksf	1.07 ksf	2.50 ksf	0.428

Overturning Stability

Load Combination...	Moments about Left Edge k-ft Overturning	Resisting	Ratio	Moments about Right Edg k-ft Overturning	Resisting	Ratio
D Only	0.00	0.00	999.000	0.00	0.00	999.000
+D+L	0.00	0.00	999.000	0.00	0.00	999.000
+D+Lr	0.00	0.00	999.000	0.00	0.00	999.000
+D+0.750Lr+0.750L	0.00	0.00	999.000	0.00	0.00	999.000
+D+0.750L	0.00	0.00	999.000	0.00	0.00	999.000
+0.60D	0.00	0.00	999.000	0.00	0.00	999.000

Sliding Stability

Load Combination...	Sliding Force	Resisting Force	Sliding Safety Ratio
D Only	0.00 k	177.72 k	999
+D+L	0.00 k	211.10 k	999
+D+Lr	0.00 k	189.49 k	999
+D+0.750Lr+0.750L	0.00 k	211.58 k	999
+D+0.750L	0.00 k	202.75 k	999
+0.60D	0.00 k	106.63 k	999

Z-Axis Footing Flexure - Maximum Values for Load Combination

Load Combination...	Mu (ft-k)	Distance from left (ft)	Tension Side	As Req'd (in^2)	Governed by	Actual As (in^2)	Phi*Mn (ft-k)	Mu / PhiMn
+0.60D	0.000	0.000	0	0.000	0	0.000	0.000	0.000
+1.40D	0.042	0.060	Bottom	6.221	Min Temp %	10.200	659.813	0.000
+1.40D	0.167	0.120	Bottom	6.221	Min Temp %	10.200	659.813	0.000
+1.40D	0.375	0.180	Bottom	6.221	Min Temp %	10.200	659.813	0.001
+1.40D	0.668	0.240	Bottom	6.221	Min Temp %	10.200	659.813	0.001
+1.40D	1.044	0.300	Bottom	6.221	Min Temp %	10.200	659.813	0.002
+1.40D	1.504	0.360	Bottom	6.221	Min Temp %	10.200	659.813	0.002
+1.40D	2.047	0.420	Bottom	6.221	Min Temp %	10.200	659.813	0.003
+1.40D	2.675	0.480	Bottom	6.221	Min Temp %	10.200	659.813	0.004

Project Title:
Engineer:
Project ID:
Project Descr:

Combined Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: F-240160 (worst)

Z-Axis Footing Flexure - Maximum Values for Load Combination

Load Combination...	Mu (ft-k)	Distance from left (ft)	Tension Side	As Req'd (in^2)	Governed by	Actual As (in^2)	Phi*Mn (ft-k)	Mu / PhiMn
+1.40D	3.387	0.540	Bottom	6.221	Min Temp %	10.200	659.813	0.005
+1.40D	4.183	0.600	Bottom	6.221	Min Temp %	10.200	659.813	0.006
+1.40D	5.064	0.660	Bottom	6.221	Min Temp %	10.200	659.813	0.008
+1.40D	6.029	0.720	Bottom	6.221	Min Temp %	10.200	659.813	0.009
+1.40D	7.078	0.780	Bottom	6.221	Min Temp %	10.200	659.813	0.011
+1.40D	8.212	0.840	Bottom	6.221	Min Temp %	10.200	659.813	0.012
+1.40D	9.431	0.900	Bottom	6.221	Min Temp %	10.200	659.813	0.014
+1.40D	10.734	0.960	Bottom	6.221	Min Temp %	10.200	659.813	0.016
+1.40D	12.123	1.020	Bottom	6.221	Min Temp %	10.200	659.813	0.018
+1.40D	13.596	1.080	Bottom	6.221	Min Temp %	10.200	659.813	0.021
+1.40D	15.154	1.140	Bottom	6.221	Min Temp %	10.200	659.813	0.023
+1.40D	16.798	1.200	Bottom	6.221	Min Temp %	10.200	659.813	0.025
+1.40D	18.527	1.260	Bottom	6.221	Min Temp %	10.200	659.813	0.028
+1.40D	20.341	1.320	Bottom	6.221	Min Temp %	10.200	659.813	0.031
+1.40D	22.241	1.380	Bottom	6.221	Min Temp %	10.200	659.813	0.034
+1.40D	24.227	1.440	Bottom	6.221	Min Temp %	10.200	659.813	0.037
+1.40D	26.298	1.500	Bottom	6.221	Min Temp %	10.200	659.813	0.040
+1.40D	28.455	1.560	Bottom	6.221	Min Temp %	10.200	659.813	0.043
+1.40D	30.697	1.620	Bottom	6.221	Min Temp %	10.200	659.813	0.047
+1.40D	33.026	1.680	Bottom	6.221	Min Temp %	10.200	659.813	0.050
+1.40D	35.441	1.740	Bottom	6.221	Min Temp %	10.200	659.813	0.054
+1.40D	37.942	1.800	Bottom	6.221	Min Temp %	10.200	659.813	0.058
+1.40D	40.529	1.860	Bottom	6.221	Min Temp %	10.200	659.813	0.061
+1.40D	43.203	1.920	Bottom	6.221	Min Temp %	10.200	659.813	0.065
+1.40D	45.963	1.980	Bottom	6.221	Min Temp %	10.200	659.813	0.070
+1.40D	48.809	2.040	Bottom	6.221	Min Temp %	10.200	659.813	0.074
+1.40D	51.743	2.100	Bottom	6.221	Min Temp %	10.200	659.813	0.078
+1.40D	54.763	2.160	Bottom	6.221	Min Temp %	10.200	659.813	0.083
+1.40D	57.870	2.220	Bottom	6.221	Min Temp %	10.200	659.813	0.088
+1.40D	61.063	2.280	Bottom	6.221	Min Temp %	10.200	659.813	0.093
+1.40D	64.344	2.340	Bottom	6.221	Min Temp %	10.200	659.813	0.098
+1.40D	67.712	2.400	Bottom	6.221	Min Temp %	10.200	659.813	0.103
+1.40D	71.167	2.460	Bottom	6.221	Min Temp %	10.200	659.813	0.108
+1.40D	74.710	2.520	Bottom	6.221	Min Temp %	10.200	659.813	0.113
+1.40D	78.340	2.580	Bottom	6.221	Min Temp %	10.200	659.813	0.119
+1.40D	82.058	2.640	Bottom	6.221	Min Temp %	10.200	659.813	0.124
+1.40D	85.863	2.700	Bottom	6.221	Min Temp %	10.200	659.813	0.130
+1.40D	89.756	2.760	Bottom	6.221	Min Temp %	10.200	659.813	0.136
+1.40D	93.736	2.820	Bottom	6.221	Min Temp %	10.200	659.813	0.142
+1.40D	97.805	2.880	Bottom	6.221	Min Temp %	10.200	659.813	0.148
+1.40D	101.962	2.940	Bottom	6.221	Min Temp %	10.200	659.813	0.155
+1.40D	106.206	3.000	Bottom	6.221	Min Temp %	10.200	659.813	0.161
+1.40D	110.539	3.060	Bottom	6.221	Min Temp %	10.200	659.813	0.168
+1.40D	114.961	3.120	Bottom	6.221	Min Temp %	10.200	659.813	0.174
+1.40D	119.471	3.180	Bottom	6.221	Min Temp %	10.200	659.813	0.181
+1.40D	124.069	3.240	Bottom	6.221	Min Temp %	10.200	659.813	0.188
+1.40D	128.756	3.300	Bottom	6.221	Min Temp %	10.200	659.813	0.195
+1.40D	133.531	3.360	Bottom	6.221	Min Temp %	10.200	659.813	0.202
+1.40D	138.395	3.420	Bottom	6.221	Min Temp %	10.200	659.813	0.210
+1.40D	143.349	3.480	Bottom	6.221	Min Temp %	10.200	659.813	0.217
+1.40D	148.391	3.540	Bottom	6.221	Min Temp %	10.200	659.813	0.225
+1.40D	153.522	3.600	Bottom	6.221	Min Temp %	10.200	659.813	0.233
+1.40D	158.743	3.660	Bottom	6.221	Min Temp %	10.200	659.813	0.241
+1.40D	164.052	3.720	Bottom	6.221	Min Temp %	10.200	659.813	0.249
+1.20D+1.60Lr+L	169.485	3.780	Bottom	6.221	Min Temp %	10.200	659.813	0.257
+1.20D+1.60Lr+L	175.010	3.840	Bottom	6.221	Min Temp %	10.200	659.813	0.265
+1.20D+1.60Lr+L	180.626	3.900	Bottom	6.221	Min Temp %	10.200	659.813	0.274
+1.20D+1.60Lr+L	186.334	3.960	Bottom	6.221	Min Temp %	10.200	659.813	0.282
+1.20D+1.60Lr+L	192.134	4.020	Bottom	6.221	Min Temp %	10.200	659.813	0.291
+1.20D+1.60Lr+L	198.026	4.080	Bottom	6.221	Min Temp %	10.200	659.813	0.300
+1.20D+1.60Lr+L	204.011	4.140	Bottom	6.221	Min Temp %	10.200	659.813	0.309
+1.20D+1.60Lr+L	209.999	4.200	Bottom	6.221	Min Temp %	10.200	659.813	0.318
+1.20D+1.60Lr+L	215.558	4.260	Bottom	6.221	Min Temp %	10.200	659.813	0.327

Project Title:
Engineer:
Project ID:
Project Descr:

Combined Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: F-240160 (worst)

Z-Axis Footing Flexure - Maximum Values for Load Combination

Load Combination...	Mu (ft-k)	Distance from left (ft)	Tension Side	As Req'd (in^2)	Governed by	Actual As (in^2)	Phi*Mn (ft-k)	Mu / PhiMn
+1.20D+1.60Lr+L	220.630	4.320	Bottom	6.221	Min Temp %	10.200	659.813	0.334
+1.20D+1.60Lr+L	225.215	4.380	Bottom	6.221	Min Temp %	10.200	659.813	0.341
+1.20D+1.60Lr+L	229.315	4.440	Bottom	6.221	Min Temp %	10.200	659.813	0.348
+1.20D+1.60Lr+L	232.929	4.500	Bottom	6.221	Min Temp %	10.200	659.813	0.353
+1.20D+1.60Lr+L	236.057	4.560	Bottom	6.221	Min Temp %	10.200	659.813	0.358
+1.20D+1.60Lr+L	238.699	4.620	Bottom	6.221	Min Temp %	10.200	659.813	0.362
+1.20D+1.60Lr+L	240.856	4.680	Bottom	6.221	Min Temp %	10.200	659.813	0.365
+1.20D+1.60Lr+L	242.527	4.740	Bottom	6.221	Min Temp %	10.200	659.813	0.368
+1.20D+1.60Lr+L	243.713	4.800	Bottom	6.221	Min Temp %	10.200	659.813	0.369
+1.20D+1.60Lr+L	244.414	4.860	Bottom	6.221	Min Temp %	10.200	659.813	0.370
+1.20D+1.60Lr+L	244.630	4.920	Bottom	6.221	Min Temp %	10.200	659.813	0.371
+1.20D+1.60Lr+L	244.361	4.980	Bottom	6.221	Min Temp %	10.200	659.813	0.370
+1.20D+1.60Lr+L	243.608	5.040	Bottom	6.221	Min Temp %	10.200	659.813	0.369
+1.40D	242.375	5.100	Bottom	6.221	Min Temp %	10.200	659.813	0.367
+1.40D	240.744	5.160	Bottom	6.221	Min Temp %	10.200	659.813	0.365
+1.40D	238.637	5.220	Bottom	6.221	Min Temp %	10.200	659.813	0.362
+1.40D	236.055	5.280	Bottom	6.221	Min Temp %	10.200	659.813	0.358
+1.40D	232.997	5.340	Bottom	6.221	Min Temp %	10.200	659.813	0.353
+1.40D	229.463	5.400	Bottom	6.221	Min Temp %	10.200	659.813	0.348
+1.40D	225.454	5.460	Bottom	6.221	Min Temp %	10.200	659.813	0.342
+1.40D	220.969	5.520	Bottom	6.221	Min Temp %	10.200	659.813	0.335
+1.40D	216.009	5.580	Bottom	6.221	Min Temp %	10.200	659.813	0.327
+1.40D	210.574	5.640	Bottom	6.221	Min Temp %	10.200	659.813	0.319
+1.40D	204.664	5.700	Bottom	6.221	Min Temp %	10.200	659.813	0.310
+1.40D	198.279	5.760	Bottom	6.221	Min Temp %	10.200	659.813	0.301
+1.40D	191.419	5.820	Bottom	6.221	Min Temp %	10.200	659.813	0.290
+1.40D	184.256	5.880	Bottom	6.221	Min Temp %	10.200	659.813	0.279
+1.40D	177.172	5.940	Bottom	6.221	Min Temp %	10.200	659.813	0.269
+1.40D	170.181	6.000	Bottom	6.221	Min Temp %	10.200	659.813	0.258
+1.40D	163.283	6.060	Bottom	6.221	Min Temp %	10.200	659.813	0.247
+1.40D	156.478	6.120	Bottom	6.221	Min Temp %	10.200	659.813	0.237
+1.40D	149.767	6.180	Bottom	6.221	Min Temp %	10.200	659.813	0.227
+1.40D	143.148	6.240	Bottom	6.221	Min Temp %	10.200	659.813	0.217
+1.40D	136.624	6.300	Bottom	6.221	Min Temp %	10.200	659.813	0.207
+1.40D	130.193	6.360	Bottom	6.221	Min Temp %	10.200	659.813	0.197
+1.40D	123.855	6.420	Bottom	6.221	Min Temp %	10.200	659.813	0.188
+1.40D	117.611	6.480	Bottom	6.221	Min Temp %	10.200	659.813	0.178
+1.40D	111.461	6.540	Bottom	6.221	Min Temp %	10.200	659.813	0.169
+1.40D	105.406	6.600	Bottom	6.221	Min Temp %	10.200	659.813	0.160
+1.40D	99.444	6.660	Bottom	6.221	Min Temp %	10.200	659.813	0.151
+1.20D+1.60Lr	93.659	6.720	Bottom	6.221	Min Temp %	10.200	659.813	0.142
+1.20D+1.60Lr	88.026	6.780	Bottom	6.221	Min Temp %	10.200	659.813	0.133
+1.20D+1.60Lr	82.484	6.840	Bottom	6.221	Min Temp %	10.200	659.813	0.125
+1.20D+1.60Lr	77.034	6.900	Bottom	6.221	Min Temp %	10.200	659.813	0.117
+1.20D+1.60Lr	71.674	6.960	Bottom	6.221	Min Temp %	10.200	659.813	0.109
+1.20D+1.60Lr	66.406	7.020	Bottom	6.221	Min Temp %	10.200	659.813	0.101
+1.20D+1.60Lr	61.229	7.080	Bottom	6.221	Min Temp %	10.200	659.813	0.093
+1.20D+1.60Lr	56.143	7.140	Bottom	6.221	Min Temp %	10.200	659.813	0.085
+1.20D+1.60Lr	51.149	7.200	Bottom	6.221	Min Temp %	10.200	659.813	0.078
+1.20D+1.60Lr	46.247	7.260	Bottom	6.221	Min Temp %	10.200	659.813	0.070
+1.20D+1.60Lr	41.435	7.320	Bottom	6.221	Min Temp %	10.200	659.813	0.063
+1.20D+1.60Lr	36.716	7.380	Bottom	6.221	Min Temp %	10.200	659.813	0.056
+1.20D+1.60Lr	32.088	7.440	Bottom	6.221	Min Temp %	10.200	659.813	0.049
+1.20D+1.60Lr	27.552	7.500	Bottom	6.221	Min Temp %	10.200	659.813	0.042
+1.20D+1.60Lr	23.108	7.560	Bottom	6.221	Min Temp %	10.200	659.813	0.035
+1.20D+1.60Lr	18.756	7.620	Bottom	6.221	Min Temp %	10.200	659.813	0.028
+1.20D+1.60Lr	14.496	7.680	Bottom	6.221	Min Temp %	10.200	659.813	0.022
+1.20D+1.60Lr	10.328	7.740	Bottom	6.221	Min Temp %	10.200	659.813	0.016
+1.20D+1.60L	-8.385	7.800	Top	6.221	Min Temp %	10.200	659.813	0.013
+1.20D+1.60L	-12.033	7.860	Top	6.221	Min Temp %	10.200	659.813	0.018
+1.20D+1.60L	-15.584	7.920	Top	6.221	Min Temp %	10.200	659.813	0.024
+1.20D+1.60L	-19.038	7.980	Top	6.221	Min Temp %	10.200	659.813	0.029
+1.20D+1.60L	-22.393	8.040	Top	6.221	Min Temp %	10.200	659.813	0.034

Project Title:
Engineer:
Project ID:
Project Descr:

Combined Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: F-240160 (worst)

Z-Axis Footing Flexure - Maximum Values for Load Combination

Load Combination...	Mu (ft-k)	Distance from left (ft)	Tension Side	As Req'd (in^2)	Governed by	Actual As (in^2)	Phi*Mn (ft-k)	Mu / PhiMn
+1.20D+1.60L	-25.651	8.100	Top	6.221	Min Temp %	10.200	659.813	0.039
+1.20D+0.50Lr+1.60L	-28.871	8.160	Top	6.221	Min Temp %	10.200	659.813	0.044
+1.20D+0.50Lr+1.60L	-32.070	8.220	Top	6.221	Min Temp %	10.200	659.813	0.049
+1.20D+0.50Lr+1.60L	-35.169	8.280	Top	6.221	Min Temp %	10.200	659.813	0.053
+1.20D+0.50Lr+1.60L	-38.165	8.340	Top	6.221	Min Temp %	10.200	659.813	0.058
+1.20D+0.50Lr+1.60L	-41.059	8.400	Top	6.221	Min Temp %	10.200	659.813	0.062
+1.20D+0.50Lr+1.60L	-43.851	8.460	Top	6.221	Min Temp %	10.200	659.813	0.066
+1.20D+0.50Lr+1.60L	-46.540	8.520	Top	6.221	Min Temp %	10.200	659.813	0.071
+1.20D+0.50Lr+1.60L	-49.127	8.580	Top	6.221	Min Temp %	10.200	659.813	0.074
+1.20D+0.50Lr+1.60L	-51.611	8.640	Top	6.221	Min Temp %	10.200	659.813	0.078
+1.20D+0.50Lr+1.60L	-53.991	8.700	Top	6.221	Min Temp %	10.200	659.813	0.082
+1.20D+0.50Lr+1.60L	-56.269	8.760	Top	6.221	Min Temp %	10.200	659.813	0.085
+1.20D+0.50Lr+1.60L	-58.443	8.820	Top	6.221	Min Temp %	10.200	659.813	0.089
+1.20D+0.50Lr+1.60L	-60.514	8.880	Top	6.221	Min Temp %	10.200	659.813	0.092
+1.20D+0.50Lr+1.60L	-62.480	8.940	Top	6.221	Min Temp %	10.200	659.813	0.095
+1.20D+0.50Lr+1.60L	-64.343	9.000	Top	6.221	Min Temp %	10.200	659.813	0.098
+1.20D+0.50Lr+1.60L	-66.101	9.060	Top	6.221	Min Temp %	10.200	659.813	0.100
+1.20D+0.50Lr+1.60L	-67.755	9.120	Top	6.221	Min Temp %	10.200	659.813	0.103
+1.20D+0.50Lr+1.60L	-69.304	9.180	Top	6.221	Min Temp %	10.200	659.813	0.105
+1.20D+1.60Lr+L	-70.978	9.240	Top	6.221	Min Temp %	10.200	659.813	0.108
+1.20D+1.60Lr+L	-72.585	9.300	Top	6.221	Min Temp %	10.200	659.813	0.110
+1.20D+1.60Lr+L	-74.086	9.360	Top	6.221	Min Temp %	10.200	659.813	0.112
+1.20D+1.60Lr+L	-75.482	9.420	Top	6.221	Min Temp %	10.200	659.813	0.114
+1.20D+1.60Lr+L	-76.773	9.480	Top	6.221	Min Temp %	10.200	659.813	0.116
+1.20D+1.60Lr+L	-77.958	9.540	Top	6.221	Min Temp %	10.200	659.813	0.118
+1.20D+1.60Lr+L	-79.037	9.600	Top	6.221	Min Temp %	10.200	659.813	0.120
+1.20D+1.60Lr+L	-80.009	9.660	Top	6.221	Min Temp %	10.200	659.813	0.121
+1.20D+1.60Lr+L	-80.876	9.720	Top	6.221	Min Temp %	10.200	659.813	0.123
+1.20D+1.60Lr+L	-81.637	9.780	Top	6.221	Min Temp %	10.200	659.813	0.124
+1.20D+1.60Lr+L	-82.290	9.840	Top	6.221	Min Temp %	10.200	659.813	0.125
+1.20D+1.60Lr+L	-82.838	9.900	Top	6.221	Min Temp %	10.200	659.813	0.126
+1.20D+1.60Lr+L	-83.278	9.960	Top	6.221	Min Temp %	10.200	659.813	0.126
+1.20D+1.60Lr+L	-83.612	10.020	Top	6.221	Min Temp %	10.200	659.813	0.127
+1.20D+1.60Lr+L	-83.839	10.080	Top	6.221	Min Temp %	10.200	659.813	0.127
+1.20D+1.60Lr+L	-83.958	10.140	Top	6.221	Min Temp %	10.200	659.813	0.127
+1.20D+1.60Lr+L	-83.970	10.200	Top	6.221	Min Temp %	10.200	659.813	0.127
+1.20D+1.60Lr+L	-83.875	10.260	Top	6.221	Min Temp %	10.200	659.813	0.127
+1.20D+1.60Lr+L	-83.672	10.320	Top	6.221	Min Temp %	10.200	659.813	0.127
+1.20D+1.60Lr+L	-83.361	10.380	Top	6.221	Min Temp %	10.200	659.813	0.126
+1.20D+1.60Lr+L	-82.942	10.440	Top	6.221	Min Temp %	10.200	659.813	0.126
+1.20D+1.60Lr+L	-82.415	10.500	Top	6.221	Min Temp %	10.200	659.813	0.125
+1.20D+1.60Lr+L	-81.780	10.560	Top	6.221	Min Temp %	10.200	659.813	0.124
+1.40D	-81.179	10.620	Top	6.221	Min Temp %	10.200	659.813	0.123
+1.40D	-80.616	10.680	Top	6.221	Min Temp %	10.200	659.813	0.122
+1.40D	-79.953	10.740	Top	6.221	Min Temp %	10.200	659.813	0.121
+1.40D	-79.189	10.800	Top	6.221	Min Temp %	10.200	659.813	0.120
+1.40D	-78.324	10.860	Top	6.221	Min Temp %	10.200	659.813	0.119
+1.40D	-77.359	10.920	Top	6.221	Min Temp %	10.200	659.813	0.117
+1.40D	-76.292	10.980	Top	6.221	Min Temp %	10.200	659.813	0.116
+1.40D	-75.124	11.040	Top	6.221	Min Temp %	10.200	659.813	0.114
+1.40D	-73.854	11.100	Top	6.221	Min Temp %	10.200	659.813	0.112
+1.20D+1.60Lr	-72.531	11.160	Top	6.221	Min Temp %	10.200	659.813	0.110
+1.20D+1.60Lr	-71.227	11.220	Top	6.221	Min Temp %	10.200	659.813	0.108
+1.20D+1.60Lr	-69.827	11.280	Top	6.221	Min Temp %	10.200	659.813	0.106
+1.20D+1.60Lr	-68.330	11.340	Top	6.221	Min Temp %	10.200	659.813	0.104
+1.20D+1.60Lr	-66.736	11.400	Top	6.221	Min Temp %	10.200	659.813	0.101
+1.20D+1.60Lr	-65.045	11.460	Top	6.221	Min Temp %	10.200	659.813	0.099
+1.20D+1.60Lr	-63.258	11.520	Top	6.221	Min Temp %	10.200	659.813	0.096
+1.20D+1.60Lr	-61.373	11.580	Top	6.221	Min Temp %	10.200	659.813	0.093
+1.20D+1.60Lr	-59.392	11.640	Top	6.221	Min Temp %	10.200	659.813	0.090
+1.20D+1.60Lr	-57.313	11.700	Top	6.221	Min Temp %	10.200	659.813	0.087
+1.20D+1.60Lr	-55.138	11.760	Top	6.221	Min Temp %	10.200	659.813	0.084
+1.20D+1.60Lr	-52.865	11.820	Top	6.221	Min Temp %	10.200	659.813	0.080

Project Title:
Engineer:
Project ID:
Project Descr:

Combined Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: F-240160 (worst)

Z-Axis Footing Flexure - Maximum Values for Load Combination

Load Combination...	Mu (ft-k)	Distance from left (ft)	Tension Side	As Req'd (in^2)	Governed by	Actual As (in^2)	Phi*Mn (ft-k)	Mu / PhiMn
+1.20D+1.60Lr	-50.495	11.880	Top	6.221	Min Temp %	10.200	659.813	0.077
+1.20D+1.60Lr	-48.027	11.940	Top	6.221	Min Temp %	10.200	659.813	0.073
+1.20D+1.60Lr	-45.462	12.000	Top	6.221	Min Temp %	10.200	659.813	0.069
+1.20D+1.60Lr	-42.799	12.060	Top	6.221	Min Temp %	10.200	659.813	0.065
+1.20D+1.60Lr	-40.039	12.120	Top	6.221	Min Temp %	10.200	659.813	0.061
+1.20D+1.60Lr	-37.181	12.180	Top	6.221	Min Temp %	10.200	659.813	0.056
+1.20D+1.60Lr	-34.225	12.240	Top	6.221	Min Temp %	10.200	659.813	0.052
+1.20D+1.60Lr	-31.171	12.300	Top	6.221	Min Temp %	10.200	659.813	0.047
+1.20D+1.60Lr	-28.019	12.360	Top	6.221	Min Temp %	10.200	659.813	0.042
+1.20D+1.60Lr	-24.770	12.420	Top	6.221	Min Temp %	10.200	659.813	0.038
+1.20D+1.60Lr	-21.422	12.480	Top	6.221	Min Temp %	10.200	659.813	0.032
+1.20D+1.60L	19.215	12.540	Bottom	6.221	Min Temp %	10.200	659.813	0.029
+1.20D+1.60L	23.874	12.600	Bottom	6.221	Min Temp %	10.200	659.813	0.036
+1.20D+1.60L	28.647	12.660	Bottom	6.221	Min Temp %	10.200	659.813	0.043
+1.20D+1.60L	33.534	12.720	Bottom	6.221	Min Temp %	10.200	659.813	0.051
+1.20D+1.60L	38.534	12.780	Bottom	6.221	Min Temp %	10.200	659.813	0.058
+1.20D+1.60L	43.647	12.840	Bottom	6.221	Min Temp %	10.200	659.813	0.066
+1.20D+1.60L	48.875	12.900	Bottom	6.221	Min Temp %	10.200	659.813	0.074
+1.20D+1.60L	54.217	12.960	Bottom	6.221	Min Temp %	10.200	659.813	0.082
+1.20D+1.60L	59.673	13.020	Bottom	6.221	Min Temp %	10.200	659.813	0.090
+1.20D+1.60L	65.243	13.080	Bottom	6.221	Min Temp %	10.200	659.813	0.099
+1.20D+1.60L	70.929	13.140	Bottom	6.221	Min Temp %	10.200	659.813	0.107
+1.20D+1.60L	76.729	13.200	Bottom	6.221	Min Temp %	10.200	659.813	0.116
+1.20D+1.60L	82.644	13.260	Bottom	6.221	Min Temp %	10.200	659.813	0.125
+1.20D+1.60L	88.675	13.320	Bottom	6.221	Min Temp %	10.200	659.813	0.134
+1.20D+1.60L	94.822	13.380	Bottom	6.221	Min Temp %	10.200	659.813	0.144
+1.20D+1.60L	101.084	13.440	Bottom	6.221	Min Temp %	10.200	659.813	0.153
+1.20D+1.60L	107.462	13.500	Bottom	6.221	Min Temp %	10.200	659.813	0.163
+1.20D+1.60L	113.956	13.560	Bottom	6.221	Min Temp %	10.200	659.813	0.173
+1.20D+1.60L	120.566	13.620	Bottom	6.221	Min Temp %	10.200	659.813	0.183
+1.20D+1.60L	127.294	13.680	Bottom	6.221	Min Temp %	10.200	659.813	0.193
+1.20D+1.60L	134.137	13.740	Bottom	6.221	Min Temp %	10.200	659.813	0.203
+1.20D+0.50Lr+1.60L	141.205	13.800	Bottom	6.221	Min Temp %	10.200	659.813	0.214
+1.20D+0.50Lr+1.60L	148.424	13.860	Bottom	6.221	Min Temp %	10.200	659.813	0.225
+1.20D+0.50Lr+1.60L	155.764	13.920	Bottom	6.221	Min Temp %	10.200	659.813	0.236
+1.20D+0.50Lr+1.60L	163.224	13.980	Bottom	6.221	Min Temp %	10.200	659.813	0.247
+1.20D+0.50Lr+1.60L	170.805	14.040	Bottom	6.221	Min Temp %	10.200	659.813	0.259
+1.20D+0.50Lr+1.60L	178.507	14.100	Bottom	6.221	Min Temp %	10.200	659.813	0.271
+1.20D+0.50Lr+1.60L	186.329	14.160	Bottom	6.221	Min Temp %	10.200	659.813	0.282
+1.20D+0.50Lr+1.60L	194.273	14.220	Bottom	6.221	Min Temp %	10.200	659.813	0.294
+1.20D+0.50Lr+1.60L	202.338	14.280	Bottom	6.221	Min Temp %	10.200	659.813	0.307
+1.20D+0.50Lr+1.60L	210.525	14.340	Bottom	6.221	Min Temp %	10.200	659.813	0.319
+1.20D+0.50Lr+1.60L	218.833	14.400	Bottom	6.221	Min Temp %	10.200	659.813	0.332
+1.20D+0.50Lr+1.60L	227.264	14.460	Bottom	6.221	Min Temp %	10.200	659.813	0.344
+1.20D+0.50Lr+1.60L	235.817	14.520	Bottom	6.221	Min Temp %	10.200	659.813	0.357
+1.20D+0.50Lr+1.60L	244.492	14.580	Bottom	6.221	Min Temp %	10.200	659.813	0.371
+1.20D+0.50Lr+1.60L	253.289	14.640	Bottom	6.221	Min Temp %	10.200	659.813	0.384
+1.20D+0.50Lr+1.60L	262.210	14.700	Bottom	6.221	Min Temp %	10.200	659.813	0.397
+1.20D+0.50Lr+1.60L	271.253	14.760	Bottom	6.221	Min Temp %	10.200	659.813	0.411
+1.20D+0.50Lr+1.60L	280.420	14.820	Bottom	6.221	Min Temp %	10.200	659.813	0.425
+1.20D+0.50Lr+1.60L	289.710	14.880	Bottom	6.221	Min Temp %	10.200	659.813	0.439
+1.20D+0.50Lr+1.60L	299.123	14.940	Bottom	6.221	Min Temp %	10.200	659.813	0.453
+1.20D+0.50Lr+1.60L	308.660	15.000	Bottom	6.221	Min Temp %	10.200	659.813	0.468
+1.20D+0.50Lr+1.60L	318.322	15.060	Bottom	6.414	Min ACI 10.5	10.200	659.813	0.482
+1.20D+0.50Lr+1.60L	328.107	15.120	Bottom	6.615	Min ACI 10.5	10.200	659.813	0.497
+1.20D+0.50Lr+1.60L	338.017	15.180	Bottom	6.819	Min ACI 10.5	10.200	659.813	0.512
+1.20D+0.50Lr+1.60L	348.051	15.240	Bottom	7.026	Min ACI 10.5	10.200	659.813	0.527
+1.20D+0.50Lr+1.60L	358.210	15.300	Bottom	7.236	Min ACI 10.5	10.200	659.813	0.543
+1.20D+0.50Lr+1.60L	368.494	15.360	Bottom	7.449	Min ACI 10.5	10.200	659.813	0.558
+1.20D+0.50Lr+1.60L	378.903	15.420	Bottom	7.664	Min ACI 10.5	10.200	659.813	0.574
+1.20D+0.50Lr+1.60L	389.437	15.480	Bottom	7.883	Min ACI 10.5	10.200	659.813	0.590
+1.20D+0.50Lr+1.60L	400.097	15.540	Bottom	8.104	Min ACI 10.5	10.200	659.813	0.606
+1.20D+0.50Lr+1.60L	410.883	15.600	Bottom	8.329	Min ACI 10.5	10.200	659.813	0.623

Project Title:
Engineer:
Project ID:
Project Descr:

Combined Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: F-240160 (worst)

Z-Axis Footing Flexure - Maximum Values for Load Combination

Load Combination...	Mu (ft-k)	Distance from left (ft)	Tension Side	As Req'd (in^2)	Governed by	Actual As (in^2)	Phi*Mn (ft-k)	Mu / PhiMn
+1.20D+0.50Lr+1.60L	421.794	15.660	Bottom	8.556	Min ACI 10.5	10.200	659.813	0.639
+1.20D+0.50Lr+1.60L	432.832	15.720	Bottom	8.786	Min ACI 10.5	10.200	659.813	0.656
+1.20D+0.50Lr+1.60L	443.996	15.780	Bottom	9.020	Min ACI 10.5	10.200	659.813	0.673
+1.20D+0.50Lr+1.60L	455.287	15.840	Bottom	9.256	Min ACI 10.5	10.200	659.813	0.690
+1.20D+0.50Lr+1.60L	466.704	15.900	Bottom	9.495	Min ACI 10.5	10.200	659.813	0.707
+1.20D+0.50Lr+1.60L	478.248	15.960	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.725
+1.20D+0.50Lr+1.60L	489.919	16.020	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.743
+1.20D+0.50Lr+1.60L	501.718	16.080	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.760
+1.20D+0.50Lr+1.60L	513.644	16.140	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.778
+1.20D+0.50Lr+1.60L	525.698	16.200	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.797
+1.20D+0.50Lr+1.60L	537.880	16.260	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.815
+1.20D+0.50Lr+1.60L	550.189	16.320	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.834
+1.20D+0.50Lr+1.60L	562.628	16.380	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.853
+1.20D+0.50Lr+1.60L	575.194	16.440	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.872
+1.20D+0.50Lr+1.60L	587.889	16.500	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.891
+1.20D+0.50Lr+1.60L	600.714	16.560	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.910
+1.20D+0.50Lr+1.60L	613.667	16.620	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.930
+1.20D+0.50Lr+1.60L	626.749	16.680	Bottom	9.667	Min for Bending	10.200	659.813	0.950
+1.20D+0.50Lr+1.60L	639.961	16.740	Bottom	9.880	Min ACI 10.5	10.200	659.813	0.970
+1.20D+0.50Lr+1.60L	652.871	16.800	Bottom	10.088	Min ACI 10.5	10.200	659.813	0.989
+1.20D+0.50Lr+1.60L	664.822	16.860	Bottom	10.281	Min ACI 10.5	10.200	659.813	1.008
+1.20D+0.50Lr+1.60L	675.808	16.920	Bottom	10.459	Min ACI 10.5	10.200	659.813	1.024
+1.20D+0.50Lr+1.60L	685.830	16.980	Bottom	10.621	Min ACI 10.5	10.200	659.813	1.039
+1.20D+0.50Lr+1.60L	694.887	17.040	Bottom	10.768	Min ACI 10.5	10.200	659.813	1.053
+1.20D+0.50Lr+1.60L	702.980	17.100	Bottom	10.900	Min ACI 10.5	10.200	659.813	1.065
+1.20D+0.50Lr+1.60L	710.110	17.160	Bottom	11.016	Min for Bending	10.200	659.813	1.076
+1.20D+0.50Lr+1.60L	716.275	17.220	Bottom	11.116	Min for Bending	10.200	659.813	1.086
+1.20D+0.50Lr+1.60L	721.478	17.280	Bottom	11.201	Min ACI 10.5	10.200	659.813	1.093
+1.20D+0.50Lr+1.60L	725.716	17.340	Bottom	11.270	Min ACI 10.5	10.200	659.813	1.100
+1.20D+0.50Lr+1.60L	728.992	17.400	Bottom	11.324	Min ACI 10.5	10.200	659.813	1.105
+1.20D+0.50Lr+1.60L	731.305	17.460	Bottom	11.361	Min ACI 10.5	10.200	659.813	1.108
+1.20D+0.50Lr+1.60L	732.655	17.520	Bottom	11.383	Min for Bending	10.200	659.813	1.110
+1.20D+0.50Lr+1.60L	733.042	17.580	Bottom	11.390	Min ACI 10.5	10.200	659.813	1.111
+1.20D+0.50Lr+1.60L	732.467	17.640	Bottom	11.380	Min ACI 10.5	10.200	659.813	1.110
+1.20D+0.50Lr+1.60L	730.930	17.700	Bottom	11.355	Min ACI 10.5	10.200	659.813	1.108
+1.20D+0.50Lr+1.60L	728.431	17.760	Bottom	11.315	Min ACI 10.5	10.200	659.813	1.104
+1.20D+0.50Lr+1.60L	724.970	17.820	Bottom	11.258	Min ACI 10.5	10.200	659.813	1.099
+1.20D+0.50Lr+1.60L	720.548	17.880	Bottom	11.186	Min ACI 10.5	10.200	659.813	1.092
+1.20D+0.50Lr+1.60L	715.164	17.940	Bottom	11.098	Min for Bending	10.200	659.813	1.084
+1.20D+0.50Lr+1.60L	708.819	18.000	Bottom	10.995	Min ACI 10.5	10.200	659.813	1.074
+1.20D+0.50Lr+1.60L	701.513	18.060	Bottom	10.876	Min ACI 10.5	10.200	659.813	1.063
+1.20D+0.50Lr+1.60L	693.246	18.120	Bottom	10.742	Min ACI 10.5	10.200	659.813	1.051
+1.20D+0.50Lr+1.60L	684.019	18.180	Bottom	10.592	Min ACI 10.5	10.200	659.813	1.037
+1.20D+0.50Lr+1.60L	673.832	18.240	Bottom	10.427	Min ACI 10.5	10.200	659.813	1.021
+1.20D+0.50Lr+1.60L	662.684	18.300	Bottom	10.246	Min ACI 10.5	10.200	659.813	1.004
+1.20D+0.50Lr+1.60L	650.576	18.360	Bottom	10.051	Min ACI 10.5	10.200	659.813	0.986
+1.20D+0.50Lr+1.60L	637.515	18.420	Bottom	9.840	Min ACI 10.5	10.200	659.813	0.966
+1.20D+0.50Lr+1.60L	624.157	18.480	Bottom	9.625	Min ACI 10.5	10.200	659.813	0.946
+1.20D+0.50Lr+1.60L	610.934	18.540	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.926
+1.20D+0.50Lr+1.60L	597.847	18.600	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.906
+1.20D+0.50Lr+1.60L	584.896	18.660	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.886
+1.20D+0.50Lr+1.60L	572.080	18.720	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.867
+1.20D+0.50Lr+1.60L	559.401	18.780	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.848
+1.20D+0.50Lr+1.60L	546.858	18.840	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.829
+1.20D+0.50Lr+1.60L	534.452	18.900	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.810
+1.20D+0.50Lr+1.60L	522.182	18.960	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.791
+1.20D+0.50Lr+1.60L	510.050	19.020	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.773
+1.20D+0.50Lr+1.60L	498.055	19.080	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.755
+1.20D+0.50Lr+1.60L	486.197	19.140	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.737
+1.20D+0.50Lr+1.60L	474.476	19.200	Bottom	9.600	Min ACI 10.5	10.200	659.813	0.719
+1.20D+0.50Lr+1.60L	462.894	19.260	Bottom	9.415	Min ACI 10.5	10.200	659.813	0.702
+1.20D+0.50Lr+1.60L	451.449	19.320	Bottom	9.175	Min ACI 10.5	10.200	659.813	0.684
+1.20D+0.50Lr+1.60L	440.142	19.380	Bottom	8.939	Min ACI 10.5	10.200	659.813	0.667

Project Title:
Engineer:
Project ID:
Project Descr:

Combined Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: F-240160 (worst)

Z-Axis Footing Flexure - Maximum Values for Load Combination

Load Combination...	Mu (ft-k)	Distance from left (ft)	Tension Side	As Req'd (in^2)	Governed by	Actual As (in^2)	Phi*Mn (ft-k)	Mu / PhiMn
+1.20D+0.50Lr+1.60L	428.974	19.440	Bottom	8.706	Min ACI 10.5	10.200	659.813	0.650
+1.20D+0.50Lr+1.60L	417.944	19.500	Bottom	8.476	Min ACI 10.5	10.200	659.813	0.633
+1.20D+0.50Lr+1.60L	407.053	19.560	Bottom	8.249	Min ACI 10.5	10.200	659.813	0.617
+1.20D+0.50Lr+1.60L	396.301	19.620	Bottom	8.026	Min ACI 10.5	10.200	659.813	0.601
+1.20D+0.50Lr+1.60L	385.689	19.680	Bottom	7.805	Min ACI 10.5	10.200	659.813	0.585
+1.20D+0.50Lr+1.60L	375.215	19.740	Bottom	7.588	Min ACI 10.5	10.200	659.813	0.569
+1.20D+0.50Lr+1.60L	364.881	19.800	Bottom	7.374	Min ACI 10.5	10.200	659.813	0.553
+1.20D+0.50Lr+1.60L	354.687	19.860	Bottom	7.163	Min ACI 10.5	10.200	659.813	0.538
+1.20D+0.50Lr+1.60L	344.633	19.920	Bottom	6.956	Min ACI 10.5	10.200	659.813	0.522
+1.20D+0.50Lr+1.60L	334.718	19.980	Bottom	6.751	Min ACI 10.5	10.200	659.813	0.507
+1.20D+0.50Lr+1.60L	324.944	20.040	Bottom	6.550	Min ACI 10.5	10.200	659.813	0.492
+1.20D+0.50Lr+1.60L	315.311	20.100	Bottom	6.352	Min ACI 10.5	10.200	659.813	0.478
+1.20D+0.50Lr+1.60L	305.818	20.160	Bottom	6.221	Min Temp %	10.200	659.813	0.463
+1.20D+0.50Lr+1.60L	296.467	20.220	Bottom	6.221	Min Temp %	10.200	659.813	0.449
+1.20D+0.50Lr+1.60L	287.256	20.280	Bottom	6.221	Min Temp %	10.200	659.813	0.435
+1.20D+0.50Lr+1.60L	278.187	20.340	Bottom	6.221	Min Temp %	10.200	659.813	0.422
+1.20D+0.50Lr+1.60L	269.259	20.400	Bottom	6.221	Min Temp %	10.200	659.813	0.408
+1.20D+0.50Lr+1.60L	260.473	20.460	Bottom	6.221	Min Temp %	10.200	659.813	0.395
+1.20D+0.50Lr+1.60L	251.829	20.520	Bottom	6.221	Min Temp %	10.200	659.813	0.382
+1.20D+0.50Lr+1.60L	243.327	20.580	Bottom	6.221	Min Temp %	10.200	659.813	0.369
+1.20D+0.50Lr+1.60L	234.967	20.640	Bottom	6.221	Min Temp %	10.200	659.813	0.356
+1.20D+0.50Lr+1.60L	226.749	20.700	Bottom	6.221	Min Temp %	10.200	659.813	0.344
+1.20D+0.50Lr+1.60L	218.675	20.760	Bottom	6.221	Min Temp %	10.200	659.813	0.331
+1.20D+0.50Lr+1.60L	210.743	20.820	Bottom	6.221	Min Temp %	10.200	659.813	0.319
+1.20D+0.50Lr+1.60L	202.954	20.880	Bottom	6.221	Min Temp %	10.200	659.813	0.308
+1.20D+0.50Lr+1.60L	195.309	20.940	Bottom	6.221	Min Temp %	10.200	659.813	0.296
+1.20D+0.50Lr+1.60L	187.807	21.000	Bottom	6.221	Min Temp %	10.200	659.813	0.285
+1.20D+0.50Lr+1.60L	180.449	21.060	Bottom	6.221	Min Temp %	10.200	659.813	0.273
+1.20D+0.50Lr+1.60L	173.234	21.120	Bottom	6.221	Min Temp %	10.200	659.813	0.263
+1.20D+0.50Lr+1.60L	166.164	21.180	Bottom	6.221	Min Temp %	10.200	659.813	0.252
+1.20D+0.50Lr+1.60L	159.238	21.240	Bottom	6.221	Min Temp %	10.200	659.813	0.241
+1.20D+0.50Lr+1.60L	152.456	21.300	Bottom	6.221	Min Temp %	10.200	659.813	0.231
+1.20D+0.50Lr+1.60L	145.819	21.360	Bottom	6.221	Min Temp %	10.200	659.813	0.221
+1.20D+0.50Lr+1.60L	139.327	21.420	Bottom	6.221	Min Temp %	10.200	659.813	0.211
+1.20D+0.50Lr+1.60L	132.980	21.480	Bottom	6.221	Min Temp %	10.200	659.813	0.202
+1.20D+0.50Lr+1.60L	126.779	21.540	Bottom	6.221	Min Temp %	10.200	659.813	0.192
+1.20D+0.50Lr+1.60L	120.722	21.600	Bottom	6.221	Min Temp %	10.200	659.813	0.183
+1.20D+0.50Lr+1.60L	114.812	21.660	Bottom	6.221	Min Temp %	10.200	659.813	0.174
+1.20D+0.50Lr+1.60L	109.047	21.720	Bottom	6.221	Min Temp %	10.200	659.813	0.165
+1.20D+0.50Lr+1.60L	103.428	21.780	Bottom	6.221	Min Temp %	10.200	659.813	0.157
+1.20D+0.50Lr+1.60L	97.955	21.840	Bottom	6.221	Min Temp %	10.200	659.813	0.148
+1.20D+0.50Lr+1.60L	92.629	21.900	Bottom	6.221	Min Temp %	10.200	659.813	0.140
+1.20D+0.50Lr+1.60L	87.450	21.960	Bottom	6.221	Min Temp %	10.200	659.813	0.133
+1.20D+0.50Lr+1.60L	82.417	22.020	Bottom	6.221	Min Temp %	10.200	659.813	0.125
+1.20D+0.50Lr+1.60L	77.531	22.080	Bottom	6.221	Min Temp %	10.200	659.813	0.118
+1.20D+0.50Lr+1.60L	72.793	22.140	Bottom	6.221	Min Temp %	10.200	659.813	0.110
+1.20D+0.50Lr+1.60L	68.202	22.200	Bottom	6.221	Min Temp %	10.200	659.813	0.103
+1.20D+0.50Lr+1.60L	63.759	22.260	Bottom	6.221	Min Temp %	10.200	659.813	0.097
+1.20D+0.50Lr+1.60L	59.463	22.320	Bottom	6.221	Min Temp %	10.200	659.813	0.090
+1.20D+0.50Lr+1.60L	55.316	22.380	Bottom	6.221	Min Temp %	10.200	659.813	0.084
+1.20D+0.50Lr+1.60L	51.316	22.440	Bottom	6.221	Min Temp %	10.200	659.813	0.078
+1.20D+0.50Lr+1.60L	47.465	22.500	Bottom	6.221	Min Temp %	10.200	659.813	0.072
+1.20D+0.50Lr+1.60L	43.763	22.560	Bottom	6.221	Min Temp %	10.200	659.813	0.066
+1.20D+0.50Lr+1.60L	40.209	22.620	Bottom	6.221	Min Temp %	10.200	659.813	0.061
+1.20D+0.50Lr+1.60L	36.805	22.680	Bottom	6.221	Min Temp %	10.200	659.813	0.056
+1.20D+0.50Lr+1.60L	33.549	22.740	Bottom	6.221	Min Temp %	10.200	659.813	0.051
+1.20D+0.50Lr+1.60L	30.443	22.800	Bottom	6.221	Min Temp %	10.200	659.813	0.046
+1.20D+0.50Lr+1.60L	27.487	22.860	Bottom	6.221	Min Temp %	10.200	659.813	0.042
+1.20D+0.50Lr+1.60L	24.680	22.920	Bottom	6.221	Min Temp %	10.200	659.813	0.037
+1.20D+0.50Lr+1.60L	22.024	22.980	Bottom	6.221	Min Temp %	10.200	659.813	0.033
+1.20D+0.50Lr+1.60L	19.517	23.040	Bottom	6.221	Min Temp %	10.200	659.813	0.030
+1.20D+0.50Lr+1.60L	17.161	23.100	Bottom	6.221	Min Temp %	10.200	659.813	0.026
+1.20D+0.50Lr+1.60L	14.956	23.160	Bottom	6.221	Min Temp %	10.200	659.813	0.023

Project Title:
Engineer:
Project ID:
Project Descr:

Combined Footing

Project File: NR134.ec6

LIC# : KW-06011763, Build:20.21.9.19 Arch-Deco Design & Construction Inc. (c) ENERCALC INC 1983-2021

DESCRIPTION: F-240160 (worst)

Z-Axis Footing Flexure - Maximum Values for Load Combination

Load Combination...	Mu (ft-k)	Distance from left (ft)	Tension Side	As Req'd (in^2)	Governed by	Actual As (in^2)	Phi*Mn (ft-k)	Mu / PhiMn
+1.20D+0.50Lr+1.60L	12.901	23.220	Bottom	6.221	Min Temp %	10.200	659.813	0.020
+1.20D+0.50Lr+1.60L	10.998	23.280	Bottom	6.221	Min Temp %	10.200	659.813	0.017
+1.20D+0.50Lr+1.60L	9.245	23.340	Bottom	6.221	Min Temp %	10.200	659.813	0.014
+1.20D+0.50Lr+1.60L	7.644	23.400	Bottom	6.221	Min Temp %	10.200	659.813	0.012
+1.20D+0.50Lr+1.60L	6.194	23.460	Bottom	6.221	Min Temp %	10.200	659.813	0.009
+1.20D+0.50Lr+1.60L	4.896	23.520	Bottom	6.221	Min Temp %	10.200	659.813	0.007
+1.20D+0.50Lr+1.60L	3.750	23.580	Bottom	6.221	Min Temp %	10.200	659.813	0.006
+1.20D+0.50Lr+1.60L	2.756	23.640	Bottom	6.221	Min Temp %	10.200	659.813	0.004
+1.20D+0.50Lr+1.60L	1.915	23.700	Bottom	6.221	Min Temp %	10.200	659.813	0.003
+1.20D+0.50Lr+1.60L	1.226	23.760	Bottom	6.221	Min Temp %	10.200	659.813	0.002
+1.20D+0.50Lr+1.60L	0.690	23.820	Bottom	6.221	Min Temp %	10.200	659.813	0.001
+1.20D+0.50Lr+1.60L	0.307	23.880	Bottom	6.221	Min Temp %	10.200	659.813	0.000
+1.20D+0.50Lr+1.60L	0.077	23.940	Bottom	6.221	Min Temp %	10.200	659.813	0.000
+1.20D+0.50Lr+1.60L	0.000	24.000	0	0.000	0	0.000	0.000	0.000

One Way Shear

Punching Shear

Load Combination...	Phi Vn	vu @ Col #1	vu @ Col #2	Phi Vn	vu @ Col #1	vu @ Col #2
+1.40D	82.16 psi	30.41 psi	51.93 psi	164.32 psi	118.48 psi	117.20 psi
+1.20D+0.50Lr+1.60L	82.16 psi	29.20 psi	61.29 psi	164.32 psi	114.23 psi	111.48 psi
+1.20D+1.60L	82.16 psi	28.07 psi	60.02 psi	164.32 psi	109.83 psi	107.03 psi
+1.20D+1.60Lr+L	82.16 psi	30.95 psi	58.27 psi	164.32 psi	120.82 psi	118.79 psi
+1.20D+1.60Lr	82.16 psi	29.70 psi	48.58 psi	164.32 psi	115.65 psi	114.69 psi
+1.20D+L	82.16 psi	27.32 psi	54.21 psi	164.32 psi	106.73 psi	104.57 psi
+1.20D	82.16 psi	26.06 psi	44.51 psi	164.32 psi	101.55 psi	100.46 psi
+1.20D+0.50Lr+L	82.16 psi	28.45 psi	55.48 psi	164.32 psi	111.13 psi	109.01 psi
+0.90D	82.16 psi	19.55 psi	33.39 psi	164.32 psi	76.17 psi	75.34 psi