THE ARCHITECT HEREBY EXPRESSLY RESERVES THE COPYRIGHT AND PROPERTY RIGHTS IN THESE PLANS. DRAWINGS ARE NOT TO BE REPRODUCED, CHANGED, OR COPIED IN ANY FORM OR MANNER WHATSOEVER WITHOUT FIRST OBTAINING WRITTEN PERMISSION AND CONSENT. NOR ARE THEY TO BE ASSIGNED TO ANY PARTY WITHOUT FIRST OBTAINING SAID WRITTEN PERMISSION AND CONSENT.

STRUCTURAL NOTES

1. FOUNDATION:

A. FOUNDATION SYSTEM CONSISTS OF SHALLOW SPREAD FOOTINGS DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 2000 PSF. FOOTINGS SHALL BEAR ON UNDISTURBED LIMEROCK.
B. PROVIDE ANY BRACING OR SHORING AS REQUIRED IN ORDER TO PREVENT SETTLEMENT OR DISPLACEMENT OF ADJACENT EXISTING FOUNDATIONS AND /OR STRUCTURES.
C. ALL FOUNDATION EXCAVATIONS SHALL BE KEPT DRY. FOUNDATION CONCRETE SHALL BE POURED ONLY UNTIL EXCAVATIONS ARE COMPLETELY DRY. TAKE EVERY PRECAUTION DURING DEWATERING OPERATIONS SO THAT EXISTING ADJACENT STRUCTURES, UTILITIES, PIPING, ETC. ARE NOT DAMAGED.
D. CENTERS OF COLUMNS SHALL COINCIDE WITH CENTERS OF FOOTINGS, UNLESS OTHERWISE NOTED IN FOUNDATION PLANS AND FOOTING DETAILS.
E. TOP OF FOUNDATION ELEVATIONS SHOWN IN PLAN ARE BASED ON THE BEST AVAILABLE INFORMATION GATHERED FROM SOIL BORING LOADS SOIL CONDITIONS ENCOUNTERED DURING FOUNDATION EXCAVATIONS MIGHT DICTATE TOP OF FOUNDATION ELEVATION TO BE LOWERED IN ORDER TO EMBED FOUNDATIONS A MINIMUM OF 1 INCHES INTO THE EXISTING LIMEROCK BEARING STRATA.
F. FOUNDATIONS MAY BE EARTH-FORMED IF SOIL CONDITIONS PERMIT. EXCAVATE TO EXACT FOUNDATION SIZES.

2. GENERAL:

A. ALL MATERIALS AND CONSTRUCTION SHALL COMPLY WITH THE FLORIDA BUILDING CODE, 2020 HVHZ, ASCE 7-16 MINIMUM DESIGN LOADS FOR BUILDINGS, THE ACI 318-14 BUILDING CODE, AND ALL APPLICABLE FEDERAL, STATE AND LOCAL ORDINANCES.
B. THESE DRAWINGS AND SPECIFICATIONS COMPLY, TO THE BEST OF MY KNOWLEDGE WITH THE FLORIDA BUILDING CODE, 2020 HVHZ.
C. THE CONTRACTOR SHALL VERIFY ALL CONDITIONS OF EXISTING STRUCTURES AFFECTING NEW CONSTRUCTION BEFORE COMMENCING ANY WORK. ANY VARIATIONS IN ACTUAL FIELD CONDITIONS/DIMENSIONS FROM THOSE SHOWN IN THE CONTRACT DRAWINGS SHALL BE REPORTED TO THE ARCHITECT/ENGINEER FOR DETERMINING THE NEED OF REDESIGN PRIOR TO CONTRACTOR'S SUBMITTAL OF SHOP WORKING DRAWINGS FOR REVIEW.
D. THESE DRAWINGS SHALL BE WORKED TOGETHER WITH ARCHITECTURAL, AIR CONDITIONING, MECHANICAL AND ELECTRICAL DRAWINGS TO LOCATE DEPRESSED SLABS, SLOPES, DRAINS, OUTLETS, RECESSES, OPENINGS, REGLETS, BOLT SETTINGS, SLEEVES, ETC. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF ARCHITECT/ ENGINEER BEFORE PROCEEDING WITH THE WORK. ANY DISCREPANCIES, OMISSIONS OR VARIATIONS FOUND ON THE DRAWINGS OR IN THE SPECIFICATIONS DISCOVERED DURING THE BIDDING PHASE SHALL BE IMMEDIATELY COMMUNICATED TO ARCHITECT/ ENGINEER.
E. WHEN PERFORMING WORK BELOW GRADE, CARE SHALL BE TAKEN TO AVOID DAMAGING ANY EXISTING UTILITIES. ALL UNKNOWN UTILITIES DISCOVERED DURING CONSTRUCTION SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER. ANY DAMAGE TO THE EXISTING UTILITIES SHALL BE REPORTED TO ALL AFFECTED PARTIES, INCLUDING THE ARCHITECT/ENGINEER.
F. GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR UPDATING HIS CONSTRUCTION DOCUMENTS WITH THE REVISED DRAWINGS AND SPECIFICATIONS, FIELD ORDERS, CHANGE ORDERS AND CLARIFICATION SKETCHES ISSUED DURING THE COURSE OF CONSTRUCTION.
G. TYPICAL DETAILS AND NOTES ON THESE DRAWINGS SHALL APPLY UNLESS SPECIFICALLY NOTED OTHERWISE. CONSTRUCTION DETAILS AND SECTIONS NOT COMPLETELY SHOWN OR NOTED SHALL BE SIMILAR TO DETAILS AND SECTIONS SHOWN OR NOTED FOR SIMILAR CONDITIONS.
H. THE GENERAL CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING, AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS AND UTILITIES IN ACCORDANCE WITH THE LOCAL BUILDING DEPARTMENT.
I. BACKFILL AROUND THE EXTERIOR PERIMETER OF WALLS SHALL NOT BE PLACED UNTIL AFTER THE WALLS ARE SUPPORTED BY THE COMPLETION OF INTERIOR FLOOR SYSTEMS. DO NOT PROCEED WITH BACKFILL UNTIL (7) DAYS AS A MINIMUM AFTER THE COMPLETION OF INTERIOR FLOOR SYSTEM UNLESS WALLS ARE ADEQUATELY BRACED. BACKFILL SHALL NOT BE PLACED UNTIL AFTER COMPLETION AND INSPECTION OF WATERPROOFING WHERE WATERPROOFING OCCUR.
J. GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR THE DISPOSAL OF ALL ACCUMULATED WATER FROM EXCAVATIONS AND DEWATERING OPERATIONS IN SUCH A WAY AS TO NOT CAUSE INCONVENIENCE TO THE WORK AND DAMAGE TO THE STRUCTURAL ELEMENTS.
K. STRUCTURAL NOTES SHALL BE USED IN CONJUNCTION WITH THE SPECIFICATIONS. IF A CONFLICT EXISTS, THE MORE STRINGENT GOVERNS.
L. GENERAL CONTRACTOR SHALL PROTECT EXISTING FACILITIES, STRUCTURES AND UTILITY LINES AND CONDUITS FROM DAMAGE. GENERAL CONTRACTOR IS SOLELY RESPONSIBLE FOR DAMAGE OR INJURY DUE TO HIS ACT OR NEGLECT.
M. GENERAL CONTRACTOR IS SOLELY RESPONSIBLE FOR JOB SAFETY, MEANS AND METHODS OF CONSTRUCTION AND CONSTRUCTION PROCEDURES.
N. DO NOT SCALE THESE DRAWINGS, USE DIMENSIONS NOTED. IF DIMENSIONS ARE MISSING CONSULT ARCHITECT/ENGINEER FOR ADVISE.

3. SHOP DRAWINGS SUBMITTALS:

A. SUBMITTALS TO STRUCTURAL ENGINEER OF RECORD:
   I. CONCRETE TEST REPORTS FOR CAST-IN-PLACE CONCRETE AS PER ACI 301-11.
   II. REINFORCING STEEL SHOP DRAWINGS.
   III. STRUCTURAL STEEL SHOP DRAWINGS AND ERECTION DRAWINGS.

B. SHOP DRAWINGS REQUIRING ENGINEERING BY A SPECIALTY ENGINEER SHALL COMPLY WITH THE FOLLOWING:
   I. THE SPECIALTY ENGINEER SHALL BE A FLORIDA REGISTERED PROFESSIONAL ENGINEER.
   II. THE SPECIALTY ENGINEER SHALL HAVE AT LEAST FIVE YEARS OF SOLID PROFESSIONAL DESIGN EXPERIENCE IN THE DESIGN AND DETAILING OF THE STRUCTURAL COMPONENT AND/OR STRUCTURAL SYSTEM BEING SUBMITTED FOR APPROVAL.
   III. THE SPECIALTY ENGINEER SHALL BE AN EMPLOYEE OR OFFICER OF A FABRICATOR, AN EMPLOYEE OR OFFICER OF ANOTHER COMPANY SUPPLYING COMPONENTS TO THE FABRICATOR.
C. ALL SHOP DRAWINGS ARE AN AID FOR FIELD PLACEMENT AND ARE SUPERSEDED BY THE STRUCTURAL DRAWINGS. IT SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO MAKE CERTAIN THAT ALL CONSTRUCTION IS IN FULL AGREEMENT WITH THE LATEST STRUCTURAL DRAWINGS.
D. THE CONTRACTOR SHALL SUPPLY THE ENGINEER THREE COPIES OF SHOP DRAWINGS A MINIMUM OF ONE WEEK PRIOR TO PLACEMENT. THE REVIEW OF SHOP DRAWINGS BY ENGINEER IS ONLY FOR GENERAL COMPLIANCE WITH THE STRUCTURAL DRAWINGS AND SPECIFICATIONS. THE REVIEW DOES NOT GUARANTEE IN ANY WAY THAT THE SHOP DRAWINGS ARE CORRECT NOR DOES IT INFER THAT THEY SUPERSEDE THE STRUCTURAL DRAWINGS. REVIEW WILL NOT INCLUDE CHECK OF DIMENSIONS, LENGTHS OR QUANTITIES.
E. THESE STRUCTURAL DRAWINGS SHALL NOT BE USED TO PRODUCE SHOP DRAWINGS WITHOUT PRIOR EXPRESSED WRITTEN APPROVAL FROM THE STRUCTURE ENGINEER OF RECORD.
F. SHOP DRAWINGS RE-SUBMITTED FOR APPROVAL SHALL BE CLEARLY CLOUDED AND NOTED. RE-REVIEW SHALL BE LIMITED TO THOSE ITEMS WHICH CAUSED THE RE-SUBMITTAL.

4. CONCRETE:

A. ALL CONCRETE WORK SHALL CONFORM TO ALL REQUIREMENTS OF ACI 301-11 'SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS.'
B. CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE AS FOLLOWS:

FOOTINGS	3,000 PSI
SLABS ON GRADE	3,000 PSI
ROOF TIE BEAMS AND BEAMS	3,000 PSI
COLUMNS AND TIE COLUMNS	3,000 PSI
ALL OTHER CONCRETE	3,000 PSI

C. FORMWORK SHALL COMPLY WITH ACI 347-04, ' RECOMMENDED PRACTICE FOR CONCRETE FORMWORK.'
D. MIX DESIGNS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO COMMENCEMENT OF ANY CONCRETE WORK. SUBMIT STATISTICAL DATA FOR EACH CLASS OF CONCRETE.
E. NO WATER SHALL BE ADDED TO THE CONCRETE AT THE JOB SITE.
F. THE OWNER SHALL CONTRACT AN INDEPENDENT TESTING LABORATORY TO PERFORM CONCRETE CYLINDER TESTS AS FOLLOWS. SIX CYLINDER TESTS FOR ANY 50 CUBIC YARDS OF CONCRETE POURED, OR FRACTION THEREOF FOR EACH CLASS OF CONCRETE POURED EACH DAY. ONE CYLINDER SHALL BE TESTED AT 3 DAYS AND 7 DAYS, THREE AT 28 DAYS, AND ONE RESERVED TO BE TESTED AT 51 DAYS IF REQUIRED. FOLLOW ASTM STANDARDS FOR SAMPLING AND TESTING. ONE SLUMP TEST SHALL BE TAKEN FOR EACH SET OF TEST CYLINDERS CAST. SLUMP TEST SHALL CONFORM WITH ASTM C 143. NO CONCRETE TEST WILL BE ACCEPTED IF CONCRETE IS TAMPERED WITH IN ANY WAY AFTER SAID TEST IS PERFORMED. REPEAT TEST IF WATER IS ADDED AFTER INITIAL SAMPLING.
G. TRANSPORTING, PLACING, CURING AND DEPOSITING OF CONCRETE SHALL COMPLY WITH ACI 301-08, 'SPECIFICATIONS FOR STRUCTURAL CONCRETE.
H. CONSTRUCTION JOINTS IN STRUCTURAL SLABS AND BEAMS SHALL BE LOCATED AT 1/3 OF THE SPAN WITH REINFORCING CONTINUOUS ACROSS THE JOINT. PROVIDE A CONTINUOUS 2 X 4 SHEAR KEY AT SLABS. AT BEAMS PROVIDE A 1 1/2" DEEP SHEAR KEY WITH A WIDTH 8" SMALLER THAN THE BEAM WIDTH AND A DEPTH 8" SMALLER THAN THE BEAM DEPTH. LOCATIONS SHALL BE APPROVED BY STRUCTURAL ENGINEER OF RECORD BEFORE POUR.
I. MAXIMUM WATER/CEMENT RATIO FOR CONCRETE CONTAINING A SUPERPLASTICIZING ADMIXTURE SHALL BE 0.40. SLUMP AFTER ADDITION OF SUPERPLASTICIZER SHALL BE 8" +/- 1".
J. MINIMUM CONCRETE COVER FOR REINFORCEMENT:
   I. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH ..... 3"
   II. CONCRETE EXPOSED TO EARTH OR WEATHER
      #7 BARS AND LARGER ..... 2"
      #5 BARS AND SMALLER ..... 1 1/2"
   III. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH EARTH:
      SLABS AND WALLS ..... 3/4",
      BEAMS AND COLUMNS ..... 1/2"
      SHEAR WALLS ..... 1 1/2"

5. REINFORCING STEEL:

A. REINFORCING STEEL SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-14.
B. REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A 615-09a, (SI) GRADE 60.
C. ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM A 185-06.
D. REINFORCING SHALL BE HELD SECURELY IN POSITION WITH STANDARD ACCESSORIES DURING PLACING OF CONCRETE, IN ACCORDANCE WITH CRSI 'MANUAL OF STANDARD PRACTICE', 12008.
E. ALL TOP REINFORCING SHALL TERMINATE WITH STANDARD HOOKS AT DISCONTINUOUS EDGES OR ENDS.
F. ALL BOTTOM BARS SHALL BEAR 1" MINIMUM OVER SUPPORTS, U.O.N.
G. ALL REINFORCING BARS MARKED CONTINUOUS SHALL BE LAPPED WITH CLASS 'B' TENSION LAP SPLICES AT CORNERS UNLESS OTHERWISE NOTED. LAP CONTINUOUS TOP BARS AT CENTER BETWEEN SUPPORTS AS REQUIRED WITH CLASS 'B' TENSION LAP SPLICES. TERMINATE CONTINUOUS BARS AT NON-CONTINUOUS ENDS WITH STANDARD HOOKS, U.O.N.
H. SLAB TOP BARS ARE SHOWN IN PLAN AS SOLID LINES. SLAB BOTTOM BARS ARE SHOWN IN PLAN AS DASHED LINES.
I. IN BEAMS WITH MULTIPLE LAYERS OF REINFORCING AT TOP OR BOTTOM SEPARATE LAYERS WITH #8 SPACER BARS FOR BAR SIZES UP TO #8 AND SAME SPACER BAR AS BAR SIZE FOR #9 BARS AND LARGER.
J. IN BEAMS 8 INCHES WIDE PROVIDE A MAXIMUM OF 2 BARS PER LAYER OF REINFORCING.
K. BEAMS INTERMEDIATE BARS SHALL BE HOOKED AT DISCONTINUOUS ENDS AND SPLICED AT SUPPORTS WITH CLASS 'B' TENSION LAP SPLICES.
L. PROVIDE A 10 TON REINFORCING STEEL ALLOWANCE TO BE USED BY STRUCTURAL ENGINEER OF RECORD AT HIS OWN DISCRETION. BALANCE OF THIS ALLOWANCE NOT USED DURING CONSTRUCTION SHALL BE CREDITED TO THE OWNER AT THE SAME COST PER TON AS ORIGINALLY CHARGED.
M. ALL TOP AND BOTTOM AND ACCESSORY REINFORCING USED IN BALCONIES AND TERRACES SHALL BE GALVANIZED.
N. FOR CLASS 'B' TENSION LAP SPLICES FOLLOW CRSI 'DESIGN HANDBOOK, 2008', CHAPTER 9.

7. CONCRETE MASONRY WORK:

A. CONCRETE MASONRY WALLS NOTED AS LOAD BEARING WALLS, SHALL BE IN PLACE BEFORE THE SLABS AND BEAMS SUPPORTED BY THEM ARE POURED AS WELL AS THE CONCRETE TIE COLUMNS FRAMING THEM.
B. CONCRETE MASONRY WALLS NOTED AS NON-LOAD BEARING WALLS SHALL BE PLACED AFTER CONCRETE FRAME SUPPORTING THEM ARE 28 DAYS OLD AND ALL SHORING AND RE-SHORING IS COMPLETELY REMOVED FROM BELOW AND ABOVE. HOLD CLEAR OF CONCRETE ABOVE UNTIL ANTICIPATED DEAD LOAD DEFLECTION OF CONCRETE SLAB OR BEAM ABOVE HAS OCCURRED. FILL JOINT WITH MORTAR AND SEAL AS REQUIRED BY ARCHITECTURAL DRAWINGS TO PREVENT WATER INTRUSION.
C. ALL CONCRETE MASONRY UNITS (CMU) SHALL CONFORM TO ASTM C 90, 'STANDARD SPECIFICATIONS FOR HOLLOW LOAD BEARING CONCRETE MASONRY UNITS', WITH A NET AREA COMPRESSIVE STRENGTH OF CONCRETE MASONRY UNITS OF 1900 PSI.
D. MORTAR SHALL CONFORM TO ASTM C 270, TYPE 'M', WITH A MINIMUM AVERAGE STRENGTH OF 2500 PSI.
E. CONCRETE MASONRY STRENGTH, f'm, SHALL BE A MINIMUM OF 1500 PSI.
F. VERTICAL REINFORCING IN CMU CELLS SHALL BE SPLICED WITH 48 BAR DIAMETER LAP SPLICES. PROVIDE CLEAN OUT HOLES AT BASE OF FILLED CELLS FOR LAP INSPECTION AND VERIFYING THAT THE CELLS HAVE BEEN FILLED SOLID WITH GROUT.
G. FILLED CELLS SHALL BE FILLED WITH 3000 PSI GROUT AS PER TMS402/602-16 FILLING OF CELLS SHALL BE DONE IN FOUR FOOT LIFTS WITH A MAXIMUM POUR OF 12 FEET. USE MECHANICAL VIBRATION TO ACHIEVE GROUT-FILLED SOLID CELLS. GROUT SHALL CONFORM TO ASTM C 471. SLUMP SHALL BE BETWEEN 8" AND 11".
H. ALL CMU WALLS SHALL BE HORIZONTALLY REINFORCED WITH STANDARD NO. 9 LADDER-TYPE GALVANIZED STEEL REINFORCING EVERY SECOND COURSE. EXTEND REINFORCING A MINIMUM OF 4 INCHES INTO TIE COLUMNS.
I. PROVIDE GALVANIZED STEEL DOVETAIL ANCHORS EVERY OTHER COURSE CONNECTING NON LOAD-BEARING WALLS TO CONCRETE COLUMNS AND SHEAR WALLS INTO THE MORTAR BED OF SOLID-GROUTED HOLLOW UNITS.
J. REINFORCING BARS IN GROUTED CELLS SHALL BE SECURED IN PLACE AT BASE OF BAR AND ABOVE BEFORE GROUTING OF CELL WITH GALVANIZED BAR POSITIONERS.
K. REINFORCING BARS BE SHALL CENTERED IN THE BLOCK CELL. DOUELS NOT LINED UP WITH THE BLOCK CELL SHALL NOT BE SLOPED MORE THAN 1 IN 7. HORIZONTAL DISTANCE BETWEEN DOUEL AND REINFORCING BAR MAY BE UP TO 8" (ONE BLOCK CELL APART.)
L. ANCHOR BOLTS SHALL BE EMBEDDED IN WALLS IN GROUTED CELLS.
M. GROUTED CELLS WHERE WEDGE ANCHORS ARE TO BE INSTALLED SHALL HAVE THE BLOCK SHELL REMOVED SO THAT WEDGE ANCHOR IS EMBEDDED IN SOLID CONCRETE GROUT. FILL ONE COURSE BELOW AND ABOVE ANCHOR LOCATION.
N. ALL CONCRETE MASONRY WORK HAS BEEN DESIGNED AND DETAILED ASSUMING CONCRETE MASONRY WORK WILL BE COMPLETELY INSPECTED BY SPECIAL OR THRESHOLD INSPECTORS.
O. REINFORCING BARS SHALL BE LOCATED AS INDICATED IN PLAN OR CALLED OUT BY NOTES IN PLANS. WHERE PLANS AND NOTES DISAGREE CONSULT ENGINEER OF RECORD FOR CLARIFICATION.
P. LAY MASONRY UNITS IN RUNNING BOND.
Q. PROVIDE 30 LBS. FELT PAPER TO ISOLATE WOOD FROM MASONRY WALLS. USE PRESSURE TREATED WOOD FOR LEDGERS IN CONTACT WITH MASONRY WALL.

7. STRUCTURAL STEEL:

A. STRUCTURAL STEEL SHALL COMPLY WITH AISC 'SPECIFICATIONS FOR DESIGN, FABRICATION AND ERECTION FOR STRUCTURAL STEEL BUILDINGS,' AISC 360-16.
B. STRUCTURAL STEEL WIDE FLANGE SHAPES AND PLATES SHALL CONFORM TO ASTM A 572, F_y 50 KSI, EXCEPT OTHER SHAPES WHICH SHALL CONFORM TO ASTM A 36, F_y 36 KSI.
C. STRUCTURAL STEEL TUBES SHALL CONFORM TO ASTM A500, GRADE B, F_{ST}=46 KSI.
D. STRUCTURAL STEEL PIPES SHALL CONFORM TO ASTM A53, TYPE S, GRADE B, F_y= 35 KSI.
E. ANCHOR BOLTS SHALL CONFORM TO EITHER ASTM A 306 OR ASTM A 36.
F. FRAMING BOLTS SHALL CONFORM TO ASTM 325, WITH HARDENED WASHERS AND HEX NUTS.
G. ALL EXTERIOR STEEL SHAPES, PLATES, NUTS, BOLTS, WASHERS SHALL BE HOT-DIPPED GALVANIZED.
H. SPLICING OF STEEL MEMBERS IS NOT ALLOWED, UNLESS SPECIFIED IN STRUCTURAL DRAWINGS OR APPROVED BY ENGINEER OF RECORD.
I. ALL BOLTS, NUTS AND WASHERS SHALL BE NEW, RUST-FREE, CLEAN AND WELL LUBRICATED.
J. BOLT HOLES THROUGH STEEL MEMBERS SHALL BE SHOP-DRILLED, CUT OR PUNCHED. DO NOT USE TORCH OR FLAME TO CUT OR ENLARGE HOLES.
K. ALL STRUCTURAL STEEL TUBE OR PIPE COLUMNS SHALL BE FILLED WITH 3000 PSI, CONCRETE GROUT. PROVIDE 1/4" DIAMETER WEEP HOLES EACH SIDE '3' FROM TOP AND BOTTOM OF COLUMN.
L. DO NOT PAINT PARTS OF STEEL MEMBERS TO BE EMBEDDED IN CONCRETE AND SURFACES TO BE IN CONTACT WITH CONCRETE.
M. FOR FIREPROOFING OF STRUCTURAL STEEL MEMBERS SEE ARCHITECTURAL DRAWINGS.
N. SEE ARCHITECTURAL MECHANICAL AND PLUMBING DRAWINGS FOR ADDITIONAL MISCELLANEOUS STRUCTURAL STEEL NOT SHOWN IN STRUCTURAL DRAWINGS.
O. FOR PAINTING OF NON-GALVANIZED STRUCTURAL STEEL SEE STRUCTURAL STEEL PROJECT SPECIFICATIONS.
P. WELDING SHALL BE DONE WITH E-70 ELECTRODES, UNLESS OTHERWISE NOTED, CONFORMING TO AWS D1.1. (WELDING OF GRADE 70 REINFORCEMENT IS NOT PERMITTED.)
Q. ALL SHOP AND FIELD WELDING SHALL BE IN ACCORDANCE WITH AWS D1.1 STRUCTURAL WELDING CODE, LATEST EDITION. ALL WELDERS SHALL BE AWS-CERTIFIED. SUBMIT WELDER CERTIFICATES TO ARCHITECT/ENGINEER FOR APPROVAL BEFORE ANY SHOP OR FIELD-WELDING IS STARTED.
R. HEADED SHEAR STUDS SHALL BE NELSON ANCHORS WITH FLUXED ENDS OR APPROVED. DEFORMED BAR ANCHORS (D.B.A.) SHALL BE NELSON, TYPE D2L, OR APPROVED. STUDS AND D.B.A. SHALL BE AUTOMATICALLY END-WELDED WITH THE MANUFACTURER'S STANDARD EQUIPMENT IN ACCORDANCE WITH THEIR RECOMMENDATIONS.
S. EXPANSION BOLTS SHALL BE HILTI KWIK BOLT 3 OR APPROVED WITH EQUIVALENT ICC ALLOWABLE TENSION AND SHEAR VALUES. EXPANSION BOLTS SHALL BE INSTALLED IN STRICT CONFORMANCE WITH MANUFACTURER'S RECOMMENDATIONS. DO NOT CUT REINFORCING IN NEW OR EXISTING CONCRETE DURING INSTALLATION.
T. THE STEEL STRUCTURE IS DESIGNED AS A WHOLE WITH THE FLOOR AND ROOF DIAPHRAGM ACTION BRACING THE FRAMES AGAINST GRAVITY AND LATERAL AND VERTICAL WIND FORCES. PROVIDE ALL TEMPORY BRACING AS REQUIRED IN ORDER TO MAINTAIN STEEL STRUCTURE STABLE UNTIL THE STRUCTURE IS COMPLETE.

8. PREFAB. WOOD TRUSSES:

A. FABRICATE ALL WOOD FRAMING MEMBERS AND PREFAB WOOD TRUSSES IN ACCORDANCE WITH THE 'NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION, 2018 EDITION.
B. WOOD FRAMING MEMBERS OTHER THAN TRUSSES SHALL BE #2 SOUTHERN PINE WITH A FIBER BENDING STRESS AS PER NDS 2018.
C. SIZES SHOWN ARE NOMINAL.
D. MEMBERS SHALL BE FREE OF CRACKS AND KNOTS.
E. MOISTURE CONTENT SHALL BE 19% OR LESS.
F. PRESSURE-TREATED WOOD SHALL BE USED ONLY WHERE SPECIFICALLY NOTED IN THE DRAWINGS. NO FRAMING MEMBERS SHALL BE OF PRESSURE-TREATED WOOD, UNLESS OTHERWISE NOTED.
G. ALL WOOD FRAMING CONNECTORS INCLUDING BOLTS, WASHERS, AND NUTS SHALL BE GALVANIZED.
H. ALL EXPOSED STEEL PLATES AND SHAPES NOTED AS CONNECTORS SHALL BE HOT-DIPPED GALVANIZED, TOUCH UP AFTER INSTALLATION WITH 'GALVICON' PAINT, OR APPROVED EQUAL.

12. WIND ANALYSIS DESIGN PARAMETERS:

A. WIND DESIGN HAS BEEN DONE IN ACCORDANCE WITH ASCE 7-16.
B. EXPOSURE 'C'
C. RISK CATEGORY II
D. INTERNAL PRESSURE COEFFICIENT, GC_{pi} = ±0.18.
E. WIND VELOCITY, V_{ult} 175 MPH.

13. LETTER OF COMPLIANCE:

I, AS THE SPECIAL INSPECTOR OF THIS BUILDING, UPON COMPLETION OF THE BUILDING AND PRIOR TO THE ISSUANCE OF THE CERTIFICATE OF OCCUPANCY, WILL FILE A SIGNED AND SEALED STATEMENT WITH THE THE ENFORCEMENT AGENCY THAT STATES THAT 'TO THE BEST OF MY KNOWLEDGE AND BELIEF, THE ABOVE DESCRIBED CONSTRUCTION OF ALL LOAD-BEARING STRUCTURAL COMPONENTS COMPLIES WITH THE PERMITTED DOCUMENTS, AND THE SHORING AND RESHORING TO THE SHORING AND RESHORING PLANS SUBMITTED TO THE ENFORCING AGENCY'.

JOAQUIN MONTESINO LICENSE No. 69969 STATE OF FLORIDA PROFESSIONAL ENGINEER

Digitally signed by JOAQUIN MONTESINO
Date: 2024.01.31 11:09:47 -05'00'

WRITTEN DIMENSIONS ON THESE DRAWINGS SHALL HAVE PRECEDENCE OVER SCALE DIMENSIONS. CONTRACTORS SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS ON THE JOB AND THIS OFFICE MUST BE NOTIFIED OF ANY VARIATION FROM THE DIMENSIONS AND CONDITIONS SHOWN BY THESE DRAWINGS

G2 CONSULTING GROUP, INC.
JOAQUIN MONTESINO, P.E
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ADDITION & RESTORATION FOR:

MAZZINI & MELINE RESIDENCE

10300 SW 70TH AVE, MIAMI, FL 33156

DRAWN BY		
DATE	--	PROJECT
04.03.23	CHECKED BY	22-84
	--	

REVISIONS

S-6

CONSTRUCTION DOCUMENTS