

I. FOUNDATIONS

I. FOUNDATIONS

- A. SOIL IS ASSIGNED TO PROVIDE 2000 PSF BEARING CAPACITY. AS FOR VISUAL INSPECTION IT APPEARS TO BE LIMEROCK AND SILICA SAND, FREE FROM ORGANIC MATERIALS. BEFORE POURING FOUNDATION CALL THE ENGINEER OF RECORD TO OBSERVE THAT THE FOUNDATION CONDITIONS ARE SIMILAR TO THOSE UPON WHICH THE DESIGN IS BASED. A LETTER WILL BE SUBMITTED TO THE CITY OF URMIA DATED, CERTIFYING THAT THE SOIL CONDITIONS ARE THE SAME AS STATED ABOVE.
- B. CENTERS OF COLLUMS SHALL COINCIDE WITH CENTERS OF FOOTINGS, UNLESS OTHERWISE NOTED IN FOUNDATION PLAN AND FOOTING DETAILS.
- C. PROVIDE ANY BRACING OR SHORING AS REQUIRED IN ORDER TO PREVENT SETTLEMENT OR DISPLACEMENT OF ADJACENT EXISTING FOUNDATIONS AND JOINT STRUCTURES.
- D. TOP OF FOUNDATION ELEVATIONS SHOWN IN PLAN ARE BASED ON THE BEST AVAILABLE INFORMATION GATHERED FROM SOIL BORING LOGS. SOIL CONDITIONS ENCOUNTERED DURING INITIAL EXCAVATION MAY BE DIFFERENT. THE FOUNDATION ELEVATION TO BE LOWERED IN ORDER TO EMBED FOUNDATIONS A MINIMUM OF 6 INCHES INTO THE EXISTING LIMEROCK BEARING STRATA.
- E. ALL FOUNDATION EXCAVATIONS SHALL BE KEPT DRY. FOUNDATION CONCRETE SHALL BE POURED DURING DRY WEATHER. EXCAVATIONS SHALL BE KEPT DRY EVERY PRECAUTION DURING DEWATERING OPERATIONS SO THAT EXISTING ADJACENT STRUCTURES, UTILITIES, PIPING, ETC. ARE NOT DAMAGED.
- F. FOUNDATION EXCAVATIONS SHALL BE INSPECTED BY ENGINEER OF RECORD AND GEOTECHNICAL ENGINEER. NO WORK SHALL BE ALLOWED UNTIL GEOTECHNICAL ENGINEER CERTIFIES THAT SOIL CONDITIONS ARE AS PER GEOTECHNICAL REPORT.
- G. FOUNDATIONS MAY BE DEEPENED IF SOIL CONDITIONS PERMIT. EXCAVATE TO EXACT FOUNDATION SIZES.
- H. SLAB ON GRADE
 - 1. FILL AND BACKFILL TO BE COMPACTED TO 98% OF THE MAXIMUM DENSITY AT OPTIMUM HUMIDITY AS DETERMINED BY THE MODIFIED PROCTOR TEST. COMPACTION LATES NOT REQUIRED TO BE RECORDED.
 - 2. ALL CONCRETE SLABS ON GRADE TO BE IN ACCORDANCE WITH THE LATEST "GUIDE FOR CONCRETE FLOOR AND SLAB CONSTRUCTION" (ACI 309).
 - 3. JOINTS SHALL BE PROVIDED WHERE INDICATED BY THE ENGINEER ON THE PLAN.
 - 4. 6 MIL. POLYETHYLENE MEMBRANE SHALL BE PROVIDED UNDER ALL SLABS FORMED ON FILL, COLLUM, BEAMS, WALLS AND ANY OTHER STRUCTURAL MEMBER PENETRATING THE SLAB.
 - 5. ALL JOINTS SHALL BE PROPERLY ISOLATED BY FUR-MOLDED JOINT FILLER (1" X 1/2" X 1/2" COMPLYING WITH ASTM D726, TYPE I).

2. CONCRETE-

- A. ALL CONCRETE WORK SHALL CONFORM TO ALL REQUIREMENTS OF ACI 301-II "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS".
- B. CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE AS FOLLOWS:
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| FOOTINGS | 3000 PSI. |
| SLABS ON GRADE | 3000 PSI. |
| STRUCTURAL SLABS AND BEAMS | 3000 PSI. |
| COLUMNS | 3000 PSI. |
| ALL OTHER CONCRETE | 3000 PSI. |
- C. ALL FOUNDATIONS AT WATER LEVEL: HIGH SHALL BE 3000 PSI. 0% CEMENT RATIO WATER AND GRADE UO 64" TYPE I (WATER REDUCING ADJUTIVE).
- D. CONTRACTOR TO CONTRACT AN INDEPENDENT TESTING LABORATORY TO PER-
FORM CONCRETE CYLINDER TESTS AS FOLLOWS: ONE CYLINDER TESTS FOR ANY
CONCRETE POURED EACH DAY. ONE CYLINDER SHALL BE TESTED AT 7 DAYS
AND 7 DAYS AT 28 DAYS AND ONE RESERVED TO BE TESTED AT 56 DAYS IF
REQUIRED. FOLLOW THE STANDARDS FOR SAMPLING AND TESTING. ONE SLUMP TEST
SHALL BE TAKEN FOR EACH SET OF TEST CYLINDERS CAST. SLUMP TEST SHALL
CONFORM WITH ASTM C 143. NO CONCRETE TEST WILL BE ACCEPTED IF CONCRETE
IS TAMPERED WITH IN ANY WAY AFTER SOLL TEST IS PERFORMED. REPEAT TEST IF
WATER IS ADDED AFTER INITIAL SAMPLING.
- E. TRANSPORTING, PLACING, CURING AND DEPOSITING OF CONCRETE SHALL COMPLY
WITH ACI 301-II: SPECIFICATIONS FOR STRUCTURAL CONCRETE.
- F. FORMWORK SHALL COMPLY WITH ACI 347-14 "RECOMMENDED
PRACTICE FOR CONCRETE WORK".
- G. NO WATER SHALL BE ADDED TO THE CONCRETE AT THE JOB SITE.
- H. MIX DESIGNS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO
CONCRETEMENT OF ANY CONCRETE WORK. SUBMIT STATISTICAL DATA FOR EACH
CLASS OF CONCRETE.
- I. MAXIMUM WATER/CEMENT RATIO FOR CONCRETE CONTAINING A SUPERPLASTICIZING
ADJUTIVE SHALL BE 0.40. SLUMP AFTER ADDITION OF SUPERPLASTICIZER
SHALL BE 6" ± 1".
- J. CONSTRUCTION JOINTS IN STRUCTURAL SLABS AND BEAMS SHALL BE LOCATED
AT 1/3 OF THE SPAN WITH REINFORCING CONTINUOUS ACROSS THE JOINT. PROVIDE
A CONTINUOUS 2" X 4" BEAM KEY AT SLABS. AT BEAMS, PROVIDE A
1" DEEP BEAM KEY AT SLAB WITH A WIDTH 8" SMALLER THAN THE BEAM WIDTH
AND A DEPTH 8" SMALLER THAN THE BEAM DEPTH.
- K. LOCATIONS SHALL BE APPROVED BY STRUCTURAL ENGINEER OF RECORD
BEFORE POUR.

K. MINIMUM CONCRETE COVER FOR REINFORCEMENT:

	BOTTOM	TOP	SIDES
FOOTINGS	3'	2'	3'
BEAMS	1½'	1½'	1½'
COLUMNS	---	---	1½'
INTERIOR SLABS	¾"	¾"	1½"
EXTERIOR SLAB	1"	1½"	1½"
GARAGE GROUND FLOOR SLAB	2"	2"	3"

3. REINFORCING STEEL:

- A. REINFORCING STEEL SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-14.
- B. REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A 615-5.
- C. MIN. COVERAGE 40.
- D. ALL UNLAPPED WIRE FABRIC SHALL CONFORM TO ASTM A 1064.
- E. REINFORCING STEEL SHALL BE HELD SECURELY IN POSITION WITH STANDARD ANCHORS.
- F. ALL REINFORCING SHALL BE DEVELOPED IN ACCORDANCE WITH CRS "MANUAL OF STANDARD PRACTICE," 2008.
- G. TOP REINFORCING SHALL BE DEVELOPED WITH STANDARD HOOKS AT DISCONTINUOUS EDGES OR ENDS.
- H. ALL BOTTOM BARS SHALL BEAR 6" MINIMUM OVER SUPPORTS. LAPS SHALL BE REINFORCED BY BARS OR CONTINUOUS SHALL BE LAPPED 30 DIA. AT SPLICES AND CORNERS UNLESS OTHERWISE NOTED. LAP CONTINUOUS TOP BARS 30 DIAMETER BETWEEN CORNERS. REINFORCE BOTTOM CONTINUOUS BARS AT NON-CONTINUOUS ENDS WITH STANDARD HOOKS, 100%.
- I. SLAB TOP BARS ARE SHOWN IN PLAN AS SOLID LINES.
- J. SLAB BOTTOM BARS ARE SHOWN IN PLAN AS DASHED LINES.
- K. BEAMS WITH MULTIPLE LAYERS SHALL BE REINFORCED WITH TOP OR BOTTOM SEPARATE LAYERS WITH 1/2 SPACER BARS FOR BAR SIZES UP TO 2# AND 3/4" SPACER BARS FOR BAR SIZE FOR 1# BARS AND LARGER.
- L. BEAMS WITH MULTIPLE LAYERS SHALL BE REINFORCED WITH 2# BARS PER LAYER OF REINFORCING.
- M. BEAMS' INTERMEDIATE BARS SHALL BE HOOKED AT DISCONTINUOUS ENDS AND SPLICED AT SUPPORTS WITH 30 BAR DIAMETER LAP SPLICES.
- N. PROVIDE A 10% REINFORCING STEEL ALLOWANCE (BY BUILDING) TO BE USED BY STRUCTURAL ENGINEER AT HIS OR HER DISCRETION. BALANCE OF STEEL NOT USED DURING CONSTRUCTION SHALL BE CREDITED TO THE OWNER AT THE SAME COST PER TON AS ORIGINALLY CHARGED.
- O. FOR SLAB 5" THICKEN LAYERS FOLLOW CRS' DESIGN HANDBOOK, 2008', 10% MINIMUM 3#.

4. CONCRETE MASONRY WORK.

- C. ALL CONCRETE MASONRY WALLS NOTED AS LOAD BEARING WALLS SHALL BE IN PLACE BEFORE THE SLABS AND BEAMS SUPPORTING BY THEM ARE POURED AS WELL AS THE CONCRETE TIE COLUINS FRAMING THEM.
- D. CONCRETE MASONRY WALLS NOTED AS LOAD BEARING WALLS SHALL BE PLACED AFTER CONCRETE FRAME SUPPORTING THEM ARE 28 DAYS OLD AND ALL SHORING AND RE-SHORING IS COMPLETELY REMOVED FROM BELOW AND ABOVE. THERE MUST BE CLEAR SPACE BETWEEN ALL CONCRETE BEAMS OR JOISTS OF SECTION OF CONCRETE SLAB OR BEAM ABOVE HAS OCCURRED. FILL WITH PORTLAND MORTAR SHALL CONFORM TO ASTM C 710, TYPE "M", WITH A MINIMUM VERTICAL STRENGTH OF 1500 PSI.
- E. ALL CMU WALLS SHALL BE HORIZONTALLY REINFORCED WITH STANDARD NO. 9 LADDER-TYPE GALVANIZED STEEL REINFORCING EVERY SECOND COURSE. EXTEND REINFORCING THROUGHOUT ENTIRE LENGTH OF COLUINS.
- F. FILLED CELLS SHALL BE FILLED WITH 3000 PSI GROUT AS PER ACI 308-10 AND AC 308-11 FILLING OF CELLS SHALL BE DONE IN FOUR FOOT LAYS WITH A MAXIMUM HEIGHT OF 6 FEET PER LAY. EACH LAYER SHALL BE THOROUGHLY CONSOLIDATED AND SEAL AS REQUIRED BY ACCEPTABLE DRAINAGE PRACTICES TO PREVENT WATER INTRUSION AND SHALL BE SECURED TO ADJACENT WALLS TO BE SET BACK BETWEEN 8" AND 10".
- G. VERTICAL REINFORCING IN CMU CELLS SHALL BE SPICED WITH 48 BAR DIAMETER LAP SPLICES. PROVIDE CLEAN OUT HOLES AT BASE OF FILLED CELLS FOR RISE UP LIFTING AND VERIFYING THAT THE GROUT HAS BEEN FILLED SOLID WITH GROUT.
- H. PROVIDE GALVANIZED STEEL DOVETAIL ANCHORS EVERY OTHER COURSE CONNECTING NON-LOAD BEARING WALLS TO LOAD BEARING WALLS.
- I. ANCHOR BOLTS SHALL BE EMBEDDED IN WALLS IN GROUPED CELLS.
- J. REINFORCING BARS BE SHALL CENTERED IN THE BLOCK CELL. DOUELS NOT LINED UP WITH THE BLOCK CELL SHALL NOT BE SLOPED MORE THAN 1 IN 6 HORIZONTAL DISTANCE BETWEEN DOEUL AND REINFORCING BAR MAY BE UP TO 8" ONE BLOCK LONG.
- K. GROUPTED CELLS WHERE WEDGE ANCHORS ARE TO BE INSTALLED SHALL HAVE THE BLOCK CELL REMOVED SO THAT WEDGE ANCHOR IS EMBEDDED IN SOLID CONCRETE GROUT. FILL GROUPTED CELLS WITH GROUT TO TOP OF ANCHOR.
- L. REINFORCING BARS IN GROUPTED CELLS SHALL BE SECURED IN PLACE AT BASE OF BAR AND ABOVE BY GROUPTED CELL WITH GALVANIZED BAR POSITONERS.
- M. ALL CONCRETE MASONRY WORK HAS BEEN DESIGNED AND DETAILED ASSUMING CONCRETE MASONRY WORK WILL BE COMPLETELY INSPECTED BY SPECIAL OR ARCHITECT'S INSPECTION.
- N. LAY MASONRY UNITS IN RUNNING BOND.
- O. PROVIDE 30 LBS. FELT PAPER TO ISOLATE WOOD FROM MASONRY WALLS USE PRESSURE TREATED WOOD FOR LEDGERS IN CONTACT WITH MASONRY WALL.
- P. REINFORCING BARS SHALL BE LOCATED AS INDICATED IN PLAN OR CALLED OUT BY NOTES IN PLAN. SEE PLANS AND NOTES DISAGREE CONSULT ENGINEER OF RECORD FOR CLARIFICATION.
- Q. CONCRETE MASONRY STRENGTH, f'_m , SHALL BE MINIMUM OF 1500 PSI.

5. STRUCTURAL STEEL

- A. STRUCTURAL STEEL SHALL COMPLY WITH AISC SPECIFICATIONS FOR DESIGN FABRICATION AND ERECTION FOR STRUCTURAL STEEL BUILDINGS NINTH EDITION.
- B. STRUCTURAL STEEL SHALL CONFORM TO ASTM A572, GRADE 50, 36 KSI.
- C. ANCHOR BOLTS SHALL CONFORM TO EITHER ASTM A 307 OR ASTM A 36.
- D. STRUCTURAL STEEL SHAPES AND PLATES SHALL CONFORM TO ASTM A 36, 36 KSI, I/LON.
- E. STRUCTURAL STEEL TUBES SHALL CONFORM TO ASTM A500, GRADE C, 36 KSI, RND.
- F. BOLT HOLES THROUGH STEEL MEMBERS SHALL BE SHOP-DRILLED, CUT OR RANCHED DO NOT USE TORCH OR FLAME CUT OR IN-LINE HOLES.
- G. ALL EXTERIOR STEEL SHAPES, PLATES, NUTS, BOLTS, WASHERS SHALL BE HOT-DIPPED GALVANIZED.
- H. FRAMING BOLTS SHALL CONFORM TO ASTM 325 WITH HARDENED WASHERS AND HEX NUTS.
- I. SPLICING OF STEEL MEMBERS IS NOT ALLOWED, UNLESS SPECIFIED IN STRUCTURAL DRAWINGS OR PERMITTED BY ENGINEER.
- J. ALL BOLTS, NUTS AND WASHERS SHALL BE NEW, RUST-FREE, CLEAN AND WELL LUBRICATED.
- K. FOR FIREPROOFING OF STRUCTURAL STEEL MEMBERS SEE ARCHITECTURAL DRAWINGS.
- L. SEE ARCHITECTURAL, MECHANICAL, AND PLUMBING DRAWINGS FOR ADDITIONAL MISCELLANEOUS STRUCTURAL STEEL NOT SHOWN IN STRUCTURAL DRAWINGS.
- M. DO NOT PART PANTS OF STEEL MEMBERS TO BE EMBEDDED IN CONCRETE AND SURFACES TO BE IN CONTACT WITH CONCRETE.
- N. FOR PAINTING OF NON-GALVANIZED STRUCTURAL STEEL, SEE STRUCTURAL STEEL PRESCRIPTIONS.
- O. ALL SHOP AND FIELD WELDING SHALL BE IN ACCORDANCE WITH AISC D11 STRUCTURAL WELDING CODE, LATEST EDITION. WELDERS SHALL BE AISC-CERTIFIED SUBMIT WELDING CERTIFICATES TO ARCHITECT/ENGINEER FOR APPROVAL BEFORE ANY SHOP OR FIELD-WELDING IS STARTED.
- P. WELDING SHALL BE DONE WITH A 10 ELECTRODES, UNLESS OTHERWISE NOTED, CONFORMING TO AISC D11.

6. COLD-FORMED STEEL MEMBERS

- A. DESIGN COLD-FORMED STEEL MEMBERS SHALL CONFORM WITH "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, AISI 66-671, LATEST EDITION, AND "DESIGN GUIDE FOR COLD-FORMED STEEL TRUSSES," AISI RG-98.
- B. RESULTS OF THE DESIGN OF STEEL MEMBERS SHALL BE SIGNED AND SEALED BY A STATE OF FLORIDA REGISTERED ENGINEER AND THE DESIGN SHALL FOLLOW DESIGN METHODOLOGIES RECOMMENDED BY AISI.
- C. MEMBERS SHALL COMPLY WITH ASTM A 653/ A 653M GALVANIZED STEEL SHEET, MINIMUM G 60 COATING, MINIMUM YIELD STRENGTH OF 55,000 PSI, MINIMUM GAUGE OF MEMBERS SHALL BE 22 GA.

- BALCONY AND TERRACE RAILINGS AND STAIR RAILINGS SHALL BE DESIGNED BY MANUFACTURER'S REGISTERED ENGINEER IN THE STATE OF FLORIDA TO RESIST A LOAD OF 150 PLF APPLIED IN ANY DIRECTION AT THE TOP OF THE RAILING OR BEAM. THE RAILING SHALL BE DESIGNED TO RESIST A LOAD OF 100 PLF APPLIED IN ANY DIRECTION AT THE TOP OF THE RAILING OR A MINIMUM LOAD OF 200 LBS HANDRAILS SHALL BE DESIGNED AND CONSTRUCTED TO RESIST A LOAD OF NOT LESS THAN 200 LBS APPLIED IN ANY DIRECTION AND AT ANY POINT ON THE RAIL. THE GUARDRAIL SYSTEM SHALL BE DESIGNED TO RESIST A LOAD OF 100 PLF APPLIED AT THE TOP OF THE GUARDRAIL. GUARDRAIL SYSTEMS LOCATED OTHER THAN IN DWELLING UNITS SHALL BE DESIGNED TO RESIST A LOAD OF 100 PLF APPLIED AT THE TOP OF THE GUARDRAIL AT THE REQUIRED GUARDRAIL HEIGHT AND A SIMULTANEOUS LOAD OF 500 PLF APPLIED VERTICALLY DOWNWARD AT THE TOP OF THE GUARDRAIL. THE GUARDRAIL SYSTEM SHALL BE DESIGNED TO RESIST A LOAD OF 100 PLF APPLIED AT THE TOP OF THE GUARDRAIL APPLIED ON A 1 SQ. FT. AREA AT ANY POINT IN THE SYSTEM INCLUDING INTERMEDIATE RAILS OR OTHER ELEMENTS SERVING THIS PURPOSE. LOADING CONDITIONS ABOVE SHALL BE APPLIED TO ALL RAILINGS AND GUARDRAILS OF THE BUILDING EXCEPT WHERE SHOWN OTHERWISE IN EACH OF THE RESPECTIVE COMPONENTS OR PARTS OF THE SUPPORTING COMPONENT.

8. DESIGN LOADS :

- A. THE DESIGN COMPLIES WITH THE REQUIREMENTS OF THE F.L.A. BLDG. CODE 2020 AND OTHER REFERENCED CODES AND SPECIFICATIONS. ALL REFERENCED CODES SHALL BE LATEST EDITION AT THE TIME OF PERMIT.
- B. WIND LOAD CRITERIA:
BASIC LOAD VELOCITY = 115 MPH ASCE 1-16.
RISK CATEGORY II
EXPOSURE CATEGORY: C
WIND DIRECTIONALITY FACTOR, K_d = 0.85 SUPERIMPOSED LOADS:
LIVE LOADS: DEAD LOADS:
ROOF = 30 PSF ROOF = 25 PSF + SYSTEM WEIGHT
FLOOR = 40 PSF (U.O.N. ON PLAN) FLOOR = 20 PSF + SYSTEM WEIGHT

9. SHORING AND RESHORING:

- A. SHORING AND RESHORING DRAININGS SHALL BE PREPARED BY A STATE OF FLORIDA REGISTERED SPECIALTY ENGINEER WITH A MINIMUM OF TEN YEARS OF EXPERIENCE IN SHORING AND RESHORING.
- B. SHORING AND RESHORING SUBMITTAL FOR APPROVAL SHALL INCLUDE AT LEAST TWO COPIES FOR THE BUILDING DEPARTMENT, ONE FOR THE ENGINEER OF RECORD, ONE FOR THE THRESHOLD INSPECTOR, AND ONE FOR THE ARCHITECT.
- C. SHORING AND RESHORING DRAININGS SHALL INCLUDE AT LEAST THE FOLLOWING ITEMS: 1. LOCATION, SIZE, TYPE, AND MATERIAL OF ALL BORING; 2. LOCATION, SIZE, TYPE, AND CAPACITY OF ALL RESHORING; 3. IDENTIFICATION OF THE TYPE OF ALL BLOCKING, WILD SILLS, TEMPORARY LATERAL BRACING, AND OTHER ACCESSORIES REQUIRED TO ADEQUATELY AND SAFELY SUPPORT AND BRACE THE STRUCTURE DURING CONSTRUCTION; 4. LOCATION, SIZE, TYPE, AND MATERIAL OF INSTALLATION LOAD RELIEF AND REMOVAL OF ALL SHORING AND RESHORING.
- D. THE MAXIMUM SUPERIMPOSED CONSTRUCTION LOAD APPLIED TO FLOORS SUPPORTING SHORING OR RESHORES SHALL NOT EXCEED 75% OF THE DESIGN LIVE LOAD SPECIFIED FOR SLABS OR BEAMS. THE MAXIMUM SUPERIMPOSED CONSTRUCTION LIVE LOAD SPECIFIED FOR BEAMS NO CONSTRUCTION LOAD SHALL BE APPLIED TO ANY MEMBER UNTIL THE CONCRETE IS A MINIMUM OF 14 DAYS OLD AND THE 1 DAY STRENGTH IS HALF OF THE SPECIFIED 28 DAY STRENGTH.
- E. SHORING MAY BE REMOVED FROM THE TOP OF THE CONCRETE SLAB PROVIDED THAT CONCRETE STRENGTH IS 10% OF THE SPECIFIED 28 DAY STRENGTH AND NOT LESS THAN 2000 PSI. RESHORE EACH BAY IMMEDIATELY AFTER FORMS ARE STRIPPED AND REMOVED. REMOVAL OF FORMS IS THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR. CONCRETE SHALL BE REMOVED IN A MANNER AS TO AVOID DAMAGE TO THE CONCRETE STRUCTURE, AND INSURE SAFETY AND PREVENT CREep DEFLECTION OF STRUCTURAL MEMBERS.
- F. DESIGN DETAIL AND ERECT FORMS, SHORING AND RESHORING IN COMPLIANCE WITH THE CAPITAL PROJECT SPECIFICATIONS AND THESE SPECIFICATIONS. SHORING AND RESHORING SHALL BE DESIGNED FOR THE WEIGHT OF THE FLOOR OR ROOF, A CONSTRUCTION LOAD OF 50 PSF AND FOR THE CUMULATIVE LOADS OF THE SUPPORTED WORK. CONCRETE SHALL BE REMOVED IN A MANNER AS TO AVOID DAMAGE TO THE CONCRETE STRUCTURE, AND INSURE SAFETY OF SAFETY OF FOR WOOD SHORES AND 2 FOR METAL SHORES.

12. GENERAL

- A. ALL MATERIALS AND CONSTRUCTION SHALL COMPLY WITH THE FLORIDA BUILDING CODE, 2020 EDITION, HAZI, ASCE 10-15 MINIMUM DESIGN LOADS FOR ROADS, THE I.C. 10-15 MINIMUM DESIGN CODE, AND ALL APPLICABLE FEDERAL, STATE AND LOCAL ORDINANCES.
- B. THESE DRAWINGS SHALL BE WORKED THROUGH WITH ARCHITECTURAL, AIR CONDITIONING, MECHANICAL AND ELECTRICAL DRAWINGS TO LOCATE DEPRESSURED GLASS BLOCK DRAINS ONLY. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF ARCHITECT/ENGINEER BEFORE PROCEEDING WITH THE WORK. ANY DISCREPANCIES OR CONFLICTS OF ANY KIND, DISCOVERED IN THE DRAWINGS OR IN THE SPECIFICATIONS DISCOVERED DURING THE BIDDING PHASE SHALL BE IMMEDIATELY COMMUNICATED TO ARCHITECT/ENGINEER.
- C. GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR UPDATING HIS CONSTRUCTION DOCUMENTS WITH THE REVISED DRAWINGS AND SPECIFICATIONS, FIELD ORDERS, CHANGE ORDERS AND CLARIFICATION SCHEDULES ISSUED DURING THE COURSE OF CONSTRUCTION.
- D. THE GENERAL CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING LAGGERS, SHORING AND PROTECTION OF ADJACENT PROPERTY, STREETS, UTILITIES AND UTILITIES IN ACCORDANCE WITH THE LOCAL BUILDING DEPARTMENT.
- E. GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR THE DISPOSAL OF ALL ACCUMULATED WATER FROM EXCAVATIONS AND Dewatering OPERATIONS AS SUCH A WAY AS TO AVOID CAUSE INCONVENIENCE TO THE WORKS AND DAMAGE TO THE STRUCTURAL ELEMENTS.
- F. GENERAL CONTRACTOR SHALL PROTECT EXISTING FACILITIES, STRUCTURES AND UTILITY LINES AND ADJUTANT PROPERTIES FROM DAMAGE OR INJURY BY HIS OR HIS SPOLE, RESPONSIBLE FOR DAMAGE OR INJURY DUE TO HIS ACT OR NEGLIGENCE.
- G. THESE DRAWINGS AND SPECIFICATIONS COMPLY TO THE BEST OF MY KNOWLEDGE WITH THE FLORIDA BUILDING CODE, 2020 EDITION, HAZI.
- H. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY WORKING STRUCTURES AFFECTING NEW CONSTRUCTION BEFORE COMMENCING ANY WORK. ANY VARIATIONS IN ACTUAL FIELD CONDITIONS/DEVIATIONS FROM THOSE SHOWN IN CONSTRUCTION DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER FOR DETERMINING THE NEED OF REDDESIGN FROM TO CONTRACTORS SUBMITTAL OF SHOP WORKING DRAWINGS FOR REVIEW.
- I. WHEN PERFORMING WORK BELOW GRADE, CARE SHALL BE TAKEN TO AVOID DAMAGING ANY EXISTING UTILITIES. ALL UNKNOWN UTILITIES DISCOVERED

- 1. WHEN PERFORMING WORK BELOW GRADE, CARE SHALL BE TAKEN TO AVOID DAMAGING ANY EXISTING UTILITIES. ALL UNKNOW UTILITIES DISCOVERED DURING CONSTRUCTION SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER. ANY DAMAGE TO UTILITIES SHALL BE IMMEDIATELY REPORTED TO ALL AFFECTED PARTIES, INCLUDING THE ARCHITECT/ENGINEER.
- 2. BACKFILL AROUND THE EXTERIOR PERIMETER OF WALLS SHALL NOT BE PLACED UNTIL AFTER THE WALLS HAVE BEEN PROTECTED BY A MINIMUM FLOOR SYSTEM. DO NOT PROCEED WITH BACKFILL UNTIL (1) DAYS AS A MINIMUM AFTER THE COMPLETION OF INTERIOR FLOOR SYSTEM UNLESS WALLS ARE ADEQUATELY PROTECTED. BACKFILL SHALL BE DONE IN ACCORDANCE WITH THE COMPLETION AND INSPECTION OF WATERPROOFING WHERE WATERPROOFING OCCURS.
- 3. TYPICAL DETAILS AND NOTES ON THESE DRAWINGS SHALL APPLY UNLESS SPECIFICALLY NOTED OTHERWISE. CONSTRUCTION DETAILS AND SECTIONS NOT COMPLETED HEREIN OR NOTED OTHERWISE ARE SIMILAR TO DETAILS AND SECTIONS SHOWN OR NOTED FOR SIMILAR CONDITIONS.
- 4. STRUCTURAL NOTES SHALL BE USED IN CONJUNCTION WITH THE SPECIFICATIONS. IF A CONFLICT EXISTS, THE MORE STRINGENT GOVERNS.
- 5. DO NOT SCALE THESE DRAWINGS; USE DIMENSIONS NOTED. IF DIMENSIONS ARE MISSING CONTACT THE ARCHITECT/ENGINEER FOR CLARIFICATION.
- 6. GENERAL CONTRACTOR IS SOLELY RESPONSIBLE FOR JOB SAFETY, MEANS AND METHODS OF CONSTRUCTION AND CONSTRUCTION PROCEDURES.

II. SHOP DRAWINGS SUBMITTALS

- A. ALL SHOP DRAWINGS ARE AN AID FOR FIELD PLACEMENT AND ARE SUPERSEDED BY THE STRUCTURAL DRAWINGS IT SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO MAKE CERTAIN THAT ALL CONSTRUCTION IS IN FULL AGREEMENT WITH THE LATEST STRUCTURAL DRAWINGS.
- B. THESE STRUCTURAL DRAWINGS SHALL NOT BE USED TO PRODUCE SHOP DRAWINGS WITHOUT PRIOR EXPRESSED WRITTEN APPROVAL FROM THE STRUCTURE ENGINEER OF RECORD
- C. SHOP DRAWINGS REQUIRING ENGINEERING BY A SPECIALTY ENGINEER SHALL COMPLY WITH THE FOLLOWING:
 - I. THE SPECIALTY ENGINEER SHALL BE A FLORIDA REGISTERED PROFESSIONAL ENGINEER
 - II. THE SPECIALTY ENGINEER SHALL HAVE AT LEAST FIVE YEARS OF SOLID PROFESSIONAL DESIGN EXPERIENCE IN THE DESIGN AND DETAILING OF THE STRUCTURAL COMPONENT AND/OR STRUCTURAL SYSTEM BEING SUBMITTED FOR APPROVAL
 - III. THE SPECIALTY ENGINEER SHALL BE AN EMPLOYEE OR OFFICER OF A FABRICATOR, AN EMPLOYEE OR OFFICER OF ANOTHER COMPANY SUPPLYING COMPONENTS TO THE FABRICATOR
 - IV. SHOP DRAWINGS RE-SUBMITTED FOR APPROVAL SHALL BE CLEARLY CLOUDED AND NOTED RE-SUBMIT SHALL BE LIMITED TO THOSE ITEMS WHICH CAUSED THE RE-SUBMITTAL.
- F. THE CONTRACTOR SHALL SUPPLY THE ENGINEER THREE COPIES OF SHOP DRAWINGS A MINIMUM OF ONE WEEK PRIOR TO PLACEMENT. THE REVIEW OF SHOP DRAWINGS BY THE ENGINEER IS ONLY FOR GENERAL COMPLIANCE WITH THE STRUCTURAL DRAWINGS AND SPECIFICATIONS. THE REVIEW DOES NOT GUARANTEE IN ANY WAY THAT THE SHOP DRAWINGS ARE CORRECT NOR DOES IT MEAN THAT THEY SUPERSEDE THE STRUCTURAL DRAWINGS. REVIEW WILL NOT INCLUDE CHECK OF DIMENSIONS, LENGTHS OR QUANTITIES.
- E. SUBMITTALS TO STRUCTURAL ENGINEER OF RECORD:
 - I. CONCRETE TEST REPORTS FOR CAST-IN-PLACE CONCRETE AS PER ACI 308-11.
 - II. REINFORCING STEEL SHOP DRAWINGS.
 - III. STRUCTURAL STEEL SHOP DRAWINGS AND ERECTION DRAWINGS.
 - IV. POST-TENSIONED CONCRETE SHOP DRAWINGS AND DESIGN DATA AS REQUIRED BY STRUCTURAL DRAWINGS.
 - V. SHORING AND RESHORING SHOP DRAWINGS AND CALCULATIONS.
 - VI. RAILING AND GUARDRAIL SHOP DRAWINGS AND CALCULATIONS.
 - VII. PRECAST JOIST SUBMITTAL, SIGNED AND SEALED BY SPECIALTY ENGINEER REGISTER IN FLORIDA
 - VIII. METAL AIR CONDITIONING COMPRESSOR STAND SHOP DRAWINGS AND NOA.

12. ALUMINUM-

- A. ALUMINUM WORK SHALL COMPLY WITH THE ALUMINUM ASSOCIATION, INC. 'SPECIFICATIONS FOR ALUMINUM STRUCTURES', 'THE COMMENTARY ON SPECIFICATIONS FOR ALUMINUM STRUCTURES', AND 'ENGINEERING DATA' FOR ALUMINUM STRUCTURES.
- B. ALUMINUM WORK SHALL COMPLY WITH THE FLORIDA BUILDING CODE, 2010
- C. ALUMINUM TUBING, BARS, AND PLATES SHALL CONFORM TO ALLOY 6061-T6.
- D. WELDING OF ALUMINUM PARTS AND MEMBERS SHALL BE DONE WITH AN INERT-GAS-SHIELDING ARC OR RESISTANCE WELDING PROCESS.

13. TERMITES NOTE

- SOIL SHALL BE TREATED FOR TERMITES PROTECTION AS PER F.B.C. 2010, SECTION R 310 .
ALL BUILDINGS SHALL HAVE PRE-CONSTRUCTION TREATMENT PROTECTION AGAINST
SUBTERRANEAN TERMITES. THE RULES AND LAWS AS ESTABLISHED BY THE FLORIDA
DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES SHALL BE DEEMED AS APPROVED
WITH PRE-CONSTRUCTION TREATMENT PROTECTION AGAINST SUBTERRANEAN
SUBTERRANEAN TERMITES. A CERTIFICATE OF COMPLIANCE SHALL BE ISSUED TO THE BUILDING
DEPARTMENT BY THE LICENSED PEST CONTROL COMPANY THAT CONTAINS THE FOLLOWING
STATEMENT: "THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF
SUBTERRANEAN TERMITES IN ACCORDANCE WITH THE RULES AND LAWS AS ESTABLISHED
BY THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES".

14. SUBMITTALS TO STRUCTURAL ENGINEER OF RECORD:

- I. CONCRETE TEST REPORTS FOR CAST-IN-PLACE CONCRETE AS PER ACI 308-11
- II. REINFORCING STEEL SHOP DRAWINGS
- III. STRUCTURAL STEEL SHOP DRAWINGS AND ERECTION DRAWINGS
- IV. POST-TENSIONED CONCRETE SHOP DRAWINGS AND DESIGN DATA AS REQUIRED BY STRUCTURAL DRAWINGS
- V. SHORING AND REINFORCING SHOP DRAWINGS AND CALCULATIONS
- VI. RAILING AND GUARDRAIL SHOP DRAWINGS AND DESIGN DATA
- VII. ALUMINUM WINDOW, SLIDING GLASS DOOR AND STOREFRONT SYSTEM SHOP DRAWINGS AND NOA
- VIII. METAL LOUVER SHOP DRAWINGS AND NOA
- IX. ALUMINUM TRELLIS SYSTEM SHOP DRAWINGS AND STRUCTURAL CALCULATIONS BY SPECIALTY ENGINEER
- X. ALUMINUM CANOPIES WITH METAL ROOF SHOP DRAWINGS AND STRUCTURAL CALCULATIONS BY SPECIALTY ENGINEER
- XI. COMPOSITE WOOD VENERE LIGHT GAUGE STEEL BEAM SUPPORT SHOP DRAWINGS AND STRUCTURAL CALCULATIONS BY SPECIALTY ENGINEER
- XII. METAL AIR CONDITIONING COMPRESSOR STAND SHOP DRAWINGS AND NOA
- XIII. EXTERIOR METAL GASK DROOP CEILING SHOP DRAWINGS AND STRUCTURAL CALCULATIONS BY SPECIALTY ENGINEER

15. PREFABRICATED WOOD TRUSSES.

- TO BE DESIGNED AND MANUFACTURED IN ACCORDANCE WITH THE FLORIDA BLDG. CODE, 2010 EDITION, THE TRUSS PLATE INSTITUTE, AND ARCHITECTURAL/STRUCTURAL DRAWINGS. SUBMIT SHOP DRAWINGS FOR REVIEW AND APPROVAL TO ENGINEER OF RECORD AND BUILDING DEPARTMENT PRIOR TO FABRICATION. SIGNED AND SEALED BY A FLORIDA REGISTERED ENGINEER.

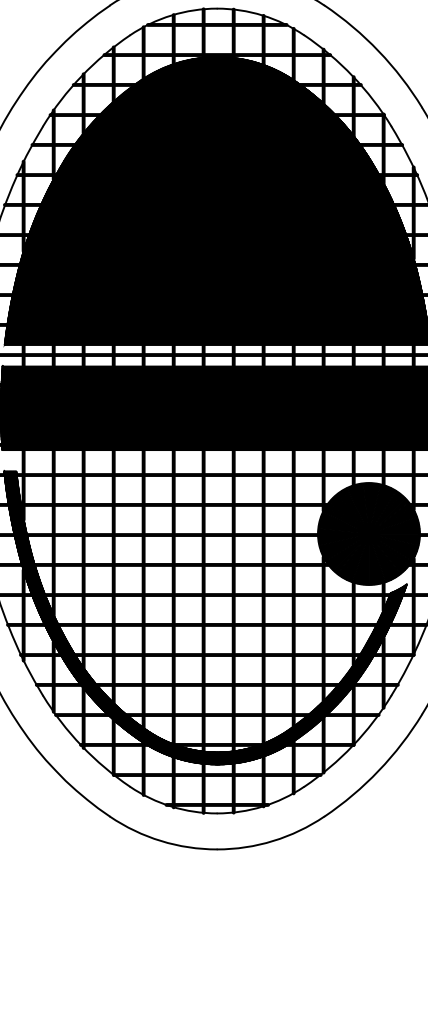
DESIGN LOADS AS FOLLOW -

- TOP CHORD #4S P.S.F. BOTTOM CHORD #10 P.S.F.
ADDITIONAL 200 LB LOAD IN BOTTOM CHORD PANEL POINT
WIND UPLIFT = ACCORDING TO F.L.A. BUILDING CODE. ASCE 7-16
- SHEATHING PLYWOOD MUST BE MINIMUM 5/8" CDX THICKNESS WITH 8d RING SHANK NAILS #4" O.C.
OR 10d COMMON NAILS #4" O.C.

ADDITION AND RENOVATION
FOR:
LISA AND CHRIS MONTERO
LOCATED AT:
6445 SW 107 STREET
PINECREST, FL. 33156

J L D
ARCHITECT
CORPORATION
AA-26003440

8592 s.w. 169th terrace
miami, florida 33157
(305) 310-5992



JOSE L. DIAZ, ARCHITECT
AR-0015547

DRAWN
CHECKED
DATE
SCALE
JOB NO.
SHEET

S-1.0

OF SHEETS

NOTICE

ARCHITECT & STRUCTURAL DRAWINGS HAVE TO BE CONSULTED TOGETHER.
CALL ARCH/ENGINEER FOR ANY DISCREPANCY BEFORE START OF WORK

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